REQUEST FOR PROPOSAL (RFP) # 2019-017

Nonstructional Component Testing at Temecula Campus For Buildings F, G, and Central Plant

MSJC
MT. SAN JACINTO COLLEGE

RFP Issued:	**February 21, 2019**
RFP Due:	**March 5, 2019 at 2:00 p.m.**
Submit Response To:	**Carole Ward** **cward@msjc.edu** **Contract Coordinator** **and** **Tammy Cunningham** **tcunningham@msjc.edu** **Director of Procurement and General**
Questions or Clarifications:	**All questions must be submitted via e-mail to: Carole Ward cward@msjc.edu and Tammy Cunningham tcunningham@msjc.edu**

1. REQUEST FOR PROPOSAL

1.1 Purpose

Mt. San Jacinto Community (MSJC) College District is requesting proposals from qualified companies to provide Special Testing Professional Services for structural attachments of architectural, mechanical and electrical components at the existing Buildings F, G and Central Plant at the MSJC Temecula Campus. The testing program is in support of the Seismic Evaluation and Upgrade project of the aforementioned buildings. The project is being reviewed by the Division of State Architect (DSA) in San Diego, California. Related Engineering Services are being provided by KPFF Los Angeles, California. The company selected to conduct the testing program for this project should be prepared and equipped to provide the special testing services as required by this RFP. The services should be provided in an expeditious and timely manner in order to meet the critical deadlines and schedules of the District listed in this RFP.

1.2 Proposal Submission

If interested in providing professional testing services for the project, proposals must be delivered to the address below, no later than **2:00 p.m. on Tuesday, March 5, 2019**. Late proposals will not be considered. The District is not responsible for late mail or postal delivery errors. Proposers shall submit one electronic version of the proposal on a flash drive, one (1) printed original proposal including any supporting documentation in a sealed box or package addressed as follows:

Attention: Tammy Cunningham
Mt. San Jacinto Community College District
Purchasing Department
Building 200, Room 223
1499 N. State Street
San Jacinto, CA 92583

1.3 Response Format

Each company is required to submit a proposal it deems appropriate to this RFP. Submittals shall be brief and concise, but shall provide sufficient clarity to meet the criteria in the evaluation process. Each Consultant shall submit one (1) electronic proposal on a flash drive and one (1) printed original proposal. The District will evaluate the Proposals based on the responsiveness to District requirements. Criteria/Evaluation Process.

NOTE for Firms teaming with Sub-Consultants: Each responding firm shall select its proposed sub-consultants based on its own criteria. However, MSJCCD reserves the right to approve sub-consultants proposed for any projects that may be awarded. Sub-

Consultants do not need to complete all the Exhibits in this RFP. Carefully read each section to determine which forms the Sub-consultants need to submit.

1.1 Questions

Consultants must carefully read the entire RFP prior to submitting questions as most questions will be answered in this RFP. All questions must be submitted in writing via e-mail to Carole Ward (cward@msjc.edu) and Tammy Cunningham (tcunningham@msjc.edu). The question deadline for this RFP is **Wednesday, February 27, 2019 by 10:00 AM**. After this deadline, the District will not answer, address, and/or review any questions that interested Consultants might submit. Responses to all questions received prior to the deadline will be provided to all Consultants.

1.2 Request for Proposals

Pre-Qualified Consultants are in no way guaranteed to receive any work from the District. Each Proposal shall describe the Consultant's experience and expertise with respect to the services, if any, which are unique to the property or project that is the subject of this RFP. In addition, the Proposal shall set forth a detailed scope of services, a completion schedule, a schedule of professionals that will be used to supervise and staff the project, and a not-to-exceed dollar amount for the services to be performed. The District will allocate work to said Pre-qualified Consultants without having to request and evaluate additional information as to Consultant's qualifications. Consultant shall assign only trained and experienced personnel, support staff, and other Consultants to the requisite tasks. Consultant shall provide cost to perform the tasks outline in the Scope of Services referenced in this RFP.

1.3 Mandatory Pre-proposal Site Visit/Review of Project Documents:

A Mandatory Pre-proposal Site Visit is scheduled for Monday, February 25, 2019 at 10:00 a.m. at the Temecula Valley Campus, 41888 Motorcar Parkway, Temecula, CA 92591.

Consultant must attend job walk. Any Consultant submitting a proposal who fails to attend the entire mandatory job walk and conference will be deemed a non-responsive bidder and will have its proposal returned unopened. Any Consultant who fails to arrive on time (**10:00 a.m**.) will not be able to participate in the Pre-Proposal site visit.

1.4 Pre-qualification of Bidders

As a condition of bidding for this Project, and in accordance with California Public Contract Code Section 20651.1, prospective bidders are required to submit to the District a completed set of prequalification documents on forms provided by the District. Prequalification documents are available at the Mt. San Jacinto Community College District, Office of Procurement and General Services, Room 223, located at 1499 N. State St., San Jacinto, California 92583 or go to the Mt. San Jacinto Community College

Purchasing Office website located at https://www.msjc.edu/Purchasing/Pages/UPCCAA.aspx to download the UPCCA Pre-Qualification Questionnaire. The prequalification documents must be submitted prior to 10:00 a.m. on February 27, 2019. Bids will not be accepted if a Contractor has not been prequalified where qualification is required. Contractors will be notified by telephone or e-mail of their prequalification status within a reasonable period of time after submission of their prequalification documents.

1.5 DIR Registration:

An consultant shall not be qualified to submit a proposal, subject to the requirements of Section 4104 of the Public Contract Code, or engage in the performance of any contract for public work, as defined in the Labor Code, unless currently registered and qualified to perform public works pursuant to Section 1725.5. It is not a violation of this section for an unregistered architect to submit a bid that is authorized by Section 7029.1 of the Business and Professions Code or by Section 10164 or 20103.5 of the Public Contract Code, provided the consultant is registered to perform public work pursuant to Section 1725.5 at the time the contract is awarded.
This Project is a public works project as defined in Labor Code section 1720. Each contractor bidding on this Project and all subcontractors (of any tier) performing any portion of the Work must comply with the Labor Code sections 1725.5 and 1771.1 and must be properly and currently registered with DIR and qualified to perform public works pursuant to Labor Code section 1725.5 throughout the duration of the Project. For more information and up to date requirements, consultants are recommended to periodically review the DI's website at www.dir.ca.gov. consultants shall be solely responsible for ensuring compliance with Labor Code section 1725.5 as well as any requirements implemented by DIR applicable to its services or its subcontractors throughout the term of the Agreement and in no event shall contractor be granted increased payment from the District or any time extensions to complete the Project as a result of contractor's efforts to maintain compliance with the Labor Code or any requirements implemented by DIR. Failure to comply with these requirements shall be deemed a material breach of this Agreement and grounds for termination for cause. The contractor and all subcontractors shall furnish certified payroll records as required pursuant Labor Code section 1776 directly to the Labor Commissioner in accordance with Labor Code section 1771.4 on at least on a monthly basis (or more frequently if required by the District or the Labor Commissioner) and in a format prescribed by the Labor Commissioner. The District reserves the right to withhold contract payments if the District is notified, or determines as the result of its own investigation, that contractor is in violation of any other the requirements set forth in Labor Code section 1720 et. Seq. at no penalty or cost to the District. Monitoring and enforcement of the prevailing wage laws and related requirements will be performed by the Labor Commissioner/Department of Labor Standards Enforcement (DLSE).

The proposals should include the following:

1. Cover Letter: Briefly describe the qualifications of the company and the proposed personnel for this project and provide a statement that you have reviewed the schedule listed in the RFP and agree to provide the necessary effort or staff allocation to meet the schedule listed in Section 4 of this RFP. (1 page max)
2. Approach to Work: provide (i) a statement of the proposed approach to the project scope of work with a description of the tasks, sub-tasks, deliverables that will be provided, and how the staff intend to coordinate and collaborate with the design team to meet the project schedule, and (ii) a description of the Quality Assurance/Control (QA/QC) plan to be followed during the duration of the work. The QA/QC plan shall address the accuracy, completeness and timeliness of all testing and related reports. (1 page max)
3. List of the main point of contact for the project team and key personnel. Clearly identify the individual(s) role(s) and responsibilities in the testing program. Include resumes of the proposed personnel with relevant testing experience. (1 page max per individual).
4. Relevant Project List: Provide project experience information of the company describing type, size, location, and any unique features or process of the project that may be relevant to this project. (1 page max).
5. Fee Proposal: Lump Sum Fee Proposal clearly listing professional testing services and assumptions. Please also include your hourly billing rates. (include number of pages as needed)

1.4 Selection Criteria

1. Timeliness and Completeness: To receive consideration, Responses to this RFP must be received by the Response Deadline. In addition, RFP Response will be evaluated with respect to organization, clarity, completeness, and responsiveness to this RFP.
2. Technical Qualifications and Competence: This includes Company and Personnel's experience, expertise, and familiarity with providing Special Testing Services required by the RFP.
3. Approach to Work: This includes your overall approach/methodology and QA/QC plan to meet the project schedule.
4. Fee: Evaluation of proposed fee structure for requested services.

2. PROJECT DESCRIPTION

2.1 The project consists of the seismic evaluation and necessary seismic upgrades of the existing Buildings F, G and Central Plant for the new MSJC Temecula Campus. The seismic evaluation is being required by DSA and the California Building Code as these existing buildings will become part of the new MSJC Temecula Campus and will have to meet the seismic standards for community colleges. The original buildings were designed under the 2001 California Building Code. Buildings G and F consist of 5-story

steel buildings with special concentric braced frame lateral system. The Central Plant is a 1-story steel ordinary concentrically braced frame.

3. BUILDING INFORMATION

3.1 The original buildings were designed under the 2001 California Building Code. A copy of the existing structural building drawings can be downloaded from the following web link for your reference:

https://kpff-la.app.box.com/s/a7nqn0g33r02kadusosjhnui4owffjuj

4. SCOPE OF WORK

4.1 The scope of work includes the following special testing. Each testing program below is described in detail in the testing drawings. A copy of these drawings can be downloaded from the same web link above.

1. Proposed Testing Program for Seismic Bracing Anchorage of Piping Systems at Building F
2. Proposed Testing Program for Attachments of Suspended Integrated Ceilings at Building G
3. Proposed Testing Program for Seismic Anchorage of MEP Equipment at Central Plant

4.2 The company selected to conduct the testing shall prepare test reports for each Testing Program listed above, stamped and signed by an engineer registered in the state of California, stating the following information:

1. A list of all samples taken from the building, including tag number
2. Location of structural element, type of element, type of material, original date of construction, type of sample;
3. A summary of tested material properties for each sample
4. A description of test method(s) utilized for each test and sample type
5. Raw test data
6. Photographs of each test specimen

4.3 The testing program and test reports shall be completed before April 5, 2019.

Exhibit A
Team Member Resume Form

Proposed Consultant Name

Title

Firm Name

Proposed Position

Years w Firm

Years w/Previous Firms

Years w/ community colleges

Availability

Education Specific to Position (School/Year/Degree/Subject):

Other Training/Experience w/MSJCCD, DSA, Community College Chancellors Office, and other State Agencies (or equivalent):

Credentials/Certifications/Licenses/Registrations/Accreditations (related to position and years acquired):
Note: Do not list any certifications, licenses, etc. that are expired or not from the State of California.

Skills Relevant to the Proposed Project:

List of Community College Districts Consultant Has Worked For:

Exhibit B

Team Member Experience Form

Provide a minimum of five (5) relevant and similar projects types completed within the last five (5) years. Use multiple sheets as necessary.

Background

Proposed Team Member Name _____ Title _____

Firm Name (at time of Project) _____

Project Details

Project Name _____ Client/District _____

Project Lead Name/Title _____ Phone _____ E-Mail _____

Address _____

DSA Project #	DSA Certified (Yes/No)	Project Scope (1)	School Type (2)	Project Start Date	Project Completion Date

Total Cost	# Change Orders	Cost of Change Orders

Change Order Notes (include description and reason):

Team Member Title and Duties for this Project:

Project Narrative (firm's role, responsibilities, challenges, how Consultant met Client/District's needs, describe project and responsibilities in detail, demonstration of how this project experience contributes to thorough knowledge of Commissioning requirements for public school buildings in California, and demonstration of how this project experience contributes to familiarity with California building code requirements relating to school sites and buildings):

(1) Project Scope: RE-Renovation/Remodel/Repurpose, ADD-Addition/Expansion, NEW-New Construction, FIX-Repair, PLAN-Planning.

(2) School Type: ES-Elementary School, MS-Middle School, K8-Kindergarten-8th Grade School, HS-High School, CCD-Community College, HE-Other College, NS-Non-School/Other

Exhibit C
Billing Rate Form

Firm Name

Billing Rates

Do rates include travel charges? [X] Yes

Note, all fees and rates must be inclusive of travel. Travel is not an acceptable reimbursable expense

Job Title	Personnel Name	Hourly Rate

Consultant's proposed **ALL INCLUSIVE NOT-TO EXCEED FEE:** $_____

Estimate of Reimbursable Expenses included in the fee stated above: _____
Estimated Amount

_____
Authorized Signature

CONSULTANT SERVICES AGREEMENT

This AGREEMENT is made and entered into this _____ day of _____ in the year 20___ by and between the MT. SAN JACINTO COMMUNITY COLLEGE DISTRICT, hereinafter referred to as "DISTRICT," and _____, hereinafter referred to as "CONSULTANT." This AGREEMENT shall include all terms and conditions set forth herein. The DISTRICT and the CONSULTANT are sometimes referred to herein individually as a "PARTY" and collectively as the "PARTIES." This AGREEMENT is made with reference to the following facts:

WHEREAS, DISTRICT desires to obtain architectural services for the NONSTRUCTURAL COMPONENT TESTING AT THE TEMECULA CAMPUS, hereinafter collectively referred to as the "PROJECT"; and

WHEREAS, CONSULTANT is fully licensed to provide architectural services in conformity with the laws of the State of California;

NOW, THEREFORE, the PARTIES hereto agree as follows:

ARTICLE I
SCOPE AND SERVICES AND RESPONSIBILITIES

1. Services to be Provided by the CONSULTANT. The CONSULTANT shall provide to the DISTRICT on the terms set forth herein all the services articulated in the CONSULTANT's proposal which is attached hereto and incorporated herein as **EXHIBIT "A"** (the "CONSULTANT's WORK PLAN"). Where the CONSULTANT's WORK PLAN consists of a proposal or quote submitted in response to a Request for Proposals ("RFP") from the DISTRICT, the CONSULTANT's WORK PLAN shall be considered to include the DISTRICT's RFP. The DISTRICT and CONSULTANT expressly agree to incorporate the terms and conditions of the DISTRICT's RFP into this AGREEMENT by this reference and the PARTIES understand that the RFP shall constitute a binding part this AGREEMENT. In the event of a discrepancy, inconsistency, or other difference between the terms of the RFP or the CONSULTANT's WORK PLAN with this AGREEMENT, the PARTIES agree that the terms of this AGREEMENT shall govern and control.

2. Classification: To the extent it is determined under applicable law that CONSULTANT fails to meet the statutory prerequisites for classification as a professional expert operating under a personal services agreement, CONSULTANT resigns any and all rights and privileges derived from this AGREEMENT and any resulting relationship, which resignation is deemed accepted under such circumstances by the DISTRICT.

3. Contract Term. The effective period of this AGREEMENT is to be _____ through _____.

4. CONSULTANT's Certifications, Representations and Warranties. CONSULTANT makes the following certifications, representations, and warranties for the benefit of the DISTRICT and CONSULTANT acknowledges and agrees that the DISTRICT, in deciding to engage CONSULTANT pursuant to this AGREEMENT, is relying upon the truth and validity of the following certifications, representations and warranties and their effectiveness throughout the term of this AGREEMENT and the course of CONSULTANT's engagement hereunder:

a. CONSULTANT is qualified in all respects to provide to the DISTRICT all of the services contemplated by this AGREEMENT and, to the extent required by any applicable laws,

CONSULTANT has all such licenses and/or governmental approvals as would be required to carry out and perform for the benefit of the DISTRICT, such services as are called for hereunder.

b. CONSULTANT, in providing the services and in otherwise carrying out its obligations to the DISTRICT under this AGREEMENT, shall, at all times, comply with all applicable federal, state, and local laws, rules, regulations, and ordinances, including workers' compensation and equal protection and non-discrimination laws.

c. The CONSULTANT will perform its services hereunder in a professional manner, using the degree of care and skill ordinarily exercised by, and consistent with, the current professional practices and standards of a professional practicing in California. The CONSULTANT will furnish, at its expense, those services that are set forth in this AGREEMENT and **EXHIBIT "A"** and represents that the services set forth in said EXHIBIT are within the technical and professional areas of expertise of the CONSULTANT or any subconsultant the CONSULTANT has engaged or will engage to perform the service(s). The DISTRICT shall request in writing if the DISTRICT desires the CONSULTANT to provide services in addition to, or different from, the services described in **EXHIBIT "A"**. The CONSULTANT shall advise the DISTRICT in writing of any services that, in the CONSULTANT's opinion, lie outside of the technical and professional expertise of the CONSULTANT.

5. CONSULTANT has been selected to perform the work herein because of the skills and expertise of key individuals. Services under this AGREEMENT shall be performed only by competent personnel under this supervision of and/or in the employment of the CONSULTANT. CONSULTANT shall conform to DISTRICT's reasonable requests regarding assignment of personnel. All personnel, including those assigned at DISTRICT's request, shall be supervised by CONSULTANT.

6. CONSULTANT shall not change any of the key personnel without prior written approval by the DISTRICT, unless said personnel cease to be employed by CONSULTANT. In either case, DISTRICT shall be allowed to interview and approve replacement personnel. CONSULTANT agrees that reassignment of any of the listed personnel during the AGREEMENT period shall only be with other professional personnel who have equivalent experience and shall require prior consultation and written approval by the DISTRICT. Any costs associated with reassignment of personnel shall be borne exclusively by CONSULTANT and CONSULTANT shall not charge the DISTRICT for the cost of training or "bringing up to speed" replacement personnel. If any designated lead or key person fails to perform to the satisfaction of the DISTRICT, then upon written notice the CONSULTANT shall immediately remove that person from the PROJECT and provide a temporary replacement. CONSULTANT shall within thirty (30) work days, provide a permanent replacement person acceptable to the DISTRICT. DISTRICT may condition its approval of replacement personnel upon a reasonable transition period wherein new personnel will learn the PROJECT and get "up to speed" at CONSULTANT's cost.

7. CONSULTANT represents that the CONSULTANT has no existing interest and will not acquire any interest, direct or indirect, which would create a conflict of interest in violation of any applicable laws, and that no person having any such interest shall be employed by CONSULTANT.

ARTICLE II
COMPENSATION TO THE CONSULTANT

1. The DISTRICT shall compensate the CONSULTANT as follows:

a. The DISTRICT agrees to pay the CONSULTANT in accordance with the fee, rate and/or price schedule information set forth in **EXHIBIT "A"** for the services performed pursuant to this AGREEMENT. In no event shall the total payment to CONSULTANT exceed

_____DOLLARS ($_____.) for performing the services required by this AGREEMENT and **EXHIBIT "A"**.

b. CONSULTANT shall invoice costs monthly, or another periodic basis approved by the DISTRICT, for the services provided pursuant to this AGREEMENT from the time the CONSULTANT begins work on the PROJECT. All costs must be supported by an invoice, receipt, or other acceptable documentation as determined by the DISTRICT.

c. Except as expressly provided herein, CONSULTANT agrees that no other compensation, fringe benefits, or other remuneration is due to CONSULTANT by the DISTRICT for services rendered under this AGREEMENT. CONSULTANT shall not apply for or receive statutory benefits available to employees of the DISTRICT because CONSULTANT is not an employee of the DISTRICT; rather, CONSULTANT is operating under a personal services agreement pursuant to Education Code section 88003.1(b)(2) and has only the rights defined by this AGREEMENT.

2. The CONSULTANT shall submit one (1) invoice monthly to the DISTRICT for the fees incurred during the billing period and reimbursable expenses (if any). Invoices for fees must reflect the date of the service, identify the individual performing the service, state the hours worked and rate charged, and describe the service performed. Invoices requesting reimbursement for reimbursable expenses incurred during the billing period must clearly list items for which reimbursement is being requested and be accompanied by proper documentation (e.g. receipts, invoices) including a copy of the DISTRICT's authorization notice for invoiced item(s). Invoices requesting payment for overtime must reflect straight time and overtime hours being charged, and must include a copy of the DISTRICT's written authorization to incur additional overtime expense. No payments will be made by the DISTRICT to the CONSULTANT for monthly invoices requesting reimbursable expenses or overtime absent the prior written authorization of the DISTRICT. The DISTRICT shall make payment to the CONSULTANT of the approved invoiced amount within forty-five (45) days of the DISTRICT's receipt of the approved invoice.

3. The DISTRICT may withhold, or on account of subsequently discovered evidence, nullify the whole or a part of any payment to such extent as may be necessary to protect the DISTRICT from loss, including costs and attorneys' fees, on account of: (1) defective or deficient work product not remedied; (2) failure of the CONSULTANT to make payments properly to its employees or subconsultants; or (3) failure of CONSULTANT to perform its services in a timely manner so as to conform to the PROJECT schedule or other time constraints.

ARTICLE III
REIMBURSABLE EXPENSES

1. Reimbursable expenses are in addition to compensation for basic and extra services, and shall be paid to the CONSULTANT at one and one-tenth (1.1) times the expenses incurred by the CONSULTANT, the CONSULTANT's employees and consultants for the following specified items unless otherwise approved by the DISTRICT in writing:

a. Approved reproduction of reports and/or other documents otherwise not covered in this AGREEMENT and approved in advance by DISTRICT.

b. Fees advanced for securing approval of authorities in connection with the services rendered pursuant to this AGREEMENT.

c. Express shipping, overnight mail, messenger, courier, or delivery services approved in advance by the DISTRICT.

d. Mileage at IRS Rate if site exceeds more than 25 miles from the DISTRICT.

e. Out of town travel approved in advance by DISTRICT.

2. Reimbursable expenses are estimated to be **_____DOLLARS ($_____)**, and this amount shall not be exceeded without the prior written approval of the DISTRICT.

ARTICLE IV
TERMINATION

1. This AGREEMENT may be terminated by either PARTY upon fourteen (14) days written notice to the other PARTY in the event of a substantial failure of performance by such other PARTY, including insolvency of CONSULTANT; or if the DISTRICT should decide to abandon or indefinitely postpone the PROJECT.

2. In the event of a termination based upon abandonment or postponement by DISTRICT, the DISTRICT shall pay to the CONSULTANT for all services performed and all expenses incurred under this AGREEMENT supported by documentary evidence, including payroll records, and expense reports up until the date of the abandonment or postponement plus any sums due the CONSULTANT for Board approved extra services. In ascertaining the services actually rendered hereunder up to the date of termination of this AGREEMENT, consideration shall be given to both completed work and work in process of completion and to complete and incomplete drawings and other documents whether delivered to the DISTRICT or in the possession of the CONSULTANT. In the event termination is for a substantial failure of performance, all damages and costs associated with the termination, including increased consultant and replacement consultant costs shall be deducted from payments to the CONSULTANT.

3. In the event a termination for cause is determined to have been made wrongfully or without cause, then the termination shall be treated as a termination for convenience in accordance with Article IV, Paragraph 4 below, and CONSULTANT shall have no greater rights than it would have had if a termination for convenience had been effected in the first instance. No other loss, cost, damage, expense or liability may be claimed, requested or recovered by CONSULTANT.

4. This AGREEMENT may be terminated without cause by DISTRICT upon twenty (20) days written notice to the CONSULTANT. In the event of a termination without cause, the DISTRICT shall pay to the CONSULTANT for all services performed and all expenses incurred under this AGREEMENT supported by documentary evidence, including payroll records, and expense reports up until the date of notice of termination plus any sums due the CONSULTANT for Board approved extra services. In ascertaining the services actually rendered hereunder up to the date of termination of this AGREEMENT, consideration shall be given to both completed work and work in process of completion and to other documents whether delivered to the DISTRICT or in the possession of the CONSULTANT.

5. In the event of a dispute between the PARTIES as to performance of the work or the interpretation of this AGREEMENT, or payment or nonpayment for work performed or not performed, the PARTIES shall attempt to resolve the dispute. Pending resolution of this dispute, CONSULTANT agrees to continue the work diligently to completion. If the dispute is not resolved, CONSULTANT agrees it will neither rescind the AGREEMENT nor stop the progress of the work, but CONSULTANT's sole remedy shall be to submit such controversy to determination by a court having competent jurisdiction of the dispute, after the PROJECT has been completed, and not before. The PARTIES may agree in writing to submit any dispute between the PARTIES to arbitration. The DISTRICT agrees to pay the CONSULTANT the undisputed amounts due under this AGREEMENT.

6. The PARTIES understand and agree that Article IV of this AGREEMENT shall govern all termination rights and procedures between the PARTIES. Any termination provision that is attached to this AGREEMENT as an Exhibit shall be void and unenforceable between the PARTIES.

ARTICLE V
ADDITIONAL CONSULTANT SERVICES

1. CONSULTANT shall notify the DISTRICT in writing of the need for additional services required due to circumstances beyond the CONSULTANT's control. CONSULTANT shall obtain written authorization from the DISTRICT before rendering such services. The DISTRICT may require CONSULTANT to perform additional services which are, in the DISTRICT's discretion, necessary. Compensation for such services shall be negotiated and approved in writing by the DISTRICT. Such services shall include:

a. Making material revisions in reports or other documents when such revisions are required by the enactment or revision of laws, rules or regulations subsequent to the preparation and completion of such documents.

b. Preparing reports and other documentation and supporting data, and providing other services in connection with PROJECT modifications required by causes beyond the control of the CONSULTANT which are not the result of the direct or indirect negligence, errors or omissions on the part of CONSULTANT;

c. If the DISTRICT requests additional shifts to complete the services articulated in **EXHIBIT "A"** where the requests for additional shifts does not arise from the direct or indirect negligence, errors or omissions on the part of CONSULTANT and the CONSULTANT's compensation is expressly conditioned on the lack of fault of the CONSULTANT;

d. Providing any other services not otherwise included in this AGREEMENT or not customarily furnished in accordance with the generally accepted practice in the CONSULTANT's industry.

ARTICLE VI
ACCOUNTING RECORDS OF THE CONSULTANT

1. Records of the CONSULTANT's direct personnel and reimbursable expenses pertaining to any extra services provided by the CONSULTANT, which are in addition to those services already required by this AGREEMENT, and any records of accounts between the DISTRICT and CONSULTANT shall be kept on a generally recognized accounting basis and shall be available to the DISTRICT or DISTRICT's authorized representative at mutually convenient times.

ARTICLE VII
REPORTS AND/OR OTHER DOCUMENTS

1. The reports and/or other documents that are prepared, reproduced, maintained and/or managed by the CONSULTANT or CONSULTANT's consultants in accordance with this AGREEMENT (regardless of medium, format, etc.) shall be and remain the property of the DISTRICT (hereinafter "PROPERTY"). The DISTRICT may provide the CONSULTANT with a written request for the return of its PROPERTY at any time. Upon CONSULTANT's receipt of the DISTRICT's written request, CONSULTANT shall return the requested PROPERTY to the DISTRICT within five (5) calendar days. Failure to comply with any such written request shall be deemed a material breach of this AGREEMENT.

ARTICLE VIII
INDEMNITY & INSURANCE

1. To the fullest extent permitted by law, CONSULTANT agrees to indemnify, and hold DISTRICT entirely harmless from all liability arising out of:

a. Workers' Compensation and Employers Liability: Any and all claims under Workers' Compensation acts and other employee benefit acts with respect to CONSULTANT's employees or CONSULTANT's subconsultant's employees arising out of CONSULTANT's work under this AGREEMENT; and

b. General Liability: Liability for damages for (1) death or bodily injury to person; (2) injury to, loss or theft of property; (3) any failure or alleged failure to comply with any provision of law or (4) any other loss, damage or expense arising under either (1), (2), or (3) above, sustained by the CONSULTANT or the DISTRICT, or any person, firm or corporation employed by the CONSULTANT or the DISTRICT upon or in connection with the PROJECT, except for liability resulting from the sole or active negligence, or willful misconduct of the DISTRICT, its officers, employees, agents or independent consultants who are directly employed by the DISTRICT;

c. Professional Liability: Any loss, injury to or death of persons or damage to property caused by any act, neglect, default or omission of the CONSULTANT, or any person, firm or corporation employed by the CONSULTANT, either directly or by independent contract, including all damages due to loss or theft, sustained by any person, firm or corporation including the DISTRICT, arising out of, or in any way connected with the services performed by CONSULTANT in accordance with this AGREEMENT, including injury or damage either on or off DISTRICT property; but not for any loss, injury, death or damages caused by the sole or active negligence, or willful misconduct of the DISTRICT.

d. The CONSULTANT, at its own expense, cost, and risk, shall defend any and all claims, actions, suits, or other proceedings, arising out of Article VIII, Paragraphs 1 (a) and (b) above, that may be brought or instituted against the DISTRICT, its officers, agents or employees, on any such claim or liability, and shall pay or satisfy any judgment that may be rendered against the DISTRICT, its officers, agents or employees in any action, suit or other proceedings as a result thereof.

e. The PARTIES understand and agree that Article VIII, Section 1 of this AGREEMENT shall be the sole indemnity, as defined by California Civil Code §2772, governing this AGREEMENT. Any other indemnity that is attached to this AGREEMENT as an Exhibit shall be void and unenforceable between the PARTIES.

f. Any attempt to limit the CONSULTANT's liability to the DISTRICT in an attached Exhibit shall be void and unenforceable between the PARTIES. In no event shall the CONSULTANT's liability be limited to any amount including, but not limited to, the amount of fees received by the CONSULTANT for performing services related to this AGREEMENT.

2. CONSULTANT shall purchase and maintain policies of insurance with an insurer or insurers, qualified to do business in the State of California and acceptable to DISTRICT which will protect CONSULTANT and DISTRICT from claims which may arise out of or result from CONSULTANT's actions or inactions relating to the AGREEMENT, whether such actions or inactions be by themselves or by any subconsultant or by anyone directly or indirectly employed by any of them, or by anyone for whose acts any of them may be liable. The aforementioned insurance shall include coverage for:

a. The CONSULTANT shall carry Workers' Compensation and Employers Liability Insurance in accordance with the laws of the State of California. However, such amount shall not be less than ONE MILLION DOLLARS ($1,000,000).

b. Comprehensive general and auto liability insurance with limits of not less than ONE MILLION DOLLARS ($1,000,000) combined single limit, bodily injury and property damage liability per occurrence, including:

1. Owned, non-owned and hired vehicles;
2. Blanket contractual;
3. Broad form property damage;
4. Products/completed operations; and
5. Personal injury.

c. Professional liability insurance, including contractual liability, with limits of ONE MILLION DOLLARS ($1,000,000), per claim. Such insurance shall be maintained during the term of this AGREEMENT and renewed for a period of at least five (5) years thereafter and/or at rates consistent with the time of execution of this AGREEMENT adjusted for inflation. In the event that CONSULTANT subcontracts any portion of CONSULTANT's duties, CONSULTANT shall require any such subconsultant to purchase and maintain insurance coverage as provided in this subparagraph. Failure to maintain professional liability insurance is a material breach of this AGREEMENT and grounds for immediate termination.

d. Each policy of insurance required in Article VIII, Section 2 (b) above shall name DISTRICT and its officers, agents and employees as additional insureds; shall state that, with respect to the operations of CONSULTANT hereunder, such policy is primary and any insurance carried by DISTRICT is excess and non-contributory with such primary insurance; shall state that not less than thirty (30) days written notice shall be given to DISTRICT prior to cancellation; and, shall waive all rights of subrogation. CONSULTANT shall notify DISTRICT in the event of material change in, or failure to renew, each policy. Prior to commencing work, CONSULTANT shall deliver to DISTRICT certificates of insurance as evidence of compliance with the requirements herein. In the event CONSULTANT fails to secure or maintain any policy of insurance required hereby, DISTRICT may, at its sole discretion, secure such policy of insurance in the name of and for the account of CONSULTANT, and in such event CONSULTANT shall reimburse DISTRICT upon demand for the cost thereof.

ARTICLE IX
MISCELLANEOUS

1. CONSULTANT, in the performance of this AGREEMENT, shall be and act as an independent contractor. CONSULTANT understands and agrees that CONSULTANT and all of CONSULTANT's employees shall not be considered officers, employees or agents of the DISTRICT, and are not entitled to benefits of any kind or nature normally provided employees of the DISTRICT and/or to which DISTRICT's employees are normally entitled, including, but not limited to, State Unemployment Compensation or Workers' Compensation. CONSULTANT assumes the full responsibility for the acts and/or omissions of CONSULTANT's employees or agents as they relate to the services to be provided under this AGREEMENT. CONSULTANT shall assume full responsibility for payment of any applicable prevailing wages and all federal, state and local taxes or contributions, including unemployment insurance, social security and income taxes for the respective CONSULTANT's employees.

2. Nothing contained in this AGREEMENT shall create a contractual relationship with or a cause of action in favor of any third party against either the DISTRICT or CONSULTANT.

3. The DISTRICT and CONSULTANT, respectively, bind themselves, their partners, officers, successors, assigns and legal representatives to the other PARTY to this AGREEMENT with respect to the terms of this AGREEMENT. CONSULTANT shall not assign this AGREEMENT.

4. This AGREEMENT shall be governed by the laws of the State of California.

5. This AGREEMENT shall not include or incorporate the terms of any general conditions, conditions, master agreement or any other boilerplate terms or form documents prepared by the CONSULTANT. The attachment of any such document to this AGREEMENT as **EXHIBIT "A"** shall not be interpreted or construed to incorporate such terms into this AGREEMENT unless the DISTRICT approves of such incorporation in a separate writing signed by the DISTRICT. Any reference to such boilerplate terms and conditions in the proposal or quote submitted by the CONSULTANT shall be null and void and have no effect upon this AGREEMENT. Proposals, quotes, statement of qualifications and other similar documents prepared by the CONSULTANT may be incorporated into this AGREEMENT as **EXHIBIT "A"** but such incorporation shall be strictly limited to those portions describing the CONSULTANT's scope of work, rate and price schedule and qualifications.

6. The PARTIES have had the opportunity to, and have to the extent each deemed appropriate, obtained legal counsel concerning the content and meaning of this AGREEMENT. Each of the PARTIES agrees and represents that no promise, inducement or agreement not herein expressed has been made to effectuate this AGREEMENT. This AGREEMENT represents the entire AGREEMENT between the DISTRICT and CONSULTANT and supersedes all prior negotiations, representations or agreements, either written or oral. This AGREEMENT may be amended or modified only by an agreement in writing signed by both the DISTRICT and the CONSULTANT.

7. The rule of construction that any ambiguities are to be resolved against the drafting PARTY shall not be employed in the interpretation of this AGREEMENT. It is expressly understood and agreed that the PARTIES to this AGREEMENT have participated equally, or have had equal opportunity to participate, in the drafting hereof.

8. Time is of the essence with respect to all provisions of this AGREEMENT.

9. If either PARTY becomes involved in litigation arising out of this AGREEMENT or the performance thereof, each PARTY shall bear its own litigation costs and expenses, including reasonable attorney's fees.

10. All exhibits referenced herein and attached hereto shall be deemed incorporated into and made a part of this AGREEMENT by each reference as though fully set forth in each instance in the text

hereof unless otherwise excluded by the terms of this AGREEMENT. In the event that the provisions of any exhibit conflict with the terms of this AGREEMENT, the terms of this AGREEMENT shall control.

11. This AGREEMENT may be executed in any number of counterparts, each of which shall be deemed an original, and the counterparts shall constitute one and the same instrument, all of which shall be sufficient evidence of this AGREEMENT.

12. Confidentiality. The CONSULTANT shall not disclose or permit the disclosure of any confidential information, except to its agents, employees and other consultants who need such confidential information in order to properly perform their duties relative to this AGREEMENT.

13. Severability. If any portion of this AGREEMENT is held as a matter of law to be unenforceable, the remainder of this AGREEMENT shall be enforceable without such provisions.

14. Notices. All notices or demands to be given under this AGREEMENT by either PARTY to the other shall be in writing and given either by: (a) personal service; or (b) by U.S. Mail, mailed either by registered, overnight, or certified mail, return receipt requested, with postage prepaid. Service shall be considered given when received if personally served or if mailed on the fifth day after deposit in any U.S. Post Office. The address to which notices or demands may be given by either PARTY may be changed by written notice given in accordance with the notice provisions of this Paragraph. At the date of this AGREEMENT, the addresses of the PARTIES are as follows:

To the DISTRICT:
Mt. San Jacinto Community College District
Attn: Beth Gomez
1499 N. State Street
San Jacinto, CA 92583
Telephone:

To the CONSULTANT:
<<Name of Contractor>>
Attn: <<Name>>
<< Address>>
<<City, State, Zip>>
Telephone:
Email:

15. Tobacco Prohibited. Any tobacco use (smoking, chewing, etc.) by anyone, is prohibited at all times on any DISTRICT property.

16. Profanity on any DISTRICT property is prohibited, including, but not limited to, racial, ethic, or sexual slurs or comments which could be considered harassment.

17. Appropriate dress is mandatory. Therefore, tank tops, cut-offs and shorts are not allowed. Additionally, what is written or pictured on clothing must comply with the requirements of acceptable language as stated above in Paragraph 16.

18. Images. If applicable, the CONSULTANT is prohibited from capturing on any visual medium images of any property, logo, student, or employee of the DISTRICT, or any image that represents the DISTRICT without express written consent from the DISTRICT.

19. Prevailing Wages. If applicable and required, CONSULTANT shall pay, and shall cause all subconsultants of every tier to pay, not less than the specified prevailing wage rates, to the extent applicable, to all workers employed to perform work or services under this AGREEMENT. CONSULTANT shall fully indemnify and defend the DISTRICT from any claims arising from CONSULTANT's failure to meet and prevailing wage requirements.

20. In accordance with California Education Code section 81655, this AGREEMENT is not a valid or enforceable obligation against the DISTRICT until approved or ratified by motion of the Governing Board of the DISTRICT duly passed and adopted.

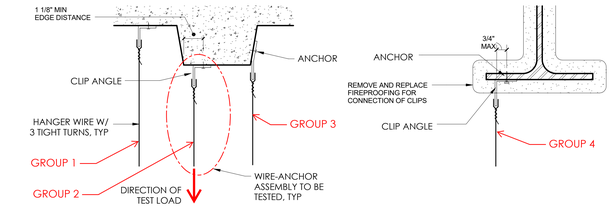
The PARTIES, through their authorized representatives, have executed this AGREEMENT as of the day and year first written above.

<<NAME OF CONSULTANT>>	**MT. SAN JACINTO COMMUNITY COLLEGE DISTRICT**
By_____	By_____
Print Name_____	Print Name_____
Title_____	Title_____
Date_____	Date_____
Address_____	
_____	
Phone_____	
Fax_____	
Tax ID#_____	
Email_____	

EXHIBIT “A”

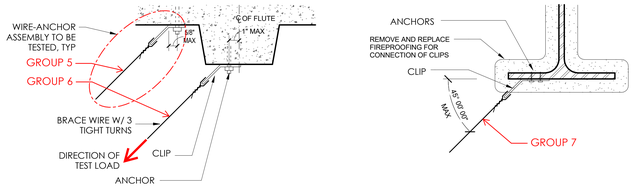
CONSULTANT ‘S WORK PLAN, SCOPE OF SERVICES, AND COMPENSATION

PROPOSED TESTING PROGRAM FOR ATTACHMENTS OF SUSPENDED INTEGRATED CEILINGS AT BUILDING G AT MT. SAN JACINTO COMMUNITY COLLEGUE - TEMECULA VALLEY CAMPUS



A METAL DECK WITH CONCRETE FILL B WIDE FLANGE BEAM

A TYPICAL CEILING HANGER WIRE ATTACHMENT TO STRUCTURE (GROUPS 1-4) - ELEVATION



A METAL DECK WITH CONCRETE FILL B WIDE FLANGE BEAM

B TYPICAL CEILING BRACE WIRE ATTACHMENT TO STRUCTURE (GROUPS 5-7) - ELEVATION

OVERVIEW
- THIS IN-SITU TESTING PROGRAM CONFORMS TO THE REQUIREMENTS OF THE CALIFORNIA BUILDING CODE 2016, SECTION 1708A AND DSA IR 25-2.

- THIS IN-SITU TESTING PROGRAM IS IN SUPPORT OF THE SEISMIC EVALUATION AND UPGRADE OF THE NONSTRUCTURAL COMPONENTS AT BUILDING G.

- THIS IN-SITU TESTING PROGRAM SHALL BE CONDUCTED IN ACCORDANCE WITH THE TEST PROCEDURE IN THESE DRAWINGS AND SHALL BE SUPERVISED BY A REGISTERED DESIGN PROFESSIONAL.

- PRIOR TO TESTING, THE TESTING AGENCY SHALL VISIT AND CONFIRM ALL TESTING LOCATIONS. WHERE TESTING CANNOT BE CONDUCTED, THE TESTING AGENCY SHALL COORDINATE AND SUBMIT ALTERNATE TESTING LOCATIONS TO THE STRUCTURAL ENGINEER OF RECORD (SEOR) FOR REVIEW AND APPROVAL.

- THE TESTING AGENCY OR CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE NONSTRUCTURAL COMPONENT DURING TESTING. SUCH MEASURES SHALL INCLUDE, NOT NOT LIMITED TO, BRACING AND SHORING.

DESCRIPTION OF THE PROCEDURE
- THE TESTING AGENCY SHALL CONFIRM THE TEST LOADS LISTED IN THE SCHEDULE WITH SEOR. TEST LOADS SHALL BE BASED ON THE ANTICIPATED DEMAND OR STRENGTH IN ACCORDANCE WITH AVAILABLE CONSTRUCTION INFORMATION.

DESCRIPTION OF SAMPLES
- THE ANCHORS OR WIRE-ANCHOR ASSEMBLIES TO BE TESTED ARE GRAPHICALLY PREPRESENTED IN DETAILS A/- AND B/-.

- IF THE WIRE-ANCHOR ASSEMBLY PER DETAILS A/- AND B/- CANNOT BE TESTED, A PULL OUT TEST OF THE ANCHOR SHALL BE CONDUCTED AT MINIMUM.

- REPRESENTATIVE ANCHORS OR WIRE-ANCHOR ASSEMBLIES FOR TESTING SHALL BE SELECTED NEAR THE LOCATIONS INDICATED PER PLAN.

- TESTING SHALL BE CONDUCTED ON EXISTING NONSTRUCTURAL COMPONENTS WHICH ARE PLANNED TO BE DEMOLISHED AS PART OF A FUTURE TENANT IMPROVEMENT PROJECT. CONFIRM AREA OF DEMOLITION WITH ARCHITECT OF RECORD (AOR) PRIOR TO START OF IN-SITE TESTING PROGRAM.

- THE TESTING AGENCY OR CONTRACTOR SHALL VERIFY THAT THE TEST SAMPLES DO NOT HAVE ANY NOTCHING, DEFECTS OR ANY OTHER TYPE OF DAMAGE THAT WOULD ADVERSELY AFFECT THE TENSILE TESTS.

- ANCHORS OR WIRE-ANCHOR ASSEMBLIES ARE CLASSIFIED IN GROUPS OF SIMILAR TYPE, SIZE, GEOMETRY, AND STRUCTURAL USE. SEE DETAILS A/- AND B/- FOR GROUPS SELECTED FOR THIS TESTING PROGRAM.

NUMBER OF SAMPLES
- A MINIMUM OF THREE (3) ANCHORS OR WIRE-ANCHOR ASSEMBLIES PER GROUP SHALL BE TESTED IN PLACE IN TENSION.

PROPERTIES TO BE DETERMINED
- CONDUCT TESTS TO ESTABLISH AN AVAILABLE STRENGTH, CONSTRUCTION QUALITY OR BOTH.

INTERPRETATION OF TEST DATA
- TESTING IN ACCORDANCE TO THIS PROGRAM SHALL BE PERMITTED TO DETERMINE THE AVAILABLE STRENGTH BASED ON A STATISTICAL DISTRIBUTION OF THE TEST RESULTS.

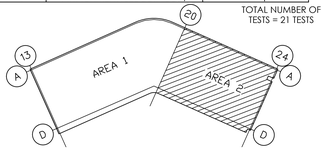
REPORT OF TESTING
THE TESTING AGENCY SHALL PREPARE A REPORT, STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA, STATING THE FOLLOWING INFORMATION RELATING TO THE TESTING:

1. A LIST OF ALL SAMPLES TAKEN FROM THE BUILDING, INCLUDING TAG NUMBER, LOCATION OF STRUCTURAL ELEMENT, TYPE OF ELEMENT, TYPE OF MATERIAL, ORIGINAL DATE OF CONSTRUCTION, TYPE OF SAMPLE;
2. A SUMMARY OF TESTED MATERIAL PROPERTIES FOR EACH SAMPLE;
3. A DESCRIPTION OF TEST METHODS UTILIZED FOR EACH TEST AND SAMPLE TYPE;
4. RAW TEST DATA;
5. PHOTOGRAPHS OF EACH TEST SPECIMEN.

TESTING PROGRAM SCHEDULE

SAMPLE ID	SYMBOL	LOCATION	DESCRIPTION	TEST LOAD	NUMBER OF TESTS
G1-1, G1-2, G1-3	■	BLDG G, LVL 2, CEILING	GROUP 1 (CEILING HANGER WIRE ATTACHMENT, SEE DETAIL A/-)	200 LBS	3
G2-1, G2-2, G2-3	●	BLDG G, LVL 2, CEILING	GROUP 1 (CEILING HANGER WIRE ATTACHMENT, SEE DETAIL A/-)	200 LBS	3
G3-1, G3-2, G3-3	✚	BLDG G, LVL 2, CEILING	GROUP 1 (CEILING HANGER WIRE ATTACHMENT, SEE DETAIL A/-)	200 LBS	3
G4-1, G4-2, G4-3	✖	BLDG G, LVL 2, CEILING	GROUP 1 (CEILING HANGER WIRE ATTACHMENT, SEE DETAIL A/-)	200 LBS	3
G5-1, G5-2, G5-3	▲	BLDG G, LVL 2, CEILING	GROUP 2 (CEILING BRACE WIRE ATTACHMENT, SEE DETAIL B/-)	500 LBS	3
G6-1, G6-2, G6-3	■	BLDG G, LVL 2, CEILING	GROUP 2 (CEILING BRACE WIRE ATTACHMENT, SEE DETAIL B/-)	500 LBS	3
G7-1, G7-2, G7-3	●	BLDG G, LVL 2, CEILING	GROUP 2 (CEILING BRACE WIRE ATTACHMENT, SEE DETAIL B/-)	500 LBS	3

TOTAL NUMBER OF TESTS = 21 TESTS



TG KEY PLAN

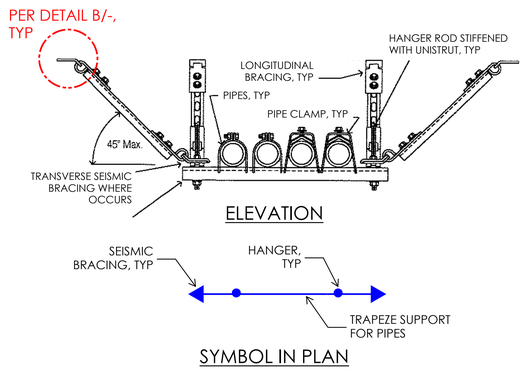
NORTH

Abbott
A Promise for Life

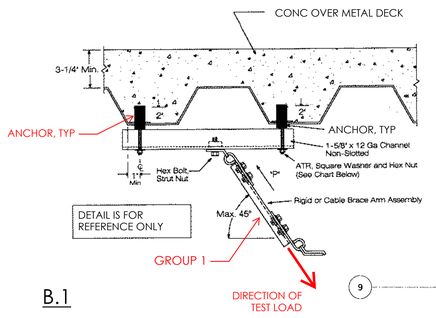
COL. A-D, 20-24,
FACILITY EQUIPMENT PLAN
SECOND FLOOR - AREA 2

NO	DATE	REVISION	DRAFTER	ENGINEER
A	11-21-08	AS-BUILT PER PROJECT #614424	A.TODANKAR	P.GOYAL

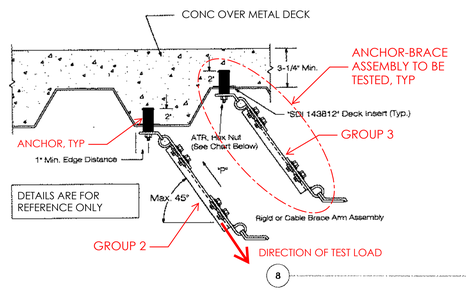
DRAWN	CHECK	ENGINEER	DATE	SCALE	FILE NO.	DRWG. NO.	REV. NO.
ANAY	MANGESH	PANKAJ	11-21-08	1/8" = 1'	tg37202.dwg	E-TG-372-02	A



A REFERENCE DETAIL FOR ATTACHMENT & BRACING OF PIPES

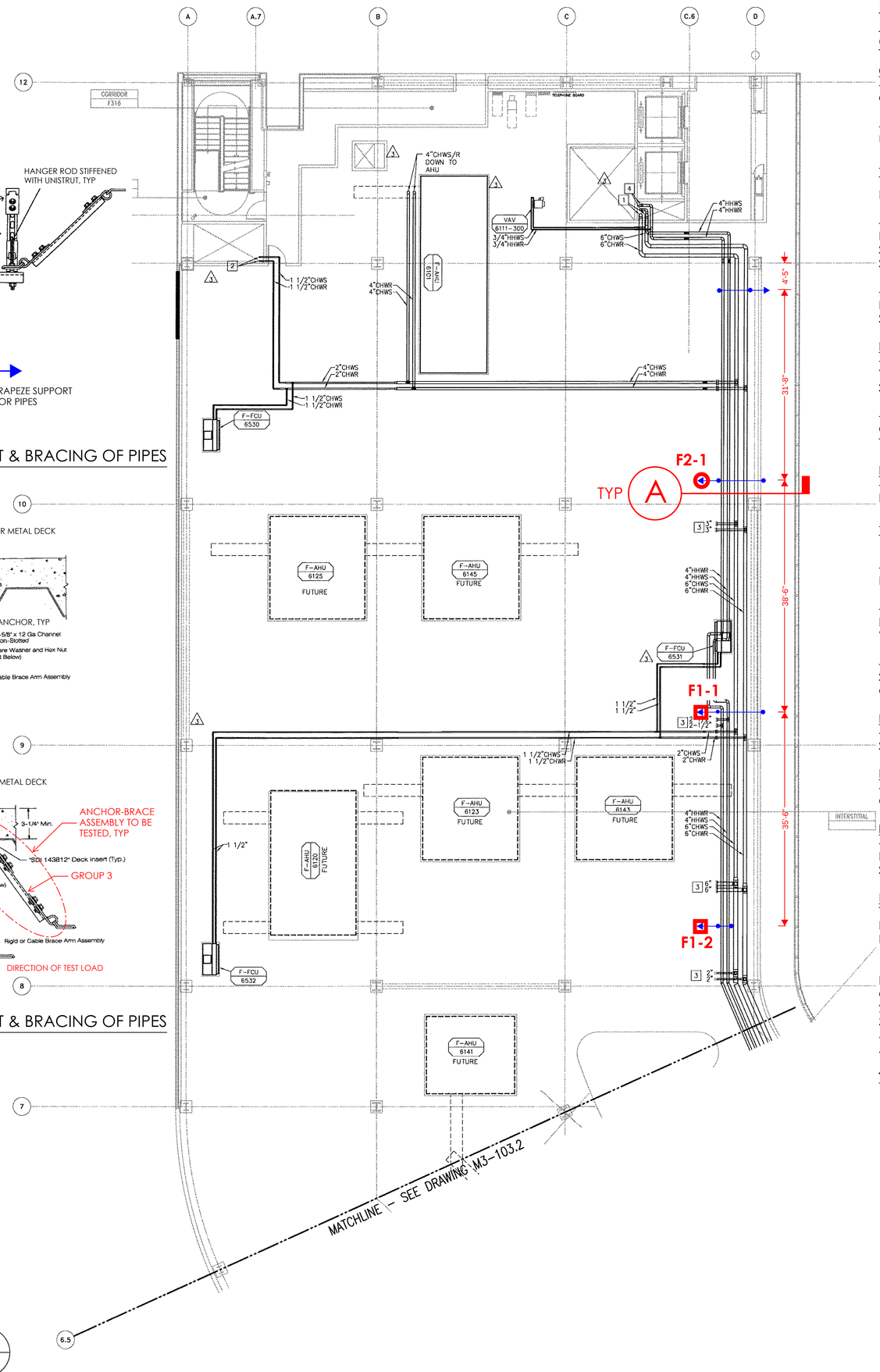


B.1



B.2

B REFERENCE DETAIL FOR ATTACHMENT & BRACING OF PIPES



1 FLOOR PLAN: LEVEL 3 PIPING BUILDING F (NORTH)

XX-XXX REF. SCALE: 1/8" = 1'-0"

PLAN NORTH

PROPOSED TESTING PROGRAM FOR SEISMIC BRACING OF PIPING AT BUILDING F AT MT. SAN JACINTO COMMUNITY COLLEGUE - TEMECULA VALLEY CAMPUS

OVERVIEW

- THIS IN-SITU TESTING PROGRAM CONFORMS TO THE REQUIREMENTS OF THE CALIFORNIA BUILDING CODE 2016, SECTION 1708A.

- THIS IN-SITU TESTING PROGRAM IS IN SUPPORT OF THE SEISMIC EVALUATION AND UPGRADE OF THE NONSTRUCTURAL COMPONENTS AT BUILDING F.

- THIS IN-SITU TESTING PROGRAM SHALL BE CONDUCTED IN ACCORDANCE WITH THE TEST PROCEDURE IN THESE DRAWINGS AND SHALL BE SUPERVISED BY A REGISTERED DESIGN PROFESSIONAL.

- PRIOR TO TESTING, THE TESTING AGENCY SHALL VISIT AND CONFIRM ALL TESTING LOCATIONS. WHERE TESTING CANNOT BE CONDUCTED, THE TESTING AGENCY SHALL COORDINATE AND SUBMIT ALTERNATE TESTING LOCATIONS TO THE STRUCTURAL ENGINEER OF RECORD (SEOR) FOR REVIEW AND APPROVAL.

- THE TESTING AGENCY OR CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE NONSTRUCTURAL COMPONENT DURING TESTING. SUCH MEASURES SHALL INCLUDE, NOT NOT LIMITED TO, BRACING AND SHORING.

DESCRIPTION OF THE PROCEDURE

- THE TESTING AGENCY SHALL CONFIRM THE TEST LOADS LISTED IN THE SCHEDULE WITH SEOR. TEST LOADS SHALL BE BASED ON THE ANTICIPATED DEMAND OR STRENGTH IN ACCORDANCE WITH AVAILABLE CONSTRUCTION INFORMATION.

- IF A TEST RESULTS IN FAILURE OF THE EXISTING ANCHOR (i.e., VISIBLE MOVEMENT OF ANCHOR OR PULL OUT), THE CONTRACTOR SHALL IMMEDIATELY TERMINATE THE TEST AND INFORM THE SEOR BEFORE RESUMING THE TEST PROGRAM.

DESCRIPTION OF SAMPLES

- THE ANCHORS OR ANCHOR-BRACE ASSEMBLIES TO BE TESTED ARE GRAPHICALLY PREPRESENTED IN DETAILS A/- AND B/-.

- IF THE ANCHOR-BRACE ASSEMBLY PER DETAIL B CANNOT BE TESTED, A PULL OUT TEST OF THE ANCHOR SHALL BE CONDUCTED AT MINIMUM.

- REPRESENTATIVE ANCHORS OR ANCHOR-BRACE ASSEMBLIES FOR TESTING SHALL BE SELECTED NEAR THE LOCATIONS INDICATED PER PLAN.

- THE TESTING AGENCY OR CONTRACTOR SHALL VERIFY THAT THE TEST SAMPLES DO NOT HAVE ANY NOTCHING, DEFECTS OR ANY OTHER TYPE OF DAMAGE THAT WOULD ADVERSELY AFFECT THE TENSILE TESTS.

- ANCHORS OR ANCHOR-BRACE ASSEMBLIES ARE CLASSIFIED IN GROUPS OF SIMILAR TYPE, SIZE, GEOMETRY, AND STRUCTURAL USE. SEE DETAILS A/- AND B/- FOR GROUPS SELECTED FOR THIS TESTING PROGRAM.

NUMBER OF SAMPLES

- A MINIMUM OF THREE (3) ANCHORS OR ANCHOR-BRACE ASSEMBLIES PER GROUP SHALL BE TESTED IN PLACE IN TENSION.

PROPERTIES TO BE DETERMINED

- CONDUCT TESTS TO ESTABLISH AN AVAILABLE STRENGTH, CONSTRUCTION QUALITY OR BOTH.

INTERPRETATION OF TEST DATA

- TESTING IN ACCORDANCE TO THIS PROGRAM SHALL BE PERMITTED TO DETERMINE THE AVAILABLE STRENGTH BASED ON A STATISTICAL DISTRIBUTION OF THE TEST RESULTS.

REPORT OF TESTING

THE TESTING AGENCY SHALL PREPARE A REPORT, STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA, STATING THE FOLLOWING INFORMATION RELATING TO THE TESTING:

1. A LIST OF ALL SAMPLES TAKEN FROM THE BUILDING, INCLUDING TAG NUMBER, LOCATION OF STRUCTURAL ELEMENT, TYPE OF ELEMENT, TYPE OF MATERIAL, ORIGINAL DATE OF CONSTRUCTION, TYPE OF SAMPLE;
2. A SUMMARY OF TESTED MATERIAL PROPERTIES FOR EACH SAMPLE;
3. A DESCRIPTION OF TEST METHODS UTILIZED FOR EACH TEST AND SAMPLE TYPE;
4. RAW TEST DATA;
5. PHOTOGRAPHS OF EACH TEST SPECIMEN.

TESTING PROGRAM SCHEDULE

SAMPLE ID	SYMBOL	LOCATION	DESCRIPTION	TEST LOAD	NUMBER OF TESTS
F1-1, F1-2, F1-3	■	BLDG F, LVL 3, CEILING	GROUP 1 (SEE DETAIL B/-)	3959 lb (4 Pipes) 1943 lb (2 Pipes)	3
F2-1, F2-2, F2-3	●	BLDG F, LVL 3, CEILING	GROUP 2 (SEE DETAIL B/-)	No Group 2	3
F3-1, F3-2, F3-3	✚	BLDG F, LVL 3, CEILING	GROUP 3 (SEE DETAIL B/-)	No Group 3	3

TOTAL NUMBER OF TESTS = 9 TESTS

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3774
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4706

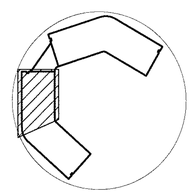
REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
4	03-22-07	FOR CONSTRUCTION
3	01-31-07	MISC. REVISIONS
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO:
DRAWN BY:
CHECKED BY:
1/8"=1'-0"
04-04-06

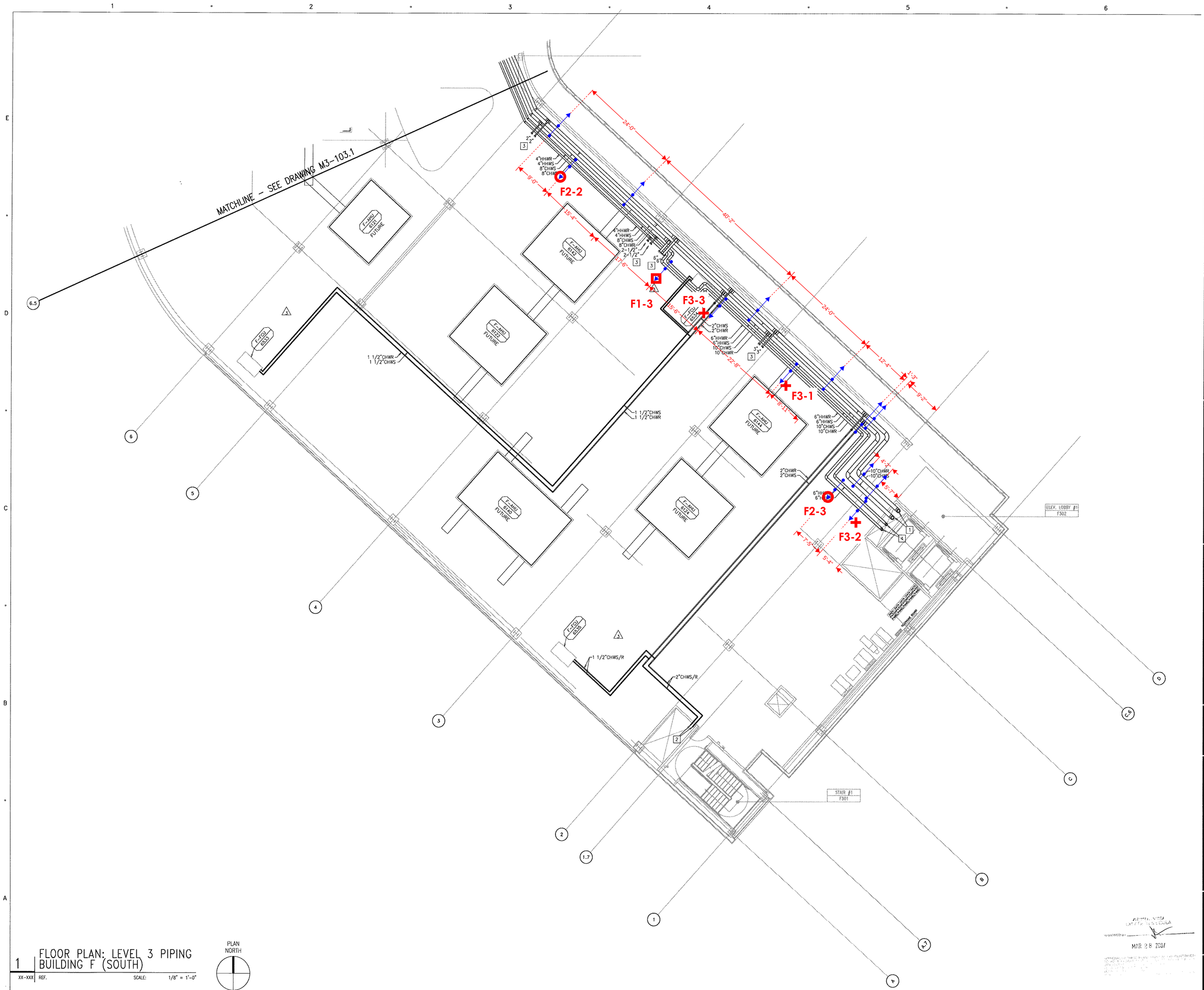
KEY PLAN



SHEET TITLE

FLOOR PLAN: LEVEL 3 PIPING BUILDING F (NORTH)

M3-103.1



1 FLOOR PLAN: LEVEL 3 PIPING BUILDING F (SOUTH)

XX-XXX REF. SCALE: 1/8" = 1'-0"

PLAN NORTH

MAR 28 2007

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
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2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

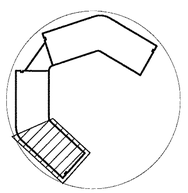
REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
4	03-22-07	FOR CONSTRUCTION
3	01-31-07	MISC. REVISIONS
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO:
DRAWN BY:
CHECKED BY:
1/8"=1'-0"
04-04-06

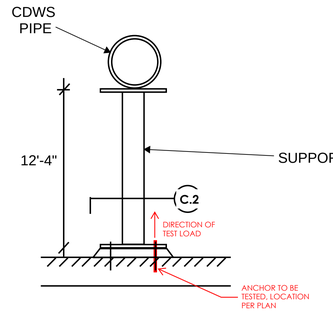
KEY PLAN



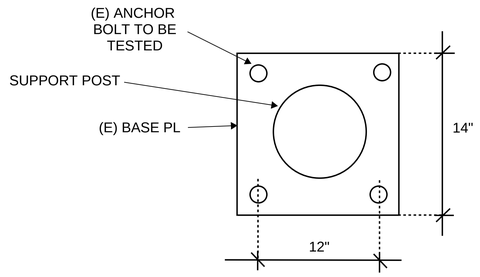
SHEET TITLE

FLOOR PLAN: LEVEL 3 PIPING BUILDING F (SOUTH)

M3-103.2

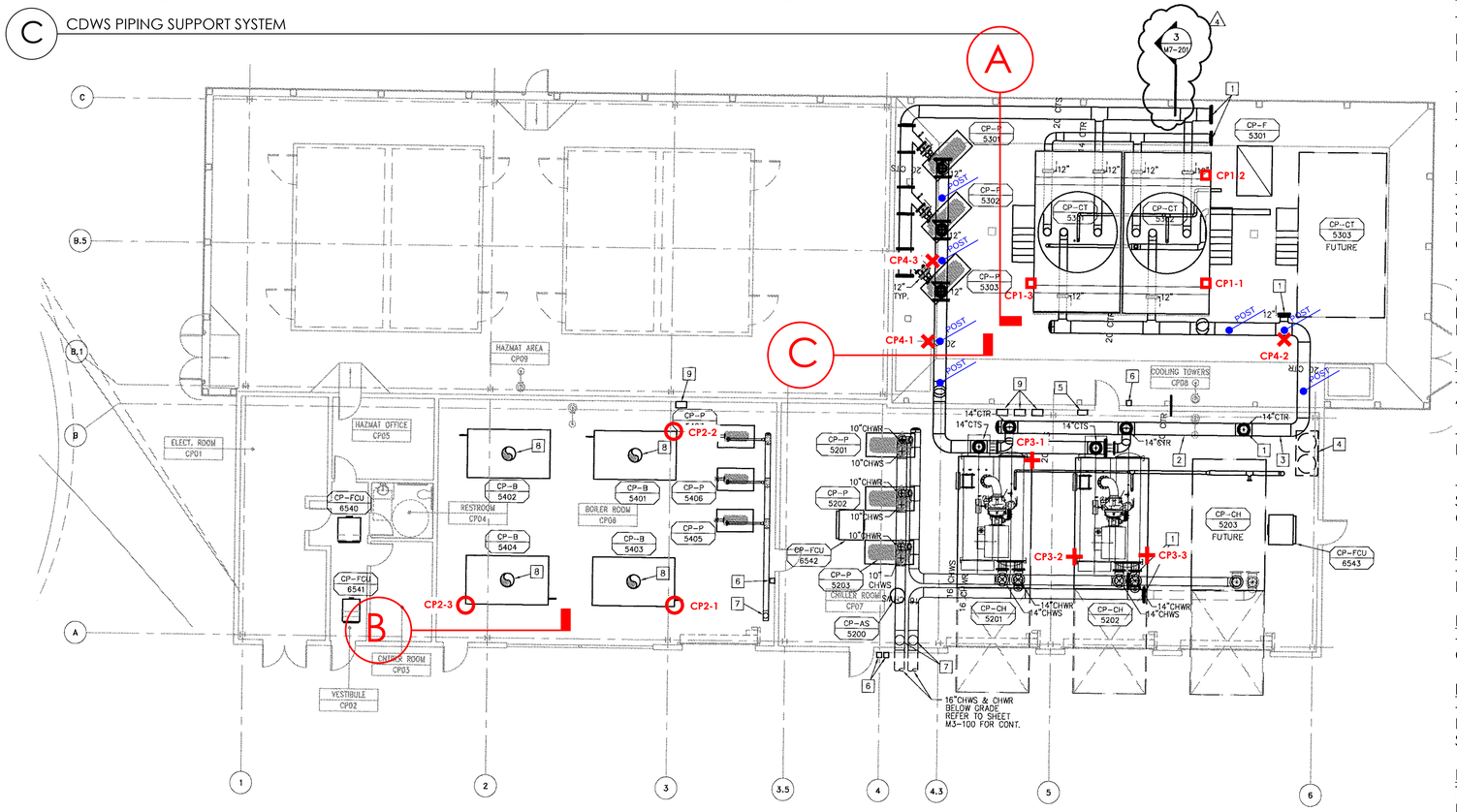


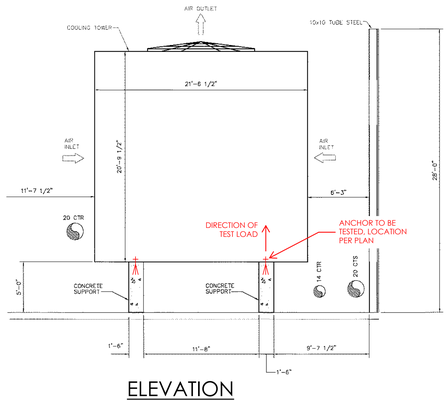
C.1 ELEVATION



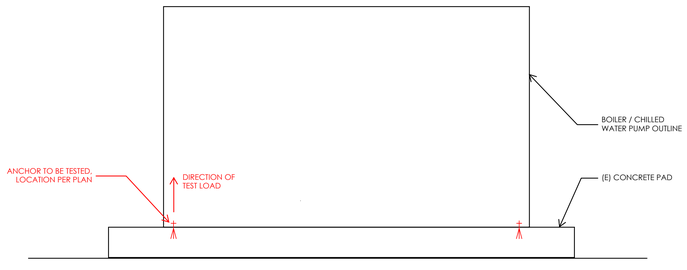
C.2 BASE PL TYP

C CDWS PIPING SUPPORT SYSTEM





ELEVATION



ELEVATION

A REFERENCE DETAIL FOR COOLING TOWER ANCHORAGE

B REFERENCE DETAIL FOR BOILER OR CHILLED WATER PUMP ANCHORAGE

1 FLOOR PLAN: LEVEL 1 PIPING – CENTRAL PLANT
REF. SCALE: 1/8" = 1'-0"

TRUE NORTH

PROPOSED TESTING PROGRAM FOR SEISMIC ANCHORAGE OF MEP EQUIPMENT AT CENTRAL PLANT AT MT. SAN JACINTO COMMUNITY COLLEGUE - TEMECULA VALLEY CAMPUS

OVERVIEW

- THIS IN-SITU TESTING PROGRAM CONFORMS TO THE REQUIREMENTS OF THE CALIFORNIA BUILDING CODE 2016, SECTION 1708A.

- THIS IN-SITU TESTING PROGRAM IS IN SUPPORT OF THE SEISMIC EVALUATION AND UPGRADE OF THE NONSTRUCTURAL COMPONENTS AT BUILDING G.

- THIS IN-SITU TESTING PROGRAM SHALL BE CONDUCTED IN ACCORDANCE WITH THE TEST PROCEDURE IN THESE DRAWINGS AND SHALL BE SUPERVISED BY A REGISTERED DESIGN PROFESSIONAL.

- PRIOR TO TESTING, THE TESTING AGENCY SHALL VISIT AND CONFIRM ALL TESTING LOCATIONS. WHERE TESTING CANNOT BE CONDUCTED, THE TESTING AGENCY SHALL COORDINATE AND SUBMIT ALTERNATE TESTING LOCATIONS TO THE STRUCTURAL ENGINEER OF RECORD (SEOR) FOR REVIEW AND APPROVAL.

- THE TESTING AGENCY OR CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE NONSTRUCTURAL COMPONENT DURING TESTING. SUCH MEASURES SHALL INCLUDE, NOT NOT LIMITED TO, BRACING AND SHORING.

DESCRIPTION OF THE PROCEDURE

- THE TESTING AGENCY SHALL CONFIRM THE TEST LOADS LISTED IN THE SCHEDULE WITH SEOR. TEST LOADS SHALL BE BASED ON THE ANTICIPATED DEMAND OR STRENGTH IN ACCORDANCE WITH AVAILABLE CONSTRUCTION INFORMATION.

- IF A TEST RESULTS IN FAILURE OF THE EXISTING ANCHOR (i.e., VISIBLE MOVEMENT OF ANCHOR OR PULL OUT), THE CONTRACTOR SHALL IMMEDIATELY TERMINATE THE TEST AND INFORM THE SEOR BEFORE RESUMING THE TEST PROGRAM.

DESCRIPTION OF SAMPLES

- THE ANCHORS TO BE TESTED ARE GRAPHICALLY PREPRESENTED IN DETAIL A/-, B/- AND C/-.

- REPRESENTATIVE ANCHORS FOR TESTING SHALL BE SELECTED NEAR THE LOCATIONS INDICATED PER PLAN.

- THE TESTING AGENCY OR CONTRACTOR SHALL VERIFY THAT THE TEST SAMPLES DO NOT HAVE ANY NOTCHING, DEFECTS OR ANY OTHER TYPE OF DAMAGE THAT WOULD ADVERSELY AFFECT THE TENSILE TESTS.

NUMBER OF SAMPLES

- A MINIMUM OF THREE (3) ANCHORS PER GROUP SHALL BE TESTED IN PLACE IN TENSION.

PROPERTIES TO BE DETERMINED

- CONDUCT TESTS TO ESTABLISH AN AVAILABLE STRENGTH, CONSTRUCTION QUALITY OR BOTH.

INTERPRETATION OF TEST DATA

- TESTING IN ACCORDANCE TO THIS PROGRAM SHALL BE PERMITTED TO DETERMINE THE AVAILABLE STRENGTH BASED ON A STATISTICAL DISTRIBUTION OF THE TEST RESULTS.

REPORT OF TESTING

THE TESTING AGENCY SHALL PREPARE A REPORT, STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA, STATING THE FOLLOWING INFORMATION RELATING TO THE TESTING:

1. A LIST OF ALL SAMPLES TAKEN FROM THE BUILDING, INCLUDING TAG NUMBER, LOCATION OF STRUCTURAL ELEMENT, TYPE OF ELEMENT, TYPE OF MATERIAL, ORIGINAL DATE OF CONSTRUCTION, TYPE OF SAMPLE;
2. A SUMMARY OF TESTED MATERIAL PROPERTIES FOR EACH SAMPLE;
3. A DESCRIPTION OF TEST METHODS UTILIZED FOR EACH TEST AND SAMPLE TYPE;
4. RAW TEST DATA;
5. PHOTOGRAPHS OF EACH TEST SPECIMEN.

TESTING PROGRAM SCHEDULE

SAMPLE ID	SYMBOL	LOCATION	DESCRIPTION	TEST LOAD	NUMBER OF TESTS
CP1-1, CP1-2, CP1-3	□	CENTRAL PLANT, LVL 1	COOLING TOWER ANCHORAGE (SEE DETAIL A/-)	6236 lb	3
CP2-1, CP2-2, CP2-3	○	CENTRAL PLANT, LVL 1	BOILER ANCHORAGE (SEE DETAIL B/-)	2404 lb	3
CP3-1, CP3-2, CP3-3	+	CENTRAL PLANT, LVL 1	CHILLED WATER PUMP ANCHORAGE (SEE DETAIL B/-)	3485 lb	3
CP4-1, CP4-2, CP4-3	×	CENTRAL PLANT, CEILING LVL 1	CDWS PIPING SUPPORT SYSTEM (SEE DETAIL C/-)	2926 lb	3

TOTAL NUMBER OF TESTS = 12 TESTS

OWNER/CLIENT

Abbott Vascular
TEMECULA EAST CAMPUS

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.448.3111

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4706

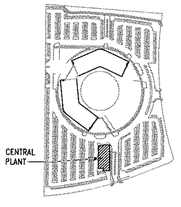
REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
4	03-22-07	FOR CONSTRUCTION
3	01-31-07	MISC. REVISIONS
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO:
DRAWN BY:
CHECKED BY:
1/8"=1'-0"
04-04-06

KEY PLAN

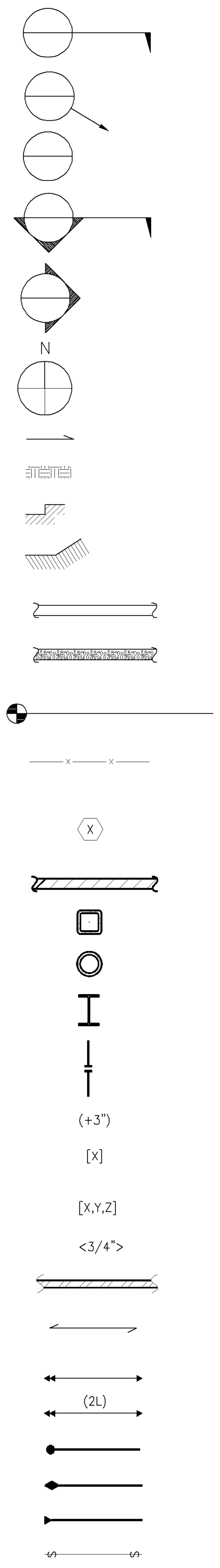


SHEET TITLE

FLOOR PLAN: LEVEL 1 PIPING CENTRAL PLANT

M3-401

SYMBOLS



- SECTION REFERENCE BUBBLE
- DETAIL REFERENCE BUBBLE WITH ARROW
- DETAIL REFERENCE BUBBLE
- FULL HEIGHT SECTION INDICATOR
- ELEVATION OF WALL OR FRAME
- NORTH ARROW
- SLOPE
- EARTH LAYER
- STEPPED SURFACE; FLOOR DEPRESSION
- SLOPPED SURFACE
- INDICATES SAND OR GROUT
- INDICATES GRAVEL
- TOP OF SLAB ELEVATION
- WELDED WIRE FABRIC (WWF LAYER)
- FOOTING TYPE
- INDICATES MASONRY WALLS
- STEEL TUBE COLUMN
- STEEL PIPE COLUMN
- WIDE FLANGE STEEL COLUMN
- MEMBER SPLICE
- TOP OF STEEL ± ELEVATION
- NUMBER OF EVENLY SPACED SHEAR STUDS
- SPECIAL STUD SPACING SEE TYPICAL STEEL DETAILS
- BEAM CAMBER AT MID-SPAN
- STEEL IN CROSS SECTION
- DIRECTION OF SPAN
- ANGLE BRACE
- DOUBLE ANGLE BRACE
- DRAG STRUT CONNECTION
- FULL HEIGHT STIFFENER CONNECTION
- MOMENT CONNECTION
- STEPPED FOOTING

ABBREVIATIONS

AB	ANCHOR BOLT
ABV	ABOVE
ACI	AMERICAN CONCRETE INSTITUTE
ADDL	ADDITIONAL
ADJ	ADJACENT
AESS	ARCHITECTURAL EXPOSED STRUCTURAL STEEL
AGGR	AGGREGATE
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ALT	ALTERNATE
ALUM	ALUMINUM
ANCH	ANCHOR
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
APA	AMERICIAN PLYWOOD ASSOCIATION
APPVD	APPROVED
APPROX	APPROXIMATE
ARCH	ARCHITECTURAL; ARCHITECT
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWPA	AMERICAN WOOD PRESERVERS ASSOCIATION
AWS	AMERICAN WELDING SOCIETY
AITC	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS
&	AND
@	AT
BLDG	BUILDING
BLK	BLOCK
BLKG	BLOCKING
BLW	BELOW
BM	BEAM
BN	BOUNDARY NAIL
BNDRY	BOUNDARY
BOT OR B	BOTTOM
BRC	BRACE
BRG	BEARING
BT	BENT
BTWN	BETWEEN
CANT	CANTILEVER
CAM OR C	CAMBER
CC	CENTER TO CENTER
CG	CENTER OF GRAVITY
CIP	CAST-IN-PLACE
CJ	CONSTRUCTION JOINT; CONTROL JOINT
CL	CENTER LINE
CLR	CLEARANCE; CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
COMP	COMPRESSION
CONC	CONCRETE
CONN	CONNECTION; CONNECT
CONSTR	CONSTRUCTION
CONT	CONTINUE; CONTINUOUS
CONTR	CONTRACTOR
CJP	COMPLETE JOINT PENETRATION WELD
CTR	CENTER
CTSK	COUNTERSINK; COUNTERSUNK
CU FT	CUBIC FOOT
d	PENNY (NAIL OR BAR DIA)
DBL	DOUBLE
DEPT	DEPARTMENT
DET	DETAIL
DF	DOUGLAS FIR/LARCH
DIA OR Ø	DIAMETER
DIAG	DIAGONAL
DIAPH	DIAPHRAGM
DIM	DIMENSION
DN	DOWN
DO	DITTO (REPEAT)
DWG	DRAWING
DWL	DOWEL
EA	EACH
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
EMBED	EMBEDMENT
EN	EDGE NAIL
ENGR	ENGINEER
EQ	EQUAL OR EQUIVALENT
EQUIP	EQUIPMENT
ES	EACH SIDE
ETC	ET CETERA
EW	EACH WAY
EXIST or (E)	EXISTING
EXT	EXTERIOR
FDN	FOUNDATION
FF	FAR FACE
FF	FINISHED FLOOR
FIN	FINISH
FJ	FLOOR JOIST
FL	FLOOR LINE
FLG	FLANGE
FLR	FLOOR
FN	FIELD NAIL
FOC	FACE OF CONCRETE
FOM	FACE OF MASONARY
FOS	FACE OF STUD
FOW	FACE OF WALL
FP	FULL PENETRATION; FIRE PROOFING
FRMG	FRAMING
FS	FULL SIZE; FAR SIDE
FT	FOOT; FEET
FTG	FOOTING
GA	GAUGE
GALV	GALVANIZED
GB	GRADE BEAM
GLB	GLUED LAMINATED BEAM
GR	GRADE
GRND	GROUND
H or HORIZ	HORIZONTAL
HDR	HEADER
HGR	HANGER
HGT	HEIGHT
HOSP	HOSPITAL
HP	HIGH POINT
HS	HIGH STRENGTH
HSH	HORIZONTALLY SLOTTED HOLES
HT	HEIGHT
HR	HARD ROCK
ID	INSIDE DIAMETER
IF	INSIDE FACE
I-JST	I-JOIST
IN	INCH
INCL	INCLUDE
INFO	INFORMATION
INSP	INSPECTION
INT	INTERIOR
JST	JOIST
JT	JOINT
K	KIPS
KSI	KIPS PER SQUARE INCH
LAB	LABORATORY
LB(S) OR #	POUND(S)
LF	LINEAL FOOT
LIN	LINEAL; LINEAR
LLBB	LONG LEGS BACK-TO-BACK
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LP	LOW POINT
LSL	LONG SLOTTED HOLES
LT WT	LIGHTWEIGHT
LVL	LEVEL
MAS	MASONRY
MATL	MATERIAL
MAX	MAXIMUM
MB	MACHINE BOLT
MC	MISCELLANEOUS CHANNEL SHAPE
MECH	MECHANICAL
MFR	MANUFACTURER
MIN	MINIMUM; MINUTE
MISC	MISCELLANEOUS
(N)	NEW
N	NORTH
NF	NEAR FACE
NIC	NOT IN CONTRACT
NORM	NORMAL
NO or #	NUMBER
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OH	OPPOSITE HAND
OPNG	OPENING
OPP	OPPOSITE
ORIG	ORIGINAL
OSB	ORIENTED STRAND BOARD
PARA OR //	PARALLEL
PC	PRECAST; PIECE
PERP	PERPENDICULAR
PI	PLYWOOD INDEX
℄	PLATE
PL	PROPERTY LINE
PLF	POUNDS PER LINEAL FOOT
PLCS	PLACES
PLY	PLYWOOD
PROP	PROPERTY
PT	POST TENSIONED
PW	PLATE WASHER
PJP	PARTIAL JOINT PENETRATION WELD
PREFAB	PREFABRICATED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PVC	POLYVINYL CHLORIDE
PVMT	PAVEMENT
#	POUND; NUMBER
REF	REFERENCE
REINF	REINFORCE; REINFORCING
REQD	REQUIRED
RF	ROOF
RJ	ROOF JOIST
RW	RETAINING WALL
ø	ROUND; DIAMETER
SCHED	SCHEDULE
SECT	SECTION
SEP	SEPERATION
SHT	SHEET
SHTG	SHEATHING
SIM	SIMILAR
SIMP	SIMPSON CONNECTOR
SLBB	SHORT LEGS BACK-TO-BACK
SOG	SLAB ON GRADE
SN	SHEAR NAIL
SPCG	SPACING
SPECS	SPECIFICATIONS
SPCL	SPECIAL
SQ	SQUARE
SS	SELECT STRUCTURAL
SSL	SHORT SLOTTED HOLES
STAGG	STAGGER
STD	STANDARD
STGR	STAGGER
STIFF	STIFFENERS
STIRR	STIRRUP
STL	STEEL
STRUCT	STRUCTURAL
STRUCT I	STRUCTURAL I
SW	SHEAR WALL
SYM	SYMMETRICAL
TB	TIE BEAM
T & B	TOP AND BOTTOM
T & G	TONGUE & GROOVE
TO	TOP OF
TOC	TOP OF CURB; TOP OF CONCRETE
TOF	TOP OF FOOTING
TEMP	TEMPERATURE; TEMPORARY
THRU	THROUGH
THK	THICKNESS/THICK
THR	THREADED
TOP or T	TOP
TOS	TOP OF STEEL
TOW	TOP OF WALL
TSG	TAPPERED STEEL GIRDER
TYP	TYPICAL
UBC	UNIFORM BUILDING CODE
UNO	UNLESS NOTED OTHERWISE
UT	ULTRA-SONIC TEST
VERT	VERTICAL
VSH	VERTICAL SLOTTED HOLES
W	W SHAPE
W/	WITH
W/O	WITHOUT
WD	WOOD
WP	WORK POINT; WATERPROOF
WT	WEIGHT; STRUCTURAL TEE SHAPE
WWF	WELDED WIRE FABRIC

STRUCTURAL STEEL SHAPES

W	W SHAPE
C	AMERICAN STD CHANNEL SHAPE
MC	MISC CHANNEL SHAPE
L	ANGLE SHAPE
WT, ST, MT	STRUCT TEE SHAPE
PIPE	STANDARD PIPE SHAPE
PIPE-X	EXTRA STRONG PIPE SHAPE
PIPE-XX	DBL EXTRA STRONG PIPE SHAPE
TS	STRUCT TUBING SHAPE

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

SYMBOLS & ABBREVIATIONS

S0-101

File: W:\2006\1061030 - Guidant Corporate Campus, Temecula, CA\Sdraft\1061030_S0.101.dwg
Plotted: 3/21/07 at 1:08pm By: SGutierrez
XREFs: 1061030_bdr.dwg

STRUCTURAL NOTES

GENERAL

1. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION. THE ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.

2. ALL DRAWINGS ARE CONSIDERED TO BE A PART OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND COORDINATION OF ALL DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCIES THAT OCCUR SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO START OF CONSTRUCTION SO THAT A CLARIFICATION CAN BE ISSUED. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT THEIR OWN EXPENSE AND AT NO EXPENSE TO THE OWNER OR ARCHITECT.

3. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK.

4. ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE FOLLOWING CODES:

   2001 CBC AND LATEST REVISIONS REFERRED TO HERE AS "THE CODE", AND ANY OTHER REGULATING AGENCIES WHICH HAVE AUTHORITY OVER ANY PORTION OF THE WORK, INCLUDING THE STATE OF CALIFORNIA DIVISION OF INDUSTRIAL SAFETY, AND THOSE CODES & STANDARDS LISTED IN THESE NOTES AND SPECIFICATIONS.

5. SEE ARCHITECTURAL DRAWINGS FOR THE FOLLOWING:

   SIZE AND LOCATION OF ALL DOOR AND WINDOW OPENINGS, EXCEPT AS NOTED.

   SIZE AND LOCATION OF ALL INTERIOR AND EXTERIOR NON-BEARING PARTITIONS.

   SIZE AND LOCATION OF ALL CONCRETE CURBS, EQUIPMENT PADS, PITS, FLOOR DRAINS, SLOPES, DEPRESSED AREAS, CHANGE IN LEVEL, CHAMFERS, GROOVES, INSERTS, ETC.

   SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS EXCEPT AS SHOWN.

   FLOOR AND ROOF FINISHES.

   DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS.

6. SEE MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR THE FOLLOWING:

   PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC., EXCEPT AS SHOWN OR NOTED.

   ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS.

   CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES.

   SIZE AND LOCATION OF MACHINE OR EQUIPMENT BASES, ANCHOR BOLTS FOR MOTOR MOUNTS.

7. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS.

8. OPENINGS, POCKETS, ETC., LARGER THAN 6" SHALL NOT BE PLACED IN CONCRETE SLABS, DECKS, WALLS, UNLESS SPECIALLY DETAILED ON THE STRUCTURAL DRAWINGS. NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS BY OTHERS SHOW OPENINGS, POCKETS, ETC., LARGER THAN 6" NOT SHOWN ON THE STRUCTURAL DRAWINGS, BUT WHICH ARE LOCATED IN STRUCTURAL MEMBERS. FOR ANY FURTHER RESTRICTIONS ON OPENINGS IN STRUCTURAL ELEMENTS, SEE APPLICABLE SECTIONS BELOW.

9. PIPES LARGER THAN 1-1/2" DIAMETER SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE EXCEPT WHERE SPECIFICALLY APPROVED. NO CONDUITS SHALL BE PLACED IN CONCRETE FILL OVER METAL DECKING.

10. ASTM SPECIFICATIONS ON THE DRAWINGS SHALL BE OF THE LATEST REVISION.

11. CONTRACTOR SHALL INVESTIGATE SITE DURING CLEARING AND EARTHWORK OPERATIONS FOR FILLED EXCAVATIONS OR BURIED STRUCTURES, SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC. IF ANY SUCH STRUCTURES ARE FOUND, STRUCTURAL ENGINEER SHALL BE NOTIFIED IMMEDIATELY.

12. CONSTRUCTION MATERIAL SHALL BE SPREAD OUT IF PLACED ON FRAMED ROOF OR FLOOR. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT. PROVIDE ADEQUATE SHORING AND/OR BRACING WHERE STRUCTURE HAS NOT ATTAINED DESIGN STRENGTH.

13. DESIGN LOADS:

LIVE LOADS:

ROOF (ALL BUILDINGS)	20	PSF REDUCIBLE
EXIT CORRIDORS (BLDG F, FLOOR 2-5) (BLDG G, FLOOR 2-5)	100	PSF REDUCIBLE
TYPICAL FLOORS, EXCLUDES PARTITION (BLDG F, FLOOR 2-5) (BLDG G, FLOOR 2-5)	80	PSF REDUCIBLE
MECHANICAL AREAS (BLDG F, FLOOR 3 & ROOF) (BLDG G, ROOF)	125	PSF NON-REDUCIBLE
LAB FLOORS (BLDG F, FLOOR 5)	100	PSF NON-REDUCIBLE
MANUFACTURING FLOORS (BLDG F, FLOOR 2 & 4)	125	PSF NON-REDUCIBLE
LOBBY FLOORS	100	PSF NON-REDUCIBLE
LOBBY ROOF	100	PSF NON-REDUCIBLE

OTHER LOADS:

PARTITION LOAD	20	PSF

14. WIND ANALYSIS PER CHAPTER 16 DIVISION II OF THE CODE BASIC WIND SPEED 70 MPH, EXPOSURE C.

15. SEISMIC ANALYSIS PER CHAPTER 16 DIVISION III OF THE CODE, UTILIZING THE DYNAMIC LATERAL FORCE PROCEDURE

    GROUND MOTION: SITE SPECIFIC RESPONSE SPECTRA PER SOILS REPORT

    Z = 0.4 ZONE 4
    I = 1.0
    R = 6.4
    Ω_o = 2.2
    SOIL PROFILE S_a
    SEISMIC SOURCE TYPE B
    DISTANCE FROM SOURCE 0.62 km

    N_a = 1.3
    N_v = 1.6
    C_a = 0.57
    C_v = 1.02

16. PLANS AND CALCULATIONS FOR PRODUCTS DESIGNED BY OTHERS (STAIRWAYS, EXTERIOR WALL SYSTEMS, ETC) MUST BE REVIEWED AND APPROVED BY THE ENGINEER OF RECORD FOR THE PROJECT. REFER TO THE "DEFERRED SUBMITTALS/DESIGN-BUILD ITEMS SECTION OF THE GENERAL NOTES.

SPECIAL INSPECTIONS

THE FOLLOWING ELEMENTS OF CONSTRUCTION SHALL HAVE CONTINUOUS INSPECTION BY A REGISTERED BUILDING INSPECTOR APPROVED BY THE CITY OF TEMECULA.

1. CONCRETE.
2. BOLTS INSTALLED IN CONCRETE.
3. PLACING OF REINFORCING STEEL AND PRESTRESSING STEEL.
4. WELDING:
   A. ALL STRUCTURAL FIELD WELDING, AND REINFORCING STEEL.
5. HIGH STRENGTH BOLTING.
6. STRUCTURAL MASONRY.
7. SEE GEOTECHNICAL ENGINEER'S REPORT FOR SPECIFIC INSPECTION REQUIREMENTS BY SOILS ENGINEER'S REPRESENTATIVE.

ALL SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 1701 OF THE CODE AND ANY ADDITIONAL REQUIREMENTS STATED IN THESE DRAWINGS AND/OR THE PROJECT SPECIFICATIONS.

FOUNDATION

1. FOUNDATION DESIGN BASED ON SOILS REPORTS BY PETRA GEOTECHNICAL INC. REPORT J.N. 158-06, DATED MAY 2, 2006 AND REPORT J.N. 159-06, DATED MAY 3, 2006.

2. FOOTINGS ARE DESIGNED BASED ON THE FOLLOWING INFORMATION:

   ALLOWABLE BEARING*:

   SPREAD FOOTING = 3000 PSF
   WALL FOOTING = 3000 PSF
   MAT FOUNDATION = 6000 PSF AVERAGE BEARING PRESSURE

   PASSIVE EARTH PRESSURE* = 250 PCF WITH MAX OF 2500 PSF
   COEFFICIENT OF FRICTION = 0.4

   MODULUS OF SUBGRADE REACTION = 200 PCI

   * ALLOWABLE BEARING AND PASSIVE EARTH PRESSSURE MAY BE INCREASED BY 1/3 FOR WIND AND SEISMIC LOAD CASES.

   FOOTINGS SHALL BEAR ON FIRM NATURAL SOILS OR PROPERLY COMPACTED FILL WHICH MEETS THE REQUIREMENTS OF THE SOILS REPORTS. MINIMUM DEPTH OF SPREAD & WALL FOOTINGS BELOW LOWEST ADJACENT FINAL GRADE SHALL BE 24". MINIMUM WIDTH OF FOOTING SHALL BE 18".

   REFER TO SOILS REPORTS FOR OVER EXCAVATION REQUIREMENTS

3. CONTRACTOR TO PROVIDE FOR DE-WATERING OF EXCAVATIONS FROM EITHER SURFACE WATER, GROUND WATER OR SEEPAGE, IF REQUIRED.

4. CONTRACTOR SHALL PROVIDE FOR DESIGN AND INSTALLATION OF ALL CRIBBING, SHEATHING AND SHORING REQUIRED AND SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH ALL NATIONAL, STATE AND LOCAL SAFETY ORDINANCES.

5. EXCAVATION FOR FOOTINGS SHALL BE APPROVED BY THE INSPECTOR OR SOILS ENGINEER PRIOR TO PLACING THE CONCRETE AND REINFORCING. CONTRACTOR TO NOTIFY THE INSPECTOR WHEN INSPECTION OF EXCAVATION IS READY. INSPECTOR TO SUBMIT LETTER OF COMPLIANCE.

6. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS BEFORE CONCRETE OR GROUT HAS ATTAINED FULL DESIGN STRENGTH. CONTRACTORS SHALL BRACE OR PROTECT ALL BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHING FLOORS ARE COMPLETELY IN PLACE AND HAVE ATTAINED FULL STRENGTH. CONTRACTOR SHALL PROVIDE FOR DESIGN, PERMITS AND INSTALLATION OF SUCH BRACING.

7. FOUNDATIONS SHALL BE PLACED AND ESTIMATED ACCORDING TO DEPTHS SHOWN ON DRAWINGS. SHOULD SOIL ENCOUNTERED AT THESE DEPTHS NOT BE APPROVED BY THE INSPECTOR OR SOILS ENGINEER, FOUNDATION ELEVATIONS WILL BE ALTERED BY CHANGE ORDER.

8. FOOTING BACKFILL AND UTILITY TRENCH BACKFILL WITHIN BUILDING AREA SHALL BE MECHANICALLY COMPACTED IN LAYERS IN ACCORDANCE WITH THE SOILS REPORT AND APPROVED BY THE SOILS ENGINEER. FLOODING WILL NOT BE PERMITTED. ALL FILLS USED TO SUPPORT FOUNDATIONS SHALL BE INSPECTED BY THE SOILS ENGINEER REPRESENTATIVE PER CODE SECTION 3317, VOLUME I.

9. ALL ABANDONED FOOTINGS, UTILITIES, ETC. SHALL BE REMOVED UNLESS NOTED OTHERWISE. NEW FOOTINGS MUST EXTEND INTO UNDISTURBED SOILS.

10. SLABS ON GRADE SHALL BE SUPPORTED ON NATURAL GRADE OR COMPACTED FILL AS PER THE RECOMMENDATIONS OF THE SOILS REPORT.

CONSTRUCTION JOINTS

1. ALL CONSTRUCTION JOINTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH CODE SECTION 1906.4 AND THE TYPICAL CONSTRUCTION JOINT DETAILS SHOWN ON THE STRUCTURAL DRAWINGS.

2. ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED TO REMOVE DUST, CHIPS, OR OTHER FOREIGN MATTER PRIOR TO PLACING THE ADJACENT CONCRETE.

3. THE CONTRACTOR SHALL SUBMIT THE PROPOSED LOCATIONS OF CONSTRUCTION JOINTS TO THE ENGINEER FOR APPROVAL BY THE STRUCTURAL ENGINEER BEFORE STARTING CONSTRUCTION.

HEADED STUDS

1. ALL HEADED STUDS WELDED TO BEAMS OR CONCRETE CONNECTIONS SHALL BE "TRUE-WELD STUDS", DIVISION OF TRU-FIT SCREW CORPORATION, CLEVELAND, OHIO OR "NELSON STUD" (ICC ER-2614), TRW FASTENERS AND ASSEMBLIES GROUP, LORAIN, OHIO, OR APPROVED EQUAL.

2. ALL HEADED STUDS SHALL BE AUTOMATICALLY END WELDED IN SHOP OR FIELD WITH EQUIPMENT RECOMMENDED BY MANUFACTURER OF STUDS.

3. STEEL SHEAR STUDS MATERIAL, WELDING AND INSPECTION, SHALL BE IN ACCORDANCE WITH AWS "STRUCTURAL WELDING CODE", AWS D1.1, SECTION 7. ALL STUDS SHALL BE 3/4" DIAMETER X 5" LONG, SPACED AT 12" O.C. MAXIMUM, UNLESS NOTED OTHERWISE.

DEFORMED BAR ANCHOR STUDS

1. ALL DEFORMED BAR ANCHORS SHALL BE NELSON D2L DEFORMED BAR ANCHORS (ICC ER-5217) OR APPROVED EQUAL.

2. ALL DEFORMED BAR ANCHORS SHALL BE AUTOMATICALLY END WELDED IN SHOP OR FIELD WITH EQUIPMENT RECOMMENDED BY MANUFACTURER OF STUDS.

3. BAR ANCHOR MATERIAL, WELDING, AND INSPECTION SHALL BE IN ACCORDANCE WITH AWS "STRUCTURAL WELDING CODE", AWS D1.1.

EXPANSION ANCHORS

1. ALL EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT 3 (ICC ESR-1385) OR APPROVED EQUAL. INSTALL PER MANUFACTURER'S RECOMMENDATIONS AND REQUIREMENTS OF ICC ES REPORT.

REINFORCING STEEL (FOR CONCRETE AND MASONRY)

1. REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 19 OF THE CODE, ASTM A615, GRADE 60 UNO.

2. BARS SHALL BE CLEAN OF RUST, GREASE, OR OTHER MATERIALS LIKELY TO IMPAIR BOND. ALL REINFORCING BAR BENDS SHALL BE MADE COLD.

3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. PROVIDE LAPS AS PER THE CODE SECTION 1912.19, 9" MINIMUM. WWF SHALL BE SUPPORTED ON APPROVED CHAIRS.

4. REINFORCING BAR SPLICES SHALL BE MADE AS INDICATED ON THE DRAWINGS. MINIMUM SPLICE LENGTH FOR REINFORCING STEEL BARS IN MASONRY SHALL BE 40 BAR DIAMETERS, 24" MINIMUM. MINIMUM SPLICE LENGTH FOR REINFORCING STEEL BARS IN CONCRETE SHALL BE AS PER THE CODE SECTION 1912. LAP ALL HORIZONTAL BARS AT CORNERS AND INTERSECTIONS. STAGGER ALL SPLICES UNLESS NOTED OTHERWISE ON PLANS.

5. ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN THE FINAL IN-PLACE INSPECTION IS MADE.

6. WHERE WELDING OF REINFORCING IS APPROVED BY THE STRUCTURAL ENGINEER, IT SHALL BE DONE BY AWS CERTIFIED WELDERS USING E90XX OR APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE REQUIREMENTS OF STRUCTURAL WELDING CODE- REINFORCING STEEL", AWS-D1.4, LATEST REVISION. REINFORCING BARS TO BE WELDED SHALL CONFORM TO THE REQUIREMENTS OF ASTM A-706.

7. BARS IN SLABS SHALL BE SECURELY SUPPORTED ON WELL-CURED CONCRETE BLOCKS OR APPROVED METAL CHAIRS, PRIOR TO PLACING CONCRETE.

8. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE "A.C.I. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", LATEST EDITION.

9. COMPLETE AND DETAILED REINFORCING PLACEMENT DRAWINGS SHALL BE PREPARED AND SUBMITTED TO THE ARCHITECT FOR APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION IN ACCORDANCE WITH THE SPECIFICATIONS AND APPLICABLE CODES. THESE DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE PRIOR TO PLACING OF CONCRETE.

10. MILL TEST REPORTS FOR GRADE 60 BARS SHALL BE SUBMITTED PRIOR TO PLACEMENT OF CONCRETE.

11. CONTINUOUS INSPECTION OF CONCRETE SHALL INCLUDE INSPECTION DURING INSTALLATION OF REINFORCING STEEL. INSPECTION SHALL BE SCHEDULED SO THAT PLACEMENT OF REINFORCING STEEL, CONDUIT, SLEEVES, AND EMBEDDED ITEMS MAY BE CORRECTED PRIOR TO PLACEMENT OF OVERLYING GRIDS OR REINFORCING STEEL.

12. ALL GRADE 60 REINFORCING STEEL SHALL BE CLEARLY MARKED TO DIFFERENTIATE THEM FROM GRADE 40 REINFORCING STEEL IF CONCURRENTLY ON SITE.

13. CONCRETE PROTECTION FOR REINFORCEMENT

    (i) CAST-IN-PLACE CONCRETE (NON-PRESTRESSED). THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

	MINIMUM COVER, IN.
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3
B. CONCRETE EXPOSED TO EARTH OR WEATHER:	
NO. 6 THROUGH NO. 18 BAR	2
NO 5 BAR, W31 OR D31 WIRE & SMALLER	1 1/2
C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS, JOISTS:	
NO. 14 AND NO. 18 BAR	1 1/2
NO. 11 BAR & SMALLER	1
BEAMS, COLUMNS:	
PRIMARY REINFORCEMENT TIES, STIRRUPS, SPIRALS	1 1/2

CONCRETE

1. ALL CONCRETE CONSTRUCTION SHALL CONFORM WITH CHAPTER 19 OF THE CODE AND WITH THE PROVISIONS OF ACI 318, LATEST EDITION.

2. REINFORCED CONCRETE IS DESIGNED BY THE "ULTIMATE STRENGTH DESIGN METHOD".

3. CONCRETE MIXES SHALL BE DESIGNED BY THE APPROVED TESTING LABORATORY AND APPROVED BY THE STRUCTURAL ENGINEER. THE COMPRESSIVE STRENGTH OF THE CONCRETE SHALL BE PROPORTIONED BASED ON SECTION 1905 OF THE CODE.

4. SCHEDULE OF STRUCTURAL CONCRETE 28-DAY STRENGTH AND TYPES (UNO):

LOCATION IN STRUCTURE	MIN STRENGTH (PSI)	MAX DENSITY (PCF)	MAX SLUMP (IN)	MAX WATER/CEMENT RATIO
CONCRETE FLOOR SLABS OVER METAL DECK	4000	110	4	0.50
ALL CONCRETE FOOTINGS AND GRADE BEAMS	4000	150	4	0.50
ALL CONCRETE WALLS, COLUMNS, BEAMS AND SLABS	4000	150	4	0.50
SLAB ON GRADE, STAIRS ON GRADE, CURBS, AND OTHER NON-STRUCTURAL CONCRETE.	3000	150	4	0.55

5. PORTLAND CEMENT SHALL CONFORM TO ASTM C-150, TYPE II.

6. AGGREGATE FOR HARDROCK CONCRETE SHALL CONFORM TO ALL REQUIREMENTS AND TESTS OF ASTM C-33 AND PROJECT SPECIFICATIONS. EXCEPTIONS MAY BE USED ONLY WITH PERMISSION OF THE STRUCTURAL ENGINEER.

7. AGGREGATE FOR LIGHT WEIGHT (110PCF) CONCRETE SHALL BE EXPANDED SHALE CONFORMING TO ASTM A330 AND PROJECT SPECIFICATIONS. EXCEPTIONS MAY BE USED ONLY WITH PERMISSION OF THE STRUCTURAL ENGINEER.

8. CONCRETE MIXING OPERATION, ETC. SHALL CONFORM TO ASTM C-94.

9. PLACEMENT OF CONCRETE SHALL CONFORM TO CODE SECTION 1905 AND PROJECT SPECIFICATIONS. CLEAN AND ROUGHEN TO 1/4" AMPLITUDE ALL CONCRETE SURFACES AGAINST WHICH NEW CONCRETE IS TO BE PLACED.

10. ALL REINFORCING BARS, ANCHOR BOLTS AND OTHER CONCRETE INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE.

11. PROVIDE SLEEVES FOR PLUMBING AND ELECTRICAL OPENINGS IN CONCRETE BEFORE PLACING. DO NOT CUT ANY REINFORCING WHICH MAY CONFLICT. CORING IN CONCRETE IS NOT PERMITTED. NOTIFY THE STRUCTURAL ENGINEER IN ADVANCE OF CONDITIONS NOT SHOWN ON THE DRAWINGS. SEE THESE DRAWINGS FOR ADDITIONAL RESTRICTIONS ON THE PLACEMENT OF OPENINGS IN SLABS AND WALLS.

12. PIPES LARGER THAN 1-1/2" DIAMETER SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE EXCEPT WHERE SPECIFICALLY APPROVED BY STRUCTURAL ENGINEER. PIPES SHALL NOT DISPLACE OR INTERRUPT REINFORCING BARS, SPACE EMBEDDED PIPES AND SLEEVES AT A MINIMUM OF 3 DIAMETERS.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED BY AN APPROVED AND LICENSED FABRICATOR IN ACCORDANCE WITH THE AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS (LATEST EDITION), AND WITH CHAPTER 22 OF THE CODE.

2. ALL STRUCTURAL STEEL SHALL CONFORM TO THE ASTM DESIGNATION AS INDICATED BELOW (UNO):

WF SHAPES, WT SHAPES	A992 OR A572, GRADE 50 (MULTI-CERT)
PLATES	
U.N.O., CONNECTION PLATES, AND MISC.	A-36
PIPE COLUMNS	A-53, GRADE B
TUBE SECTIONS	A-500, GRADE B
BOLTS	A-325/A-490
BOLTS IN CONCRETE/MASONRY	A-307/A-449/A-354
ANGLES, CHANNELS	A-36

3. THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP DRAWINGS FOR ALL STEEL FOR ARCHITECTS AND STRUCTURAL ENGINEERS REVIEW AND APPROVAL BEFORE FABRICATION.

4. BOLT HOLES USED IN STEEL SHALL BE 1/16" LARGER IN DIAMETER THAN NOMINAL SIZE OF BOLT USED, EXCEPT AS NOTED.

5. ALL STRUCTURAL STEEL SURFACES THAT ARE ENCASED IN CONCRETE OR MASONRY, SPRAY ON FIREPROOFING, OR ARE ENCASED BY BUILDING FINISH, SHALL BE LEFT UNPAINTED.

6. ALL WELDING IS TO BE DONE BY CERTIFIED WELDERS USING E70XX ELECTRODES (UNO). ALL WELDS SHALL BE IN CONFORMITY WITH THE PROJECT SPECIFICATIONS AND THE CODE FOR WELDING IN BUILDING CONSTRUCTION (AWS D1.1 LATEST REVISION) OF THE AMERICAN WELDING SOCIETY. SEE SPECIAL INSPECTIONS SECTION FOR WELDING INSPECTION REQUIREMENTS.

7. WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE, USE MINIMUM SIZE WELDS AS SPECIFIED IN AISC MANUAL OF STEEL CONSTRUCTION 9TH EDITION, SECTION J2.

8. ALL EXPOSED STRUCTURAL STEEL AND MISCELLANEOUS METAL SHALL BE HOT DIP GALVANIZED AFTER FABRICATION.

9. THE USE OF E70T-4 WELDING WIRE IS NOT PERMITTED.

10. ALL WELD FILLER MATERIAL SPECIFIED AS "NOTCH TOUGH" SHALL HAVE A MINIMUM CHARPY-V NOTCH (CVN) VALUE OF 20 FT-LBS AT A TEMPERATURE OF -20° F.

11. 100% UT TEST FOR ALL COMPLETE PENETRATION GROOVE WELDS.

12. THE SEISMIC DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL BE ACCORDANCE WITH PART I (LRFD) AND PART III (ASD) OF THE SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS, APRIL 15, 1997, PUBLISHED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC). THESE PROVISIONS SHALL BE APPLIED IN CONJUNCTION WITH CHAPTER 22, DIVISION II 91.2210.

13. DISCONTINUITIES IN WELD CREATED BY ERRORS OR BY FABRICATION OR ERECTION OPERATIONS, SUCH AS TACK WELDS, ERECTION AIDS, AIR ARC GOUGING AND FLAME CUTTING, SHALL BE REPAIRED AS REQUIRED BY THE ENGINEER OF RECORD.

14. FOR ARCHITECTURAL EXPOSED STRUCTURAL STEEL (AESS) SPECIFIED ON THE DRAWINGS, REFERENCE THE "AISC CODE OF STANDARD PRACTICE".

SPECIAL STEEL REQUIREMENTS (AISC 13TH EDITION)

1. ALL ROLLED STEEL MEMBERS CONFORMING TO ASTM GROUP 4 OR GROUP 5 AND ALL PLATES EXCEEDING 2" IN THICKNESS FOR BUILT UP SECTIONS MUST COMPLY WITH THESE SPECIAL STEEL REQUIREMENTS, U.N.O..

2. ALL MEMBERS BELONGING TO LATERAL FORCE RESISTING FRAMES THAT ARE TO HAVE FULL PENETRATION GROOVE WELDS SHALL BE SUPPLIED WITH PART 16 NOTCH TESTING IN ACCORDANCE WITH ASTM A6 AS INDICATED AND MODIFIED BY SECTION A3.1C.

3. ALL SPLICES IN HEAVY SECTIONS BELONGING TO LATERAL FORCE RESISTING FRAMES SHALL COMPLY WITH SECTION J1.5.

4. BEAM COPES AND WELD ACCESS HOLES SHALL COMPLY WITH SECTION J1.6.

5. ALL WELD MATERIAL SHALL COMPLY WITH SECTION J2.6, AND J2.7.

6. THE THERMAL CUTTING OF ALL MEMBERS SHALL COMPLY WITH SECTION M2.2.

STEEL DECK

1. ROOF AND FLOOR DECKS SHALL BE AS NOTED ON THE DRAWINGS. PROPERTIES ARE AS FOLLOWS:

DECK SIZE AND GAUGE	I (in4)	S (in3)
3" X 20 GA DECK	.896	.534
1 1/2" X 20 GA DECK	0.216	.235
1 1/2" X 18 GA DECK	0.302	.322

2. DECK SHOP DRAWINGS SHALL BE SUBMITTED TO ENGINEER FOR APPROVAL PRIOR TO FABRICATION AND SHALL INDICATE STUD LAYOUT.

3. THE AMERICAN IRON AND STEEL INSTITUTE "SPECIFICATIONS FOR THE DESIGN OF LIGHT GAUGE STEEL STRUCTURAL MEMBERS" SHALL GOVERN THE DESIGN OF ALL DECK UNITS, STEEL DECK AND ALL OF ITS CLOSURES AND FLASHINGS SHALL CONFORM TO ASTM A653, GRADE 38, FY 38,000 PSI MIN.

4. ACCEPTABLE STEEL DECK MANUFACTURERS ARE AS FOLLOWS:

   VERCO MANUFACTURING, INC. (ICBO #2078)
   H.H. ROBERTSON COMPANY (ICBO #2739)
   ASC PACIFIC (ICBO #2757)

5. UNITS SHALL BE CONTINUOUS OVER THREE OR MORE SPANS, EXCEPT WHERE THE FRAMING DOES NOT PERMIT. SHORING MAY BE REQUIRED AT NON-CONTINUOUS SPANS. DECK SHOP DRAWINGS SHALL INDICATE WHERE SHORING WILL BE REQUIRED. DECK SHALL BEAR 2" MINIMUM AT ALL SUPPORTS.

6. ALL WELDING OF STEEL DECK SHALL BE DONE BY CERTIFIED LIGHT GAGE WELDERS IN ACCORDANCE WITH AWS "SPECIFICATIONS FOR WELDING SHEET STEEL IN STRUCTURES", AWS D1.3-81.

7. UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE END OF THE UNITS AND AT INTERMEDIATE SUPPORTS AND TO THE STEEL SUPPORTS AT THE SIDE BOUNDARIES BY 3/4" DIAMETER PUDDLE WELDS AT 1'-0" O.C. SHEAR STUDS WELDED THROUGH DECK MAY BE USED IN PLACE OF 3/4" DIAMETER PUDDLE WELDS.

8. THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BETWEEN SUPPORTS BY BUTTON PUNCHING AT 3'-0" O.C. MAX. CONTRACTOR MAY DECREASE SPACING OF SIDE LAP ATTACHMENTS TO ACCOMMODATE CONSTRUCTION LOADING AS REQUIRED.

9. PROVIDE FLASHING AND CLOSURE PLATES AT ENDS OF ALL UNITS, AROUND COLUMNS, AND AT ALL PERIMETER LOCATIONS REQUIRING CONCRETE.

METAL STUDS

1. ALL LIGHTGAGE METAL FRAMING SHALL BE AS NOTED BELOW:
   EXTERIOR STUDS: GALVANIZED

2. ALL LIGHTGAGE METAL FRAMING CONSTRUCTION SHALL BE IN ACCORDANCE WITH AISI "SPECIFICATIONS FOR DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" LATEST EDITION.

3. ALL LIGHTGAGE METAL FRAMING SHALL CONFORM WITH THE FOLLOWING:

GALVANIZED STUDS: 12, 14 AND 16 GAGE	ASTM A653, GRADE 50 Fy = 50,000 psi)
GALVANIZED STUDS: 18 AND 20 GAGE	ASTM A653, GRADE 33 (Fy = 33,000 psi)
GALVANIZED TRACK END CLOSURES, BRIDGING AND ACCESSORIES	ASTM A653, GRADE 33 (Fy = 33,000 psi)

4. AS A MINIMUM, DOUBLE VERTICAL STUDS SHALL BE STITCH WELDED TOGETHER ON BOTH FLANGES WITH 1/16" GROOVE WELDS X 1" LONG AT 12" ON CENTER, U.N.O.

5. MINIMUM STUD PROPERTIES SHALL BE AS INDICATED ON PLANS.

6. ALL SHEET METAL SCREWS SHALL CONSIST OF 'DARTS' BRAND SELF-DRILLING / SELF-TAPPING STEEL SCREWS (ICC ER-5202). INSTALLATION OF SCREWS SHALL BE IN CONFORMANCE WITH ICC REPORT # ER-5202 AND SHALL PROTRUDE THROUGH THE ATTACHED MEMBERS THREE FULL THREADS BEYOND THE ATTACHED MEMBERS.

7. AS A MINIMUM, ANCHOR STUD TRACKS WITH HILTI 0.145" DIAMETER X-DNI (ICBO # 2388) LOW VELOCITY POWDER DRIVEN INSERTS @ 16" OC OR APPROVED EQUAL. THE INSERTS SHALL HAVE A MINIMUM 1 1/2" EMBEDMENT INTO THE CONCRETE AND SHALL BE LOCATED A MINIMUM 3" FROM ANY CONCRETE EDGE. THE INSERTS SHALL BE PLACED ADJACENT TO EACH WALL STUD AT A MAXIMUM DISTANCE OF 2".

8. CONTRACTOR SHALL COMPLY WITH ALL OF THE MANUFACTURER'S INSTALLATION SPECIFICATIONS AND RECOMMENDATIONS. ALL METAL STUDS SHALL HAVE STIFFENED FLANGES. SEE DRAWINGS FOR SPECIFIC DETAILS ON CONNECTIONS, BRACING, BRIDGING, ETC. CONTRACTOR SHALL PROVIDE ALL ACCESSORIES INCLUDING. BUT NOT LIMITED TO, TRACKS, CLIPS, AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE AND PROPER INSTALLATION, AND AS RECOMMENDED BY THE MANUFACTURER FOR THE STEEL MEMBERS USED.

9. ALL METAL STUDS TO BE MANUFACTURED BY MEMBERS OF THE STEEL STUD MANUFACTURER'S ASSOCIATION, SSMA (ICBO #4943P), U.N.O.

STRUCTURAL OBSERVATION:

1. THE STRUCTURAL ENGINEER OF RECORD OR ONE OF HIS/HER QUALIFIED REPRESENTATIVES IS REQUIRED TO PERFORM PERIODIC STRUCTURAL OBSERVATION OF THE STRUCTURAL SYSTEM DURING CONSTRUCTION IN ACCORDANCE WITH THE CODE. THE CONTRACTOR IS RESPONSIBLE FOR FACILITATING A TIMELY AND SAFE ACCESS TO THE STRUCTURAL ELEMENTS TO BE OBSERVED.

2. AS PER THE CODE REQUIREMENTS, A MEETING BETWEEN THE STRUCTURAL ENGINEER OF RECORD, THE GENERAL CONTRACTOR, AFFECTED SUBCONTRACTORS AND DEPUTY INSPECTORS, SHALL BE PLANNED AND CONDUCTED PRIOR TO START OF CONSTRUCTION. THE PURPOSE OF THE MEETING SHALL BE TO IDENTIFY THE MAJOR STRUCTURAL ELEMENTS AND CONNECTIONS THAT ARE EFFECTED BY THE STRUCTURAL OBSERVATION REQUIREMENTS, AND TO REVIEW SCHEDULING OF THE REQUIRED OBSERVATIONS. A RECORD OF THE MEETING WILL BE INCLUDED IN THE FIRST OBSERVATION REPORT SUBMITTED TO THE BUILDING INSPECTOR, AS REQUIRED BY THE CODE.

3. IN GENERAL, THE STRUCTURAL OBSERVER IS REQUIRED TO PERFORM SITE VISITS AT THOSE STEPS IN THE PROGRESS OF THE WORK THAT ALLOW FOR CORRECTION OF DEFFICIENCIES WITHOUT SUBSTANTIAL EFFORT FOR UNCOVERING OF THE WORK INVOLVED. AT A MINIMUM, THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL ENGINEER:

   A) PERIODIC OBSERVATION OF FOUNDATION EXCAVATIONS
   B) PERIODIC OBSERVATION OF PLACEMENT OF REINFORCING STEEL IN FOOTINGS, SLABS AND WALLS.
   C) PERIODIC OBSERVATION OF THE PLACEMENT OF CONCRETE IN FOOTINGS, SLABS AND WALLS.
   D) PERIODIC OBSERVATION OF ERECTION OF STRUCTURAL STEEL.
   E) PERIODIC OBSERVATION OF WELDING AND BOLTING OF CONNECTIONS.

4. COPIES OF EACH OBSERVATION REPORT, SIGNED AND STAMPED BY THE STRUCTURAL ENGINEER, WILL BE SENT TO THE BUILDING INSPECTOR'S OFFICE, TO THE OWNER, TO THE DEPUTY INSPECTOR, AND TO THE GENERAL CONTRACTOR. A COPY OF EACH OF THESE REPORTS SHALL BE ATTACHED TO THE APPROVED PLANS.

5. A FINAL OBSERVATION REPORT MUST BE SUBMITTED WHICH SHOWS THAT ALL OBSERVED DEFICIENCIES WERE RESOLVED AND THE STRUCTURAL SYSTEM GENERALLY CONFORMS WITH THE APPROVED PLANS AND SPECIFICATIONS. THE DEPARTMENT OF BUILDING AND SAFETY WILL NOT ACCEPT THE STRUCTURAL WORK WITHOUT THIS FINAL OBSERVATION

MASONRY

1. CONCRETE BLOCK SHALL BE HOLLOW LOAD-BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C-90, GRADE N. MEDIUM WEIGHT UNITS. USE OPEN END UNITS FOR STACKED BOND PATTERN.
2. CEMENT SHALL BE AS SPECIFIED FOR CONCRETE.
3. REINFORCING BARS - SEE NOTES UNDER "REINFORCING STEEL" FOR REQUIREMENTS.
4. MORTAR SHALL BE TYPE S WITH PROPORTIONS OF 4 -1/2 PARTS SAND, 1/2 PART HYDRATED LIME, 1 PART PORTLAND CEMENT AND SHALL COMPLY WITH UBC STD. 21-15. MORTAR SHALL HAVE A 28 DAY STRENGTH OF 2500 PSI MINIMUM. MORTAR COMPONENT TESTS ARE NOT REQUIRED.
5. GROUT SHALL CONSIST OF 3 PARTS SAND, 2 PARTS PEA GRAVEL, AND 1 PART PORTLAND CEMENT, AND SHALL COMPLY WITH UBC STD. 21-14. GROUT COMPONENT TESTS ARE NOT REQUIRED.
6. PROVIDE A MINIMUM OF 1/2" CLEAR BETWEEN MAIN REINFORCING AND MASONRY UNITS.
7. ALL CELLS SHALL BE GROUTED SOLID.
8. DESIGN f'm = 1500 PSI FOR CMU CONSTRUCTION, AS PER TABLE 21 D, OF THE CODE. PRISM TESTING SHALL BE PERFORMED AS PER THE REQUIREMENTS OF THE CODE, SECTION 2105.3.3.

DEFERRED SUBMITTALS/DESIGN-BUILD ITEMS

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE FOLLOWING LIST OF DESIGN-BUILD ITEMS NOT DESIGNED BY KPFF CONSULTING ENGINEERS. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND STRUCTURAL CALCULATIONS SIGNED BY AND BEARING THE SEAL OF A REGISTERED CIVIL OR STRUCTURAL ENGINEER LICENSED IN THE STATE OF CALIFORNIA FOR REVIEW BY THE ARCHITECT AND KPFF CONSULTING ENGINEERS. AFTER A REVIEW FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT BY THE ARCHITECT AND KPFF CONSULTING ENGINEERS, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY REQUIRED SUBMITTAL AND APPROVALS BY THE BUILDING DEPARTMENT. PRE-APPROVED STANDARD DETAILS MAY BE USED PROVIDED THEY ARE AN ACCEPTABLE APPROVED STANDARD BY THE GOVERNING BUILDING OFFICIAL AND APPLICABLE CODE FOR THIS PROJECT.

1. GLASS CURTAIN WALL, WINDOW WALL, STOREFRONT
2. EXTERIOR METAL PALNEL WALL SYSTEM AND FRAMING
3. ELEVATOR GUIDE RAILS
4. EQUIPMENT ANCHORAGE
5. DUCTS, PIPES, ELECTRICAL TRAYS, ETC.

THE DESIGN OF THESE ELEMENTS SHALL COMPLY WITH THE GOVERNING CODE FOR THIS PROJECT AND ALL PROJECT SPECIFICATIONS AND CRITERIA INDICATED ON THE CONSTRUCTION DOCUMENTS. ADDITIONAL SUPPORT FRAMING, BRACING, STIFFENER PLATES, AND/OR OTHER STABILIZING ELEMENTS SHALL BE PROVIDED AT CONNECTIONS TO THE MAIN STRUCTURE TO PREVENT ECCENTRIC LOADING, TWISTING, WARPING, ROTATION, AND/OR TORSION OF THE MAIN STRUCTURAL MEMBERS. THE DESIGN OF THESE ELEMENTS SHALL MAKE PROVISIONS FOR ACCOMODATING THE APPROPRIATE SEISMIC/WIND FORCES AND DISPACEMENTS AS WELL AS FLOOR DEFLECTIONS DUE TO GRAVITY LOADS OF THE MAIN STRUCTURE.

DESIGN CRITERIA FOR EXTERIOR WALL SYSTEMS DESIGN:

A. ACCOMODATE A MAXIMUM INELASTIC RESPONSE DISPLACEMENT (INTER-STORY DRIFT) $\Delta_m = 3.8"$

B. ALLOW FOR A MAXIMUM FLOOR/ROOF LIVE LOAD DEFLECTION Δ_{VERT} =

$$\begin{cases} = \ell/360 \text{ (FOR TYPICAL BEAMS)} \\ = \tfrac{1}{2}" \text{ (FOR BEAMS AT BLDG EXTERIOR)} \end{cases}$$

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Plotted: 3/21/07 at 1:08pm By: SGutierrez
XREFs: 1061030_bdr.dwg

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

ALDRIN J. ORUE
No. S4399
Exp. 12/31/07

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

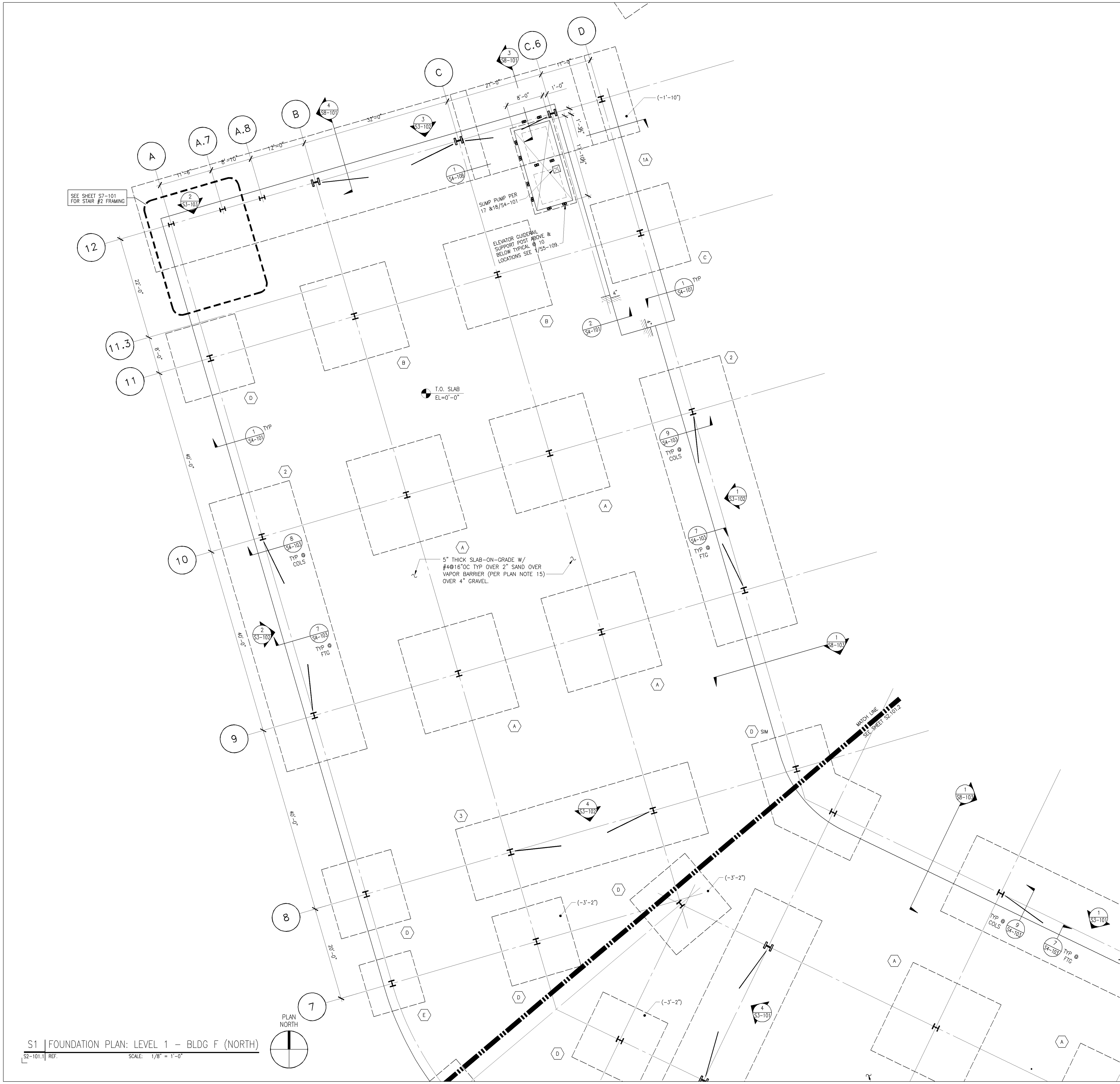
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

GENERAL STRUCTURAL NOTES

S1-101



S1 | FOUNDATION PLAN: LEVEL 1 – BLDG F (NORTH)

S2-101.1 | REF. SCALE: 1/8" = 1'-0"

FOUNDATION PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. (-X'-XX") INDICATES TOP OF CONCRETE FOOTING BELOW THE ADJACENT TOP OF SLAB ELEVATION. ALL FOOTINGS NOT NOTED SHALL BE (-1'-6") BELOW THE ADJACENT TOP OF SLAB ELEVATION.
7. X" INDICATES SLAB DEPRESSION
8. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
9. # INDICATES FOOTING CALLOUT PER SCHEDULE ON DETAIL 1/S4-103.
10. SEE ARCHITECTURAL FOR CONCRETE SLAB DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
11. VERIFY LOCATIONS, DIMENSIONS, AND TYPE OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT. REFER TO DETAILS 7 & 15/S4-101.
12. VERIFY LOCATIONS AND DIMENSIONS OF SUMP PITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
13. SEE SOILS REPORTS FOR OVER EXCAVATION REQUIREMENTS.
14. INDICATES STEPPED FOOTING PER DETAIL 12/S4-101.
15. VAPOR BARRIER SHALL HAVE A PERMEANCE OF 0.01 PERMS OR LESS, MEETING ASTM E-1745 (CLASS "A"), BE LOCATED PER ACI LOCATION GUIDELINES, AND BE INSTALLED PER ASTM E-1643.

OWNER/CLIENT

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TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CIVIL ENGINEER
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ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

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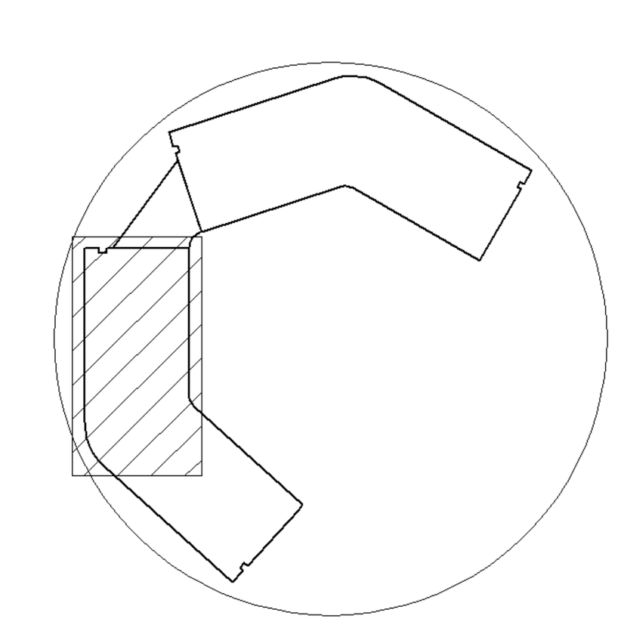
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN



SHEET TITLE

FOUNDATION PLAN: LEVEL 1 – BUILDING F (NORTH)

S2-101.1

S1 | FRAMING PLAN: LEVEL 1 – BLDG F (SOUTH)
S2-101.2 | REF. SCALE: 1/8" = 1'-0"

FOUNDATION PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. (-X'-XX") INDICATES TOP OF CONCRETE FOOTING BELOW THE ADJACENT TOP OF SLAB ELEVATION. ALL FOOTINGS NOT NOTED SHALL BE (-1'-6") BELOW THE ADJACENT TOP OF SLAB ELEVATION.
7. X" INDICATES SLAB DEPRESSION
8. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
9. # INDICATES FOOTING CALLOUT PER SCHEDULE ON DETAIL 1/S4-103.
10. SEE ARCHITECTURAL FOR CONCRETE SLAB DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
11. VERIFY LOCATIONS, DIMENSIONS, AND TYPE OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT. REFER TO DETAILS 7 & 15/S4-101.
12. VERIFY LOCATIONS AND DIMENSIONS OF SUMP PITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
13. SEE SOILS REPORTS FOR OVER EXCAVATION REQUIREMENTS.
14. INDICATES STEPPED FOOTING PER DETAIL 12/S4-101.
15. VAPOR BARRIER SHALL HAVE A PERMEANCE OF 0.01 PERMS OR LESS, MEETING ASTM E-1745 (CLASS "A"), BE LOCATED PER ACI LOCATION GUIDELINES, AND BE INSTALLED PER ASTM E-1643.

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BENNITT + MITCHELL
LANDSCAPE ARCHITECT
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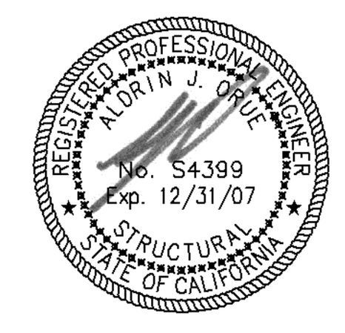
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

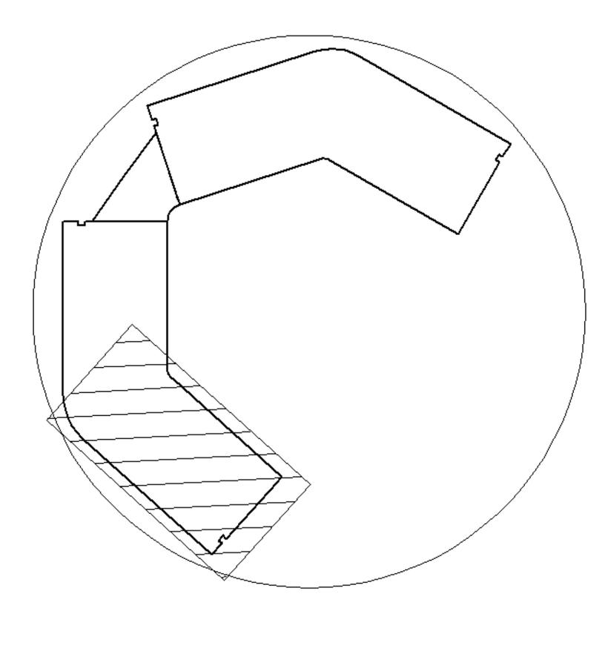


ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

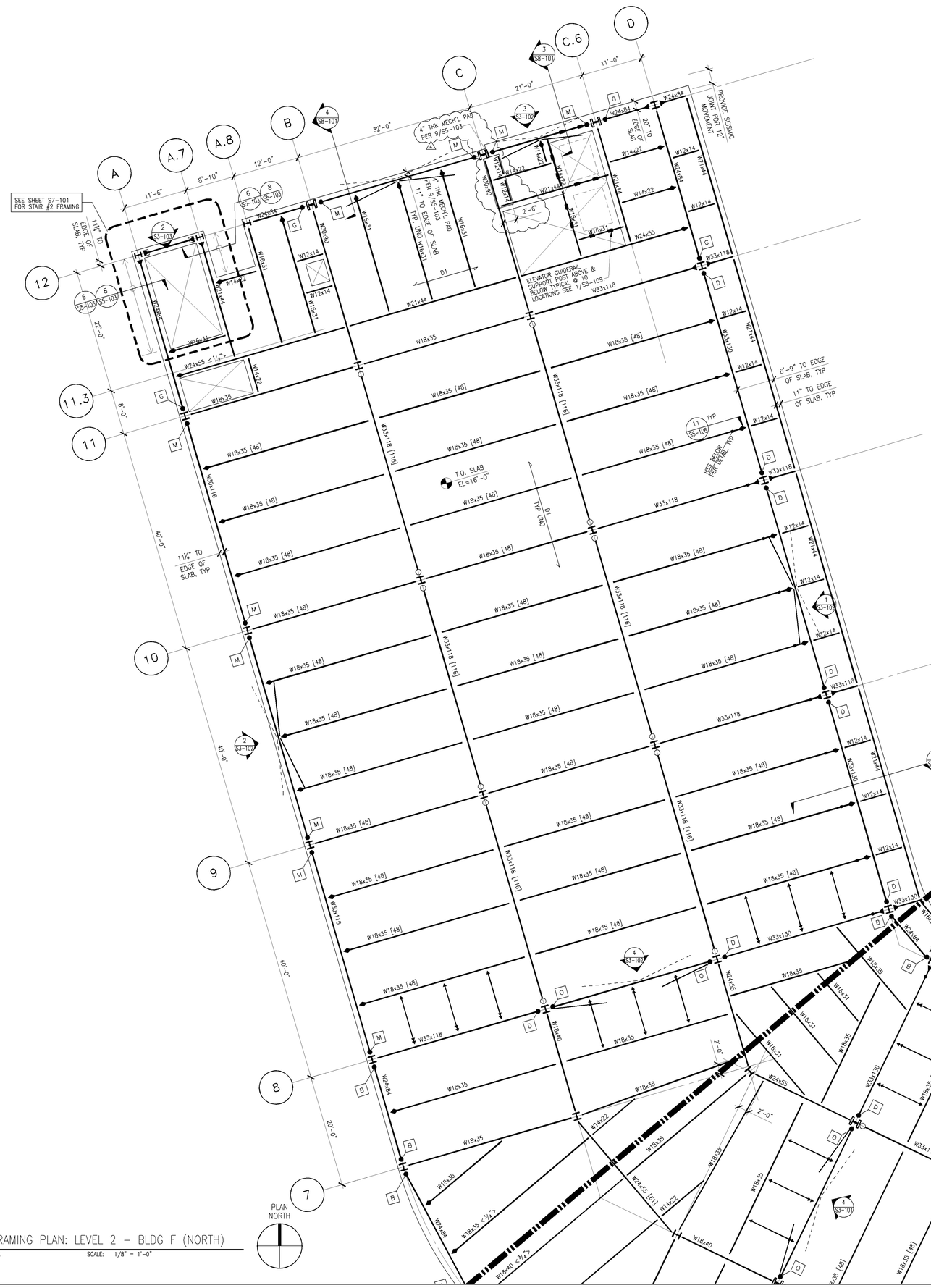
KEY PLAN



SHEET TITLE

FOUNDATION PLAN: LEVEL 1 – BUILDING F (SOUTH)

S2-101.2



FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.
   TOS (X'-XX")INDICATES TOP OF STEEL ELEVATION WHERE NOTED.
   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. "X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

S1 | FRAMING PLAN: LEVEL 2 – BLDG F (NORTH)
S2-102.1 REF. SCALE: 1/8" = 1'-0"

OWNER/CLIENT

Abbott Vascular
TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3774
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER ALAN N. J. [illegible] No. [illegible] EXP. 12/31/07 STRUCTURAL STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
4	08-30-07	BULLETIN 010 MODS.
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

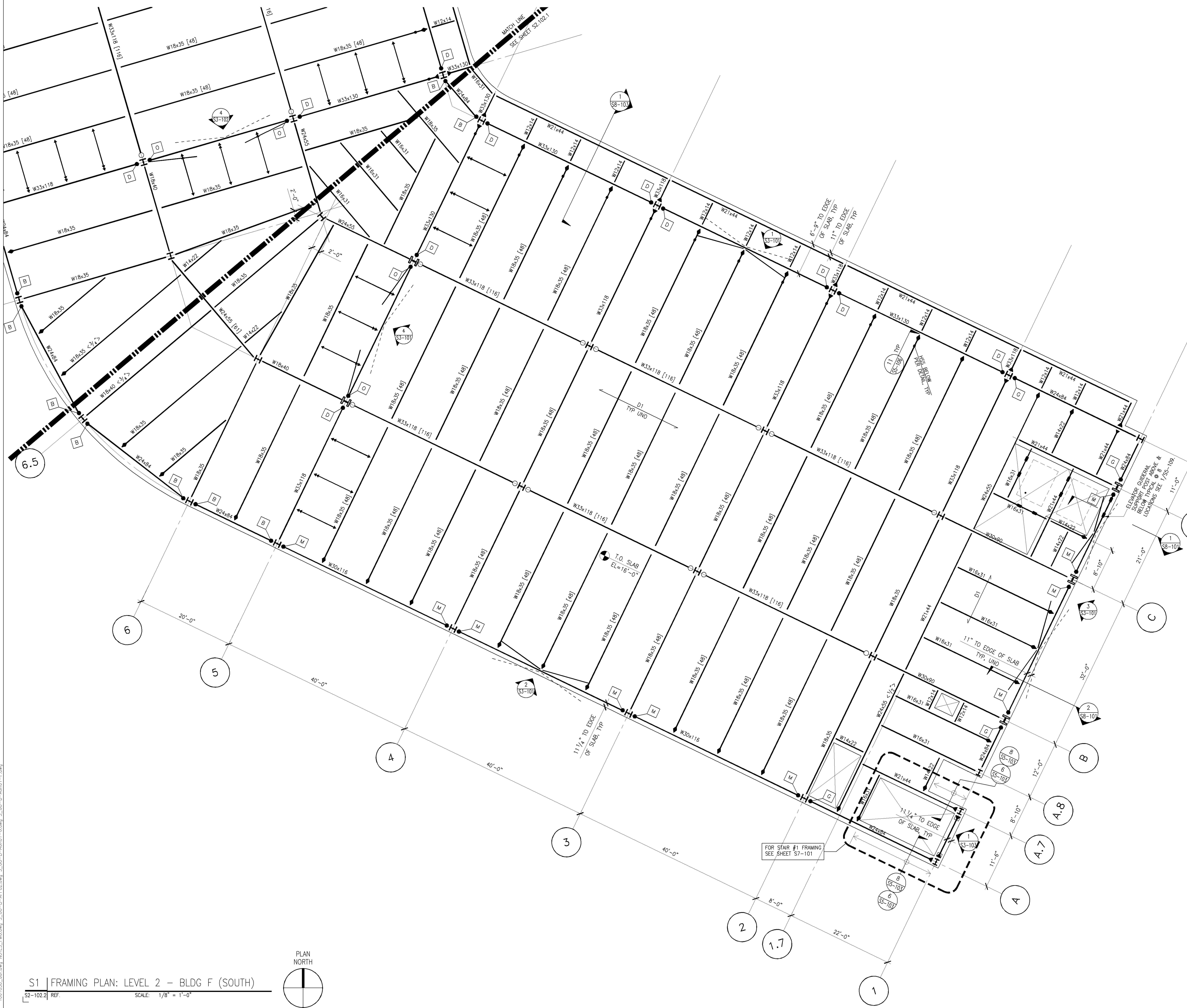
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

FRAMING PLAN: LEVEL 2 – BUILDING F (NORTH)

S2-102.1



S1 | FRAMING PLAN: LEVEL 2 – BLDG F (SOUTH)
S2-102.2 | REF. SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.
   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.
   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

OWNER/CLIENT

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
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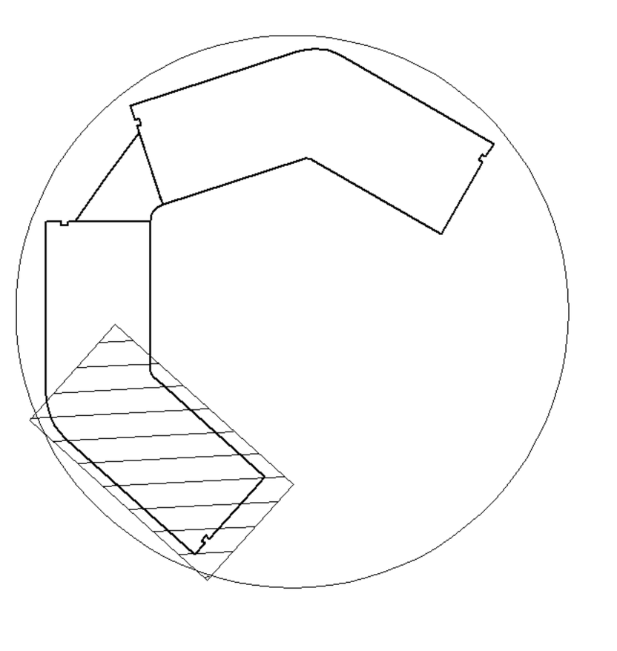
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

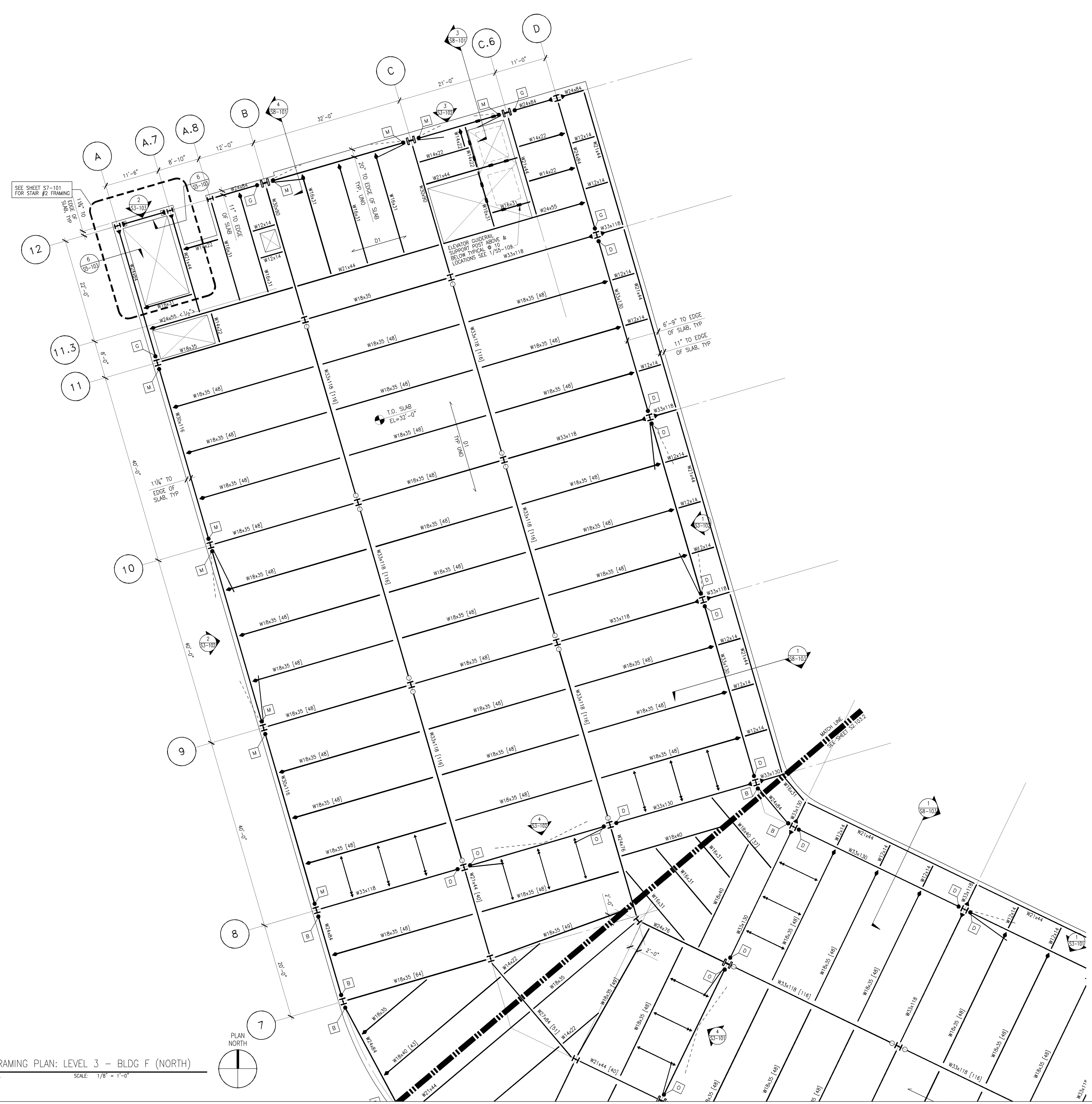
KEY PLAN



SHEET TITLE

FRAMING PLAN: LEVEL 2 – BUILDING F (SOUTH)

S2-102.2



S1 | FRAMING PLAN: LEVEL 3 – BLDG F (NORTH)

S2-103.1 | REF. | SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.
25. PROVIDE CONCRETE MECHANICAL EQUIPMENT PADS PER DETAILS 9 & 10/S5-103 AS REQUIRED BY MECHANICAL AND ARCHITECTURAL DRAWINGS.

OWNER/CLIENT

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PROJECT NO: 60004775
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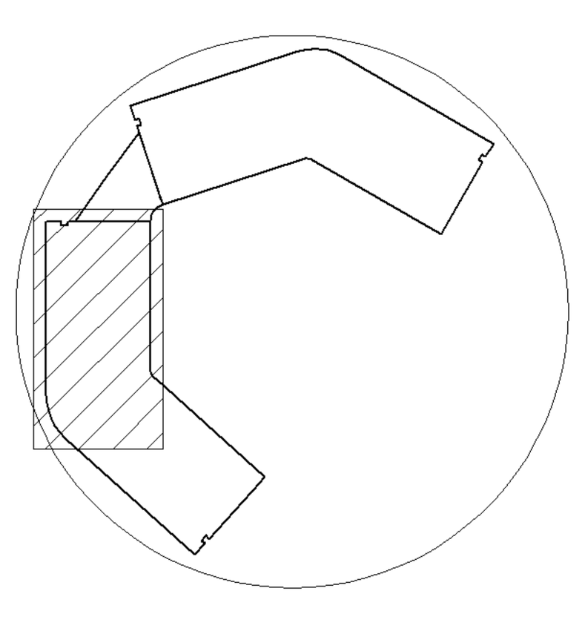
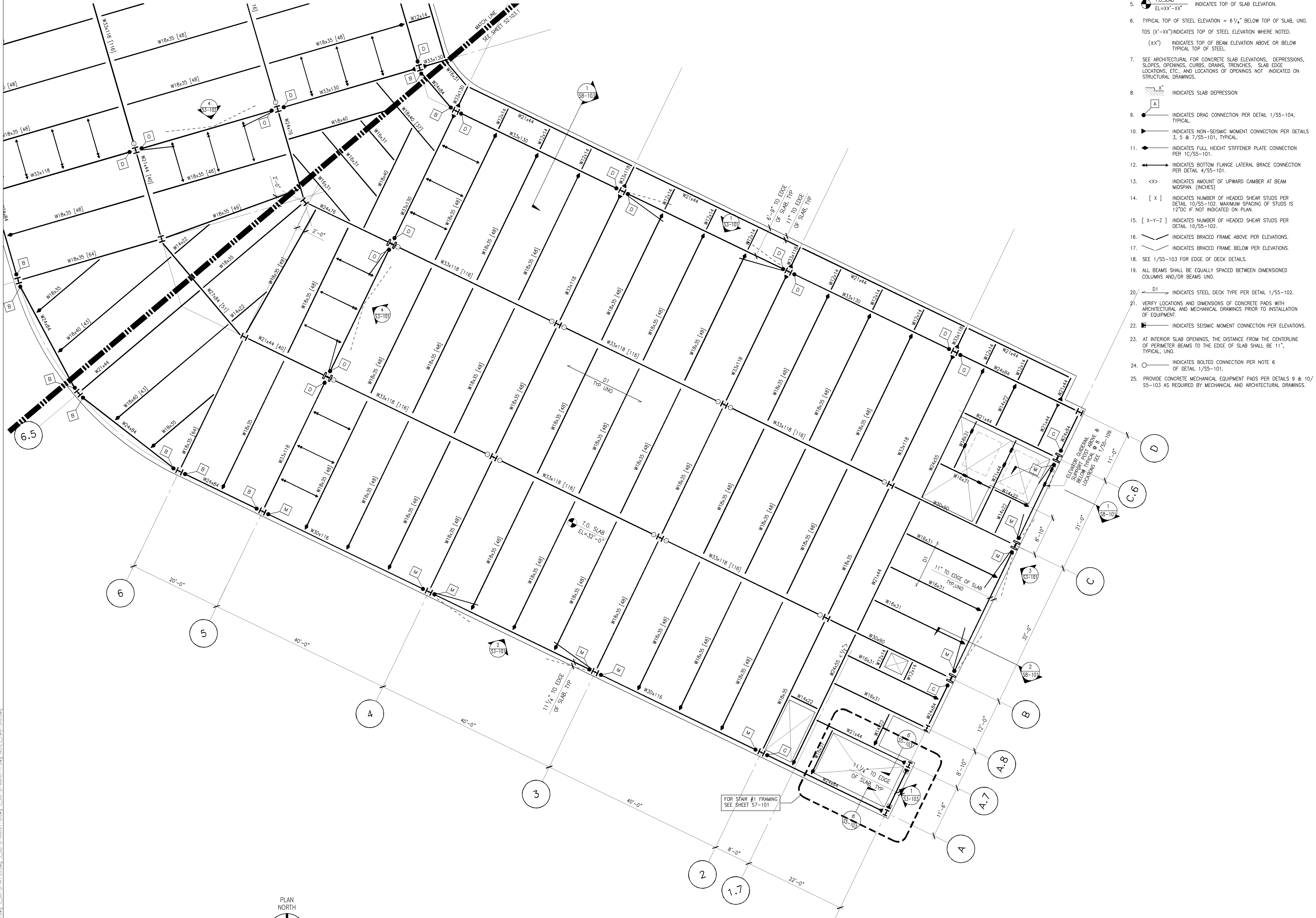
KEY PLAN



SHEET TITLE

FRAMING PLAN: LEVEL 3 – BUILDING F (NORTH)

S2-103.1



FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.
   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.
   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A — INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.
25. PROVIDE CONCRETE MECHANICAL EQUIPMENT PADS PER DETAILS 9 & 10/S5-103 AS REQUIRED BY MECHANICAL AND ARCHITECTURAL DRAWINGS.

PLAN NORTH

S1 | FRAMING PLAN: LEVEL 3 – BLDG F (SOUTH)
S2-103.2 | REF. SCALE: 1/8" = 1'-0"

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
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KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
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CARLSBAD, CA 92010
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LANDSCAPE ARCHITECT
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PROJECT NO: 60004775
DRAWN BY:
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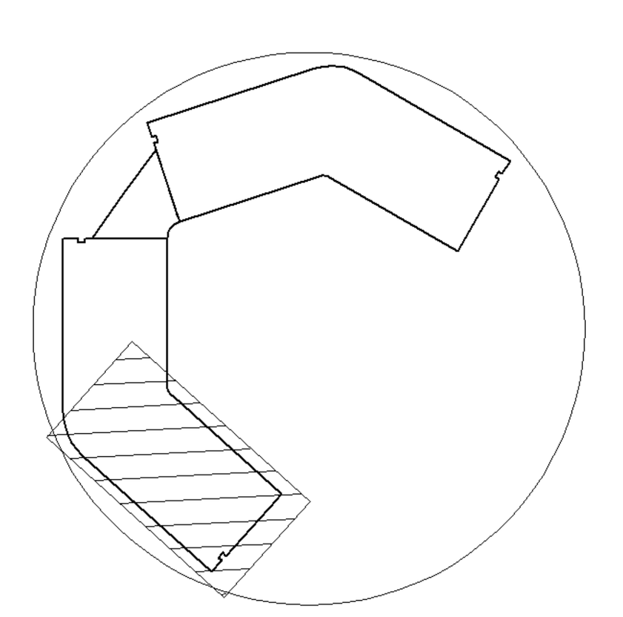
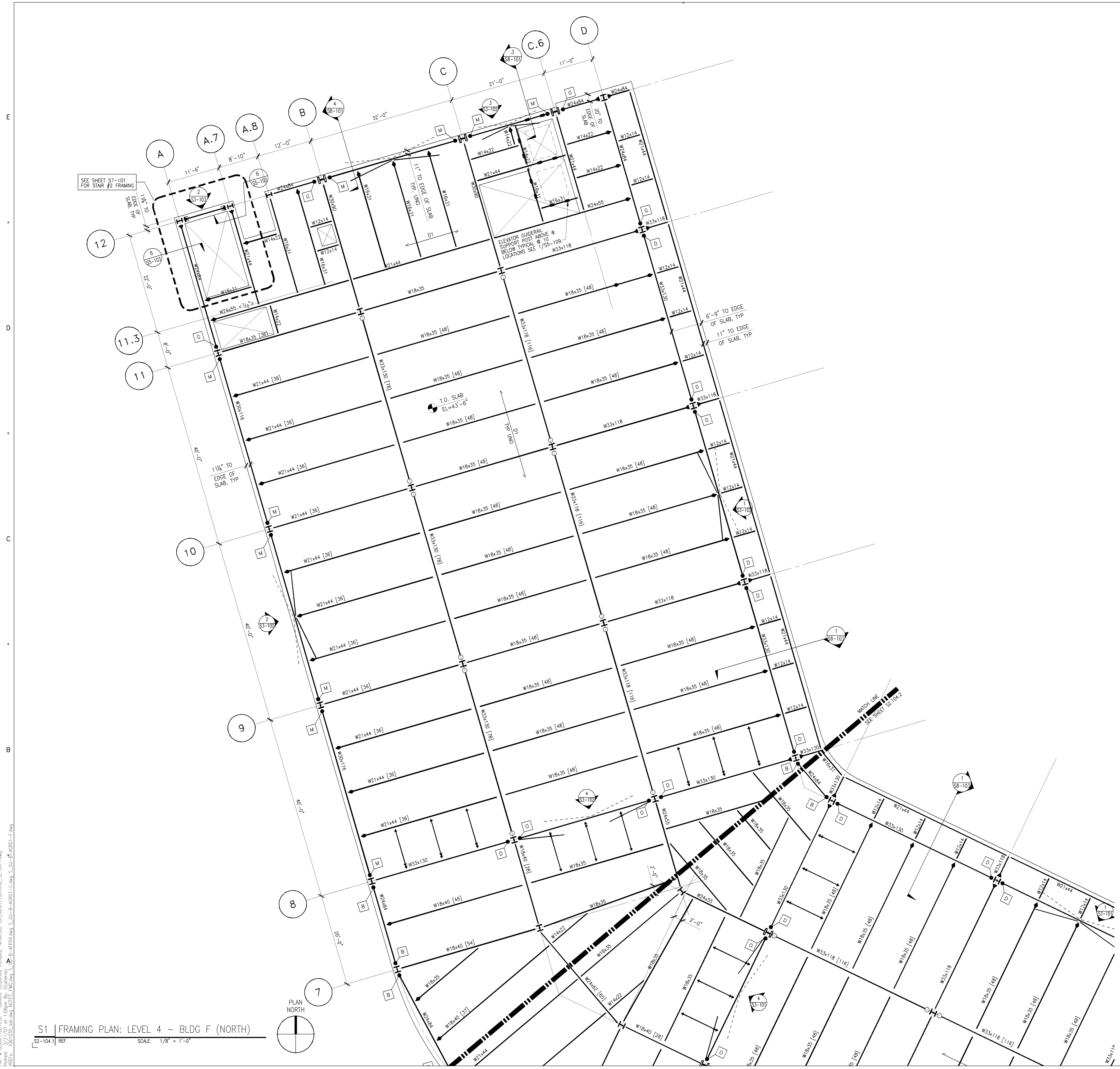
KEY PLAN



SHEET TITLE

FRAMING PLAN: LEVEL 3 – BUILDING F (SOUTH)

S2-103.2



S1 | FRAMING PLAN: LEVEL 4 – BLDG F (NORTH)

S2-104.1 | REF. SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

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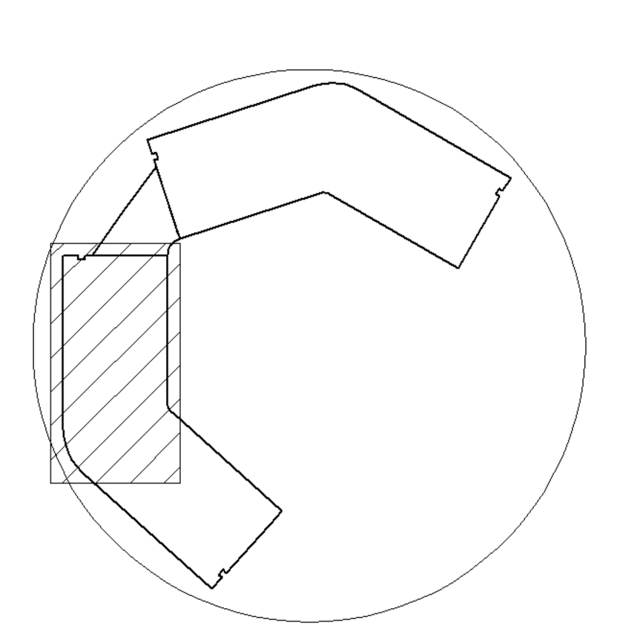

SHEET TITLE

FRAMING PLAN: LEVEL 4 – BUILDING F (NORTH)

S2-104.1

PLAN
NORTH

S1 | FRAMING PLAN: LEVEL 4 – BLDG F (SOUTH)

S2-104.2 | REF. SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

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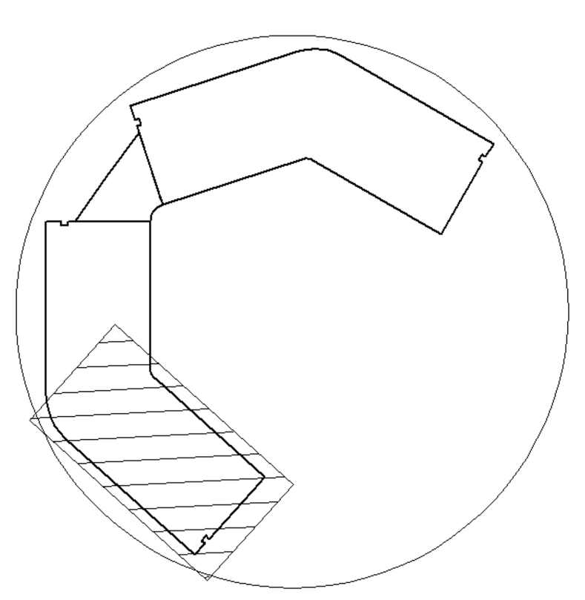
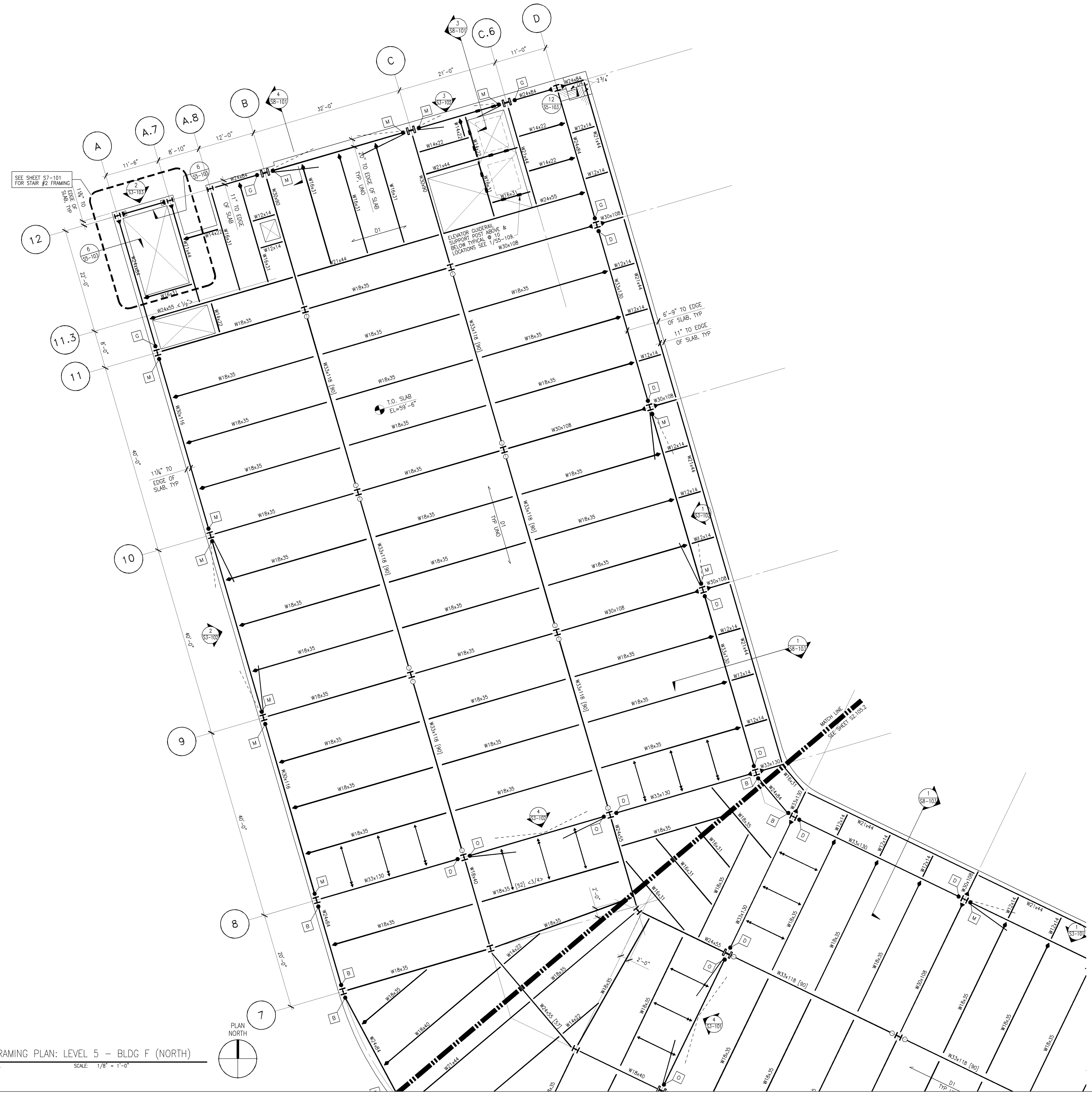


SHEET TITLE

FRAMING PLAN: LEVEL 4 – BUILDING F (SOUTH)

S2-104.2



S1 FRAMING PLAN: LEVEL 5 – BLDG F (NORTH)

S2-105.1 REF. SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

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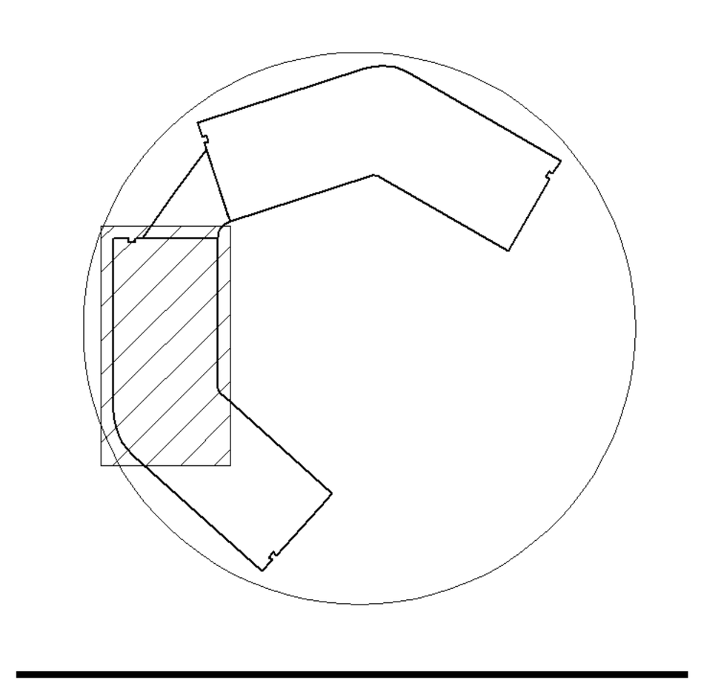
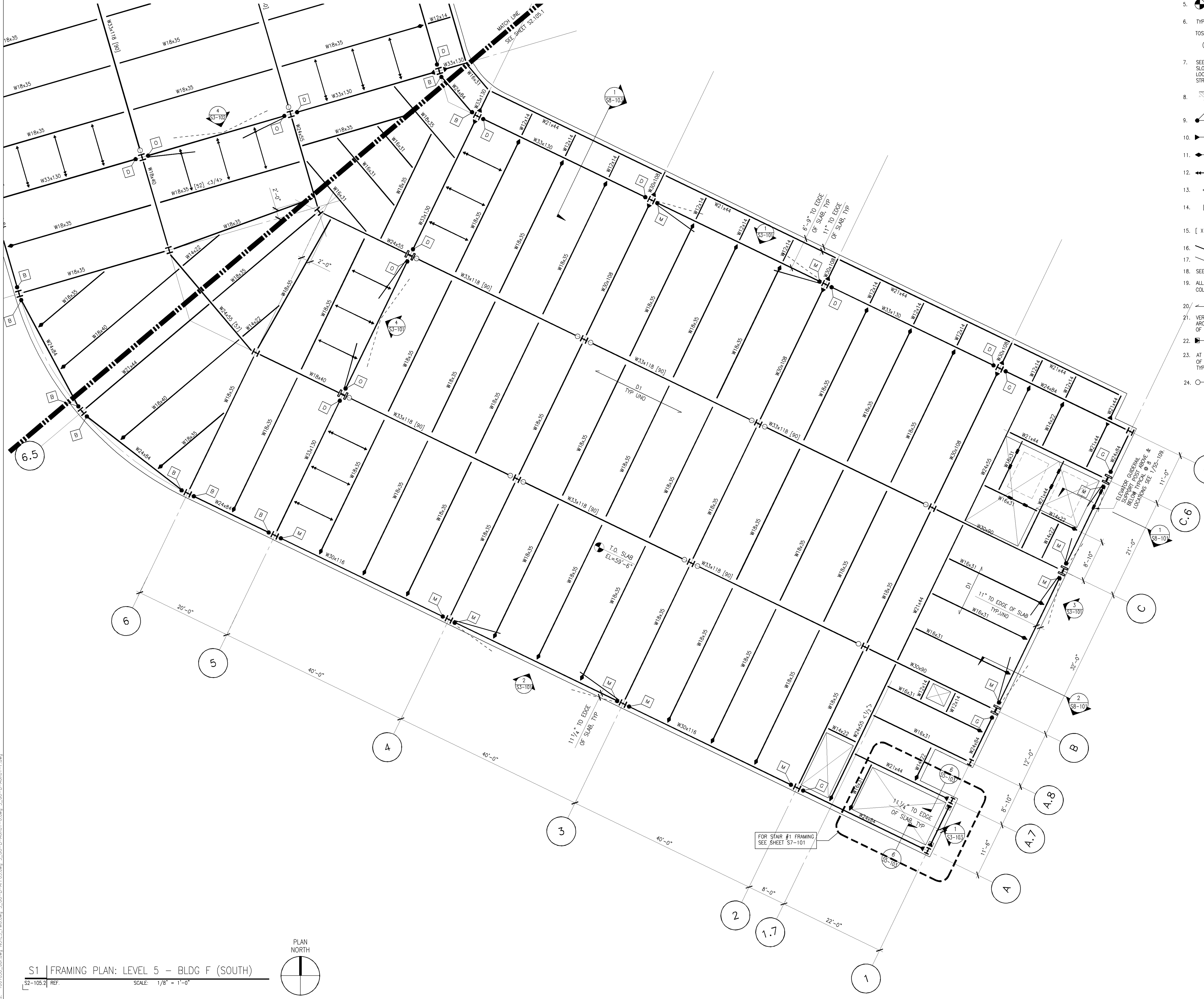


SHEET TITLE

FRAMING PLAN: LEVEL 5 – BUILDING F (NORTH)

S2-105.1



S1 | FRAMING PLAN: LEVEL 5 – BLDG F (SOUTH)
S2-105.2 | REF. SCALE: 1/8" = 1'-0"

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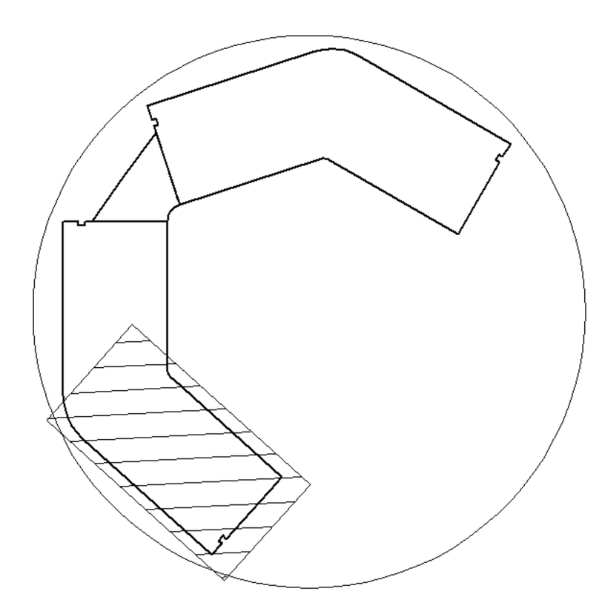
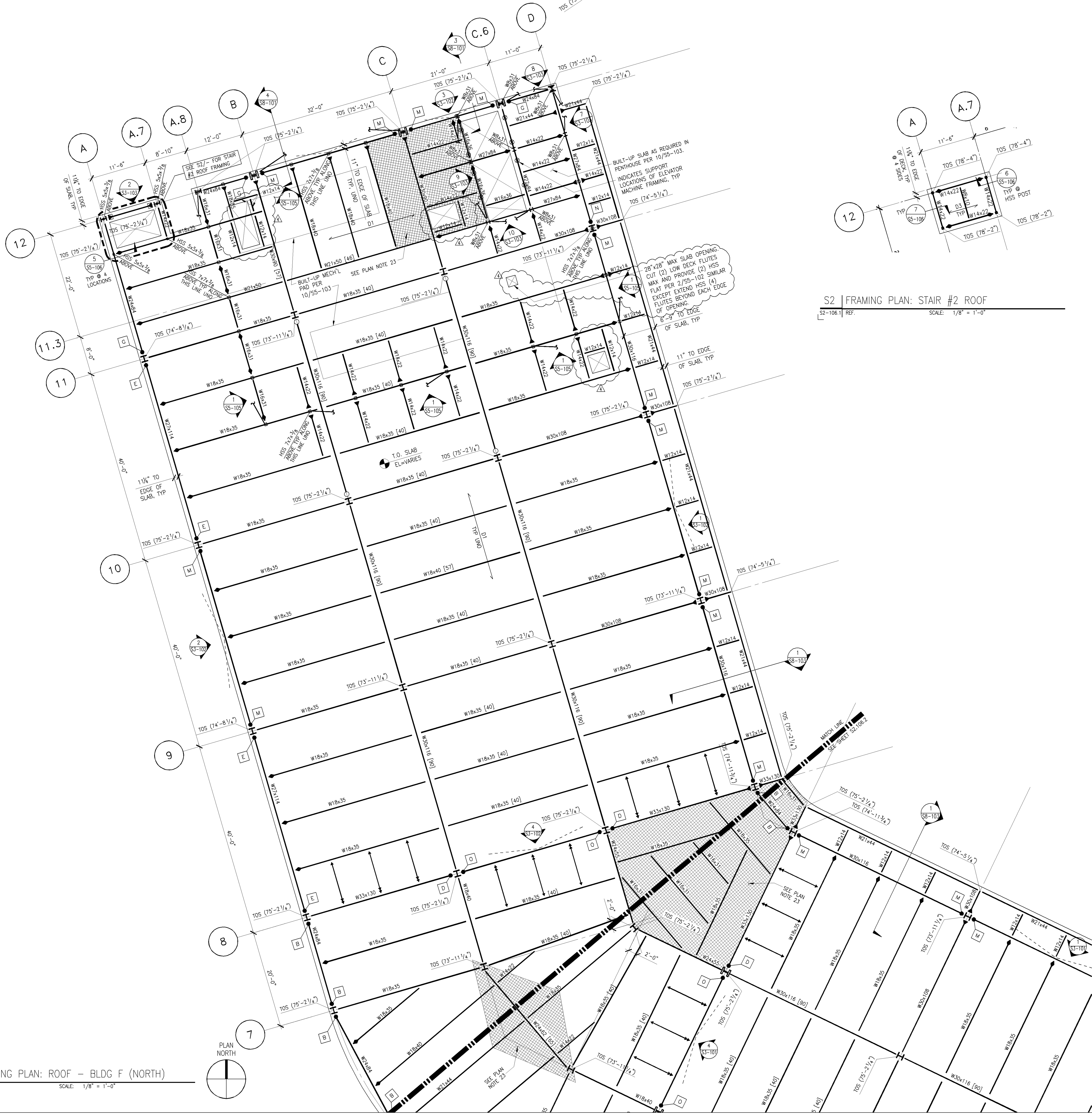


SHEET TITLE

FRAMING PLAN: LEVEL 5 – BUILDING F (SOUTH)

S2-105.2



S1 | FRAMING PLAN: ROOF – BLDG F (NORTH)
S2-106.1 | REF. SCALE: 1/8" = 1'-0"

S2 | FRAMING PLAN: STAIR #2 ROOF
S2-106.1 | REF. SCALE: 1/8" = 1'-0"

ROOF FRAMING NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.
   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.
   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. INDICATES AREAS OF TAPERED RIGID INSULATION PER ARCH. WHERE ROOF SLOPES ARE NOT OBTAINABLE USING THE SLOPED STRUCTURAL STEEL FRAMING, PROVIDE TAPERED RIGID INSULATION AS REQUIRED TO ACHIEVE THE SLOPES INDICATED ON THE ARCHITECTURAL DRAWINGS. PLEASE NOTE TAPERED RIGID INSULATION MAY BE NECESSARY IN ADDITIONAL ISOLATED AREAS NOT INDICATED ON PLAN. THE USE OF ADDITIONAL LIGHT WEIGHT CONCRETE FILL (BEYOND THE THICKNESS INDICATED ON PLAN) TO ACHIEVE ROOF SLOPES IS NOT PERMITTED WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
24. IF REQUIRED TO MAINTAIN CONTACT BETWEEN DECK AND STRUCTURAL STEEL FRAMING AND TO AVOID EXCESSIVE DECK WARPING, REFER TO DETAIL 7/S5-103.
25. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
26. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

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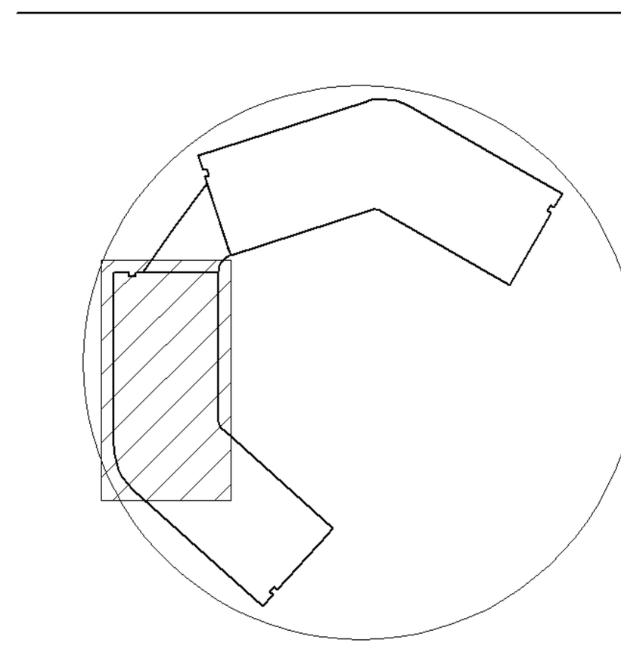
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

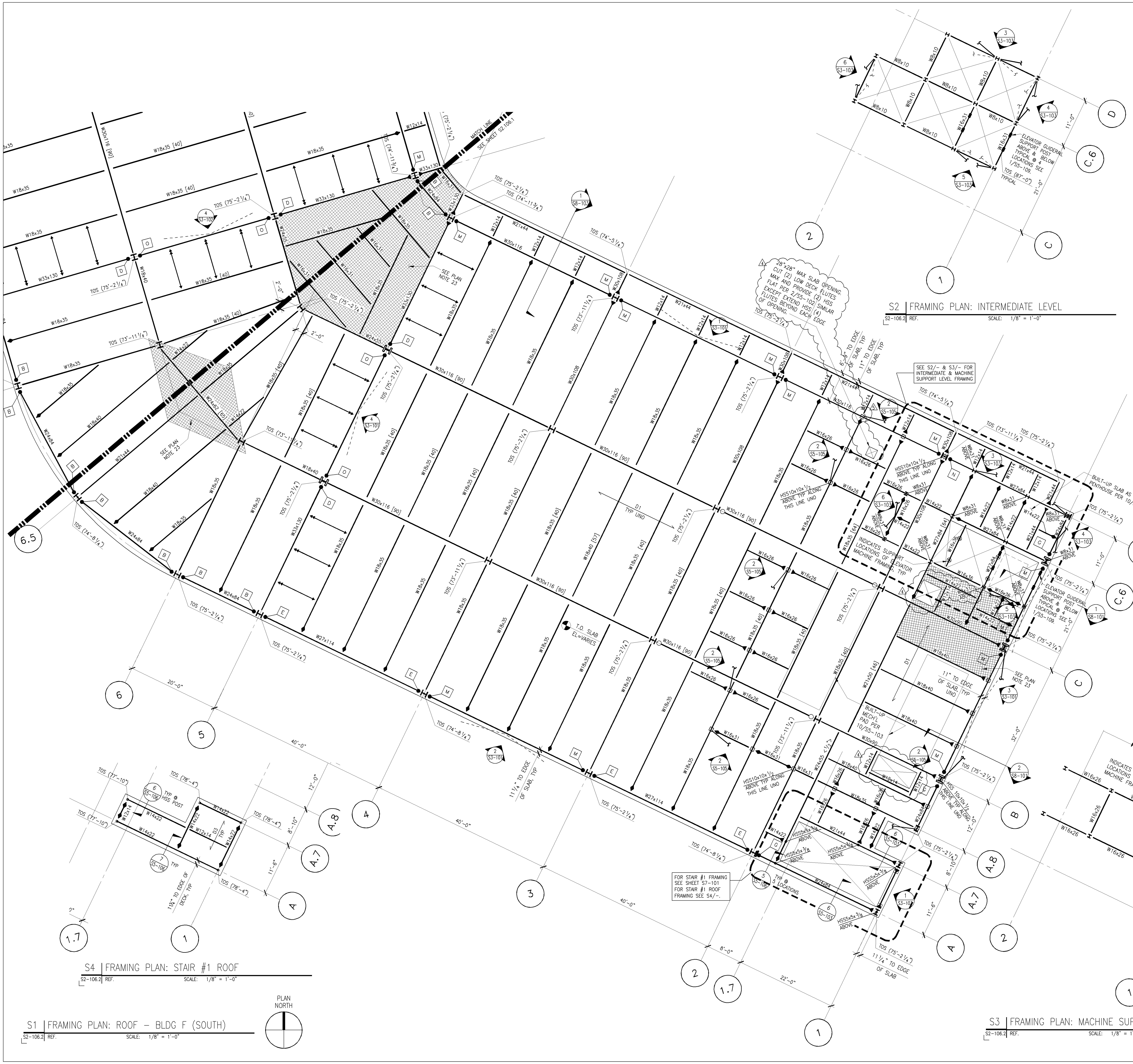
KEY PLAN



SHEET TITLE

FRAMING PLAN: ROOF – BUILDING F (NORTH)

S2-106.1



S1 | FRAMING PLAN: ROOF – BLDG F (SOUTH)
S2-106.2 REF. SCALE: 1/8" = 1'-0"

S2 | FRAMING PLAN: INTERMEDIATE LEVEL
S2-106.2 REF. SCALE: 1/8" = 1'-0"

S3 | FRAMING PLAN: MACHINE SUPPORT LEVEL
S2-106.2 REF. SCALE: 1/8" = 1'-0"

S4 | FRAMING PLAN: STAIR #1 ROOF
S2-106.2 REF. SCALE: 1/8" = 1'-0"

ROOF FRAMING NOTES:

1. SEE SHEET S1–101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4–101 & S4–102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5–101, S5–102, S5–103 & S5–104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3–201, S3–202, S3–211, S3–212 & S3–213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'–XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.
   TOS (X'–XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.
   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5–104, TYPICAL.
10. INDICATES NON–SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5–101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5–101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5–101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5–102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X–Y–Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5–102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5–103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5–102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. INDICATES AREAS OF TAPERED RIGID INSULATION PER ARCH. WHERE ROOF SLOPES ARE NOT OBTAINABLE USING THE SLOPED STRUCTURAL STEEL FRAMING, PROVIDE TAPERED RIGID INSULATION AS REQUIRED TO ACHIEVE THE SLOPES INDICATED ON THE ARCHITECTURAL DRAWINGS. PLEASE NOTE TAPERED RIGID INSULATION MAY BE NECESSARY IN ADDITIONAL ISOLATED AREAS NOT INDICATED ON PLAN. THE USE OF ADDITIONAL LIGHT WEIGHT CONCRETE FILL (BEYOND THE THICKNESS INDICATED ON PLAN) TO ACHIEVE ROOF SLOPES IS NOT PERMITTED WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
24. IF REQUIRED TO MAINTAIN CONTACT BETWEEN DECK AND STRUCTURAL STEEL FRAMING AND TO AVOID EXCESSIVE DECK WARPING, REFER TO DETAIL 7/S5–103.
25. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
26. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5–101.

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Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591–4628

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DMJM DESIGN | AECOM
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LOS ANGELES, CA 90071
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CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
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ESCANDIDO, CA 92029
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STRUCTURAL ENGINEER
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Tel: 760.496.3714
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TORRANCE, CA 90503
Tel: 310.328.4734
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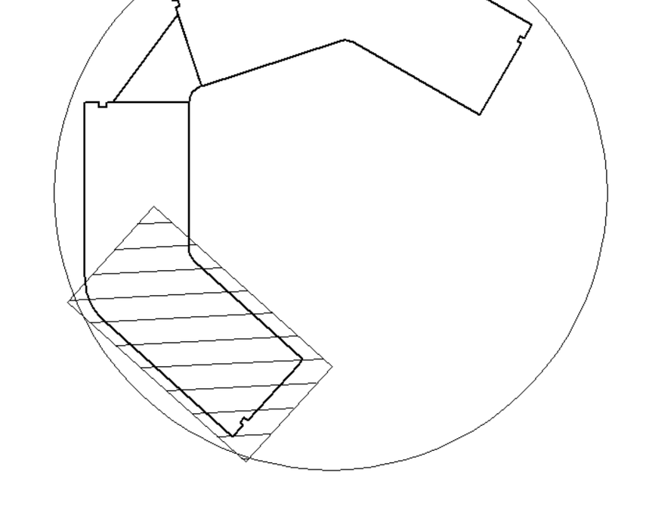
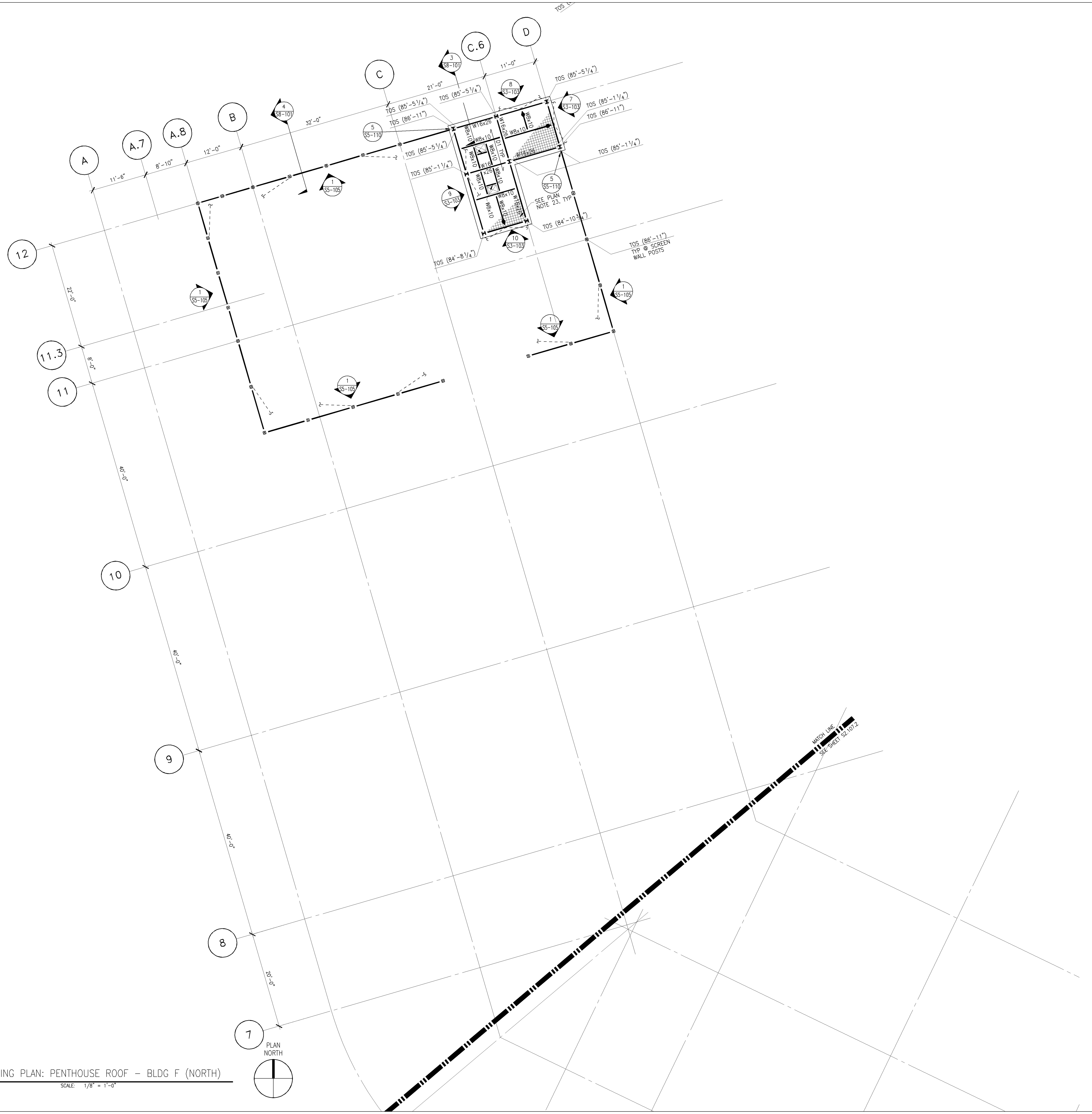
KEY PLAN



SHEET TITLE

FRAMING PLAN: ROOF – BUILDING F (SOUTH)

S2-106.2



S1 | FRAMING PLAN: PENTHOUSE ROOF – BLDG F (NORTH)

S2-107.1 | REF. SCALE: 1/8" = 1'-0"

ROOF FRAMING NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
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24. IF REQUIRED TO MAINTAIN CONTACT BETWEEN DECK AND STRUCTURAL STEEL FRAMING AND TO AVOID EXCESSIVE DECK WARPING, REFER TO DETAIL 7/S5-103.
25. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
26. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

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TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

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LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CONSULTANTS

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PROJECT NO: 60004775
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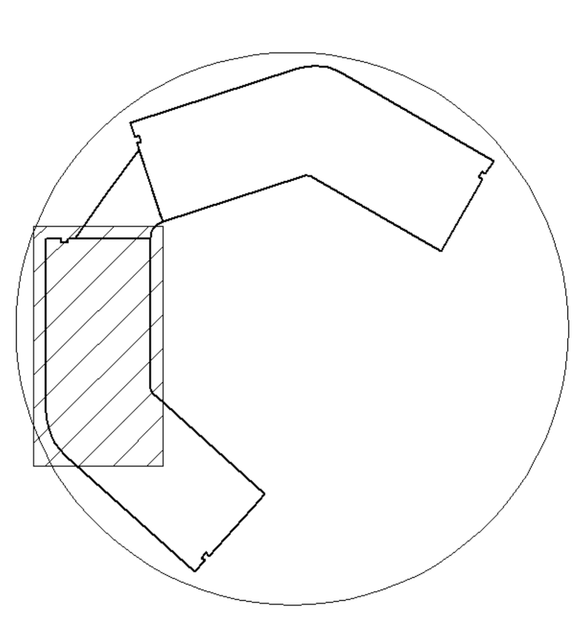
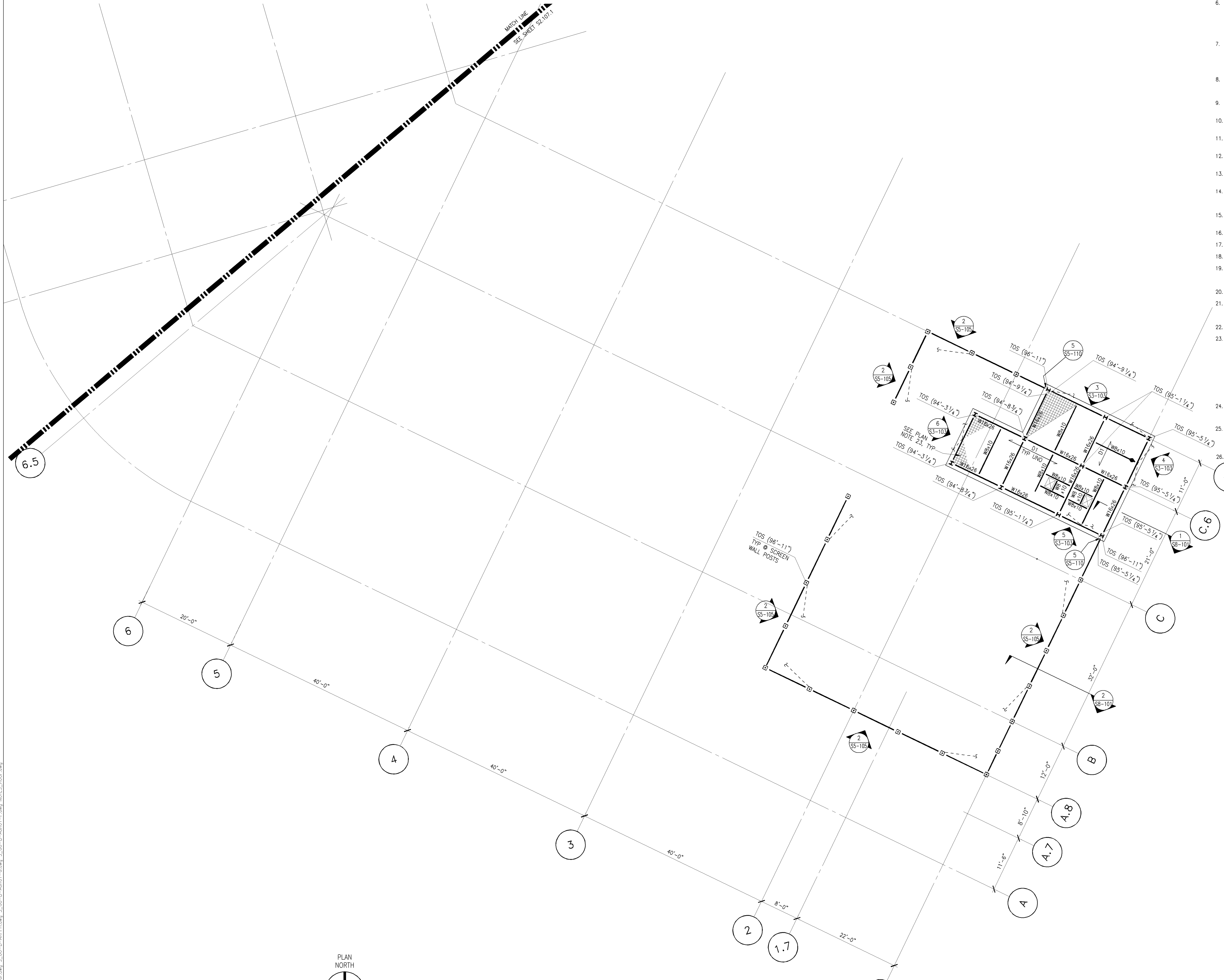
KEY PLAN



SHEET TITLE

FRAMING PLAN: PENTHOUSE ROOF – BUILDING F (NORTH)

S2-107.1



S1 | FRAMING PLAN: PENTHOUSE ROOF – BLDG F (SOUTH)

S2-107.2 REF. SCALE: 1/8" = 1'-0"

ROOF FRAMING NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
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15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. INDICATES AREAS OF TAPERED RIGID INSULATION PER ARCH. WHERE ROOF SLOPES ARE NOT OBTAINABLE USING THE SLOPED STRUCTURAL STEEL FRAMING, PROVIDE TAPERED RIGID INSULATION AS REQUIRED TO ACHIEVE THE SLOPES INDICATED ON THE ARCHITECTURAL DRAWINGS. PLEASE NOTE TAPERED RIGID INSULATION MAY BE NECESSARY IN ADDITIONAL ISOLATED AREAS NOT INDICATED ON PLAN. THE USE OF ADDITIONAL LIGHT WEIGHT CONCRETE FILL (BEYOND THE THICKNESS INDICATED ON PLAN) TO ACHIEVE ROOF SLOPES IS NOT PERMITTED WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
24. IF REQUIRED TO MAINTAIN CONTACT BETWEEN DECK AND STRUCTURAL STEEL FRAMING AND TO AVOID EXCESSIVE DECK WARPING, REFER TO DETAIL 7/S5-103.
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26. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

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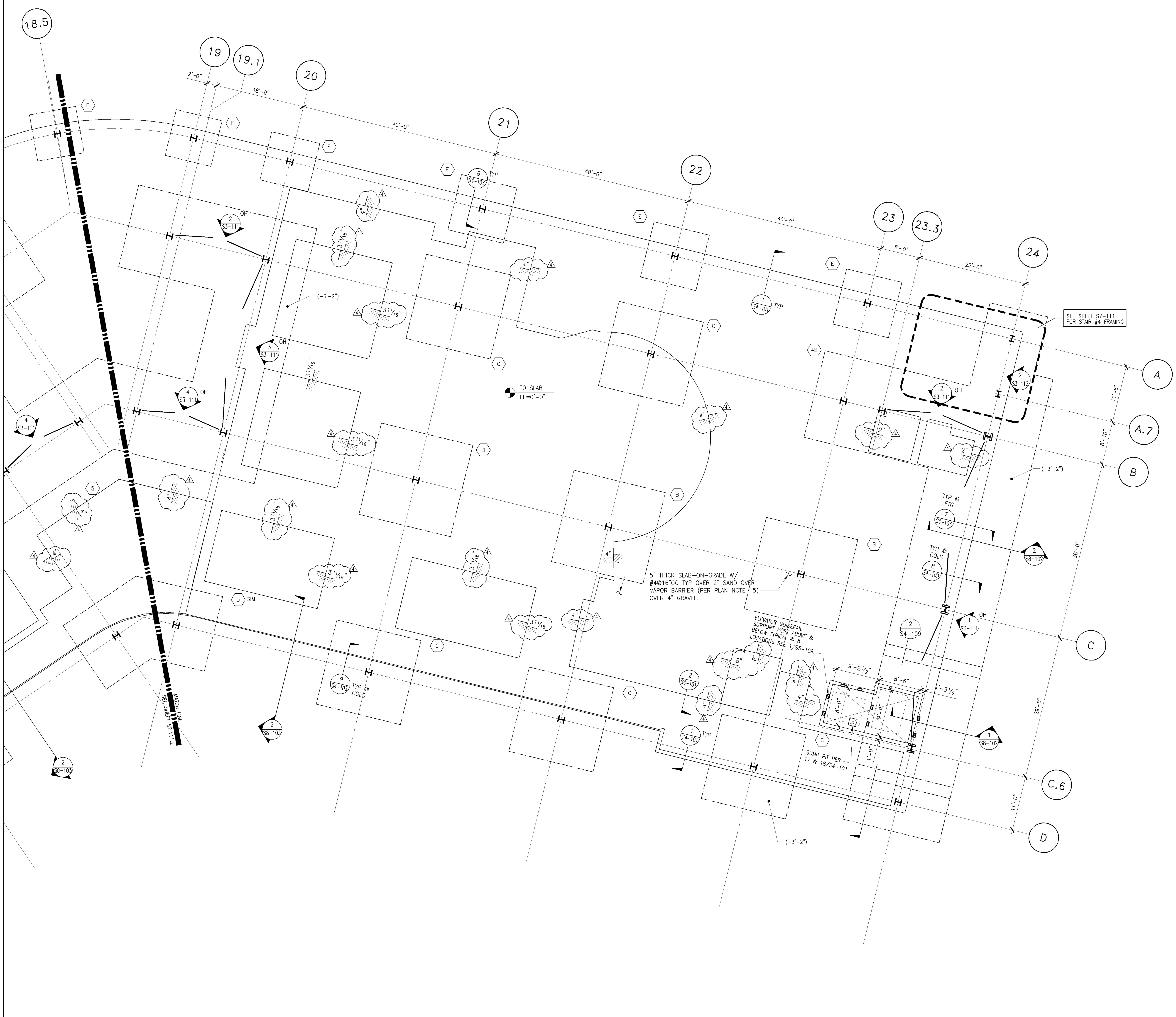
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

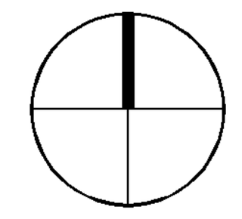
FRAMING PLAN: PENTHOUSE ROOF – BUILDING F (SOUTH)

S2-107.2



S1 | FOUNDATION PLAN: LEVEL 1 – BLDG G (EAST)

S2-111.1 | REF. SCALE: 1/8" = 1'-0"



FOUNDATION PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. (-X'-XX") INDICATES TOP OF CONCRETE FOOTING BELOW THE ADJACENT TOP OF SLAB ELEVATION. ALL FOOTINGS NOT NOTED SHALL BE (-1'-6") BELOW THE ADJACENT TOP OF SLAB ELEVATION.
7. X" INDICATES SLAB DEPRESSION
8. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
9. # INDICATES FOOTING CALLOUT PER SCHEDULE ON DETAIL 1/S4-103.
10. SEE ARCHITECTURAL FOR CONCRETE SLAB DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
11. VERIFY LOCATIONS, DIMENSIONS, AND TYPE OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT. REFER TO DETAILS 7 & 15/S4-101.
12. VERIFY LOCATIONS AND DIMENSIONS OF SUMP PITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
13. SEE SOILS REPORTS FOR OVER EXCAVATION REQUIREMENTS.
14. INDICATES STEPPED FOOTING PER DETAIL 12/S4-101.
15. VAPOR BARRIER SHALL HAVE A PERMEANCE OF 0.01 PERMS OR LESS, MEETING ASTM E-1745 (CLASS "A"), BE LOCATED PER ACI LOCATION GUIDELINES, AND BE INSTALLED PER ASTM E-1643.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

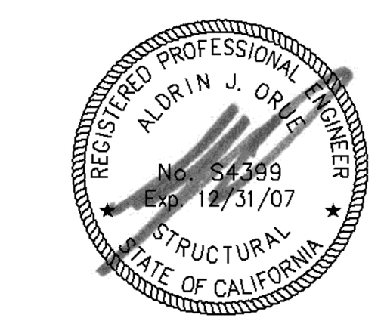
EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

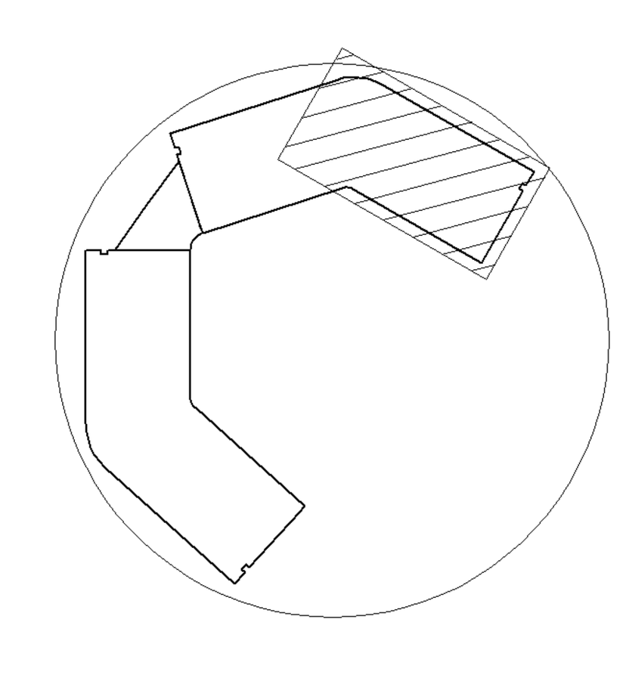


ISSUE

MARK	DATE	DESCRIPTION
4	08-30-07	BULLETIN 010 MODS.
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

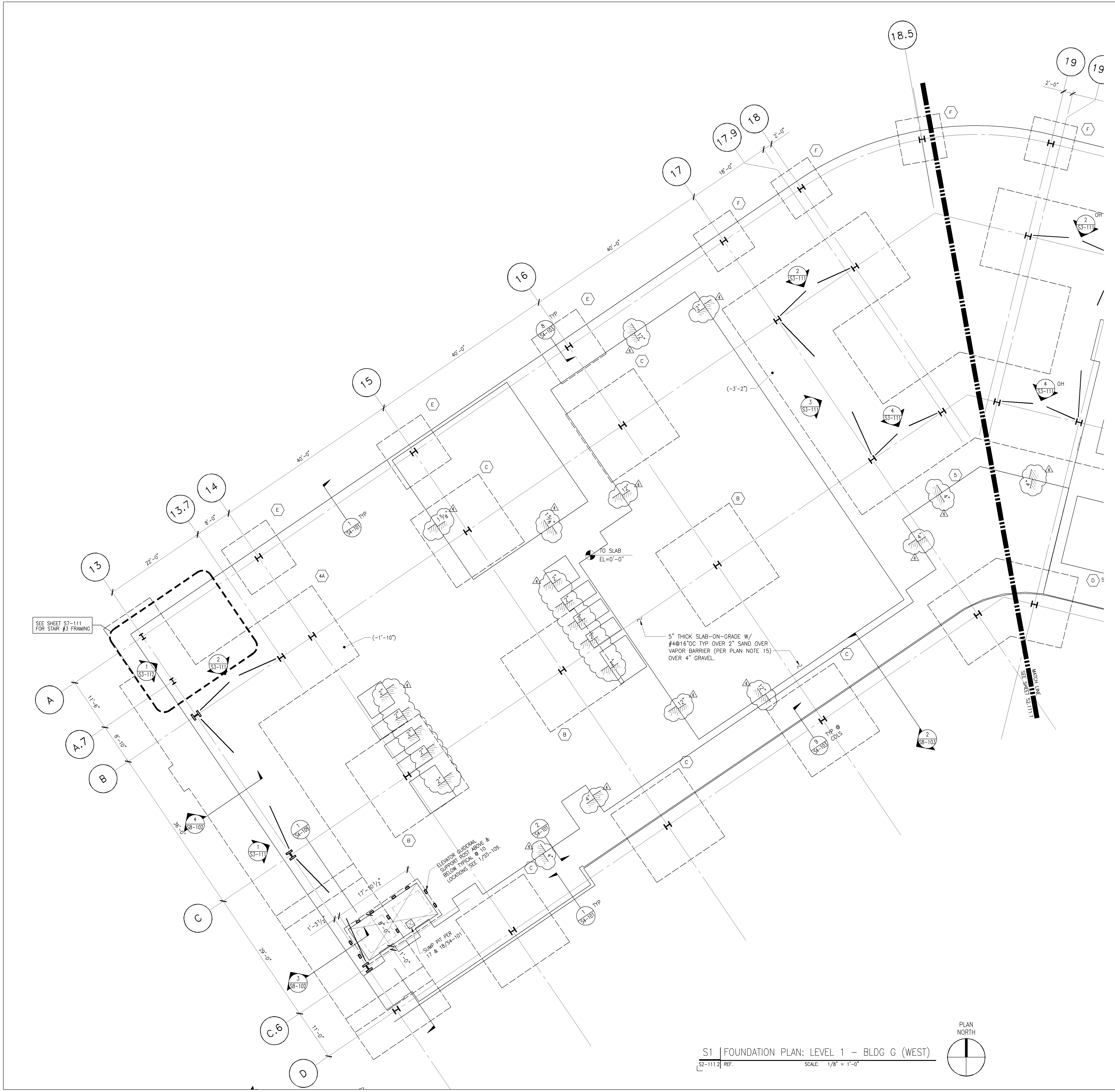
KEY PLAN



SHEET TITLE

FOUNDATION PLAN: LEVEL 1 – BUILDING G (EAST)

S2-111.1



S1 | FOUNDATION PLAN: LEVEL 1 – BLDG G (WEST)
S2-111.2 REF. SCALE: 1/8" = 1'-0"

FOUNDATION PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. (-X'-XX") INDICATES TOP OF CONCRETE FOOTING BELOW THE ADJACENT TOP OF SLAB ELEVATION. ALL FOOTINGS NOT NOTED SHALL BE (-1'-6") BELOW THE ADJACENT TOP OF SLAB ELEVATION.
7. X" INDICATES SLAB DEPRESSION
8. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
9. # INDICATES FOOTING CALLOUT PER SCHEDULE ON DETAIL 1/S4-103.
10. SEE ARCHITECTURAL FOR CONCRETE SLAB DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
11. VERIFY LOCATIONS, DIMENSIONS, AND TYPE OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT. REFER TO DETAILS 7 & 15/S4-101.
12. VERIFY LOCATIONS AND DIMENSIONS OF SUMP PITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
13. SEE SOILS REPORTS FOR OVER EXCAVATION REQUIREMENTS.
14. INDICATES STEPPED FOOTING PER DETAIL 12/S4-101.
15. VAPOR BARRIER SHALL HAVE A PERMEANCE OF 0.01 PERMS OR LESS, MEETING ASTM E-1745 (CLASS "A"), BE LOCATED PER ACI LOCATION GUIDELINES, AND BE INSTALLED PER ASTM E-1643.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
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Temecula, CA 92591-4628

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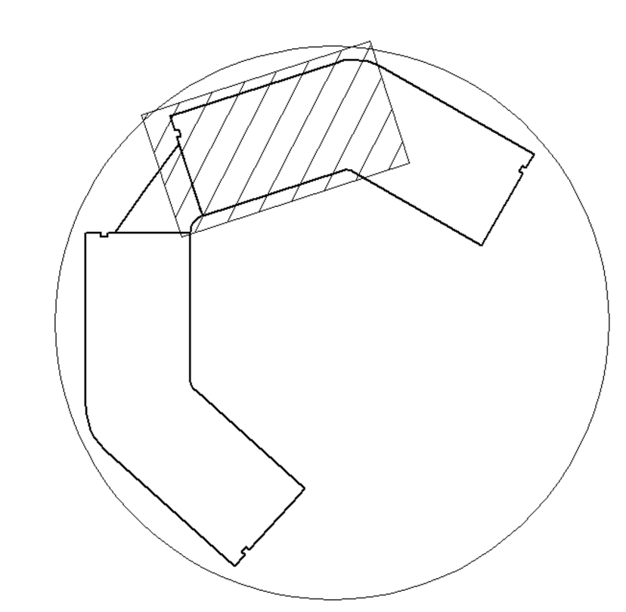
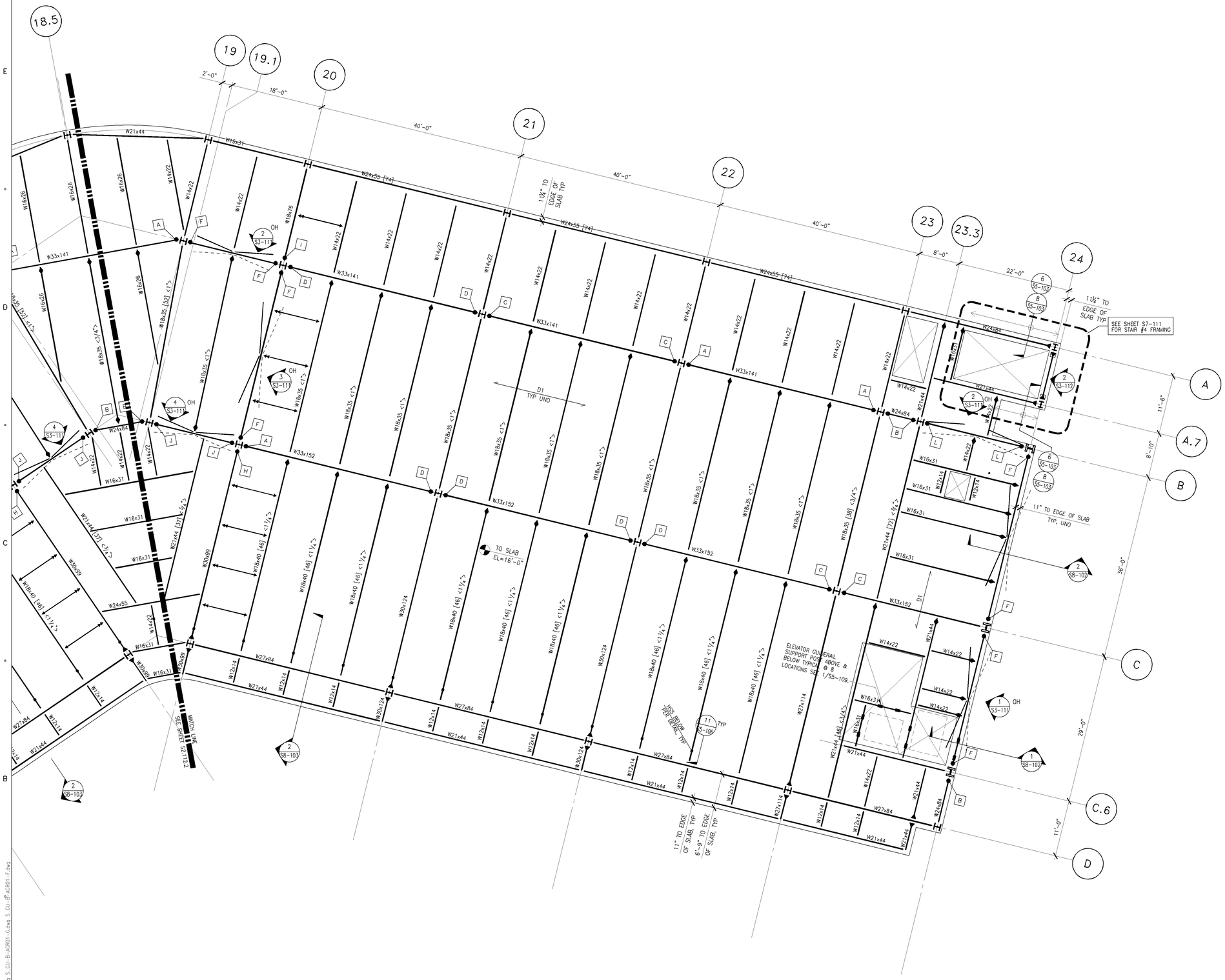
KEY PLAN



SHEET TITLE

FOUNDATION PLAN: LEVEL 1 – BUILDING G (WEST)

S2-111.2



FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6¼" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX")INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A ●—— INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. ▶—— INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. ◆—— INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. ↞——→ INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. ╲╱ INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. ⋰⋱ INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. ←D1→ INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. ▶—— INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. ○—— INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

S1 | FRAMING PLAN: LEVEL 2 – BLDG G (EAST)
S2-112.1 | REF. SCALE: 1/8" = 1'-0"

File: W:\2006\1061030 – Guidant Corporate Campus, Temecula, CA\Sdraft\1061030_S2.112.1.dwg
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REGISTRATION

ALDRIN J. ORUE
No. S4399
Exp. 12/31/07

ISSUE

MARK	DATE	DESCRIPTION
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1	10-23-06	BLDG & SAFETY SUBMITTAL

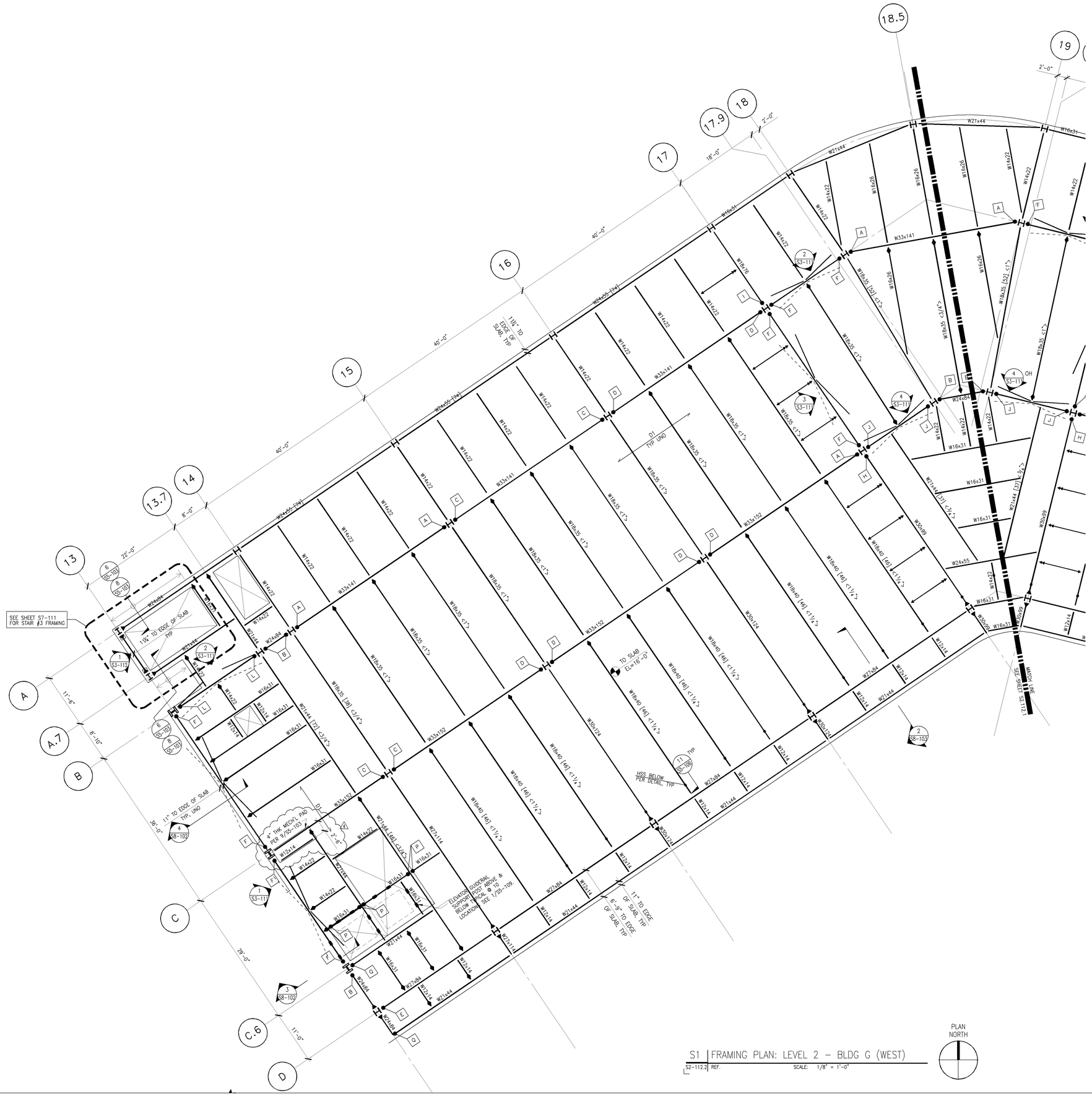
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

FRAMING PLAN:
LEVEL 2 –
BUILDING G (EAST)

S2-112.1



S1 | FRAMING PLAN: LEVEL 2 – BLDG G (WEST)

S2-112.2 REF. SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. ▶—— INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. ◆—— INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. ◀◀——▶ INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. ╲╱ INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. ╲╱ INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. ←D1→ INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. ▶—— INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. ○—— INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

OWNER/CLIENT

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REGISTRATION

REGISTERED PROFESSIONAL ENGINEER
ALDRIN J. ORDE
No. S4399
Exp. 12/31/07
STRUCTURAL
STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
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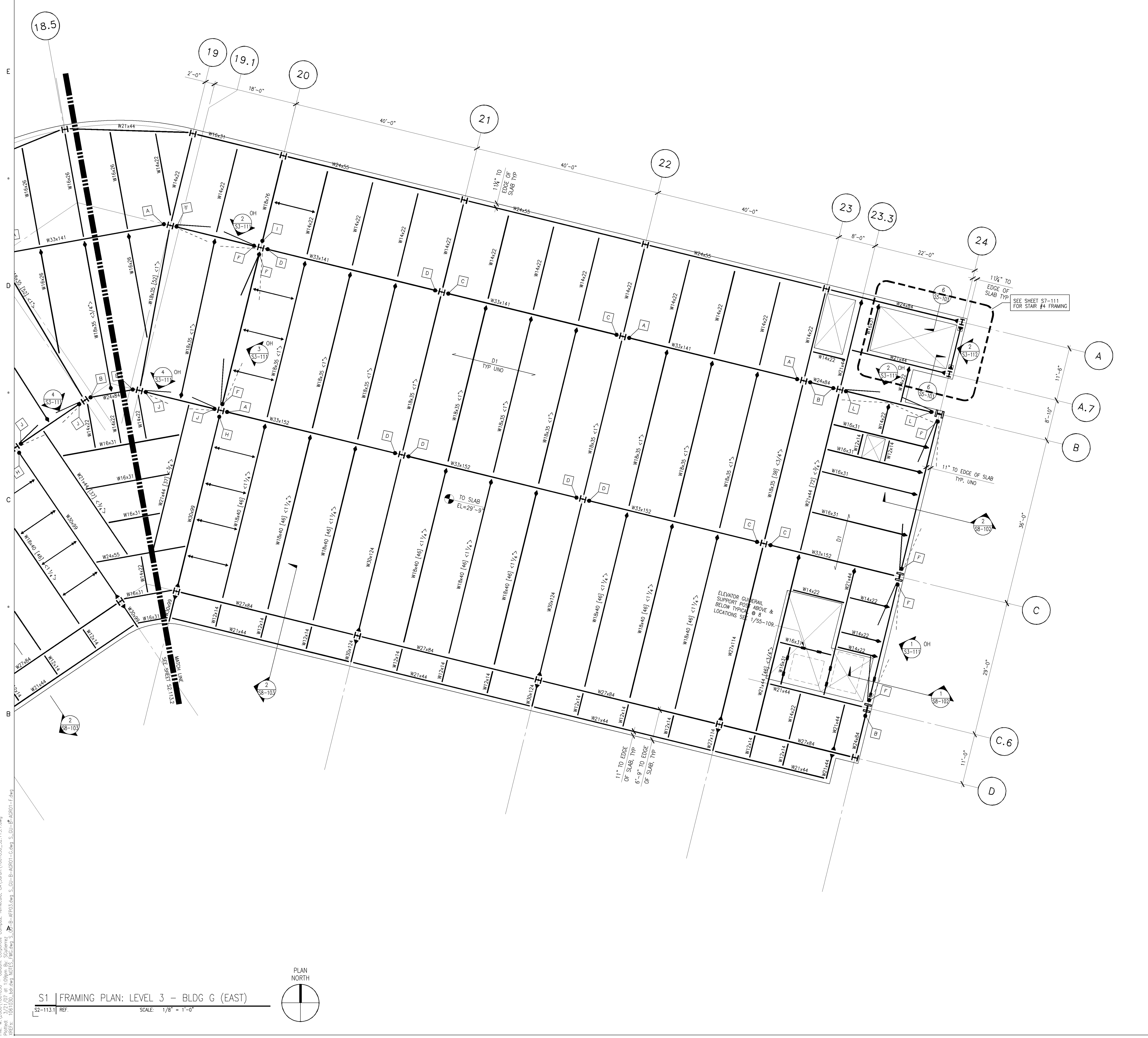
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

FRAMING PLAN:
LEVEL 2 –
BUILDING G (WEST)

S2-112.2



S1 | FRAMING PLAN: LEVEL 3 – BLDG G (EAST)

S2-113.1 | REF. SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

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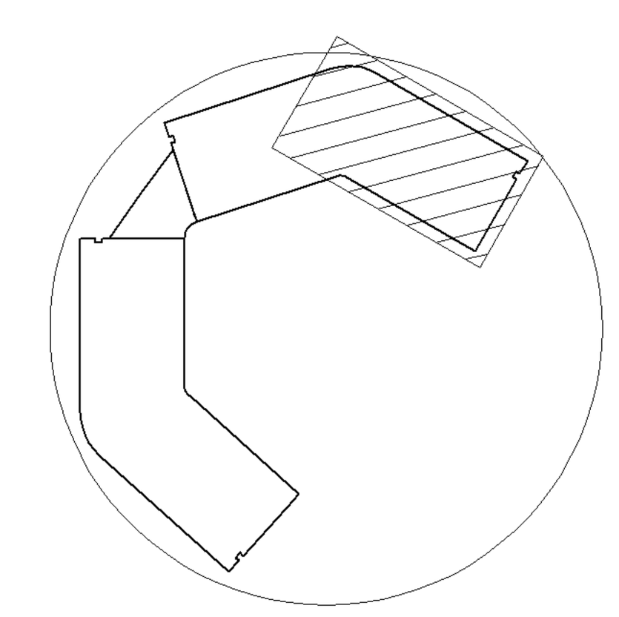
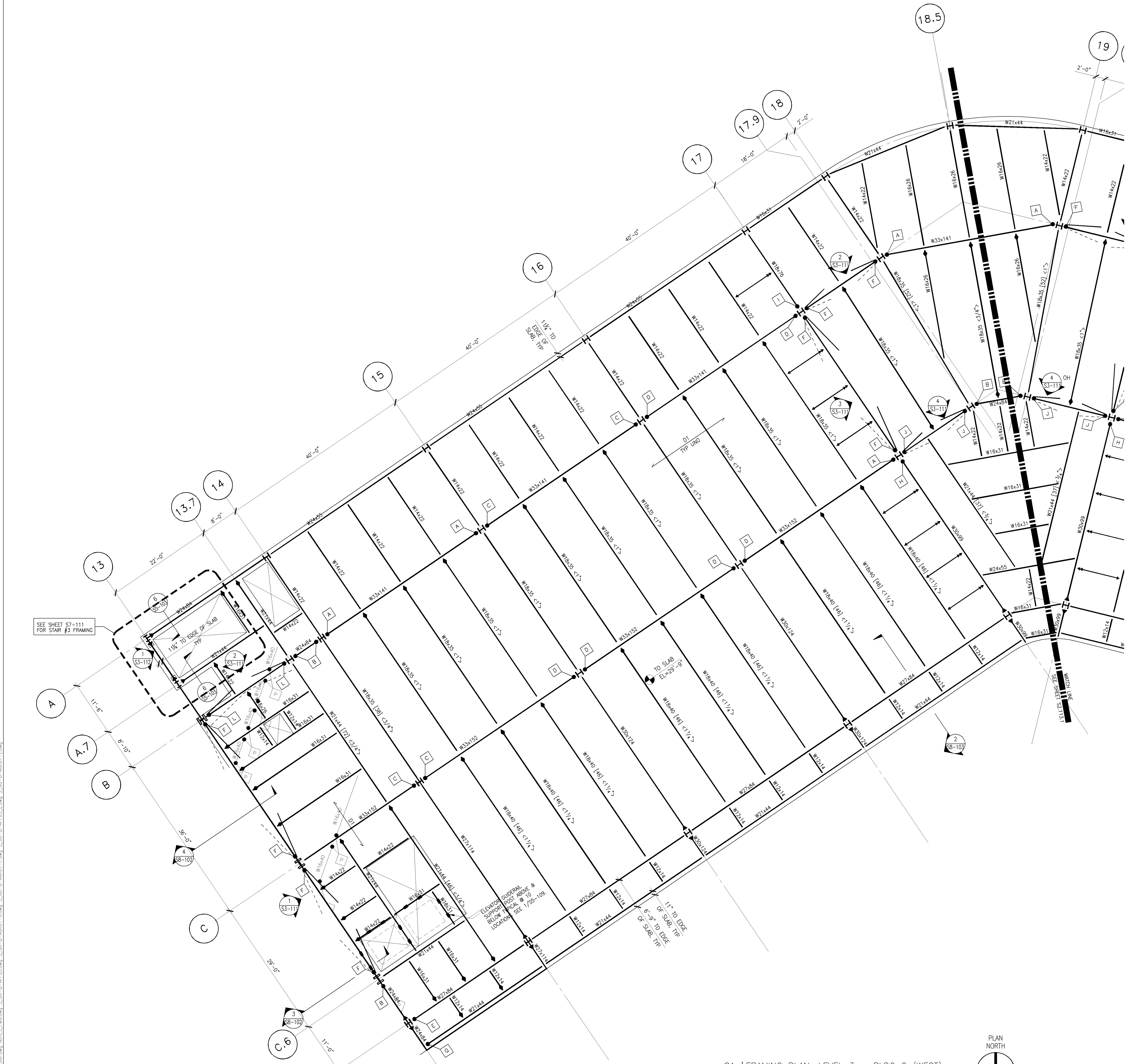


SHEET TITLE

FRAMING PLAN: LEVEL 3 – BUILDING G (EAST)

S2-113.1



S1 | FRAMING PLAN: LEVEL 3 – BLDG G (WEST)
S2-113.2 | REF. SCALE: 1/8" = 1'-0"

PLAN NORTH

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.
   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.
   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
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20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
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KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
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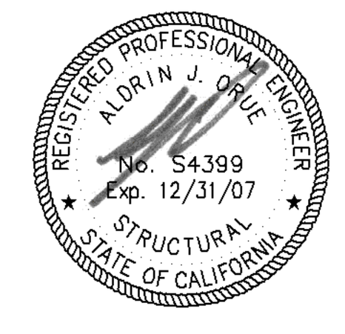
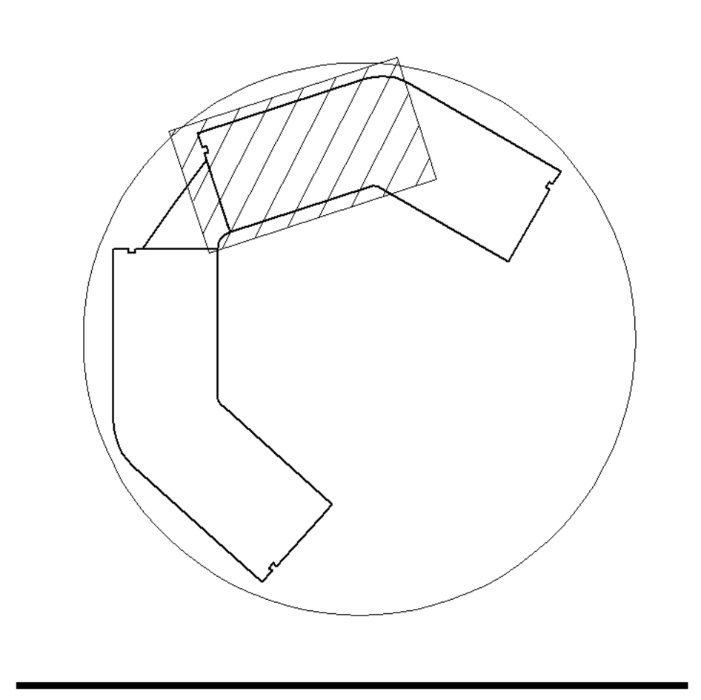


ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

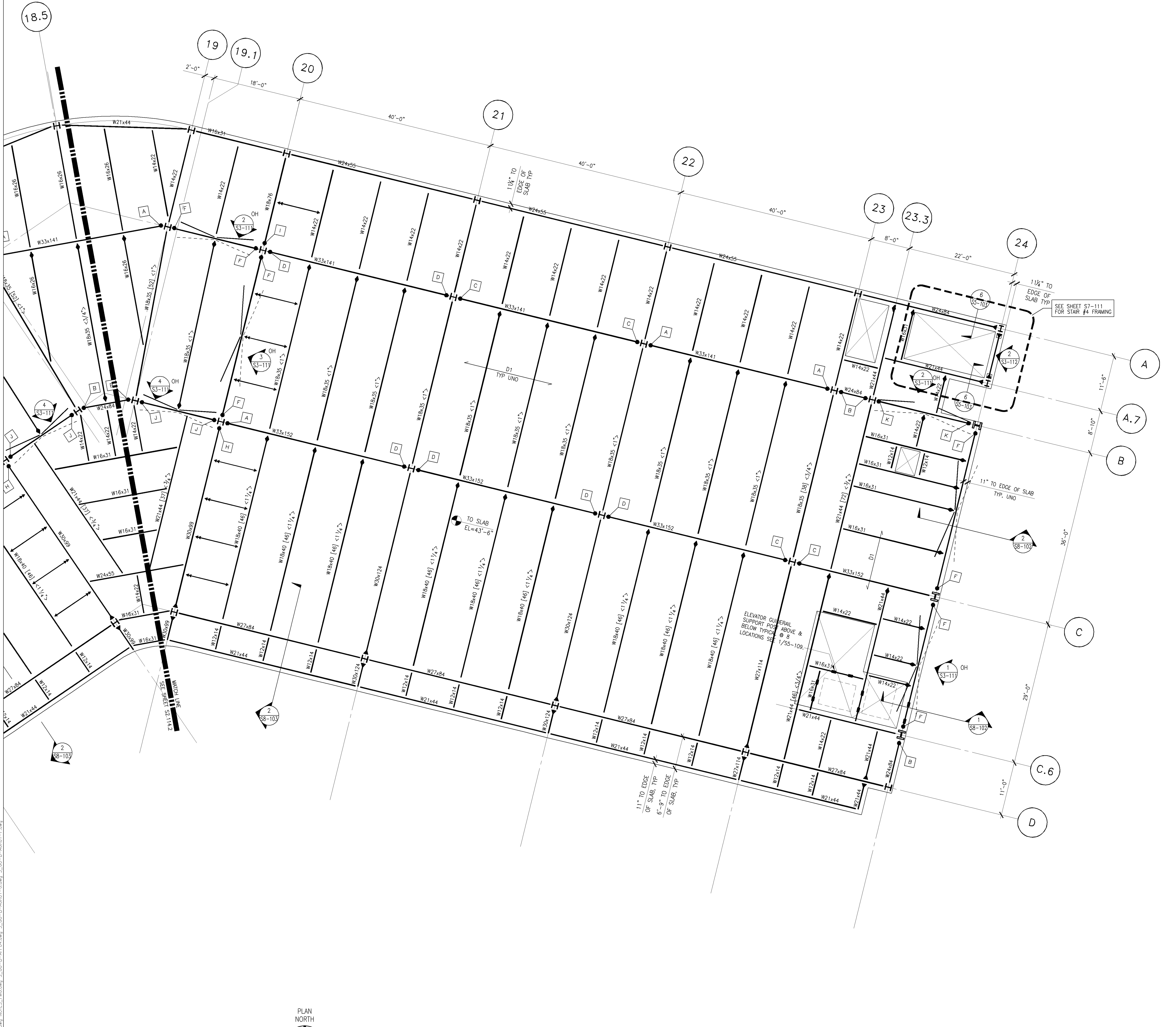
KEY PLAN



SHEET TITLE

FRAMING PLAN: LEVEL 3 – BUILDING G (WEST)

S2-113.2



S1 | FRAMING PLAN: LEVEL 4 – BLDG G (EAST)

S2-114.1 | REF. SCALE: 1/8" = 1'-0"

PLAN NORTH

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

OWNER/CLIENT

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DMJM DESIGN | AECOM

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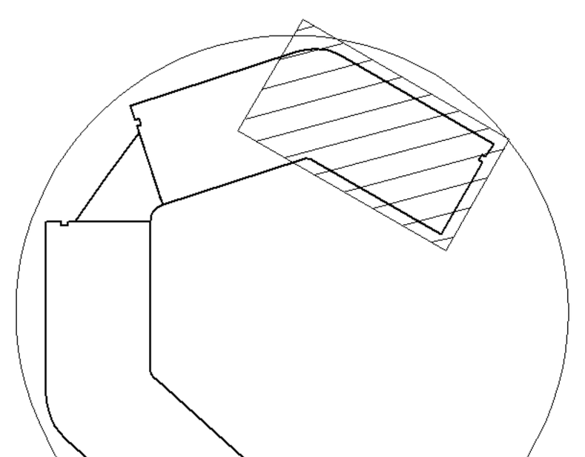
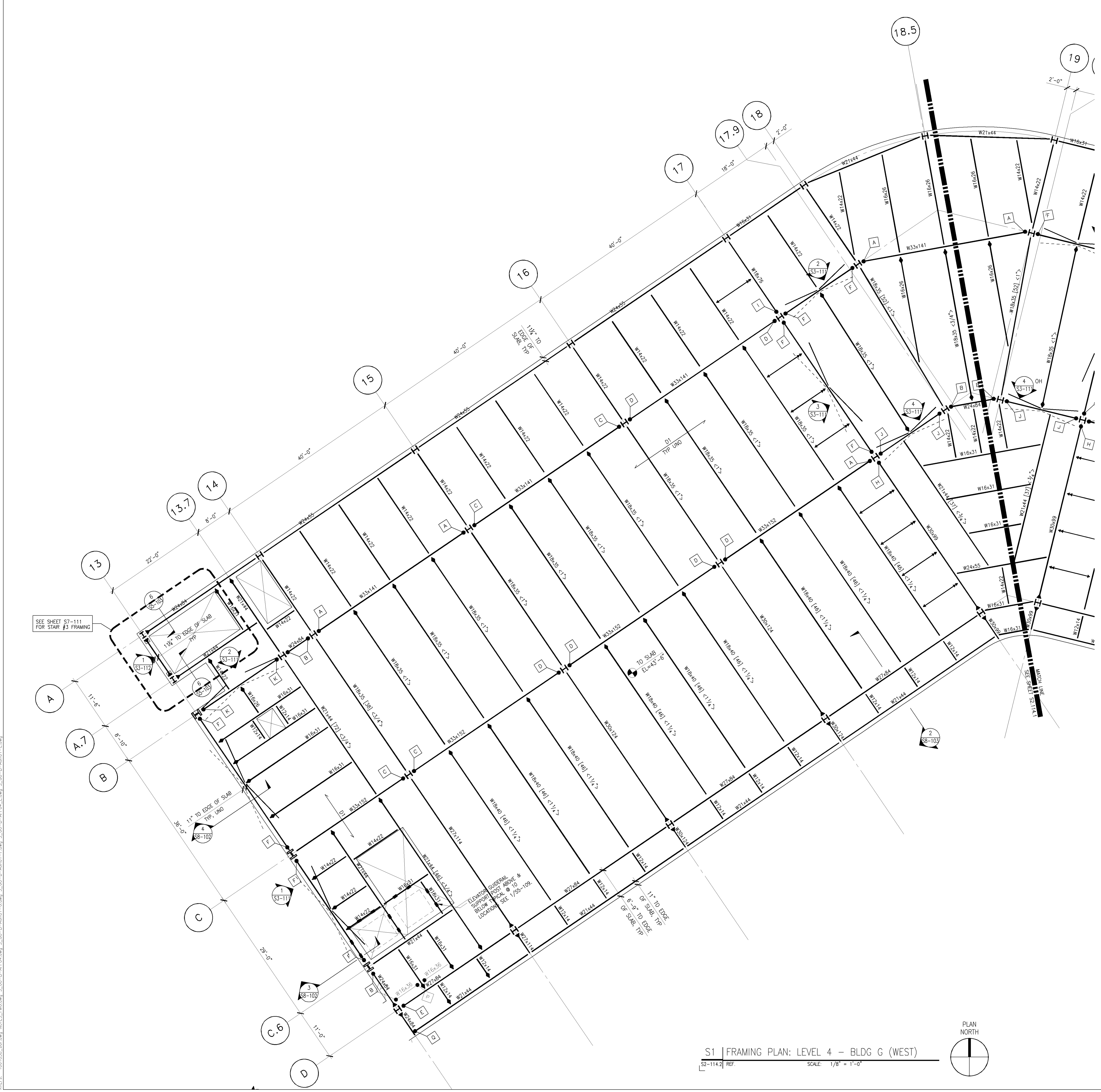


SHEET TITLE

FRAMING PLAN: LEVEL 4 – BUILDING G (EAST)

S2-114.1



S1 | FRAMING PLAN: LEVEL 4 – BLDG G (WEST)

S2-114.2 REF. SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
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12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
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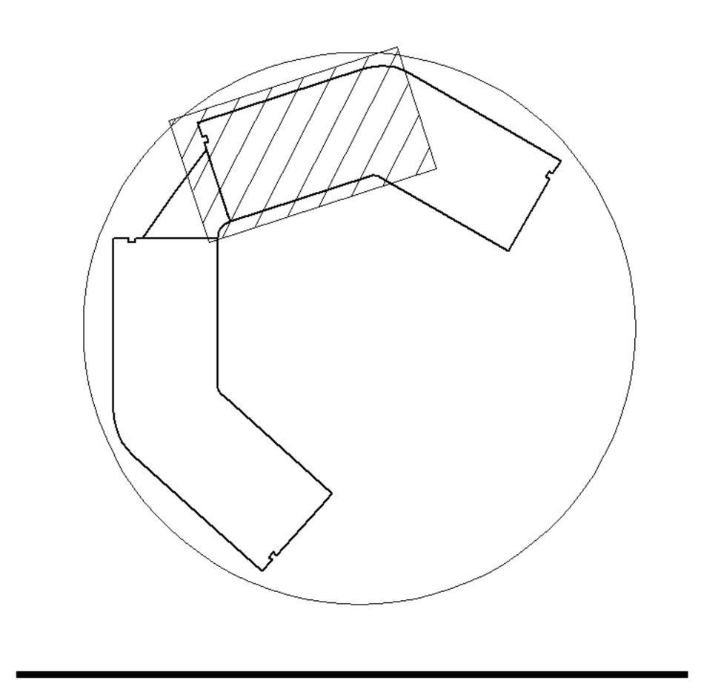
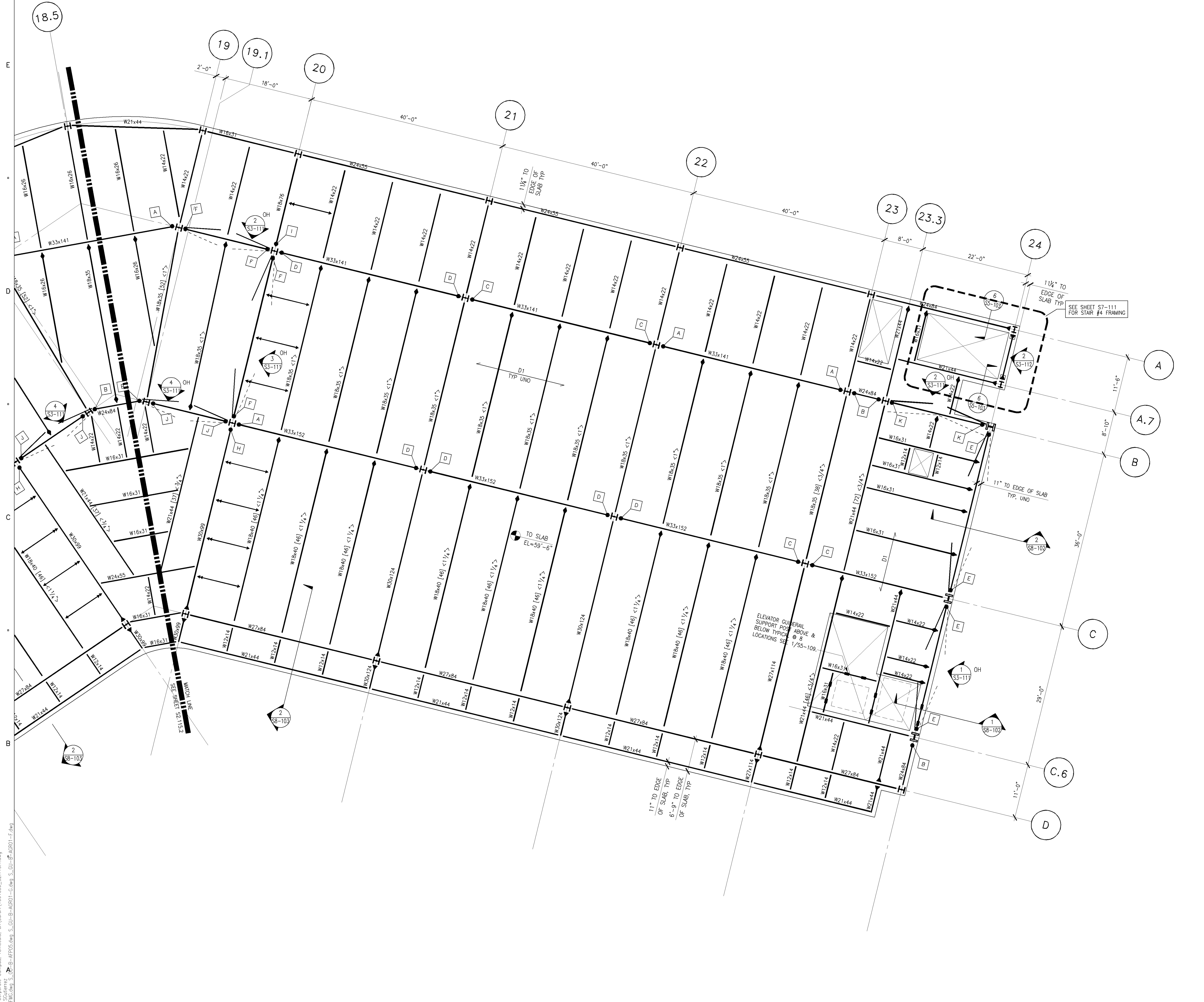


SHEET TITLE

FRAMING PLAN: LEVEL 4 – BUILDING G (WEST)

S2-114.2



FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
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24. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

S1 | FRAMING PLAN: LEVEL 5 - BLDG G (EAST)

S2-115.1 | REF. SCALE: 1/8" = 1'-0"

PLAN NORTH

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
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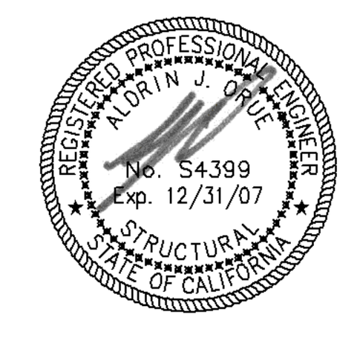

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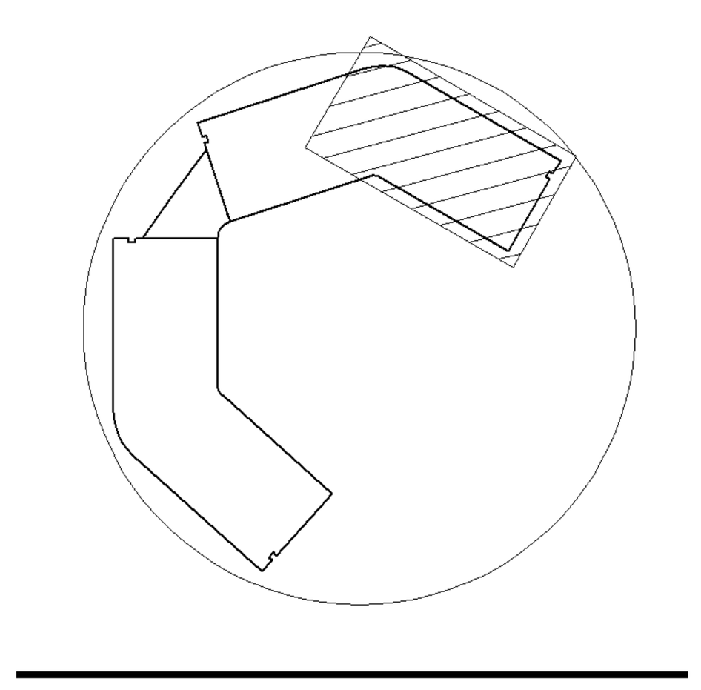
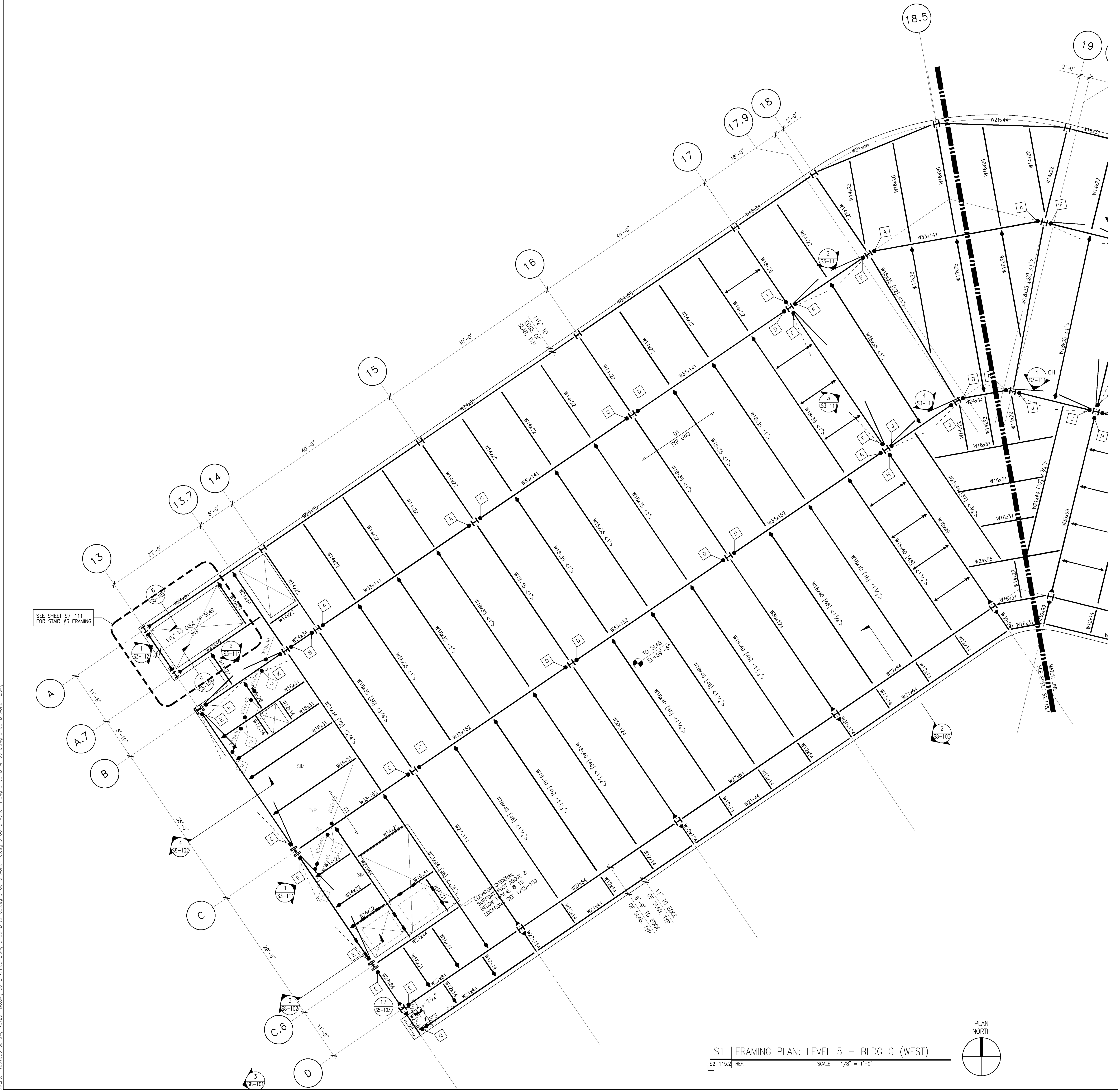


SHEET TITLE

FRAMING PLAN: LEVEL 5 - BUILDING G (EAST)

S2-115.1



S1 | FRAMING PLAN: LEVEL 5 – BLDG G (WEST)

S2-115.2 REF. SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
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19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
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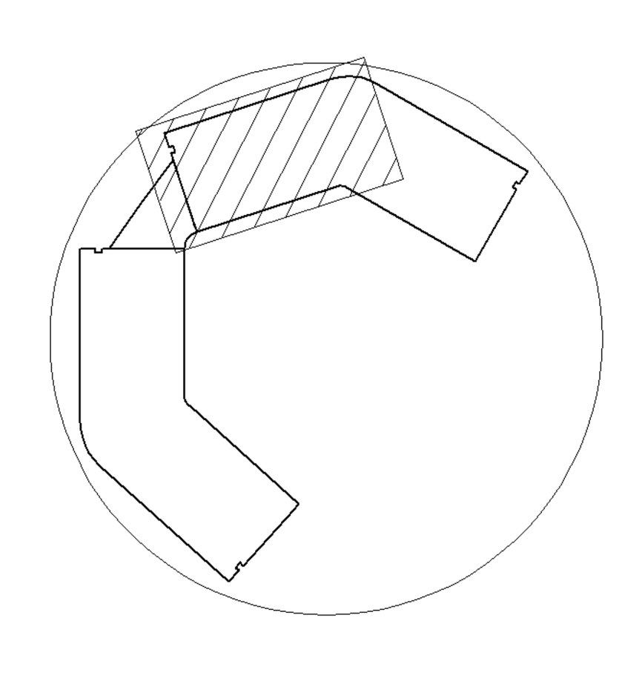

SHEET TITLE

FRAMING PLAN: LEVEL 5 – BUILDING G (WEST)

S2-115.2

S1 | FRAMING PLAN: ROOF – BLDG G (EAST)
S2-116.1 REF. SCALE: 1/8" = 1'-0"

S4 | FRAMING PLAN: STAIR #4 ROOF
S2-116.1 REF. SCALE: 1/8" = 1'-0"

ROOF FRAMING NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
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5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = $6\frac{1}{4}$" BELOW TOP OF SLAB, UNO.
   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.
   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
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17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
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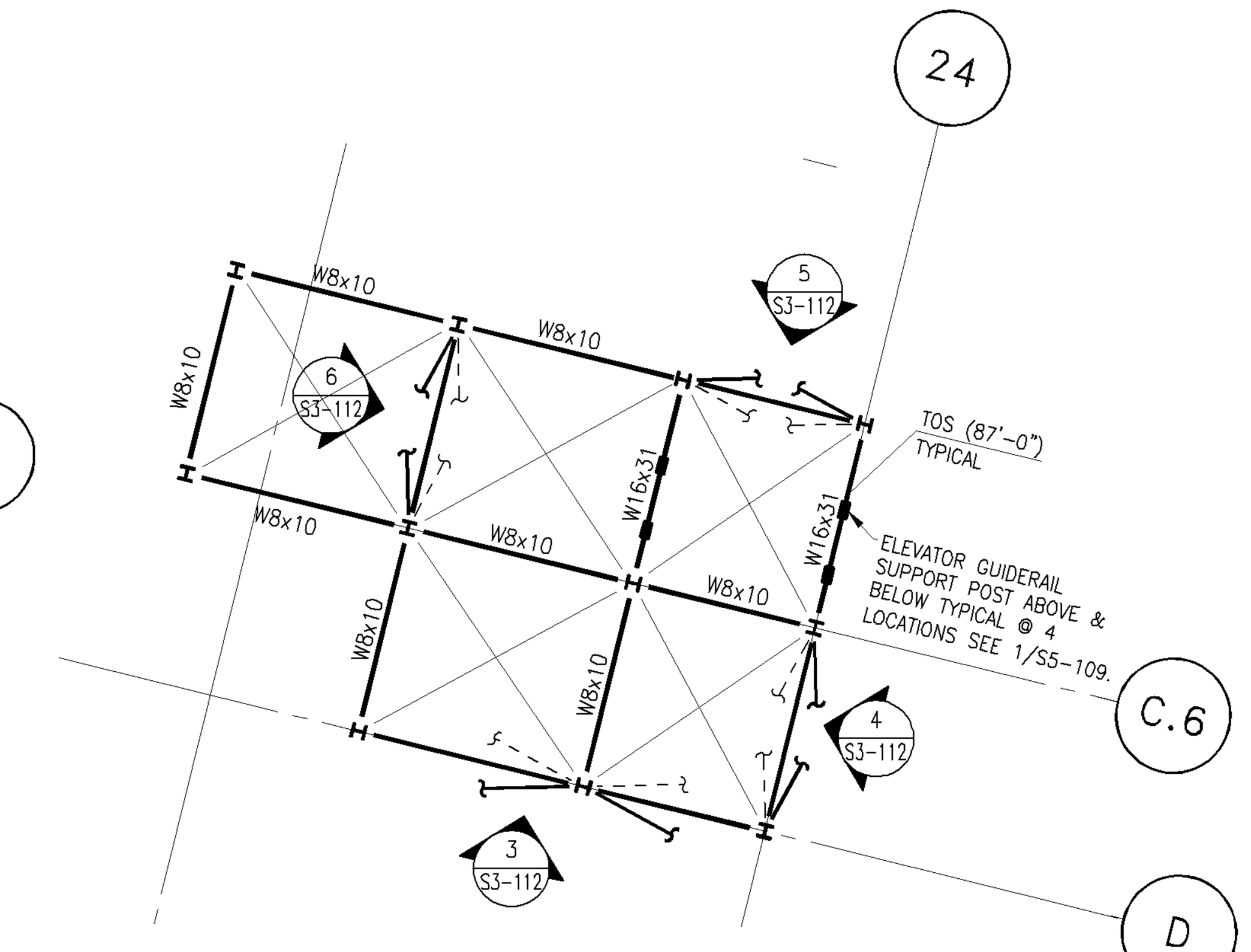
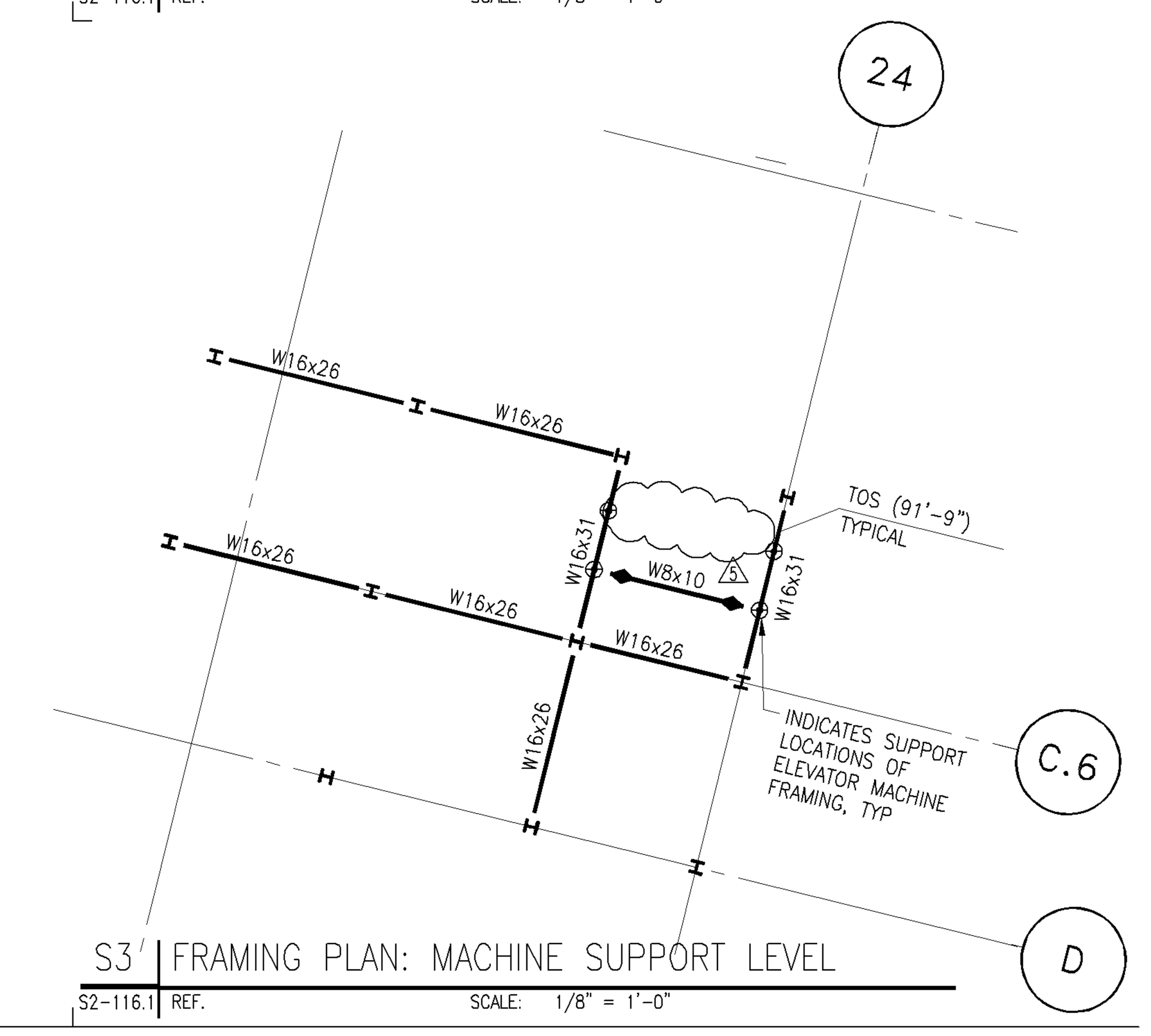
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. INDICATES AREAS OF TAPERED RIGID INSULATION PER ARCH. WHERE ROOF SLOPES ARE NOT OBTAINABLE USING THE SLOPED STRUCTURAL STEEL FRAMING, PROVIDE TAPERED RIGID INSULATION AS REQUIRED TO ACHIEVE THE SLOPES INDICATED ON THE ARCHITECTURAL DRAWINGS. PLEASE NOTE TAPERED RIGID INSULATION MAY BE NECESSARY IN ADDITIONAL ISOLATED AREAS NOT INDICATED ON PLAN. THE USE OF ADDITIONAL LIGHT WEIGHT CONCRETE FILL (BEYOND THE THICKNESS INDICATED ON PLAN) TO ACHIEVE ROOF SLOPES IS NOT PERMITTED WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
24. IF REQUIRED TO MAINTAIN CONTACT BETWEEN DECK AND STRUCTURAL STEEL FRAMING AND TO AVOID EXCESSIVE DECK WARPING, REFER TO DETAIL 7/S5-103.
25. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
26. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.



S2 | FRAMING PLAN: INTERMEDIATE LEVEL
S2-116.1 REF. SCALE: 1/8" = 1'-0"



S3 | FRAMING PLAN: MACHINE SUPPORT LEVEL
S2-116.1 REF. SCALE: 1/8" = 1'-0"

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

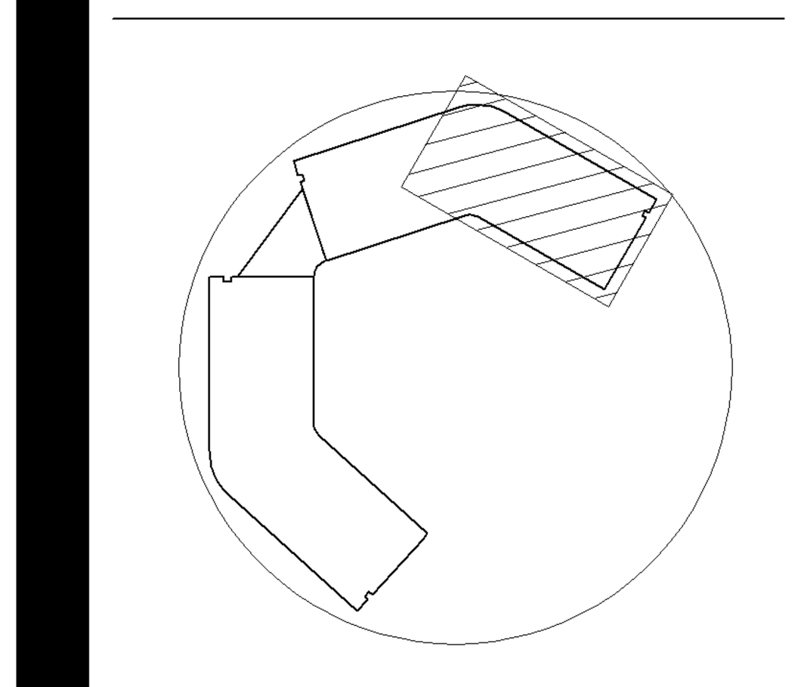
REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
5	08-30-07	PREVIOUS RFI MODS.
4	08-30-07	BULLETIN 010 MODS.
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

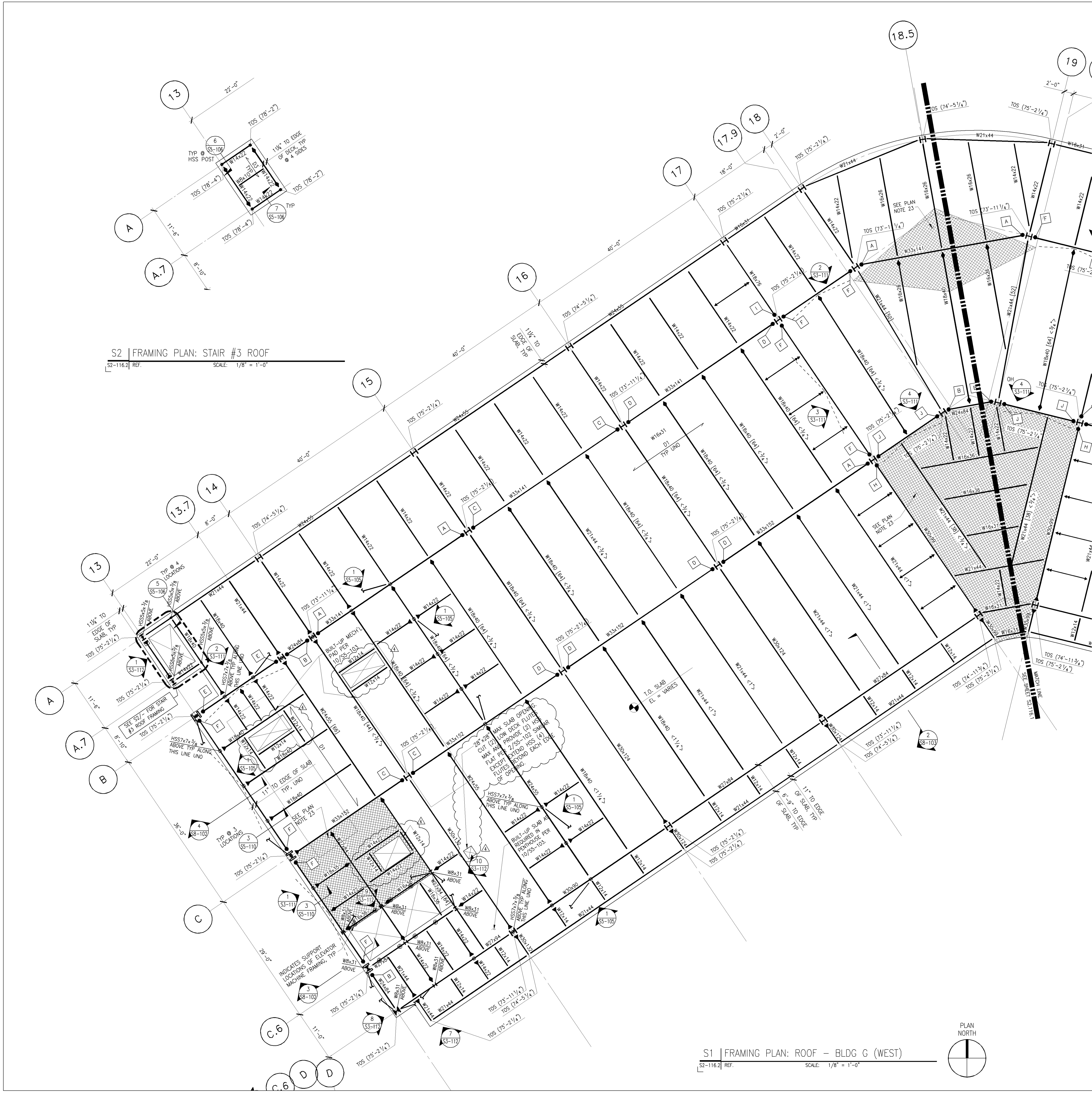
KEY PLAN



SHEET TITLE

FRAMING PLAN: ROOF – BUILDING G (EAST)

S2-116.1



S2 | FRAMING PLAN: STAIR #3 ROOF

S2-116.2 | REF. SCALE: 1/8" = 1'-0"

S1 | FRAMING PLAN: ROOF - BLDG G (WEST)

S2-116.2 | REF. SCALE: 1/8" = 1'-0"

PLAN NORTH

ROOF FRAMING NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. INDICATES AREAS OF TAPERED RIGID INSULATION PER ARCH. WHERE ROOF SLOPES ARE NOT OBTAINABLE USING THE SLOPED STRUCTURAL STEEL FRAMING, PROVIDE TAPERED RIGID INSULATION AS REQUIRED TO ACHIEVE THE SLOPES INDICATED ON THE ARCHITECTURAL DRAWINGS. PLEASE NOTE TAPERED RIGID INSULATION MAY BE NECESSARY IN ADDITIONAL ISOLATED AREAS NOT INDICATED ON PLAN. THE USE OF ADDITIONAL LIGHT WEIGHT CONCRETE FILL (BEYOND THE THICKNESS INDICATED ON PLAN) TO ACHIEVE ROOF SLOPES IS NOT PERMITTED WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
24. IF REQUIRED TO MAINTAIN CONTACT BETWEEN DECK AND STRUCTURAL STEEL FRAMING AND TO AVOID EXCESSIVE DECK WARPING, REFER TO DETAIL 7/S5-103.
25. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
26. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

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LOS ANGELES, CA 90071
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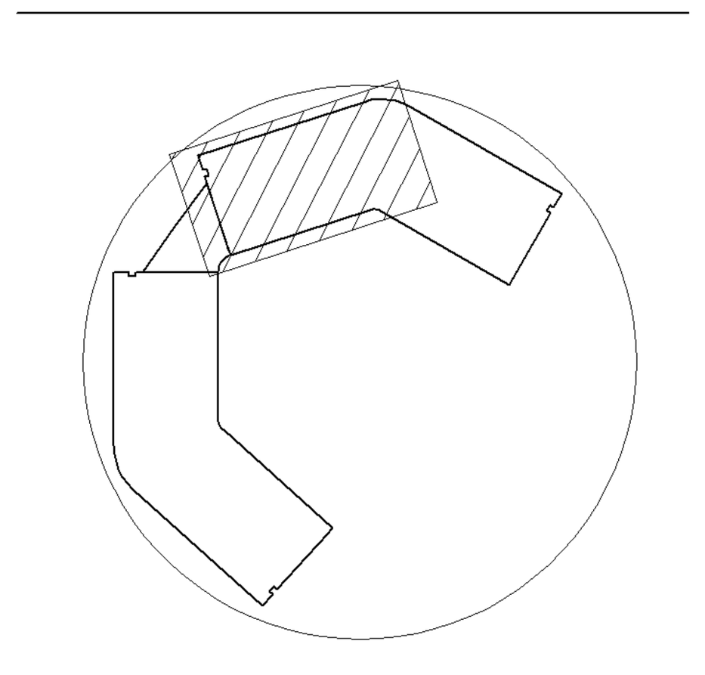
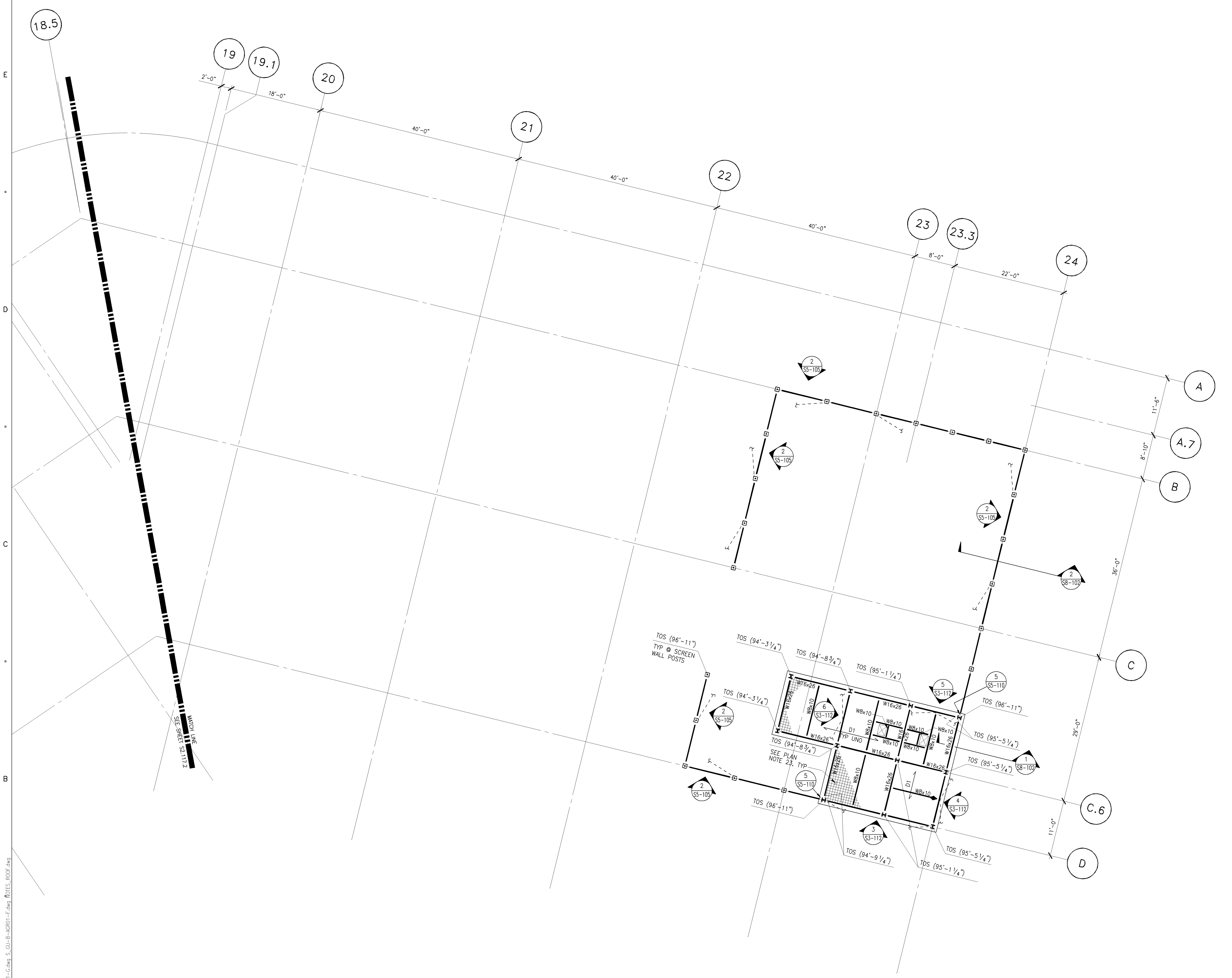


SHEET TITLE

FRAMING PLAN: ROOF - BUILDING G (WEST)

S2-116.2



S1 | FRAMING PLAN: PENTHOUSE ROOF – BLDG G (EAST)

S2-117.1 | REF. SCALE: 1/8" = 1'-0"

PLAN
NORTH

ROOF FRAMING NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. INDICATES AREAS OF TAPERED RIGID INSULATION PER ARCH. WHERE ROOF SLOPES ARE NOT OBTAINABLE USING THE SLOPED STRUCTURAL STEEL FRAMING, PROVIDE TAPERED RIGID INSULATION AS REQUIRED TO ACHIEVE THE SLOPES INDICATED ON THE ARCHITECTURAL DRAWINGS. PLEASE NOTE TAPERED RIGID INSULATION MAY BE NECESSARY IN ADDITIONAL ISOLATED AREAS NOT INDICATED ON PLAN. THE USE OF ADDITIONAL LIGHT WEIGHT CONCRETE FILL (BEYOND THE THICKNESS INDICATED ON PLAN) TO ACHIEVE ROOF SLOPES IS NOT PERMITTED WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
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25. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
26. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

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CONSULTANTS

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CIVIL ENGINEER
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ESCONDIDO, CA 92029

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
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BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

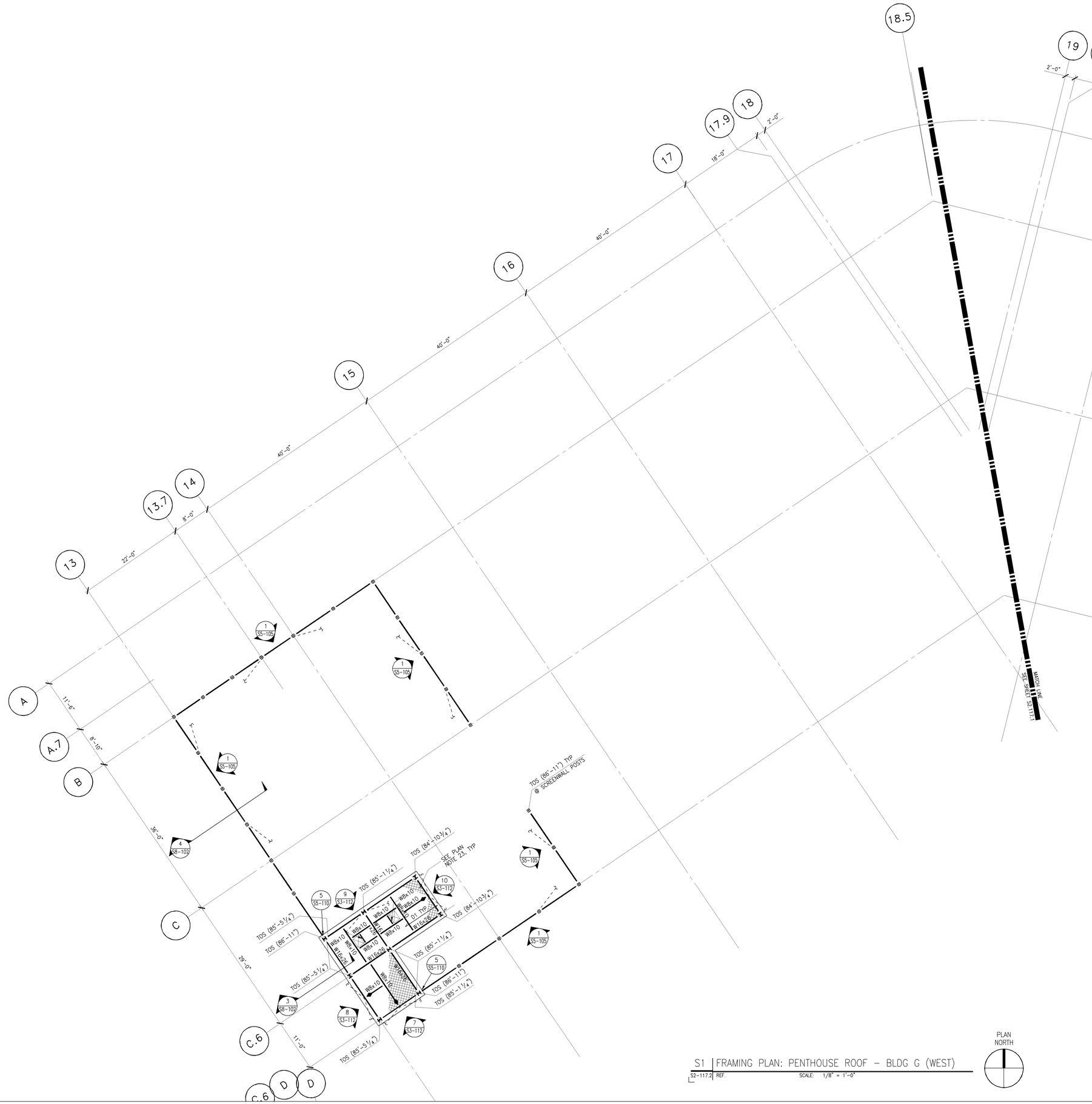
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

FRAMING PLAN: PENTHOUSE ROOF – BUILDING G (EAST)

S2-117.1



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S1 | FRAMING PLAN: PENTHOUSE ROOF - BLDG G (WEST)
S2-117.2 | REF. SCALE: 1/8" = 1'-0"

ROOF FRAMING NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. ◐ T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6¼" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX")INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. ▨ X" INDICATES SLAB DEPRESSION
9. ●── A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. ▶── INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. ◆── INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. ◀◀──▶ INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. ╲╱ INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. ╲╱ INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. ←D1→ INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. ▶── INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. ▨ INDICATES AREAS OF TAPERED RIGID INSULATION PER ARCH. WHERE ROOF SLOPES ARE NOT OBTAINABLE USING THE SLOPED STRUCTURAL STEEL FRAMING, PROVIDE TAPERED RIGID INSULATION AS REQUIRED TO ACHIEVE THE SLOPES INDICATED ON THE ARCHITECTURAL DRAWINGS. PLEASE NOTE TAPERED RIGID INSULATION MAY BE NECESSARY IN ADDITIONAL ISOLATED AREAS NOT INDICATED ON PLAN. THE USE OF ADDITIONAL LIGHT WEIGHT CONCRETE FILL (BEYOND THE THICKNESS INDICATED ON PLAN) TO ACHIEVE ROOF SLOPES IS NOT PERMITTED WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
24. IF REQUIRED TO MAINTAIN CONTACT BETWEEN DECK AND STRUCTURAL STEEL FRAMING AND TO AVOID EXCESSIVE DECK WARPING, REFER TO DETAIL 7/S5-103.
25. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
26. ○── INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

OWNER/CLIENT

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Temecula, CA 92591-4628

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LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

ALDRIN J. ORUE
No. S4399
Exp. 12/31/07

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

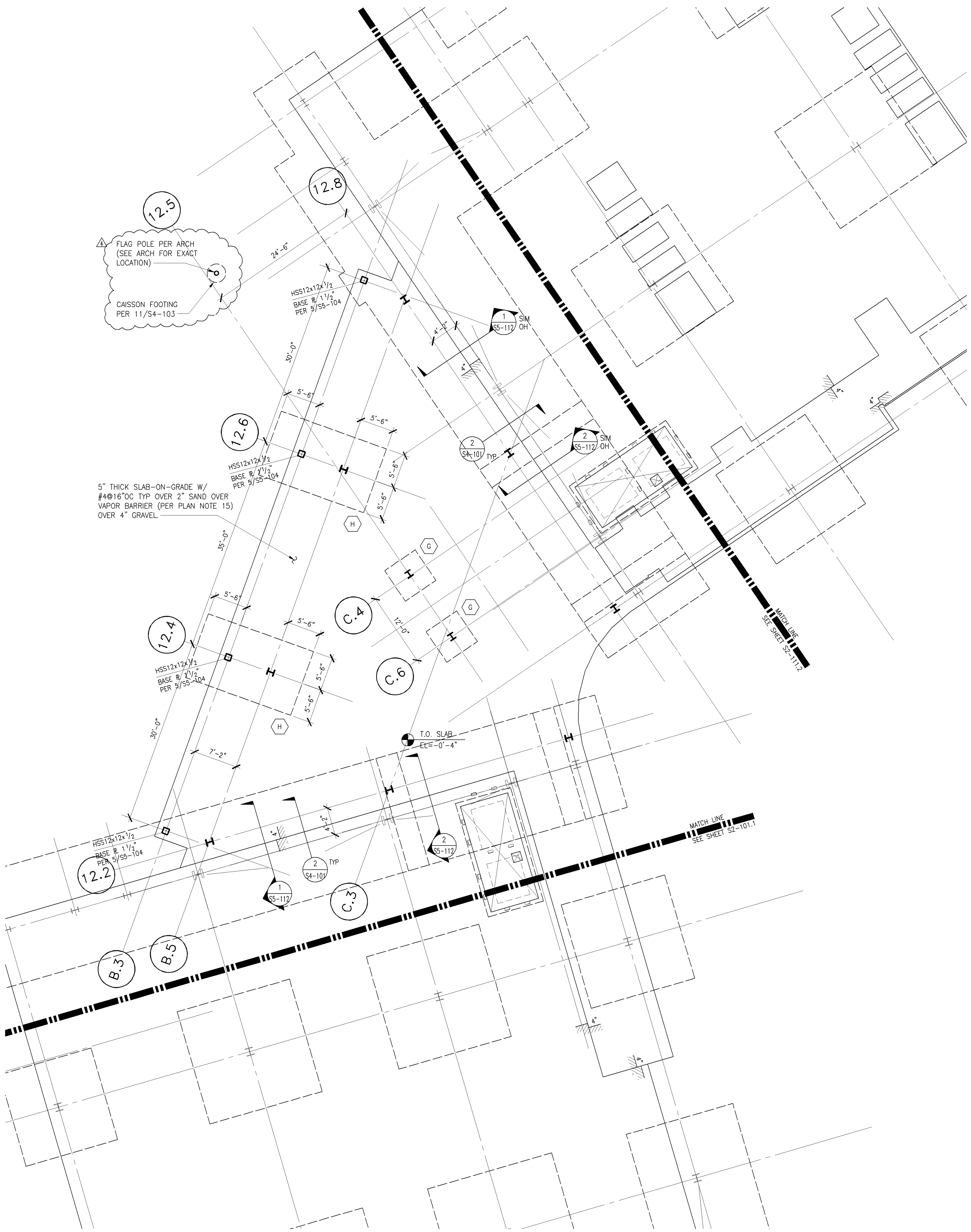
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

FRAMING PLAN:
PENTHOUSE ROOF -
BUILDING G (WEST)

S2-117.2



PLAN NORTH

S1 | FOUNDATION PLAN: LOBBY LEVEL 1

S2-301 | REF. SCALE: 1/8" = 1'-0"

FOUNDATION PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. (-X'-XX") INDICATES TOP OF CONCRETE FOOTING BELOW THE ADJACENT TOP OF SLAB ELEVATION. ALL FOOTINGS NOT NOTED SHALL BE (-1'-6") BELOW THE ADJACENT TOP OF SLAB ELEVATION.
7. X" INDICATES SLAB DEPRESSION
8. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
9. # INDICATES FOOTING CALLOUT PER SCHEDULE ON DETAIL 1/S4-103.
10. SEE ARCHITECTURAL FOR CONCRETE SLAB DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
11. VERIFY LOCATIONS, DIMENSIONS, AND TYPE OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT. REFER TO DETAILS 7 & 15/S4-101.
12. VERIFY LOCATIONS AND DIMENSIONS OF SUMP PITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
13. SEE SOILS REPORTS FOR OVER EXCAVATION REQUIREMENTS.
14. INDICATES STEPPED FOOTING PER DETAIL 12/S4-101.
15. VAPOR BARRIER SHALL HAVE A PERMEANCE OF 0.01 PERMS OR LESS, MEETING ASTM E-1745 (CLASS "A"), BE LOCATED PER ACI LOCATION GUIDELINES, AND BE INSTALLED PER ASTM E-1643.

OWNER/CLIENT

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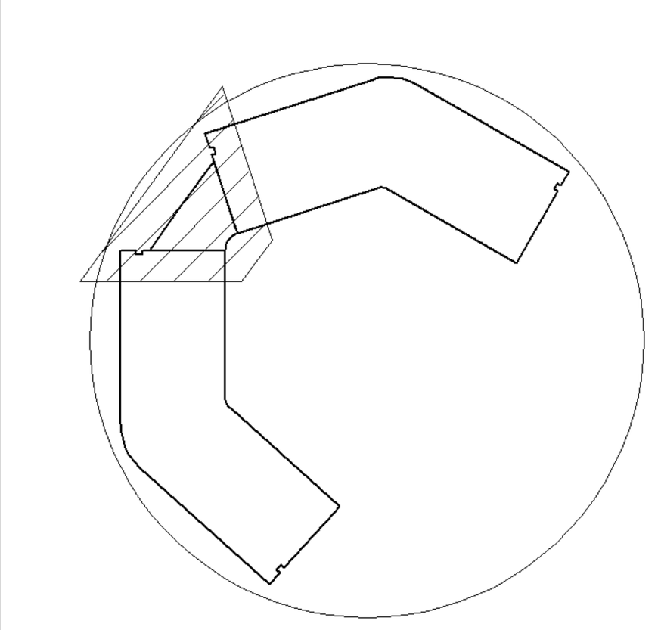
CHECKED BY:

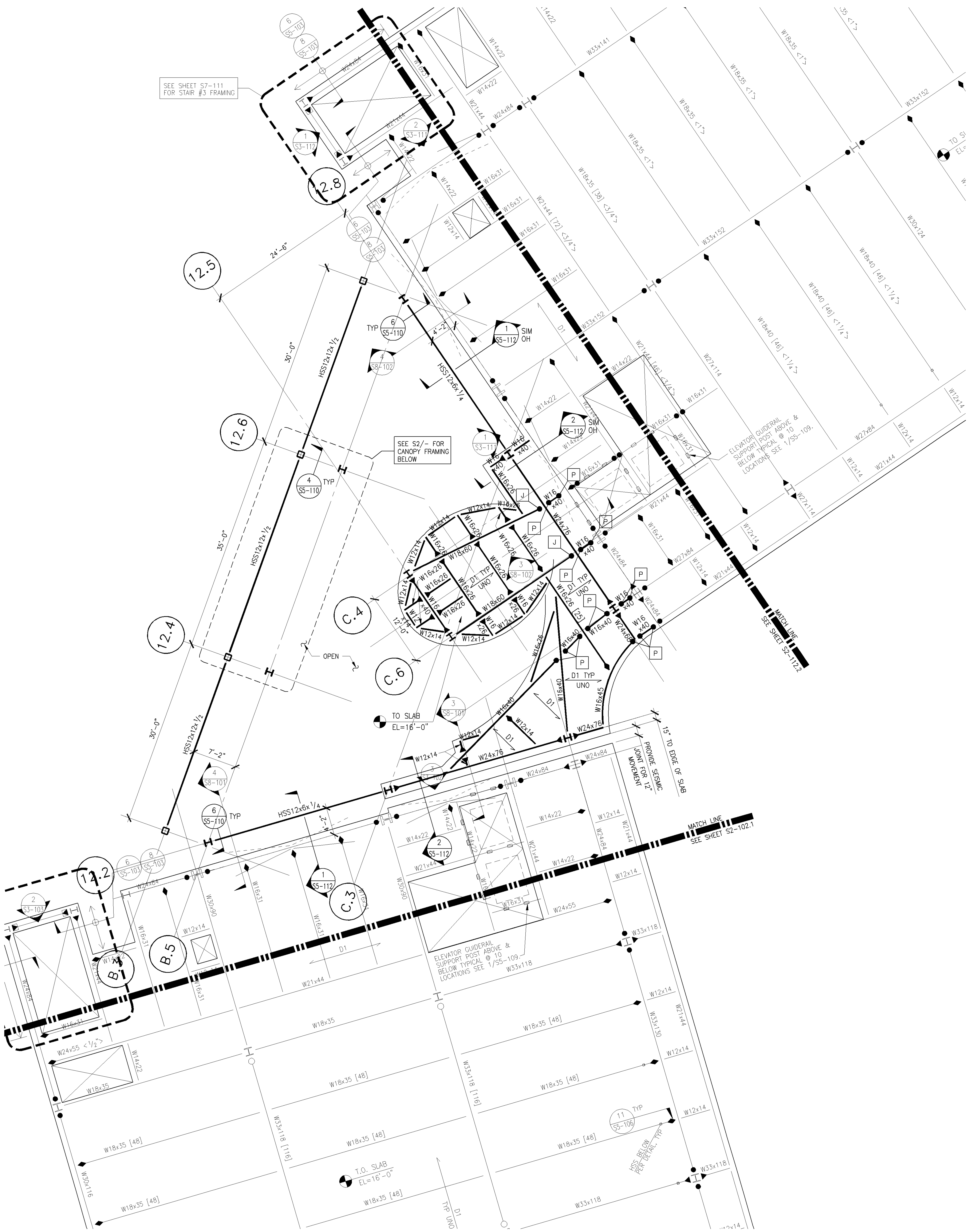
KEY PLAN



SHEET TITLE

FOUNDATION PLAN: LEVEL 1 - LOBBY

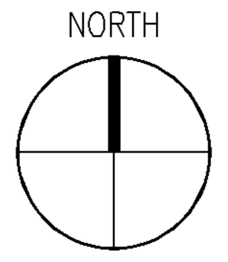
S2-301

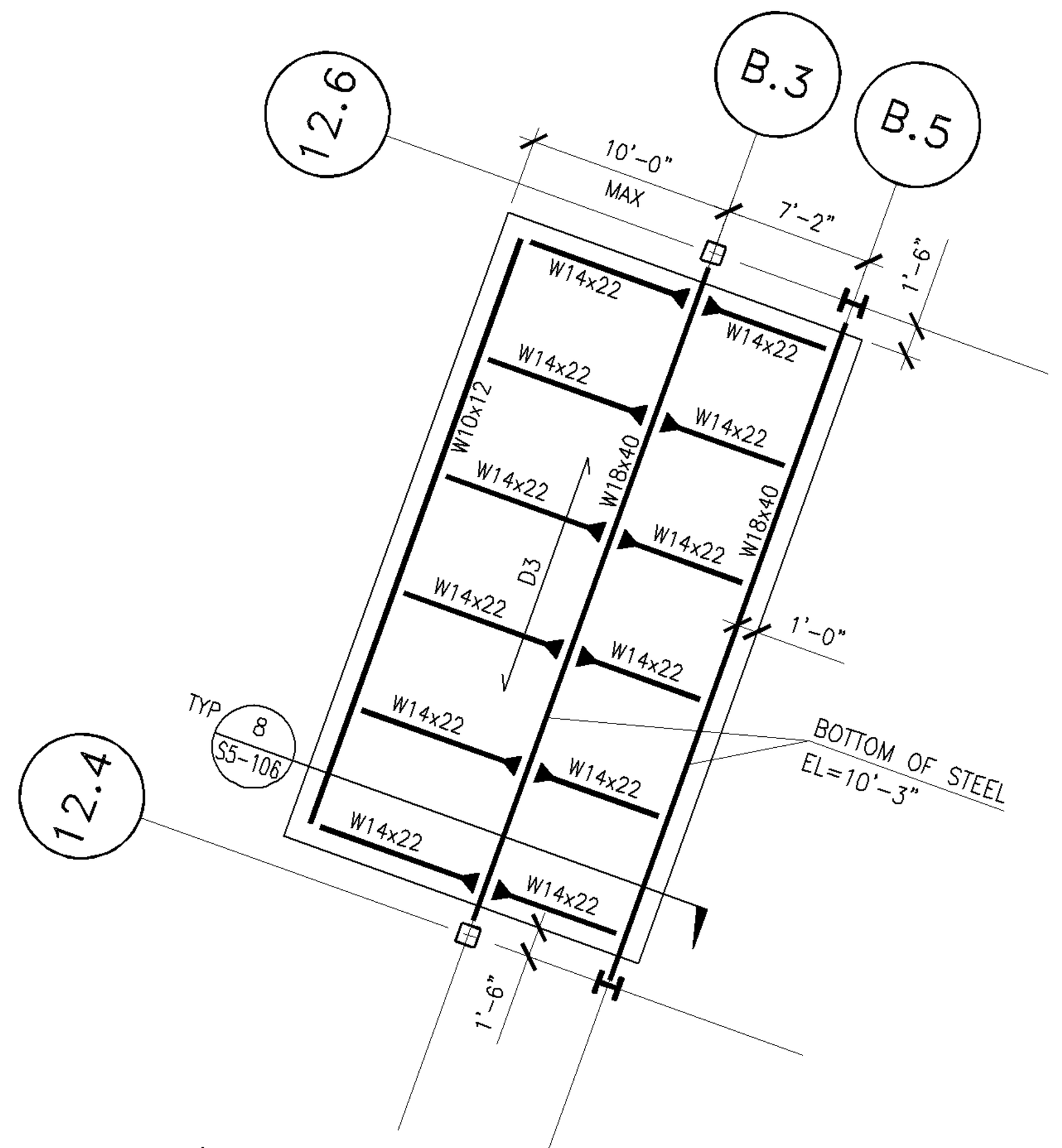


S1 | FRAMING PLAN: LOBBY LEVEL 2

S2-302 | REF. SCALE: 1/8" = 1'-0"

PLAN NORTH





S2 | FRAMING PLAN: CANOPY

S2-302 | REF. SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX")INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

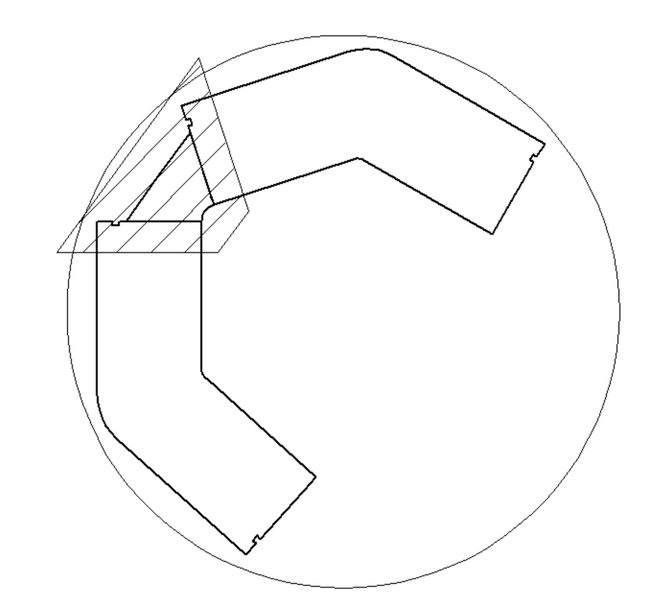
REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN



SHEET TITLE

FRAMING PLAN:
LEVEL 2 - LOBBY

S2-302

S1 | FRAMING PLAN: LOBBY LEVEL 3

S2-303 | REF. SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A ●—— INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. ▶—— INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. ◆—— INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. ◀◀——▶ INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. ╲╱ INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. ╲╱ INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. ▶—— INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. ○—— INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
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CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER
ALDRIN J. ORUE
No. S4399
Exp. 12/31/07
STRUCTURAL
STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

FRAMING PLAN:
LEVEL 3 - LOBBY

S2-303

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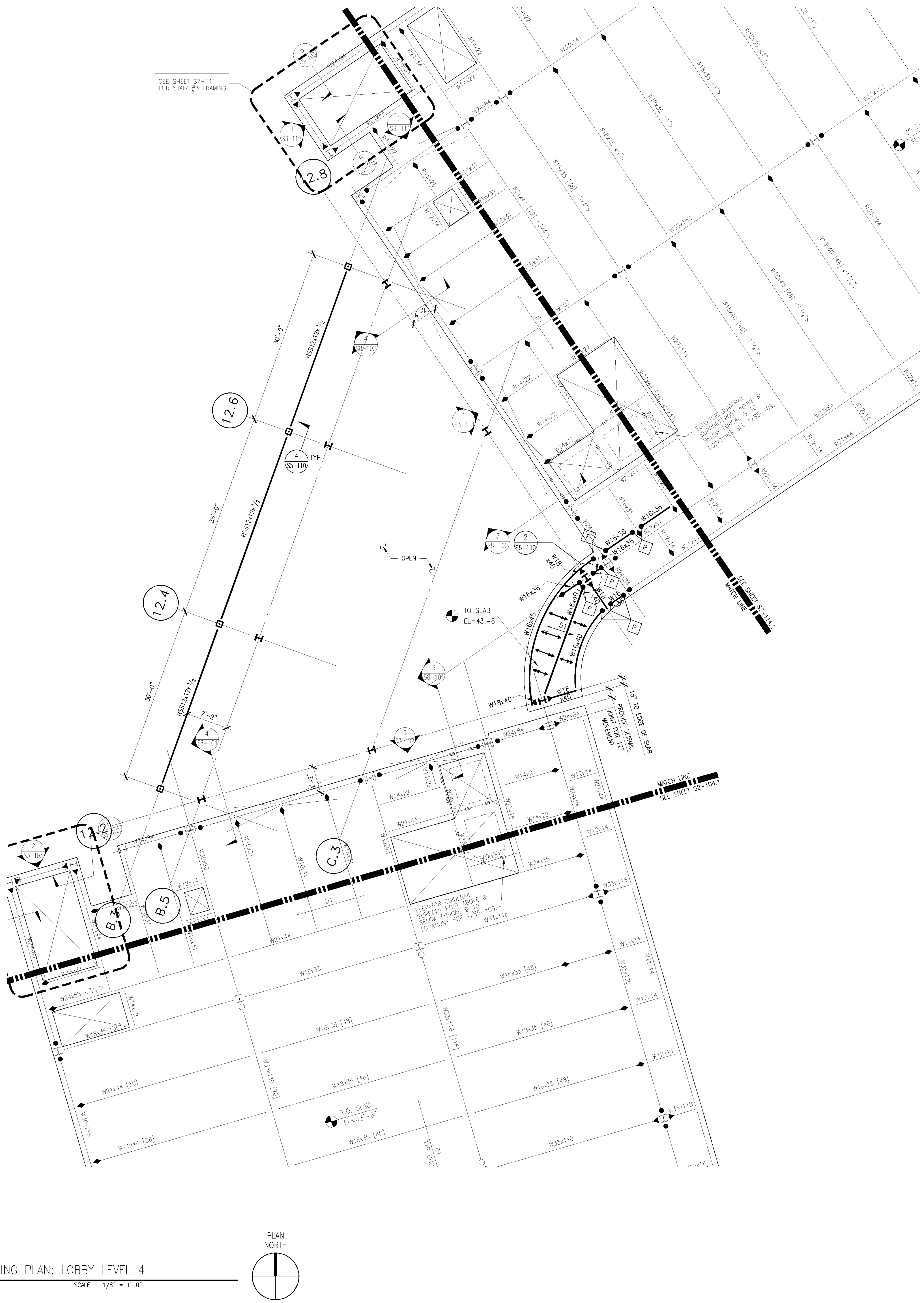

S1 | FRAMING PLAN: LOBBY LEVEL 4

S2-304 | REF. | SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.
   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.
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7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
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10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
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17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

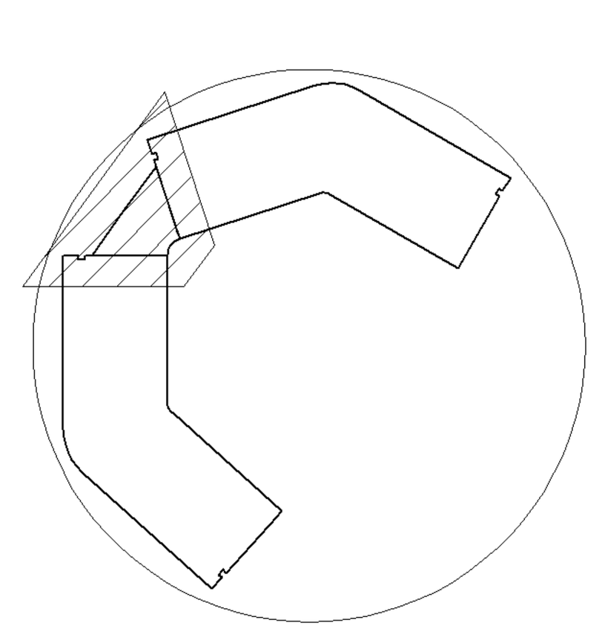
REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

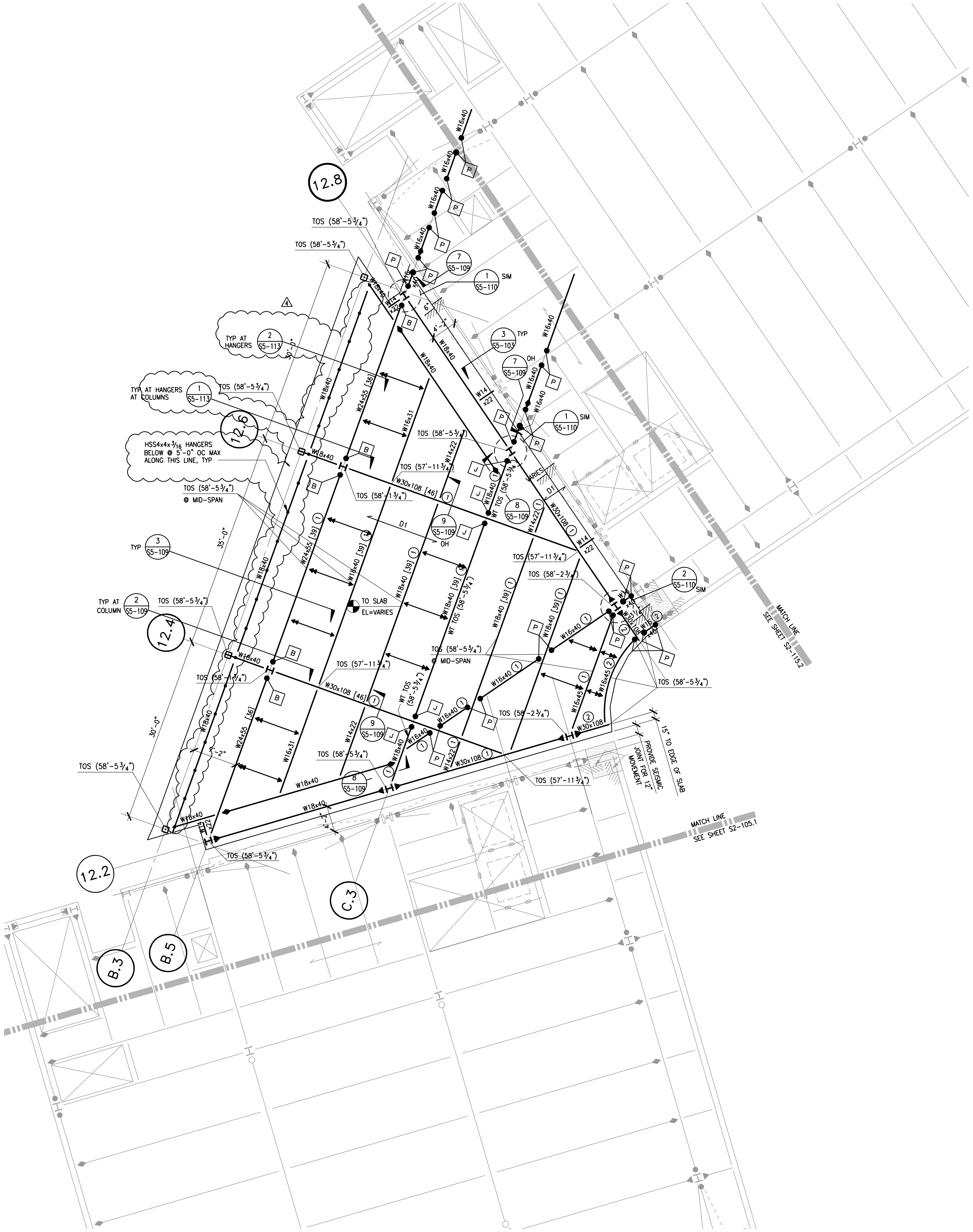
KEY PLAN



SHEET TITLE

FRAMING PLAN:
LEVEL 4 - LOBBY

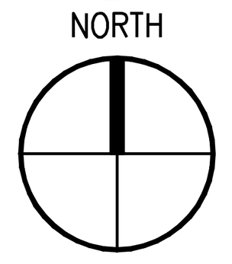
S2-304



S1 | FRAMING PLAN: LOBBY ROOF

S2-305 | REF. SCALE: 1/8" = 1'-0"

PLAN NORTH



FRAMING PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB / EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TYPICAL TOP OF STEEL ELEVATION = 6 1/4" BELOW TOP OF SLAB, UNO.

   TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.

   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS.
18. SEE 1/S5-103 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECK TYPE PER DETAIL 1/S5-102.
21. VERIFY LOCATIONS AND DIMENSIONS OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT.
22. INDICATES SEISMIC MOMENT CONNECTION PER ELEVATIONS.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. INDICATES BOLTED CONNECTION PER NOTE 6 OF DETAIL 1/S5-101.
25. (1) INDICATES LEVEL Wx BEAM PER PLAN WITH TAPERED WT6x13 ON TOP, TOS ELEVATION OF Wx BEAM = 57'-11 3/4". TOS ELEVATION OF WT=VARIES, AS NOTED ON PLAN. SEE DETAIL 7/S5-103.
26. (2) INDICATES LEVEL OR SLOPED Wx BEAM PER PLAN WITH CONSTANT WT3x7.5 ON TOP. TOS ELEVATION=PER PLAN. SEE DETAIL 7/S5-103.

OWNER/CLIENT

Abbott Vascula

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECO

515 SOUTH FLOWER ST., 8TH FLC
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.86
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
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CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

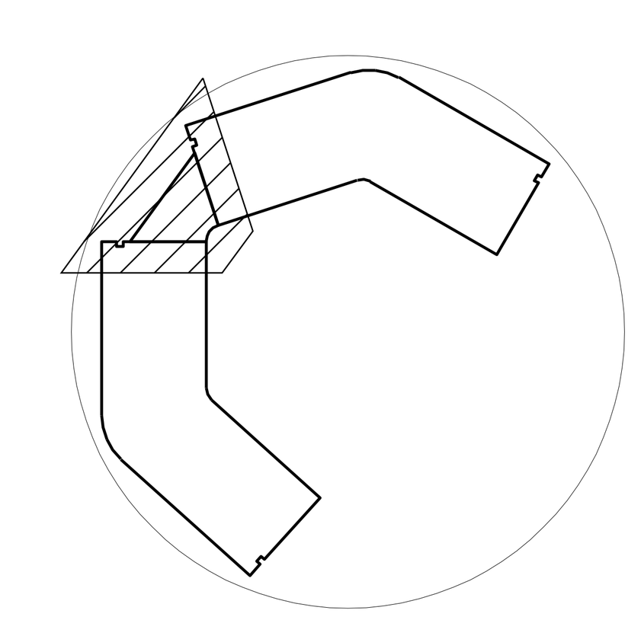
REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
4	01-16-08	BULLETIN 013 SUBMITT
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBM

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

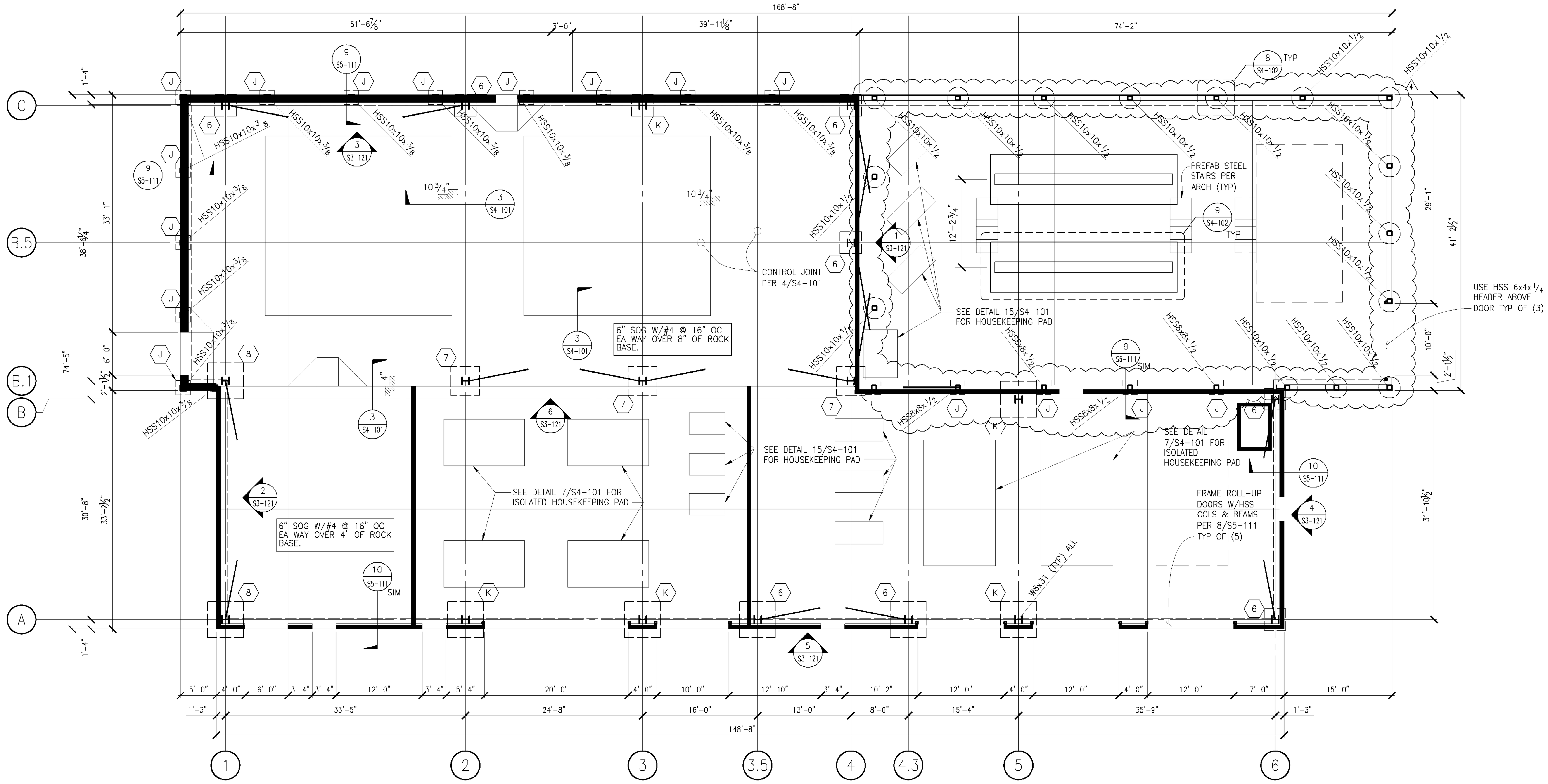
KEY PLAN



SHEET TITLE

FRAMING PLAN: ROOF – LOBBY

S2-305



S1 | FOUNDATION PLAN: CENTRAL PLANT
S2-401 | REF. SCALE: 1/8" = 1'-0"

PLAN NORTH

FOUNDATION PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & & S3-213 FOR COLUMN SCHEDULE.
4. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
5. (-X'-XX") INDICATES TOP OF CONCRETE FOOTING BELOW THE ADJACENT TOP OF SLAB ELEVATION. ALL FOOTINGS NOT NOTED SHALL BE (-1'-6") BELOW THE ADJACENT TOP OF SLAB ELEVATION.
6. X" INDICATES SLAB DEPRESSION
7. INDICATES BRACED FRAME ABOVE PER ELEVATIONS. SEE SHEET S3-121.
8. # INDICATES FOOTING CALLOUT PER SCHEDULE ON DETAIL 1/S4-103.
9. SEE ARCHITECTURAL FOR CONCRETE SLAB DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
10. VERIFY LOCATIONS, DIMENSIONS, AND TYPE OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT. REFER TO DETAILS 7 & 15/S4-101.
11. VERIFY LOCATIONS AND DIMENSIONS OF SUMP PITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
12. SEE SOILS REPORTS FOR OVER EXCAVATION REQUIREMENTS.
13. INDICATES 4" HIGH x 1'-1" WIDE CURB
14. INDICATES 4" HIGH x 7-1/4" WIDE CURB
15. INDICATES 4" HIGH x 6" WIDE CURB
16. INDICATES HSS 4x4x1/4" UNO.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

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STRUCTURAL ENGINEER
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PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

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CARLSBAD, CA 92010
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Fax: 760.496.3711

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LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
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REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
4	08-30-07	BULLETIN 010 MODS.
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

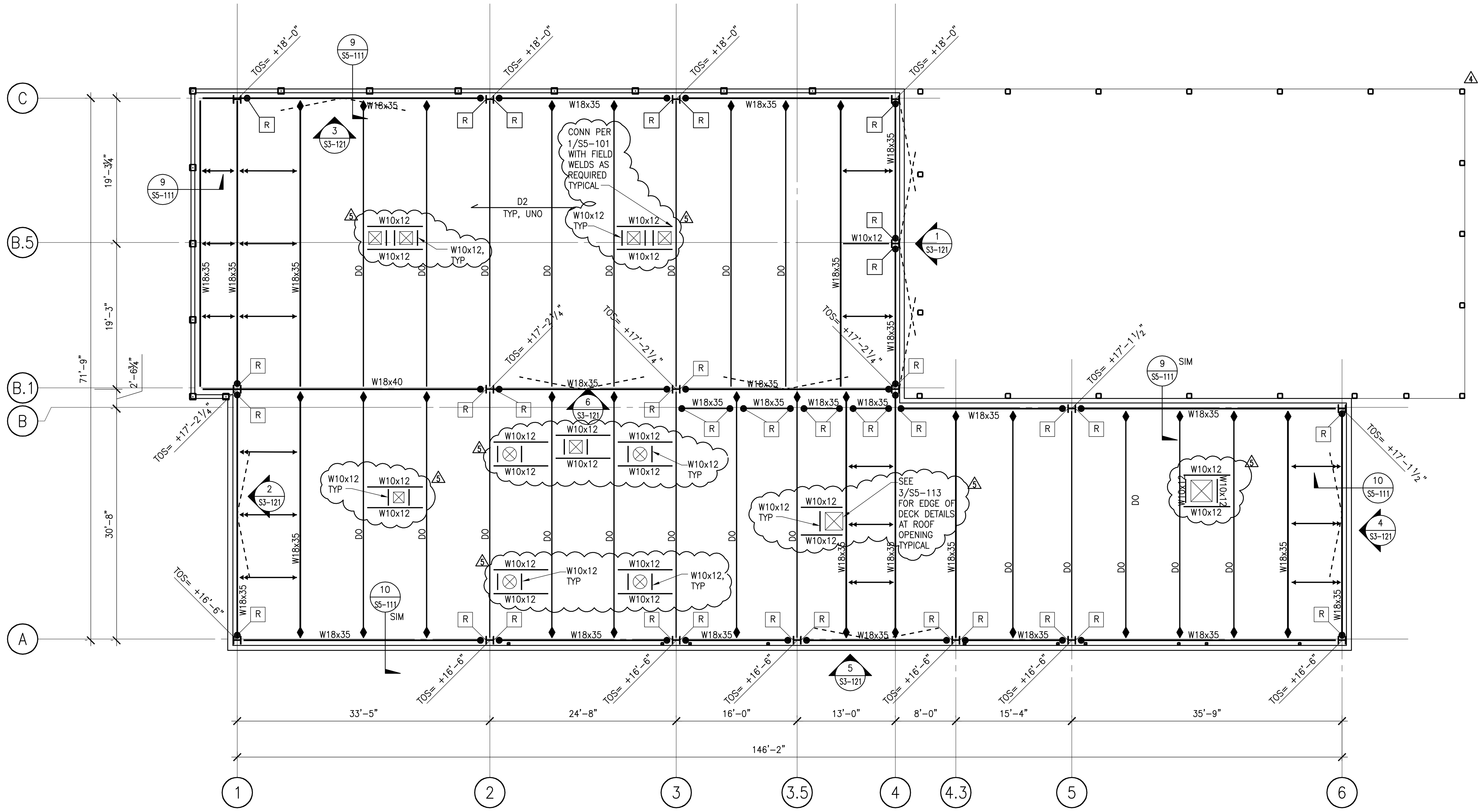
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

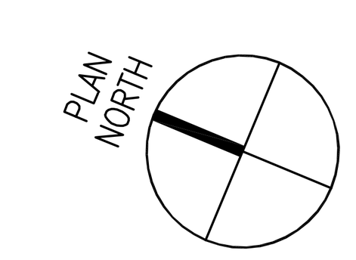
FOUNDATION PLAN: CENTRAL PLANT

S2-401



S1 | ROOF FRAMING PLAN: CENTRAL PLANT

S2-402 | REF. SCALE: 1/8" = 1'-0"



ROOF FRAMING NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. TOS (X'-XX") INDICATES TOP OF STEEL ELEVATION WHERE NOTED.
   (±X") INDICATES TOP OF BEAM ELEVATION ABOVE OR BELOW TYPICAL TOP OF STEEL.
7. SEE ARCHITECTURAL FOR CONCRETE SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
8. X" INDICATES SLAB DEPRESSION
9. A INDICATES DRAG CONNECTION PER DETAIL 1/S5-104, TYPICAL.
10. INDICATES NON-SEISMIC MOMENT CONNECTION PER DETAILS 3, 5 & 7/S5-101, TYPICAL.
11. INDICATES FULL HEIGHT STIFFENER PLATE CONNECTION PER 1C/S5-101.
12. INDICATES BOTTOM FLANGE LATERAL BRACE CONNECTION PER DETAIL 4/S5-101.
13. <X> INDICATES AMOUNT OF UPWARD CAMBER AT BEAM MIDSPAN. (INCHES)
14. [X] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102. MAXIMUM SPACING OF STUDS IS 12"OC IF NOT INDICATED ON PLAN.
15. [X-Y-Z] INDICATES NUMBER OF HEADED SHEAR STUDS PER DETAIL 10/S5-102.
16. INDICATES BRACED FRAME ABOVE PER ELEVATIONS. SEE SHEET S3-121.
17. INDICATES BRACED FRAME BELOW PER ELEVATIONS. SEE SHEET S3-121.
18. SEE 7/S5-106 FOR EDGE OF DECK DETAILS.
19. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN DIMENSIONED COLUMNS AND/OR BEAMS UNO.
20. D1 INDICATES STEEL DECR TYPE PER DETAIL 1/S5-102.
21. INDICATES AREAS OF TAPERED RIGID INSULATION PER ARCH. WHERE ROOF SLOPES ARE NOT OBTAINABLE USING THE SLOPED STRUCTURAL STEEL FRAMING, PROVIDE TAPERED RIGID INSULATION AS REQUIRED TO ACHIEVE THE SLOPES INDICATED ON THE ARCHITECTURAL DRAWINGS. PLEASE NOTE TAPERED RIGID INSULATION MAY BE NECESSARY IN ADDITIONAL ISOLATED AREAS NOT INDICATED ON PLAN. THE USE OF ADDITIONAL LIGHT WEIGHT CONCRETE FILL (BEYOND THE THICRNESS INDICATED ON PLAN) TO ACHIEVE ROOF SLOPES IS NOT PERMITTED WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
22. IF REQUIRED TO MAINTAIN CONTACT BETWEEN DECR AND STRUCTURAL STEEL FRAMING AND TO AVOID DECR WARPING, REFER TO DETAIL 7/S5-103.
23. AT INTERIOR SLAB OPENINGS, THE DISTANCE FROM THE CENTERLINE OF PERIMETER BEAMS TO THE EDGE OF SLAB SHALL BE 11", TYPICAL, UNO.
24. SEE 11/S5-111 FOR METAL STUD HEADER SCHEDULE.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
5	12-21-07	BULLETIN 015 - HAZMAT
4	08-30-07	BULLETIN 010 MODS.
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

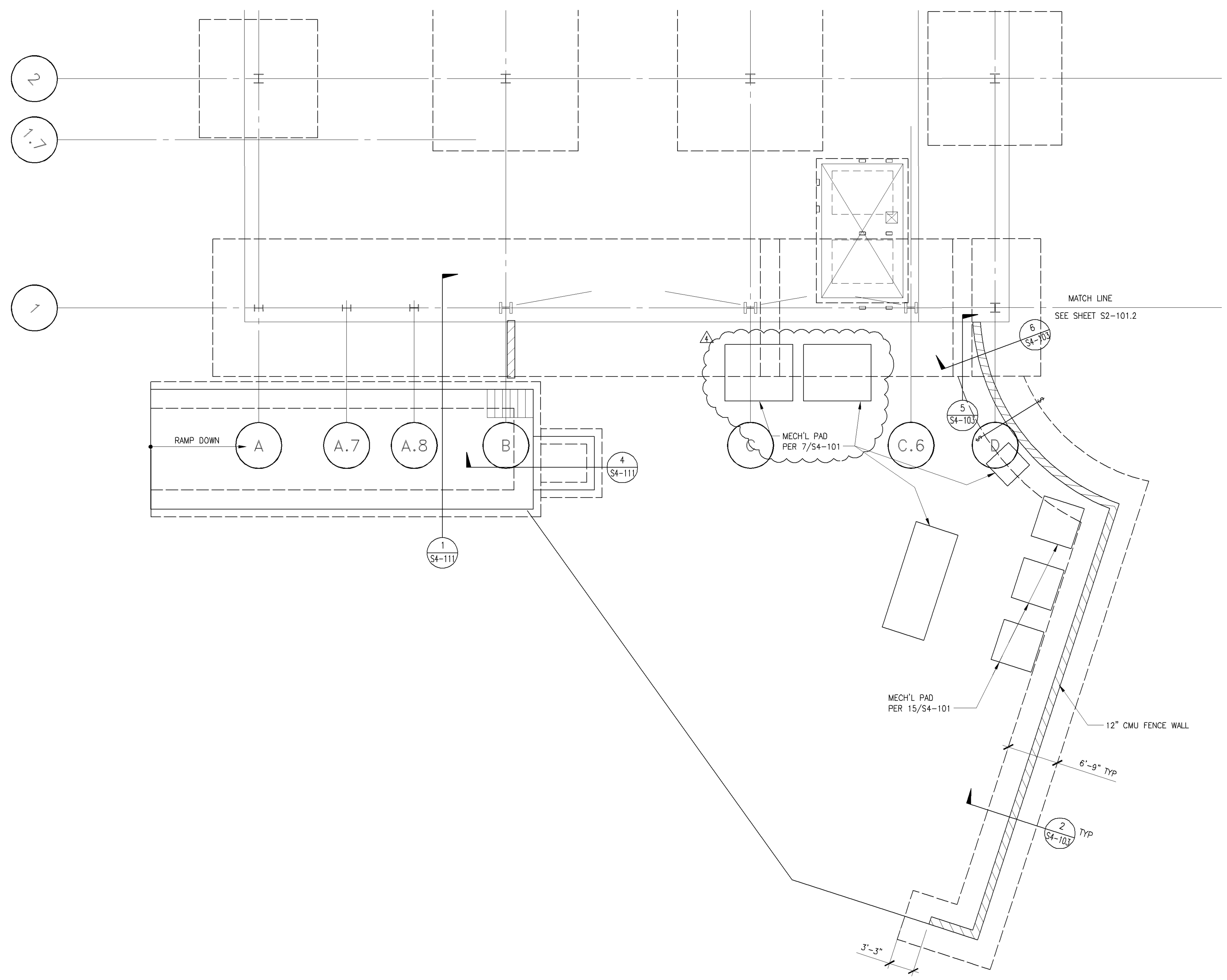
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

ROOF FRAMING PLAN: CENTRAL PLANT

S2-402



S1 | FOUNDATION PLAN: SERVICE YARD – BUILDING F

S2-401 | REF. SCALE: 1/8" = 1'-0"

PLAN NORTH

FOUNDATION PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. (-X'-XX") INDICATES TOP OF CONCRETE FOOTING BELOW THE ADJACENT TOP OF SLAB ELEVATION. ALL FOOTINGS NOT NOTED SHALL BE (-1'-6") BELOW THE ADJACENT TOP OF SLAB ELEVATION.
7. X" INDICATES SLAB DEPRESSION
8. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
9. # INDICATES FOOTING CALLOUT PER SCHEDULE ON DETAIL 1/S4-103.
10. SEE ARCHITECTURAL FOR CONCRETE SLAB DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
11. VERIFY LOCATIONS, DIMENSIONS, AND TYPE OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT. REFER TO DETAILS 7 & 15/S4-101.
12. VERIFY LOCATIONS AND DIMENSIONS OF SUMP PITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
13. SEE SOILS REPORTS FOR OVER EXCAVATION REQUIREMENTS.
14. INDICATES STEPPED FOOTING PER DETAIL 12/S4-101.
15. VAPOR BARRIER SHALL HAVE A PERMEANCE OF 0.01 PERMS OR LESS, MEETING ASTM E-1745 (CLASS "A"), BE LOCATED PER ACI LOCATION GUIDELINES, AND BE INSTALLED PER ASTM E-1643.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

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4	08-30-07	BULLETIN 010 MODS.
3	03-22-07	FOR CONSTRUCTION
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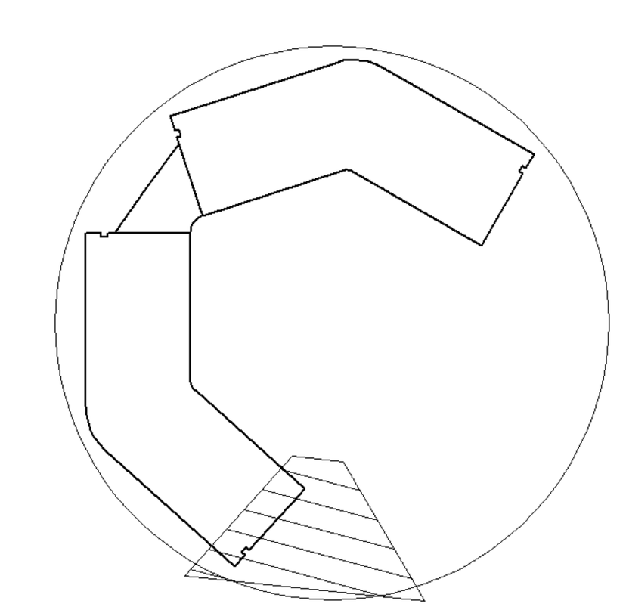
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

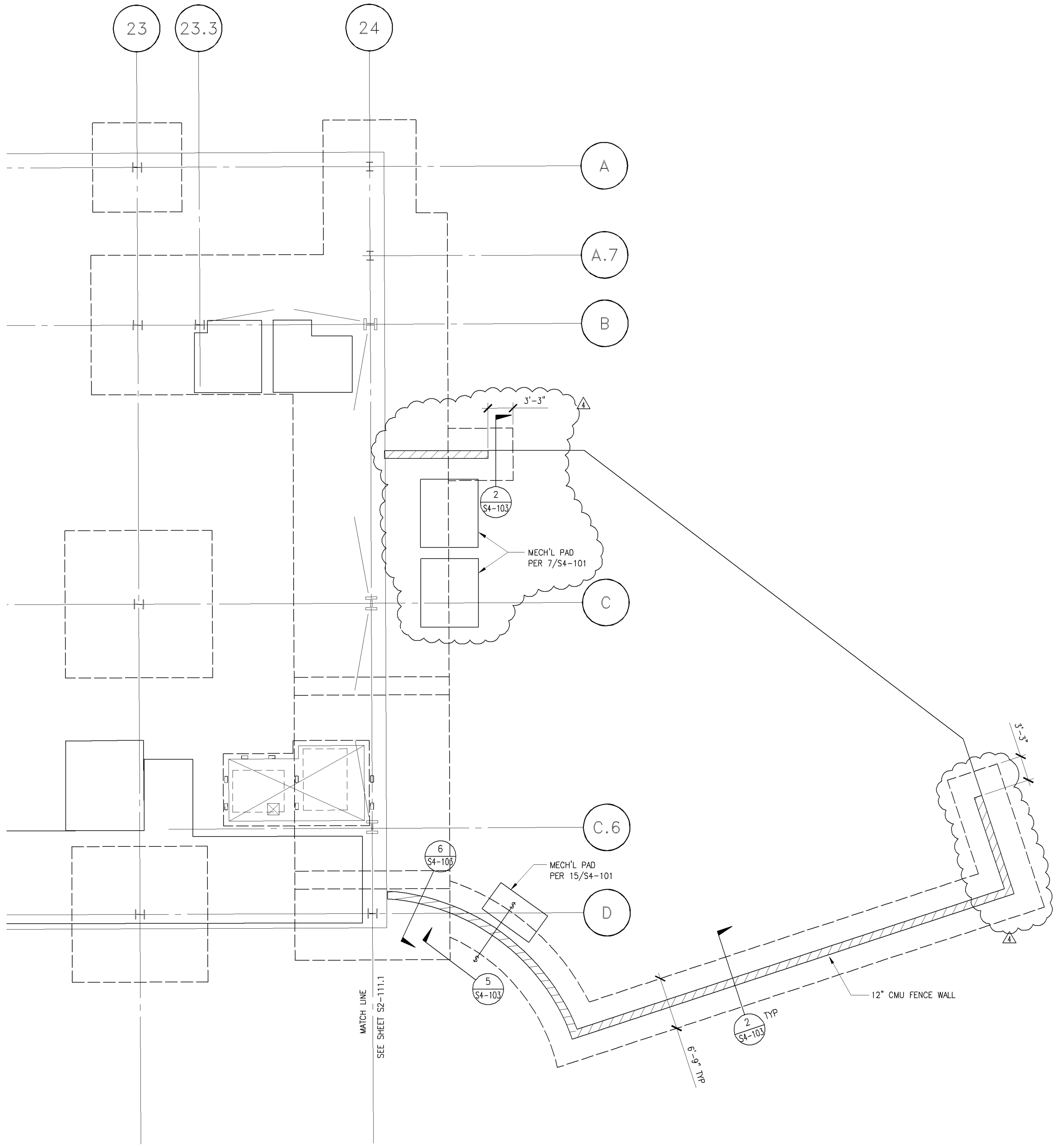
KEY PLAN



SHEET TITLE

FOUNDATION PLAN: SERVICE YARD – BUILDING F

S2-501



S1 | FOUNDATION PLAN: SERVICE YARD – BUILDING G

S2-411 | REF. SCALE: 1/8" = 1'-0"

PLAN NORTH

FOUNDATION PLAN NOTES:

1. SEE SHEET S1-101 FOR STRUCTURAL GENERAL NOTES.
2. SEE SHEETS S4-101 & S4-102 FOR TYPICAL CONCRETE DETAILS.
3. SEE SHEETS S5-101, S5-102, S5-103 & S5-104 FOR TYPICAL STEEL DETAILS.
4. SEE SHEETS S3-201, S3-202, S3-211, S3-212 & & S3-213 FOR COLUMN SCHEDULE.
5. T.O.SLAB EL=XX'-XX" INDICATES TOP OF SLAB ELEVATION.
6. (-X'-XX") INDICATES TOP OF CONCRETE FOOTING BELOW THE ADJACENT TOP OF SLAB ELEVATION. ALL FOOTINGS NOT NOTED SHALL BE (-1'-6") BELOW THE ADJACENT TOP OF SLAB ELEVATION.
7. X" INDICATES SLAB DEPRESSION
8. INDICATES BRACED FRAME ABOVE PER ELEVATIONS.
9. # INDICATES FOOTING CALLOUT PER SCHEDULE ON DETAIL 1/S4-103.
10. SEE ARCHITECTURAL FOR CONCRETE SLAB DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC., AND LOCATIONS OF OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS.
11. VERIFY LOCATIONS, DIMENSIONS, AND TYPE OF CONCRETE PADS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS PRIOR TO INSTALLATION OF EQUIPMENT. REFER TO DETAILS 7 & 15/S4-101.
12. VERIFY LOCATIONS AND DIMENSIONS OF SUMP PITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
13. SEE SOILS REPORTS FOR OVER EXCAVATION REQUIREMENTS.
14. INDICATES STEPPED FOOTING PER DETAIL 12/S4-101.
15. VAPOR BARRIER SHALL HAVE A PERMEANCE OF 0.01 PERMS OR LESS, MEETING ASTM E-1745 (CLASS "A"), BE LOCATED PER ACI LOCATION GUIDELINES, AND BE INSTALLED PER ASTM E-1643.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
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Fax: 626.578.9121

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2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
4	08-30-07	BULLETIN 010 MODS.
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
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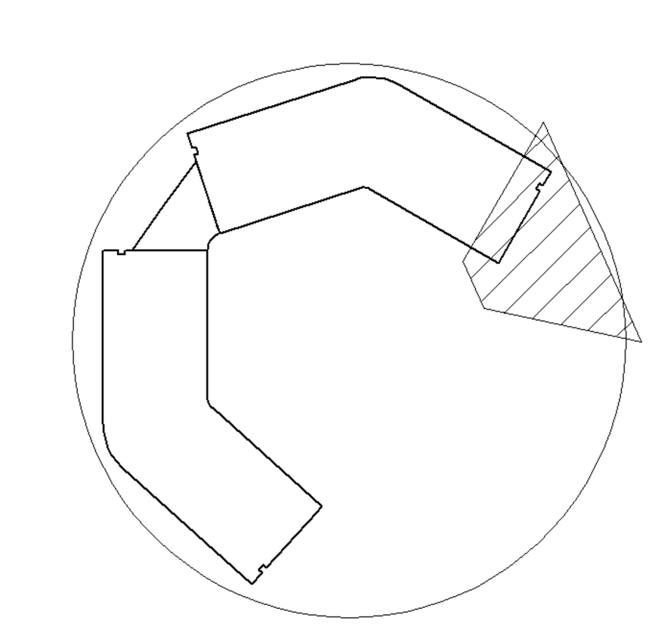
KEY PLAN



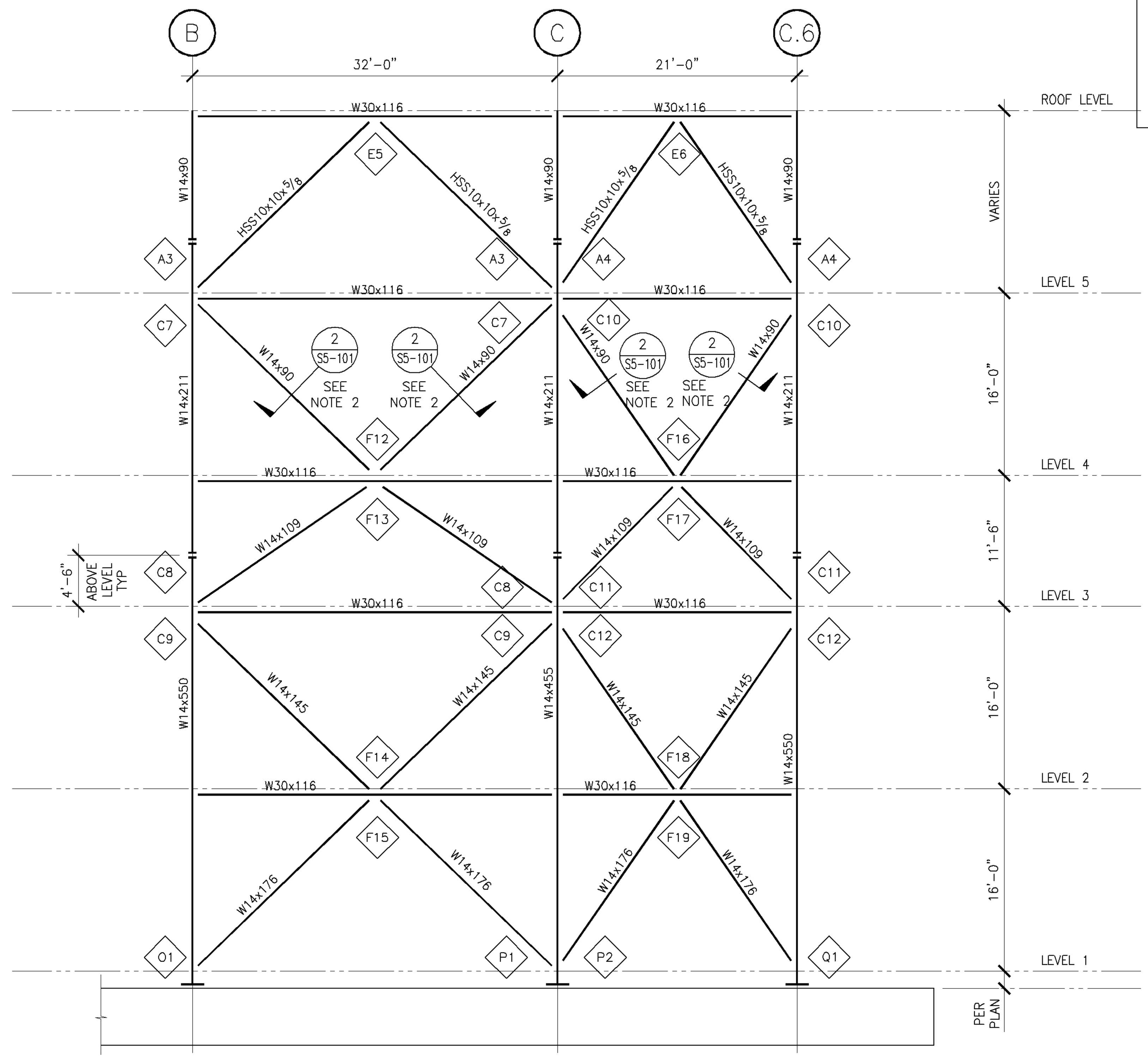
SHEET TITLE

FOUNDATION PLAN: SERVICE YARD – BUILDING G

S2-511

BRACED FRAME NOTE:

1. INDICATES BRACE FRAME CONNECTION ID PER DETAILS ON SHEETS S6-101 THROUGH S6-109.
2. PROVIDE STIFFENER PLATES PER 2/S5-101 & @ 2'-0" FROM END OF COVER PLATE @ EA END & 4'-0" OC, MAX IN BETWEEN.

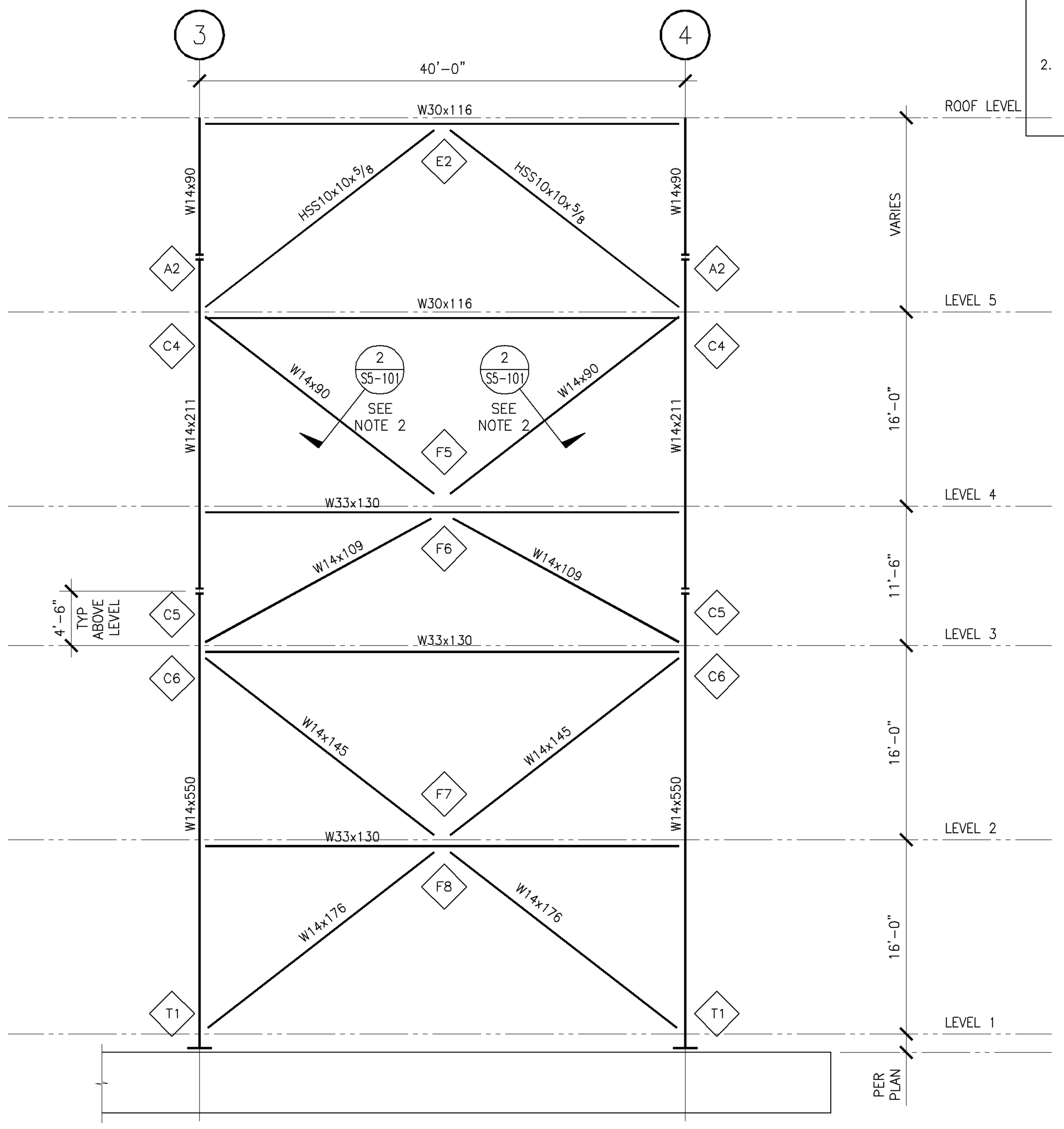


BRACED FRAME ELEVATION - GRID LINE 1

1/8"=1'-0" 3

BRACED FRAME NOTE:

1. INDICATES BRACE FRAME CONNECTION ID PER DETAILS ON SHEETS S6-101 THROUGH S6-109.
2. PROVIDE STIFFENER PLATES PER 2/S5-101 & @ 2'-0" FROM END OF COVER PLATE @ EA END & 4'-0" OC, MAX IN BETWEEN.

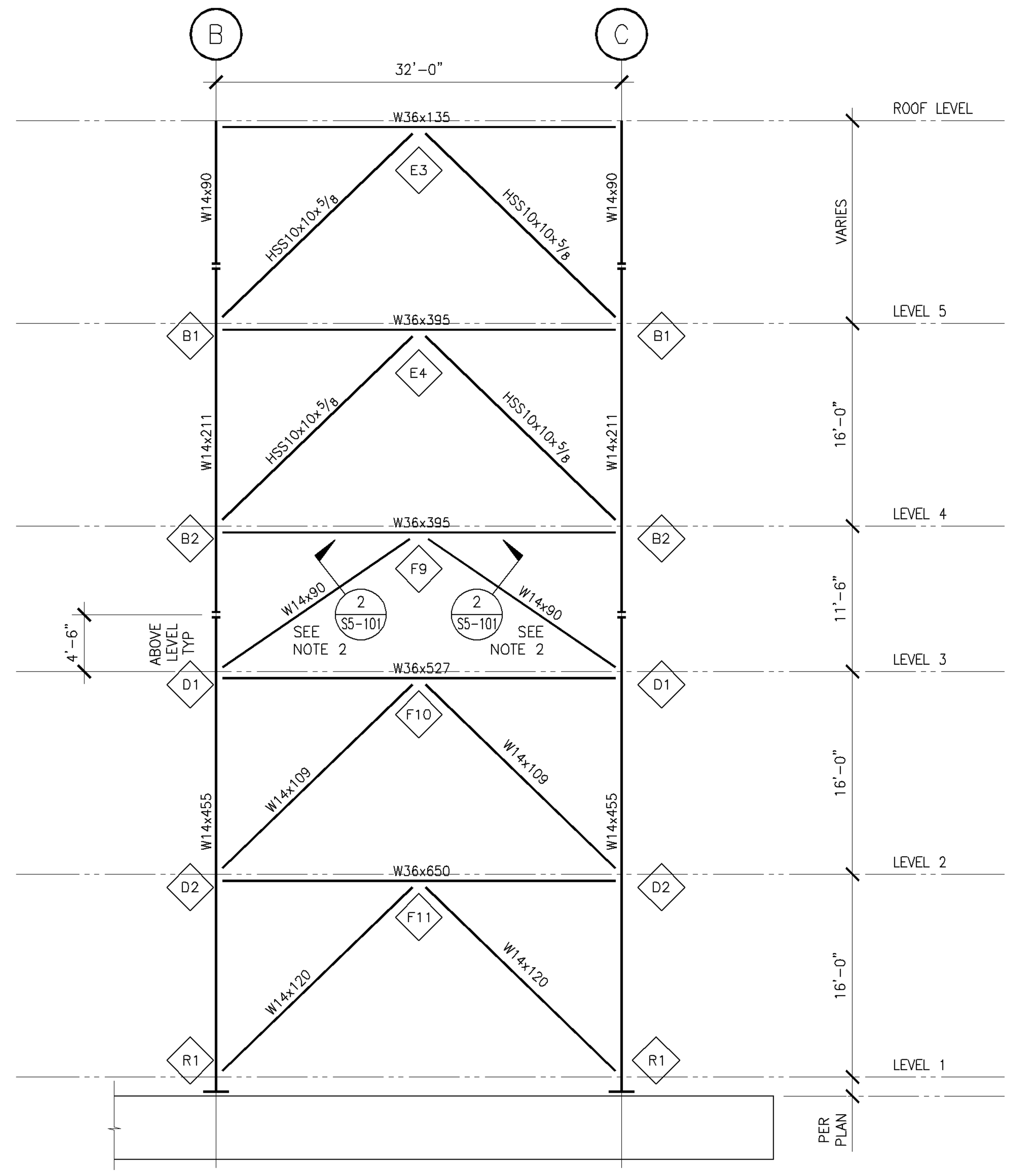


BRACED FRAME ELEVATION - GRID LINE D

1/8"=1'-0" 1

BRACED FRAME NOTE:

1. INDICATES BRACE FRAME CONNECTION ID PER DETAILS ON SHEETS S6-101 THROUGH S6-109.
2. PROVIDE STIFFENER PLATES PER 2/S5-101 & @ 2'-0" FROM END OF COVER PLATE @ EA END & 4'-0" OC, MAX IN BETWEEN.

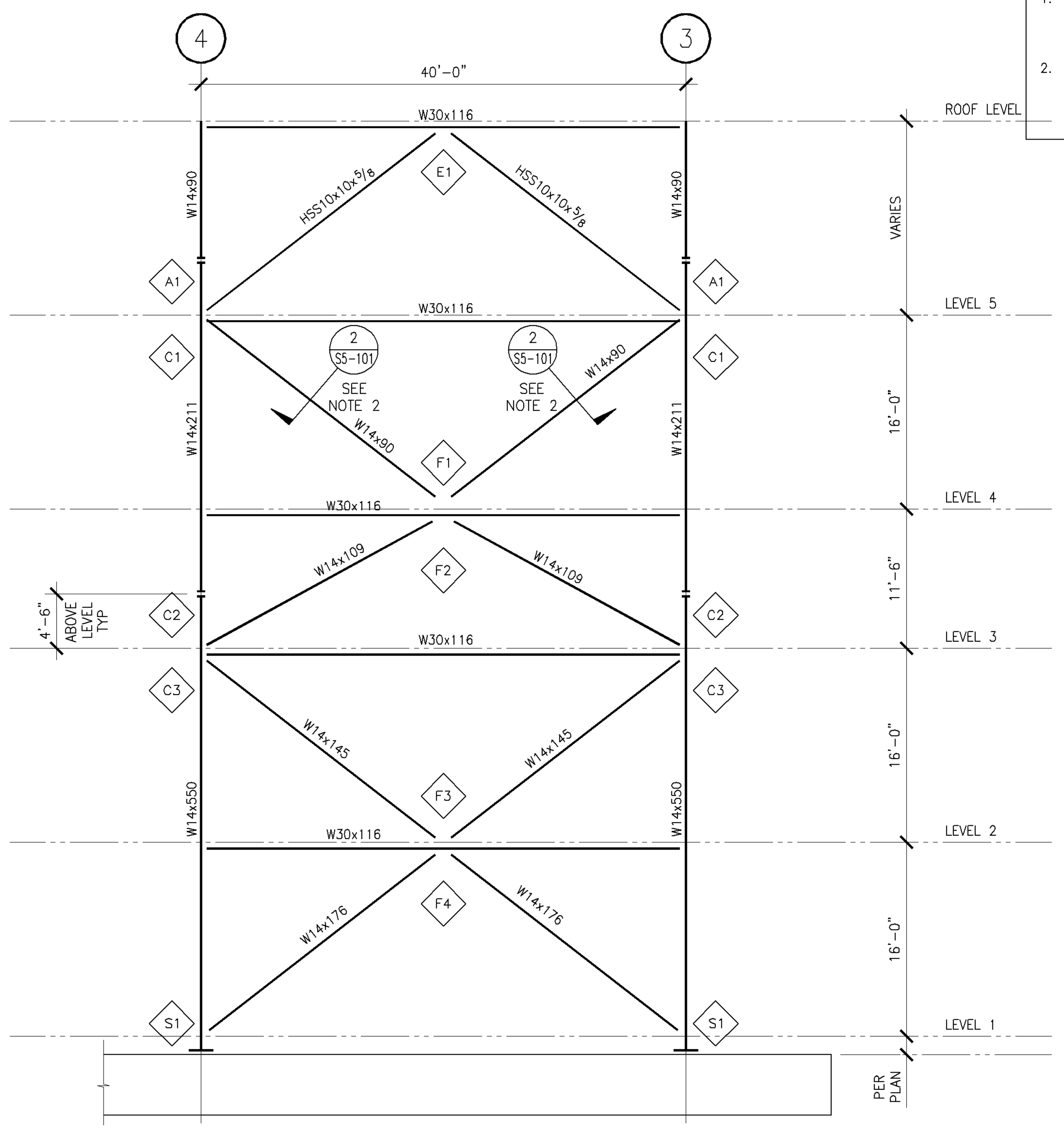


BRACED FRAME ELEVATION - GRID LINE 5

1/8"=1'-0" 4

BRACED FRAME NOTE:

1. INDICATES BRACE FRAME CONNECTION ID PER DETAILS ON SHEETS S6-101 THROUGH S6-109.
2. PROVIDE STIFFENER PLATES PER 2/S5-101 & @ 2'-0" FROM END OF COVER PLATE @ EA END & 4'-0" OC, MAX IN BETWEEN.



BRACED FRAME ELEVATION - GRID LINE A

1/8"=1'-0" 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

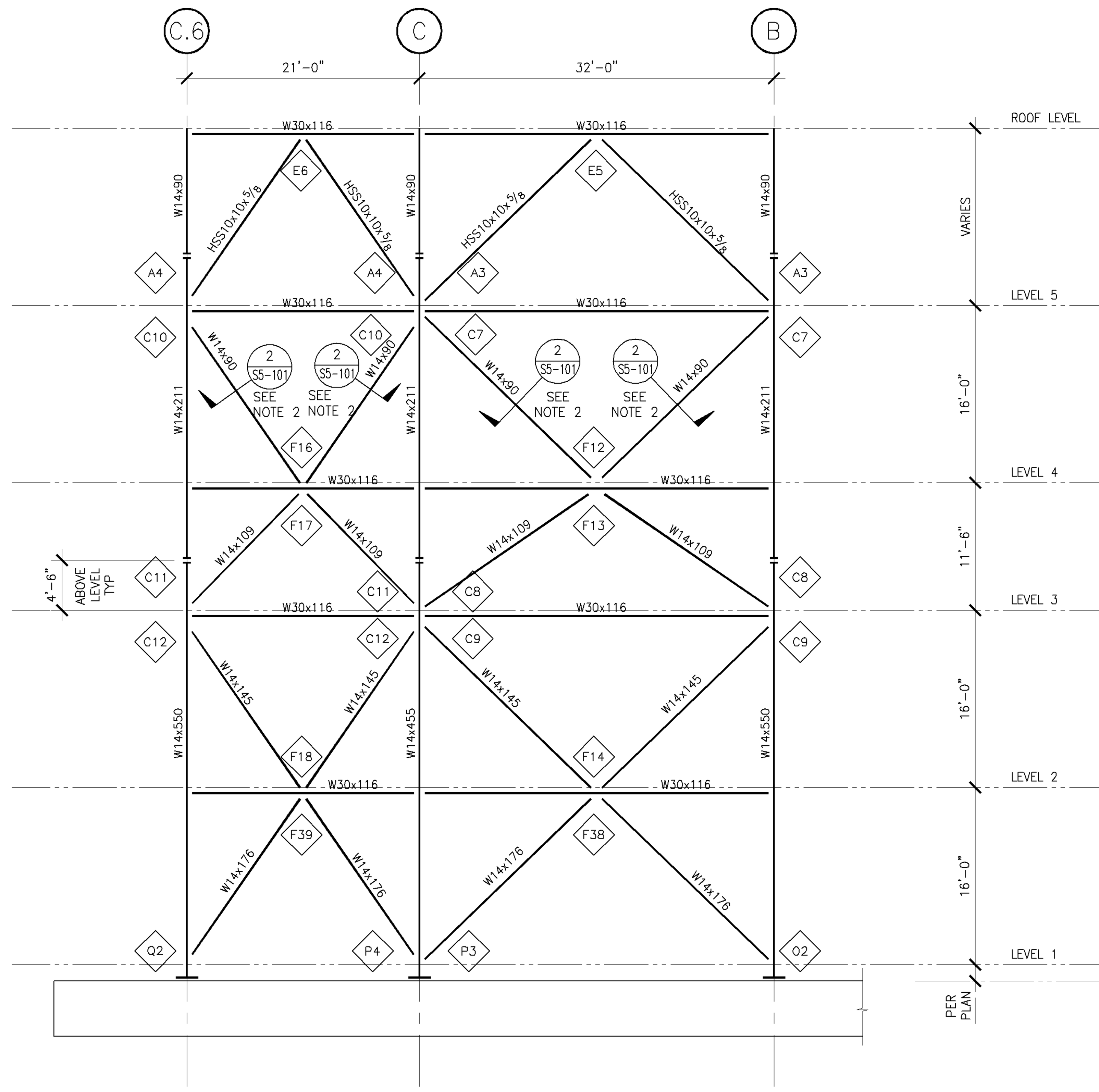
SHEET TITLE

BRACED FRAME ELEVATIONS - BUILDING F

S3-101

BRACED FRAME NOTE:

1. ◇# INDICATES BRACE FRAME CONNECTION ID PER DETAILS ON SHEETS S6-101 THROUGH S6-109.
2. PROVIDE STIFFENER PLATES PER 2/S5-101 & @ 2'-0" FROM END OF COVER PLATE @ EA END & 4'-0" OC, MAX IN BETWEEN.

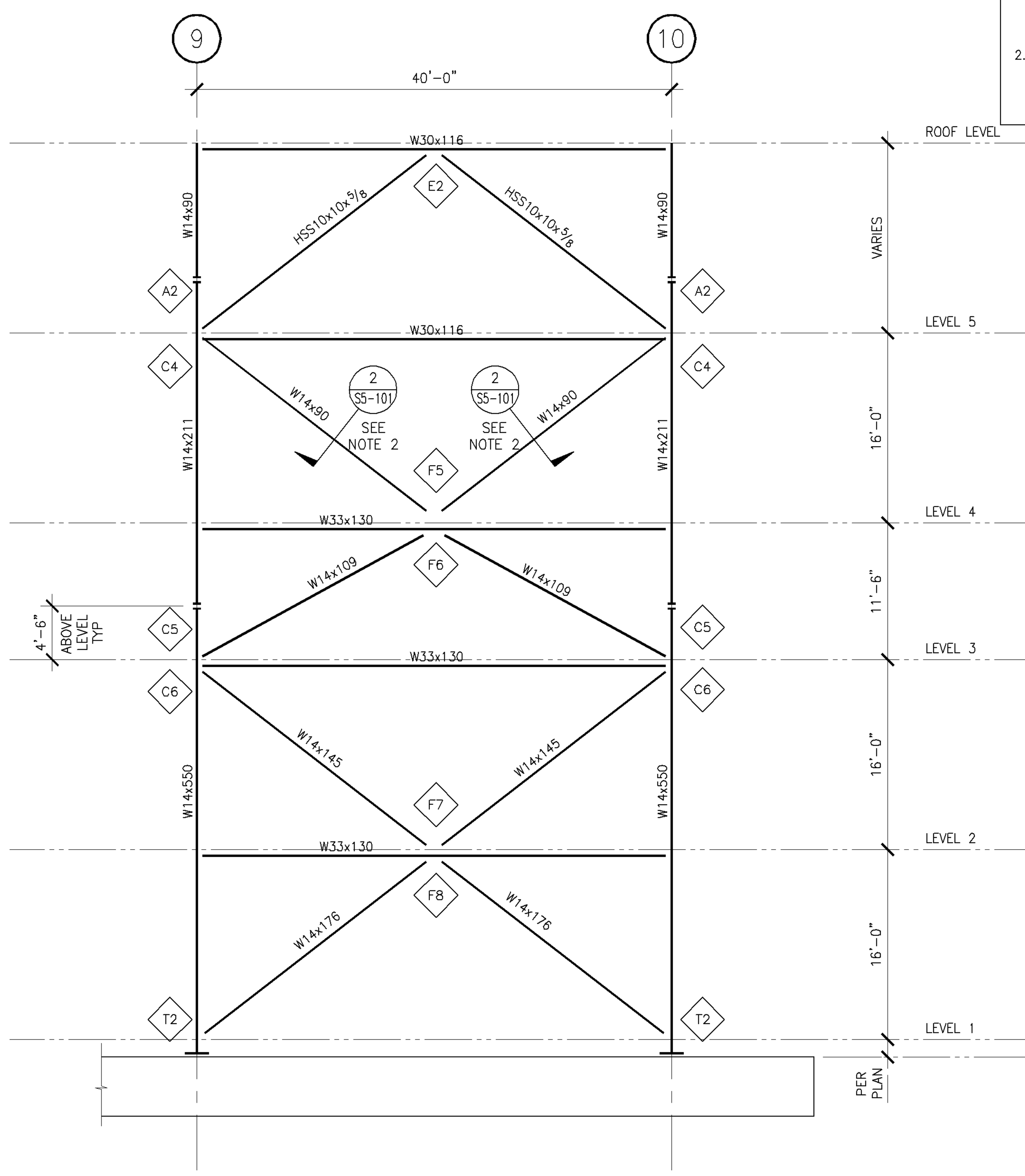


BRACED FRAME ELEVATION - GRID LINES 12

1/8"=1'-0" 3

BRACED FRAME NOTE:

1. ◇# INDICATES BRACE FRAME CONNECTION ID PER DETAILS ON SHEETS S6-101 THROUGH S6-109.
2. PROVIDE STIFFENER PLATES PER 2/S5-101 & @ 2'-0" FROM END OF COVER PLATE @ EA END & 4'-0" OC, MAX IN BETWEEN.

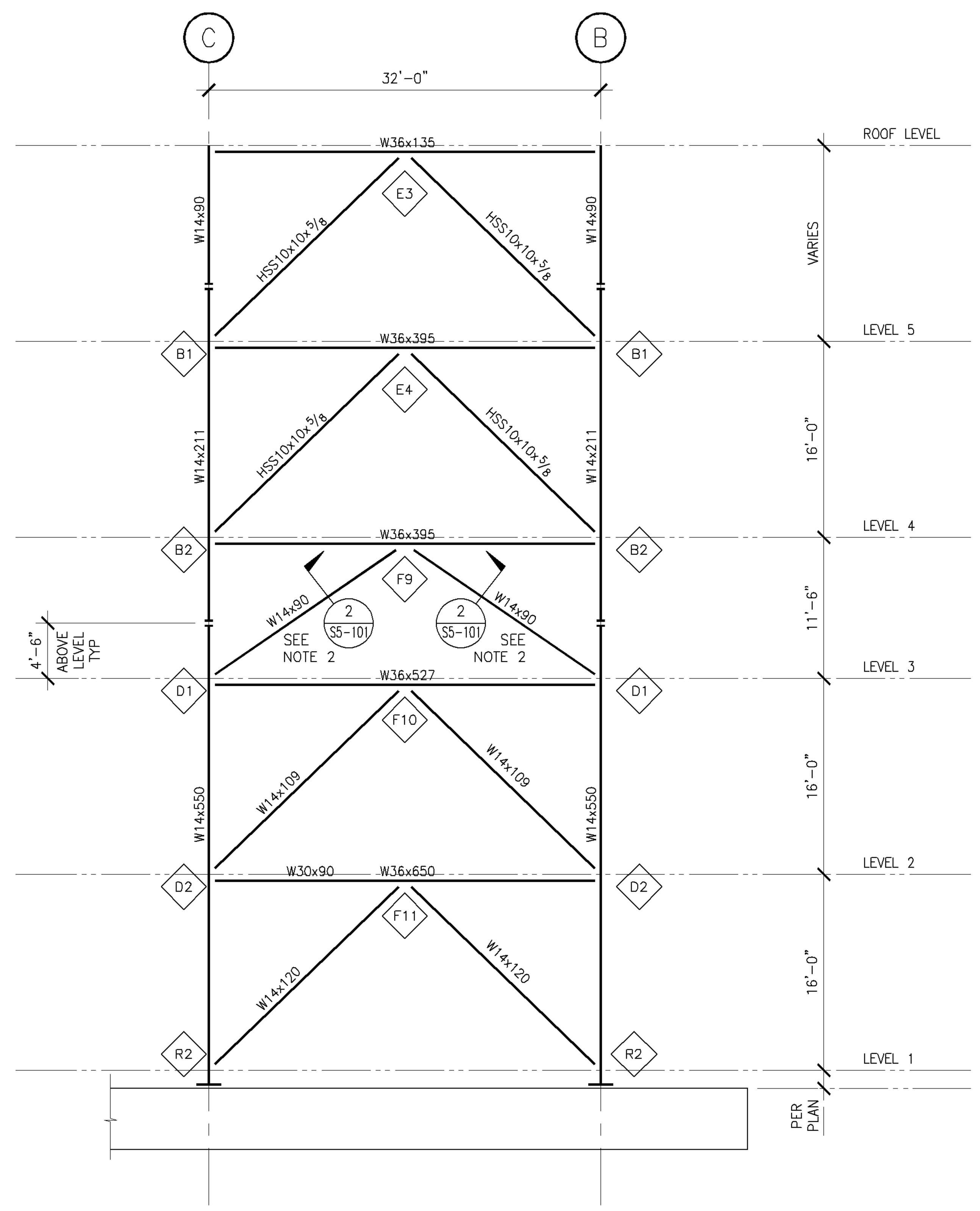


BRACED FRAME ELEVATION - GRID LINE D

1/8"=1'-0" 1

BRACED FRAME NOTE:

1. ◇# INDICATES BRACE FRAME CONNECTION ID PER DETAILS ON SHEETS S6-101 THROUGH S6-109.
2. PROVIDE STIFFENER PLATES PER 2/S5-101 & @ 2'-0" FROM END OF COVER PLATE @ EA END & 4'-0" OC, MAX IN BETWEEN.

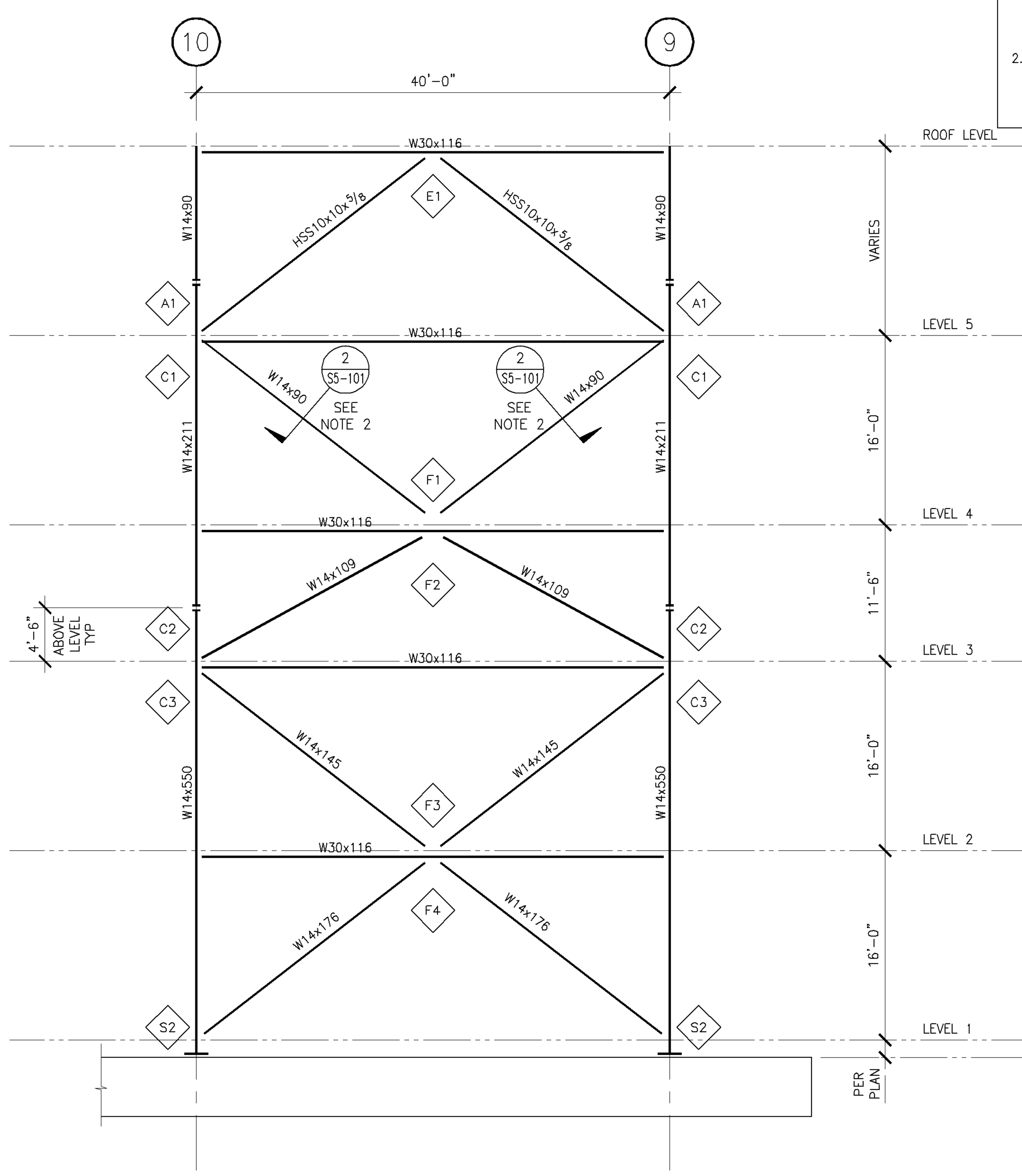


BRACED FRAME ELEVATION - GRID LINE 8

1/8"=1'-0" 4

BRACED FRAME NOTE:

1. ◇# INDICATES BRACE FRAME CONNECTION ID PER DETAILS ON SHEETS S6-101 THROUGH S6-109.
2. PROVIDE STIFFENER PLATES PER 2/S5-101 & @ 2'-0" FROM END OF COVER PLATE @ EA END & 4'-0" OC, MAX IN BETWEEN.



BRACED FRAME ELEVATION - GRID LINE A

1/8"=1'-0" 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
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1	10-23-06	BLDG & SAFETY SUBMITTAL

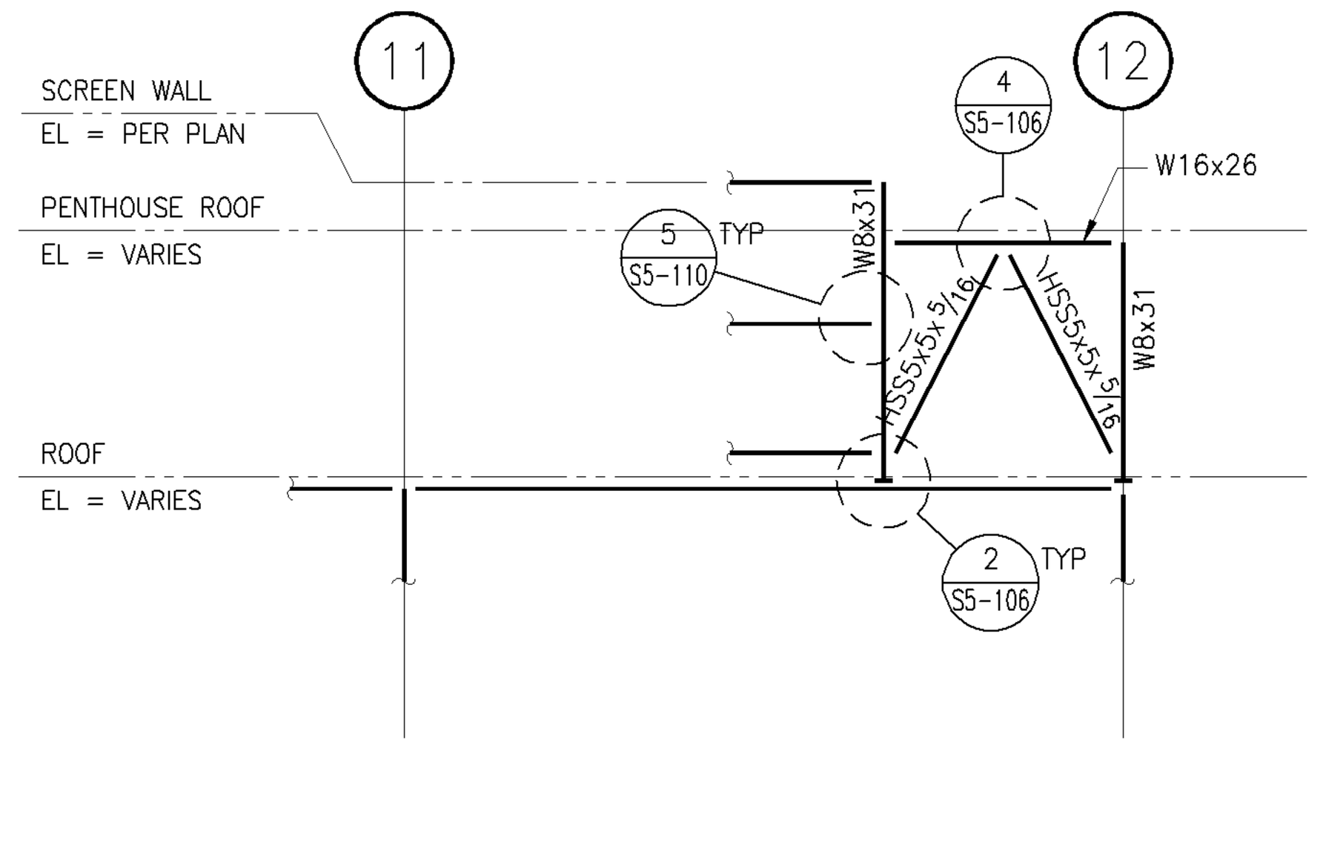
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

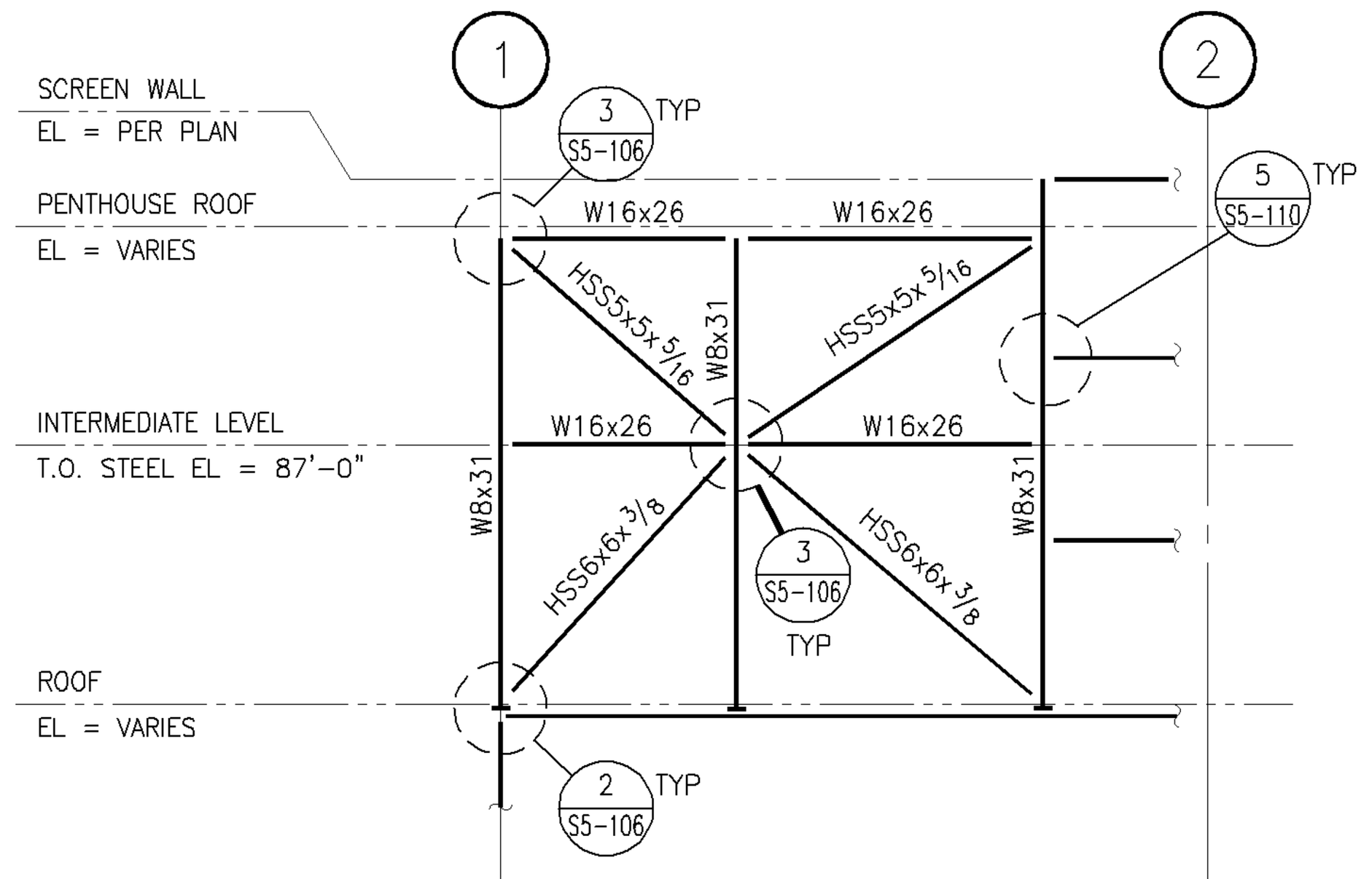
SHEET TITLE

BRACED FRAME ELEVATIONS - BUILDING F

S3-102

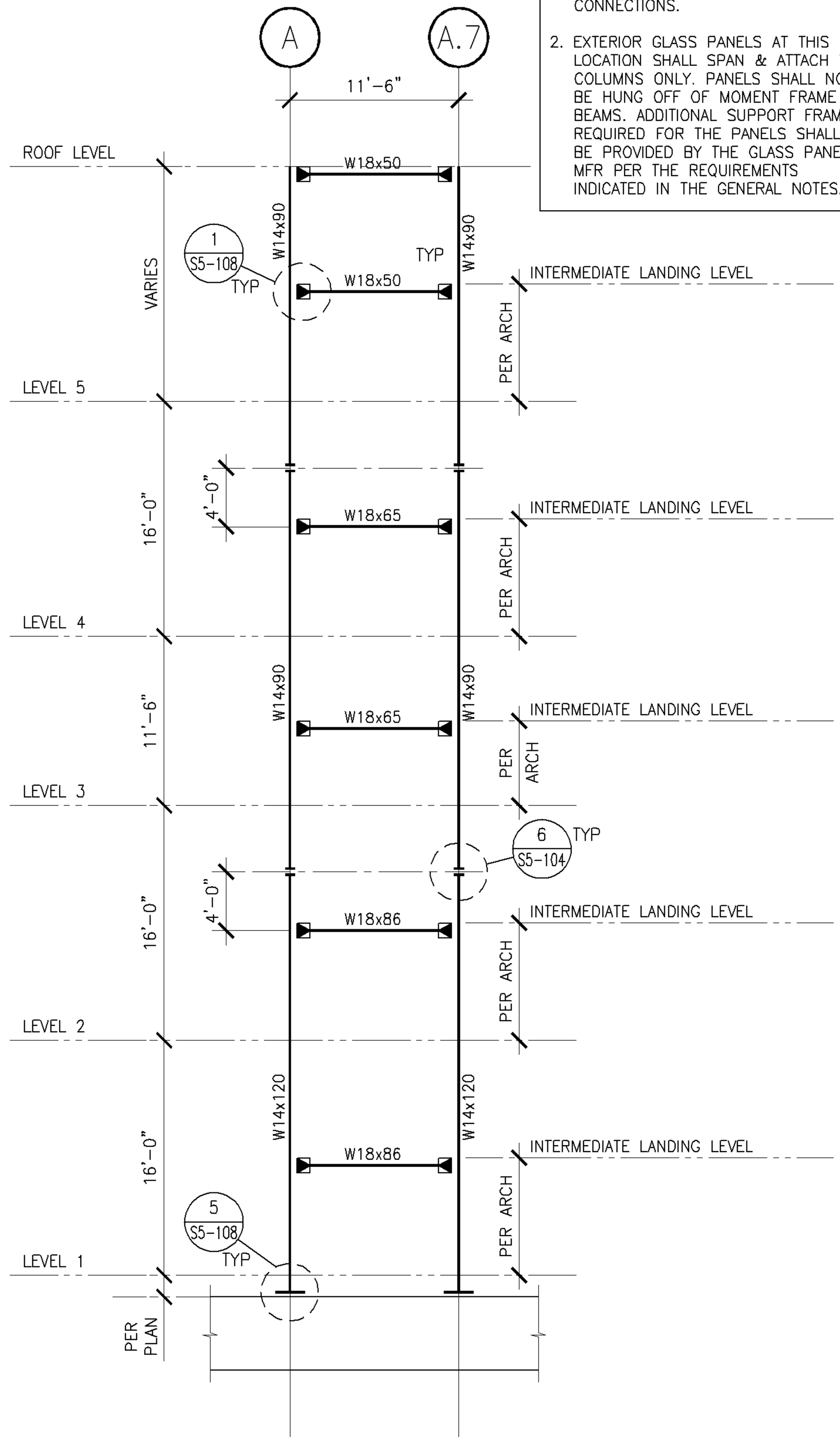


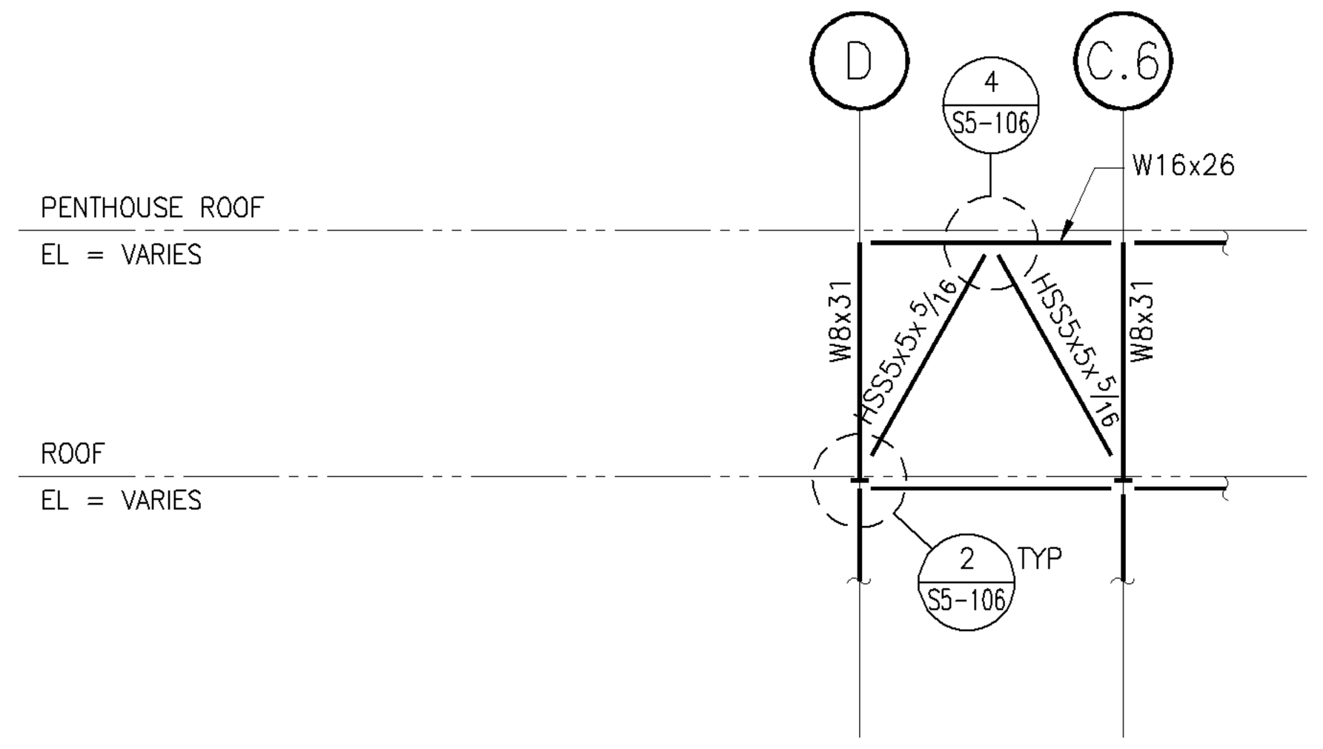
PENTHOUSE FRAME ELEVATION - GRID LINE D 1/8"=1'-0" 7



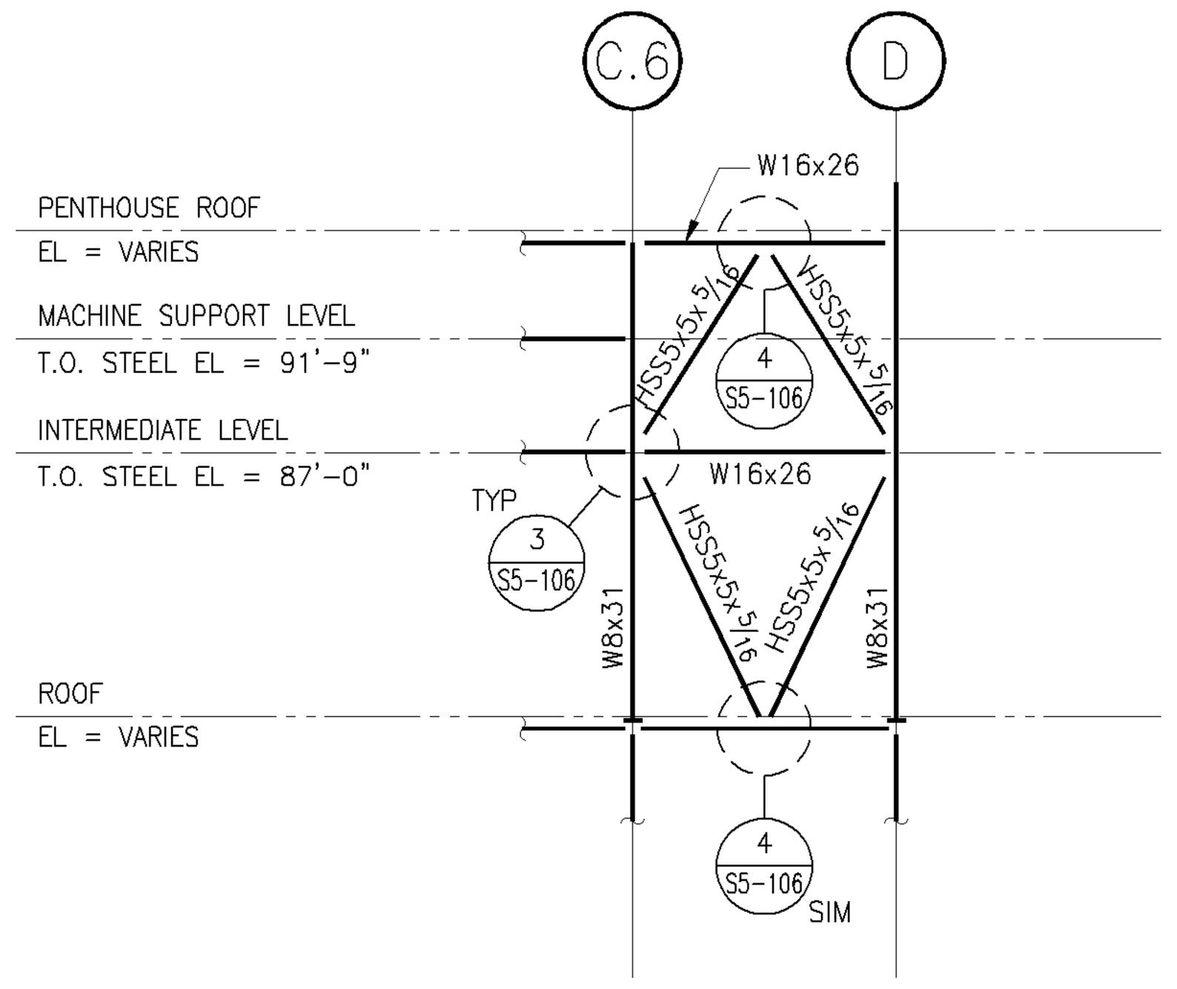
PENTHOUSE FRAME ELEVATION - GRID LINE D 1/8"=1'-0" 3

NOTE:
1. SEE 3/S5-108 FOR WELDING GENERAL NOTES FOR MOMENT FRAME CONNECTIONS.
2. EXTERIOR GLASS PANELS AT THIS LOCATION SHALL SPAN & ATTACH TO COLUMNS ONLY. PANELS SHALL NOT BE HUNG OFF OF MOMENT FRAME BEAMS. ADDITIONAL SUPPORT FRAMING REQUIRED FOR THE PANELS SHALL BE PROVIDED BY THE GLASS PANEL MFR PER THE REQUIREMENTS INDICATED IN THE GENERAL NOTES.



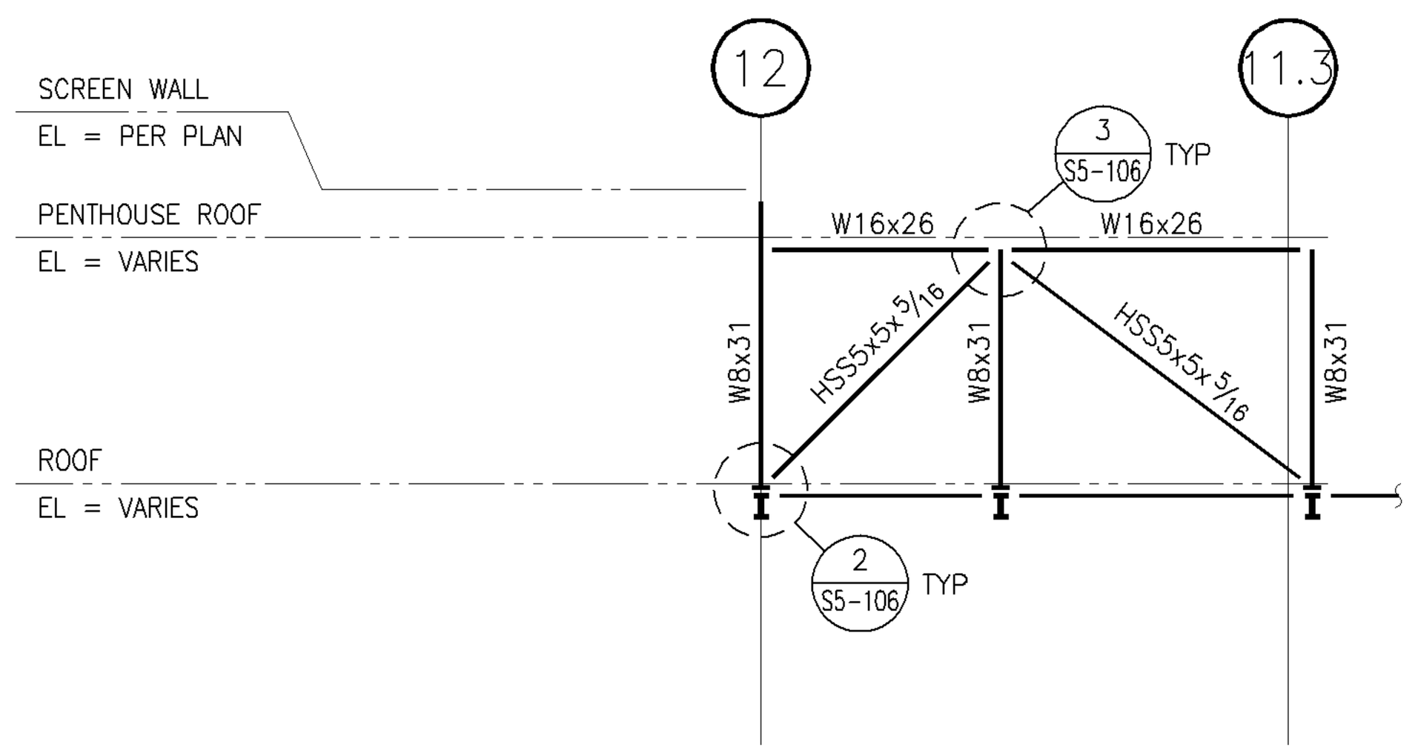


PENTHOUSE FRAME ELEVATION - GRID LINE 12 1/8"=1'-0" 8

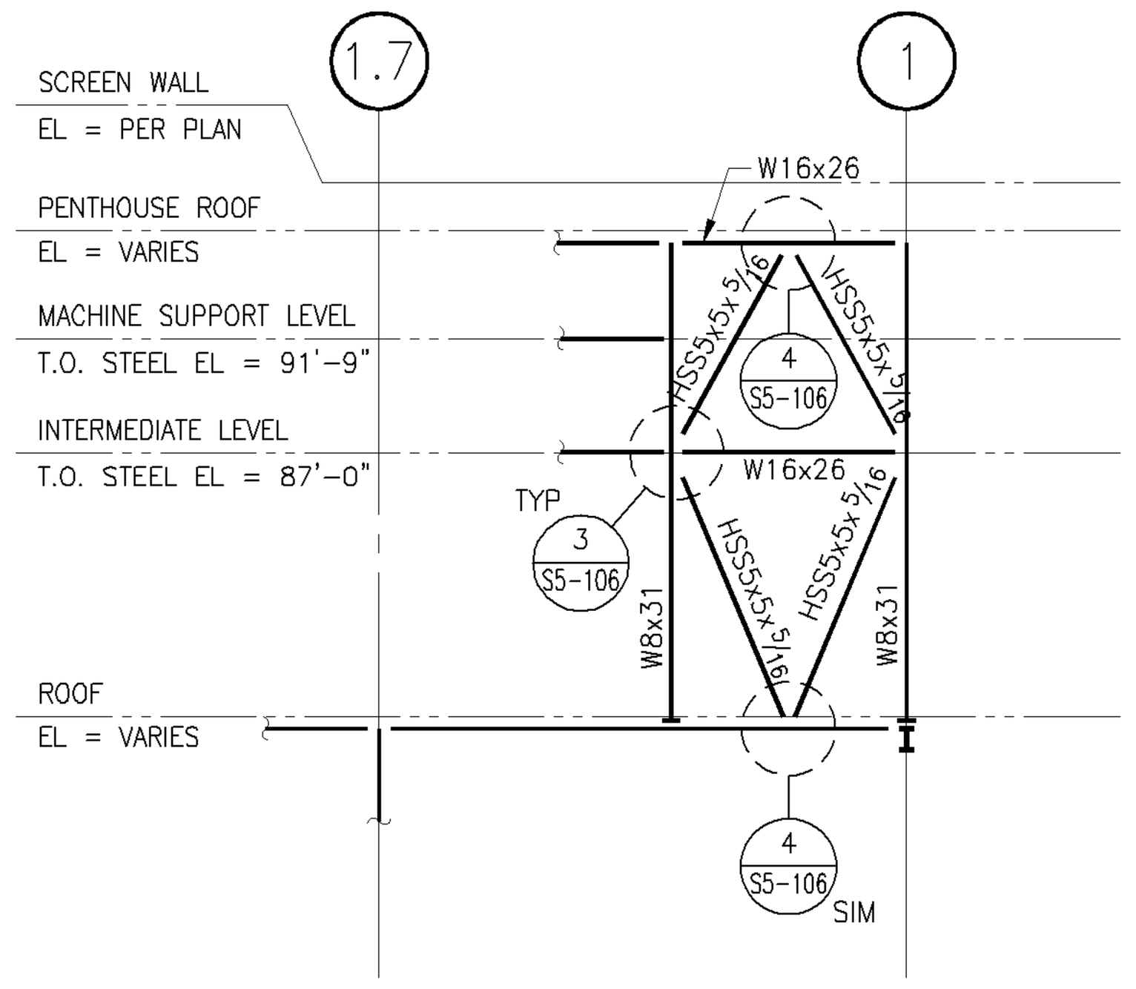


PENTHOUSE FRAME ELEVATION - GRID LINE 1 1/8"=1'-0" 4

MOMENT FRAME ELEVATION - GRID LINE 1 1/8"=1'-0" 1

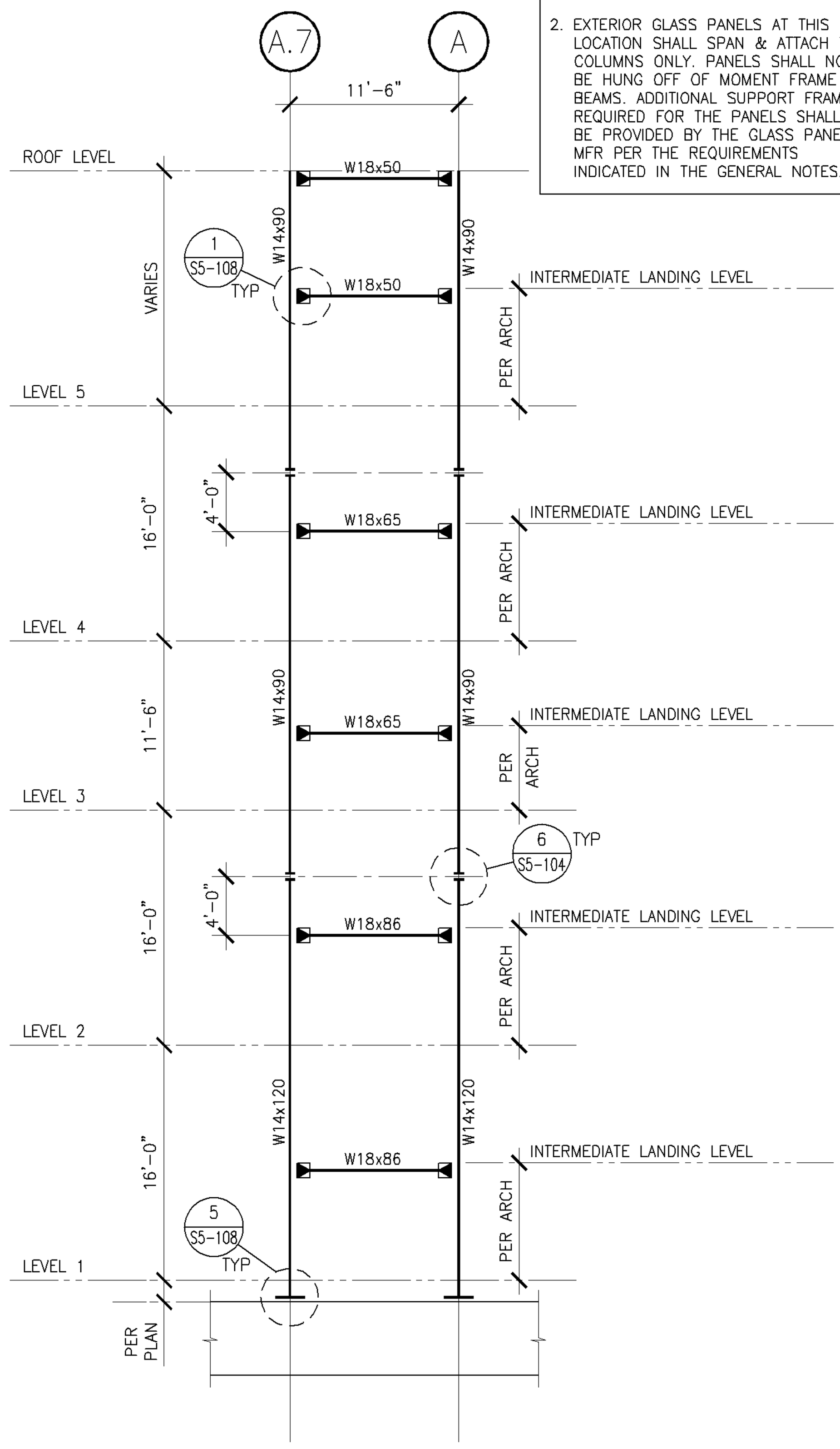


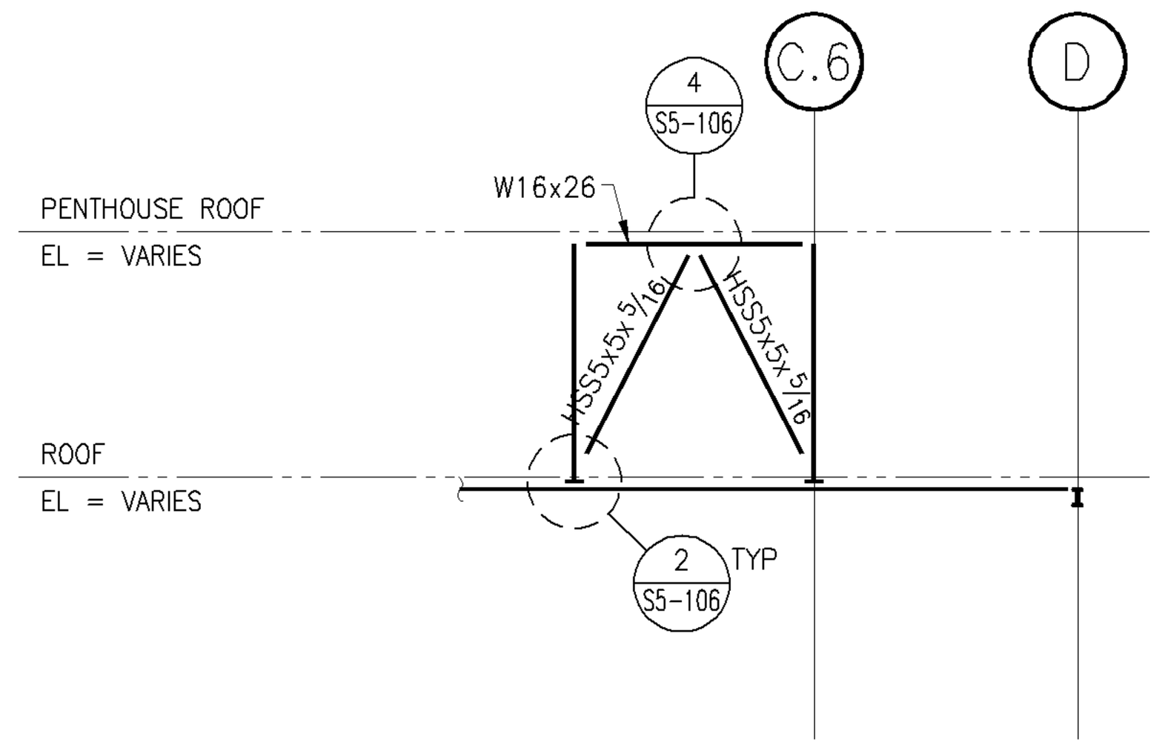
PENTHOUSE FRAME ELEVATION - BETWEEN GRID LINES C.8 & C.6 1/8"=1'-0" 9



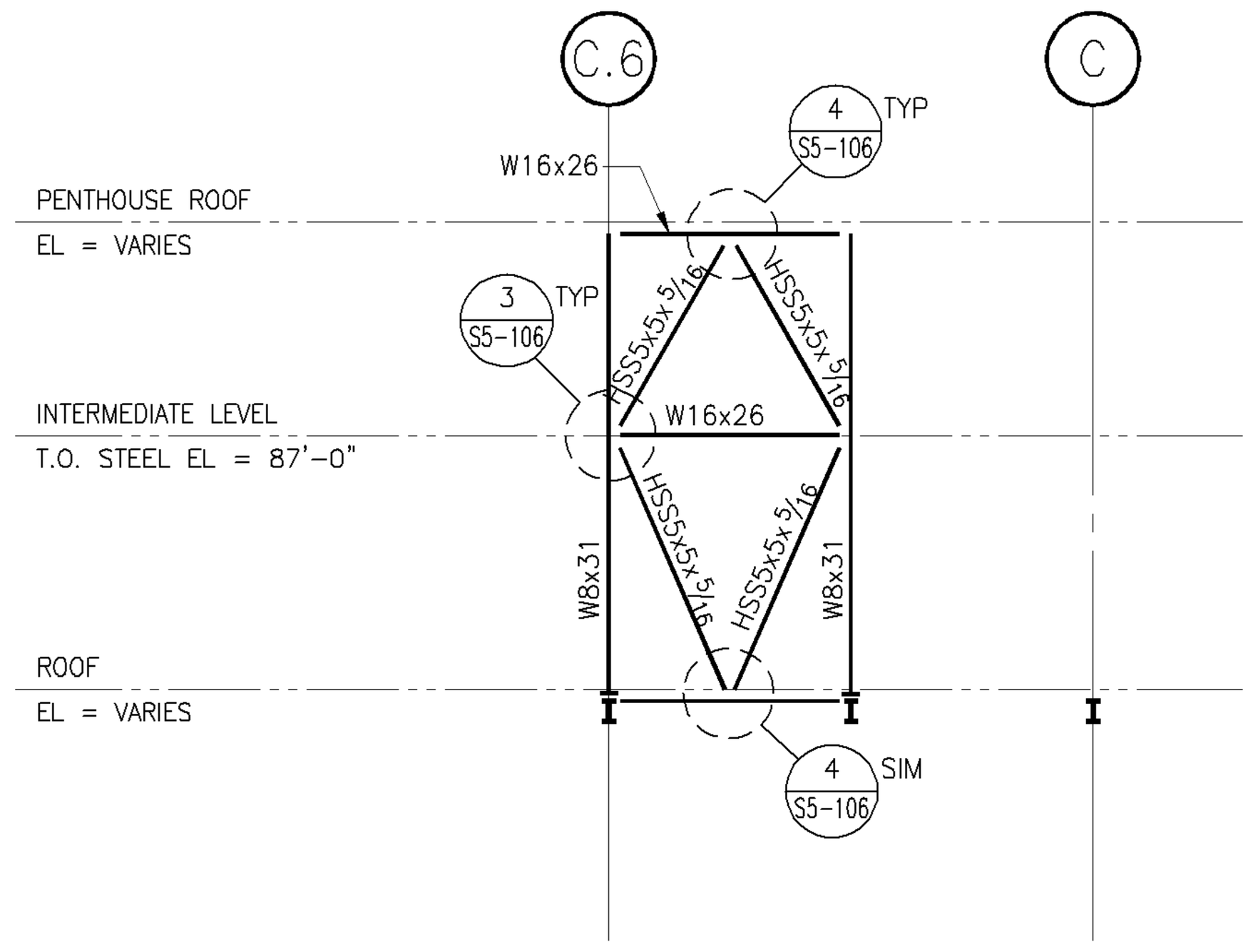
PENTHOUSE FRAME ELEVATION - BETWEEN GRID LINES C & C.6 1/8"=1'-0" 5

NOTE:
1. SEE 3/S5-108 FOR WELDING GENERAL NOTES FOR MOMENT FRAME CONNECTIONS.
2. EXTERIOR GLASS PANELS AT THIS LOCATION SHALL SPAN & ATTACH TO COLUMNS ONLY. PANELS SHALL NOT BE HUNG OFF OF MOMENT FRAME BEAMS. ADDITIONAL SUPPORT FRAMING REQUIRED FOR THE PANELS SHALL BE PROVIDED BY THE GLASS PANEL MFR PER THE REQUIREMENTS INDICATED IN THE GENERAL NOTES.





PENTHOUSE FRAME ELEVATION - GRID LINE 11.3 1/8"=1'-0" 10



PENTHOUSE FRAME ELEVATION - GRID LINE 2.1 1/8"=1'-0" 6

MOMENT FRAME ELEVATION - GRID LINE 12 1/8"=1'-0" 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

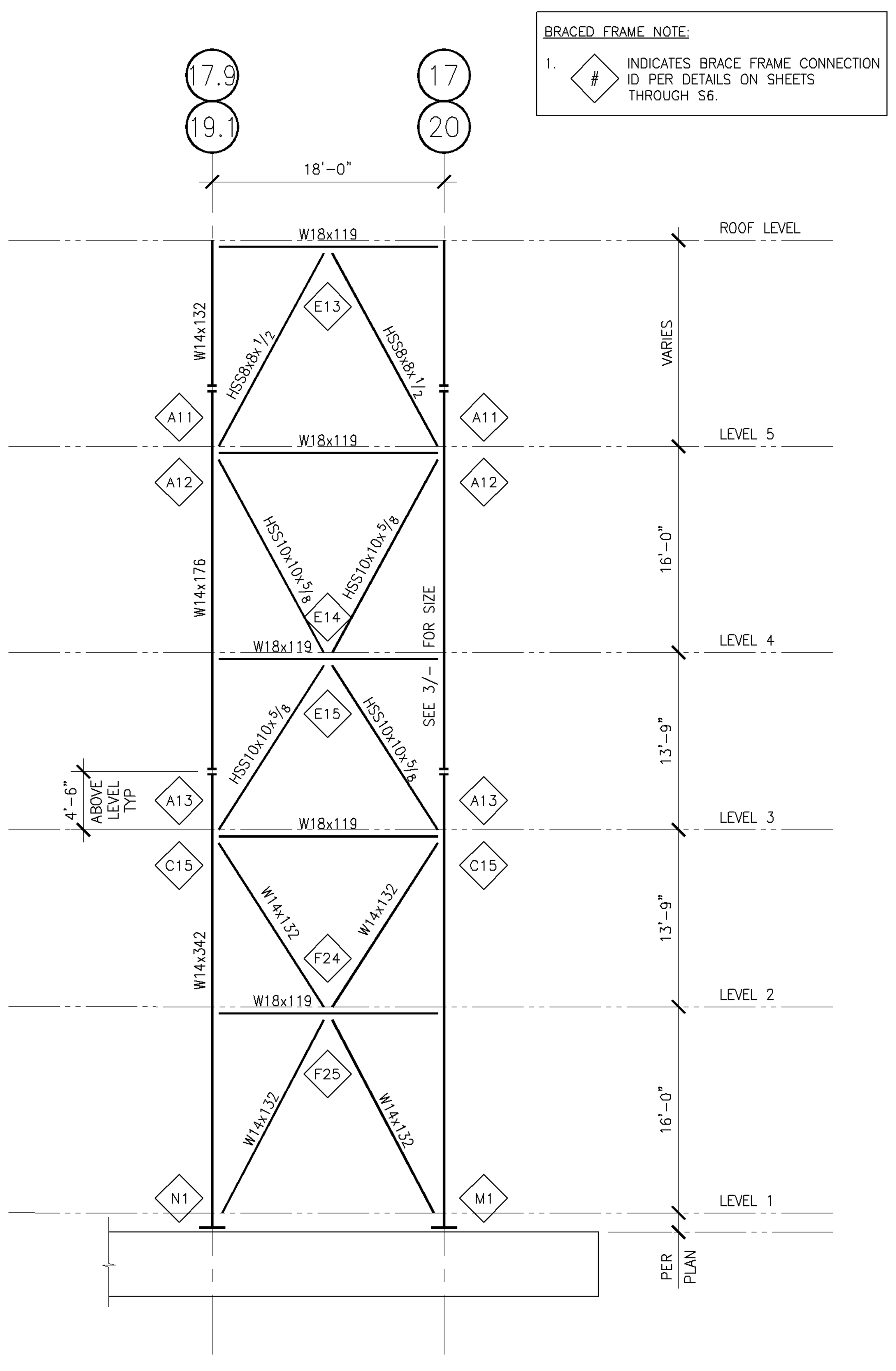
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

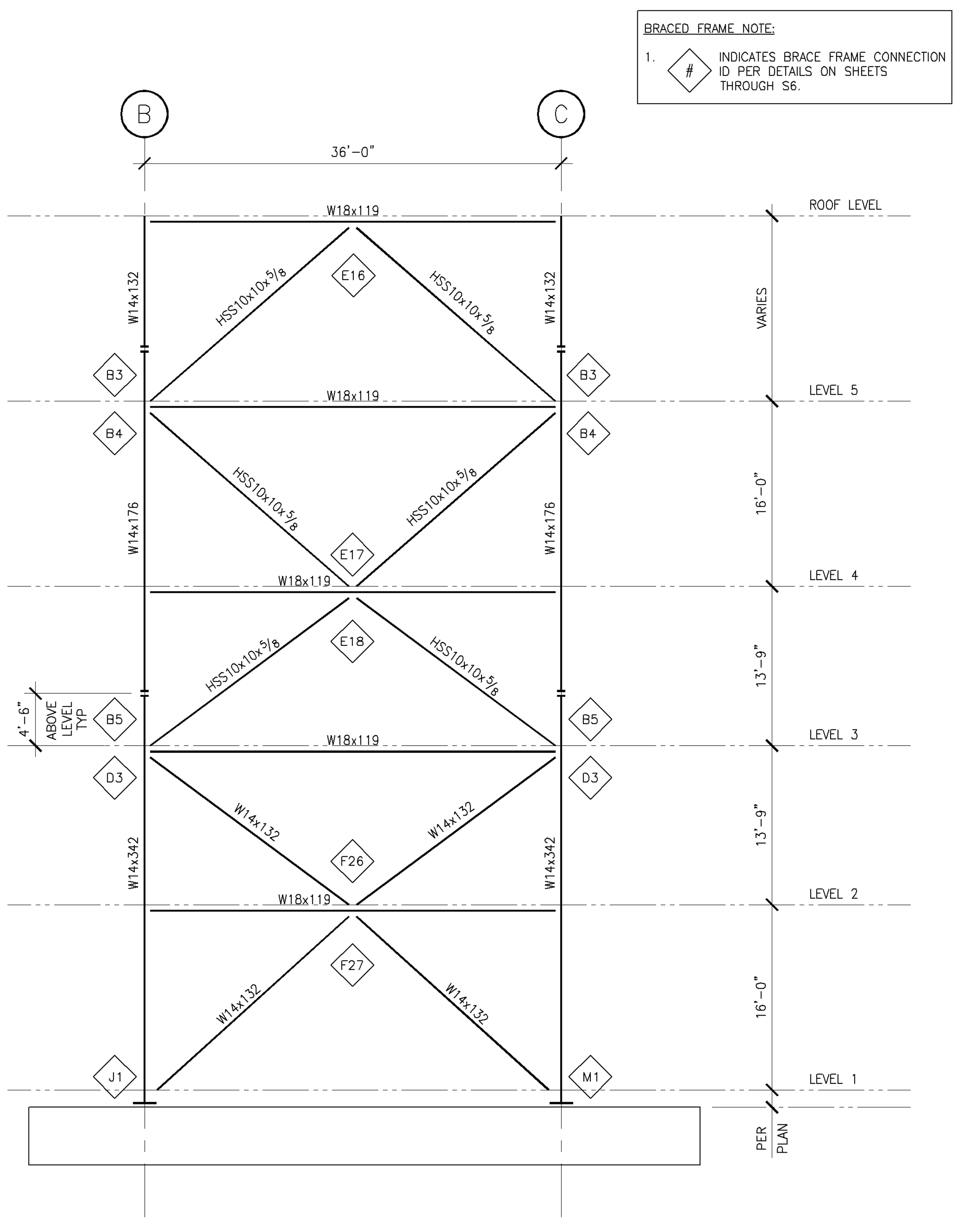
SHEET TITLE

FRAME ELEVATIONS - BUILDING F

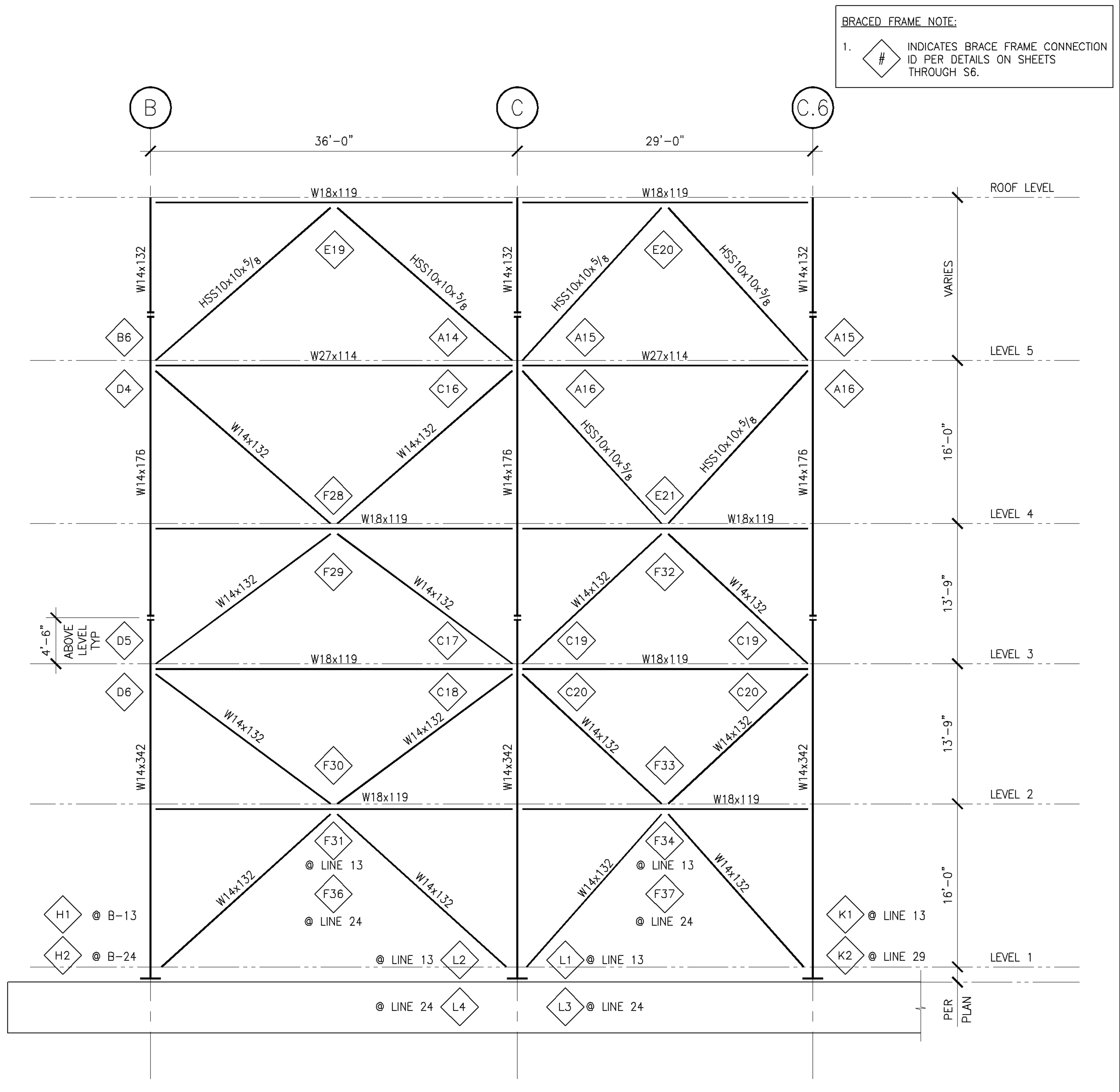
S3-103



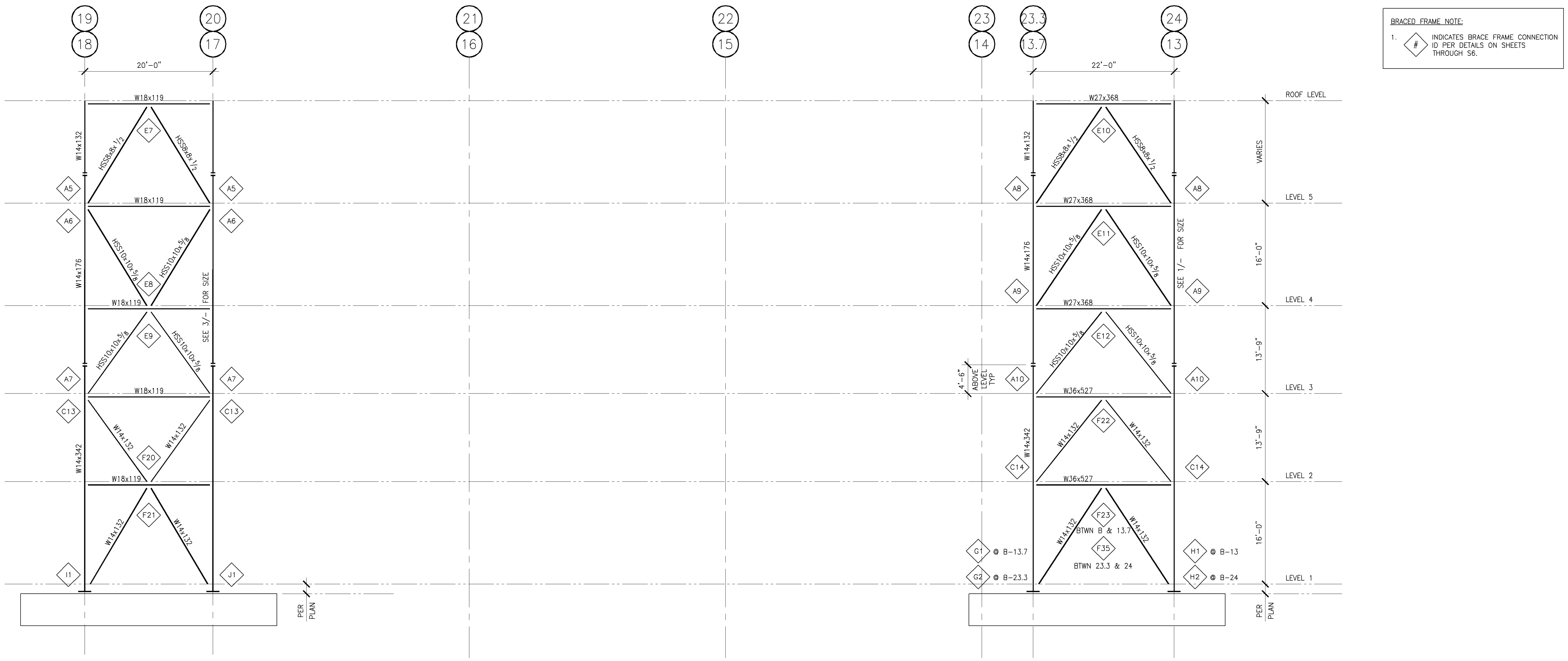
BRACED FRAME ELEVATION - GRID LINE C   1/8"=1'-0"   4



BRACED FRAME ELEVATION - GRID LINES 17 & 20   1/8"=1'-0"   3



BRACED FRAME ELEVATION - GRID LINES 13 & 24   1/8"=1'-0"   1



BRACED FRAME ELEVATION - GRID LINE B   1/8"=1'-0"   2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

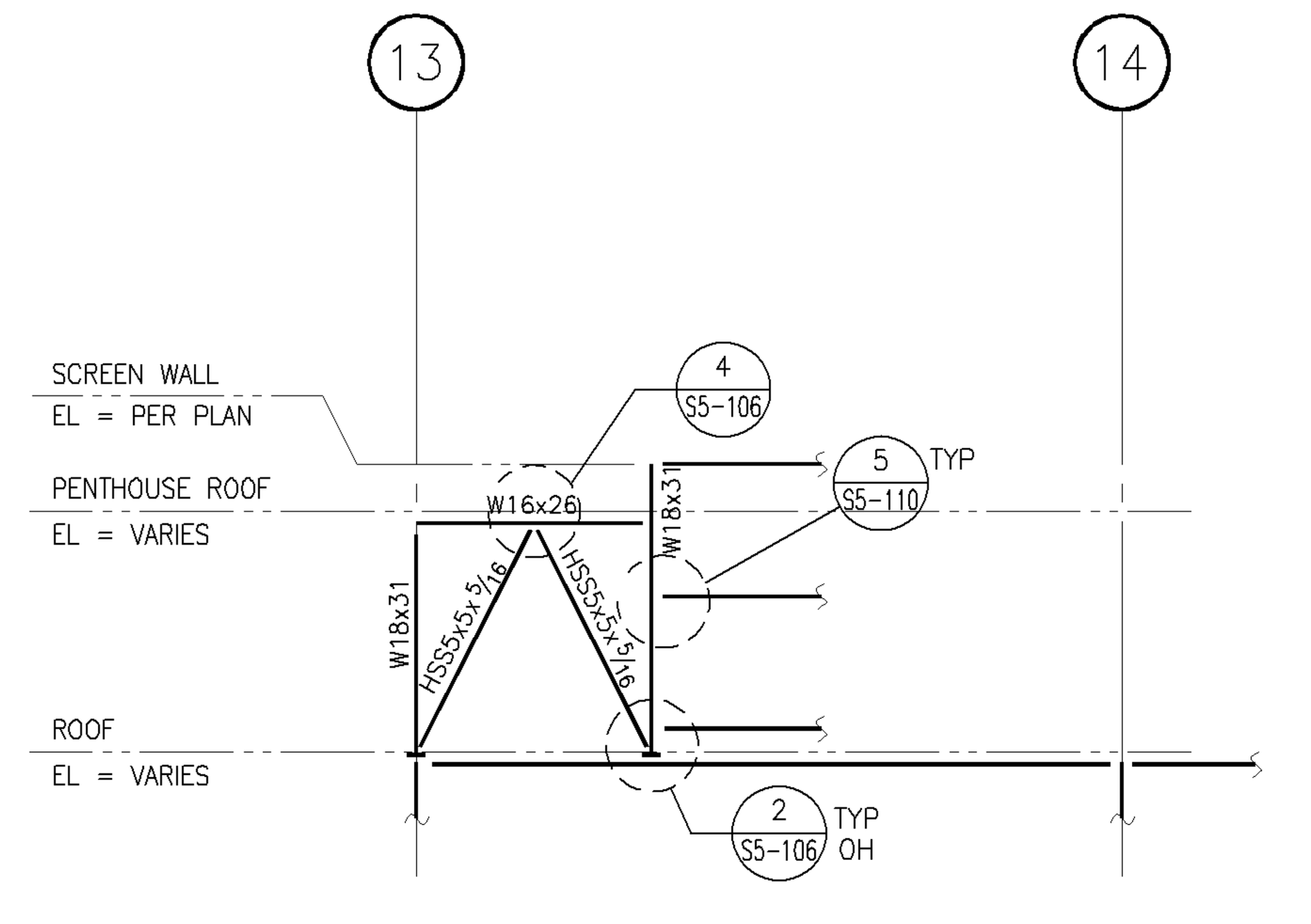
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

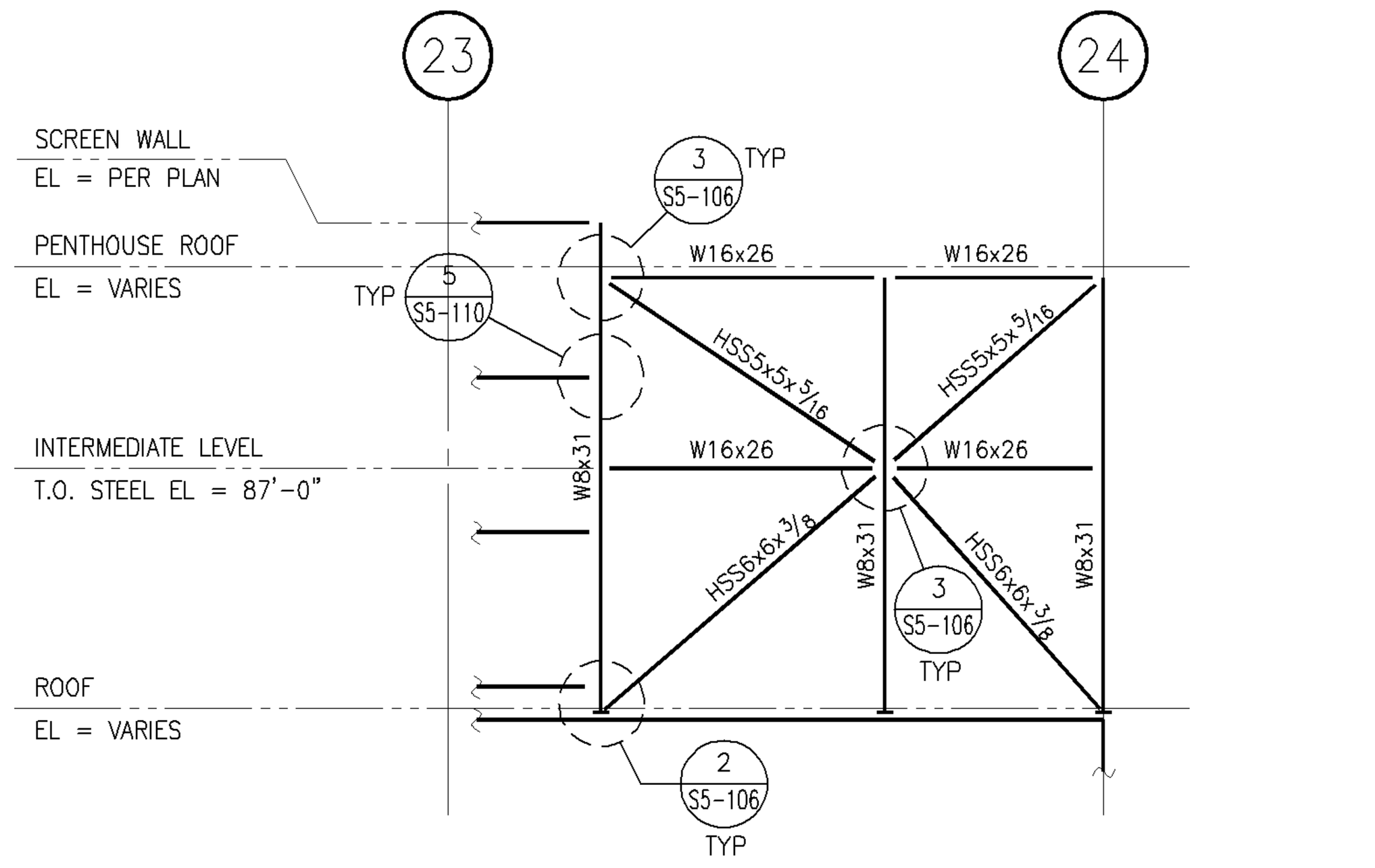
SHEET TITLE

BRACED FRAME ELEVATIONS - BUILDING G

S3-111

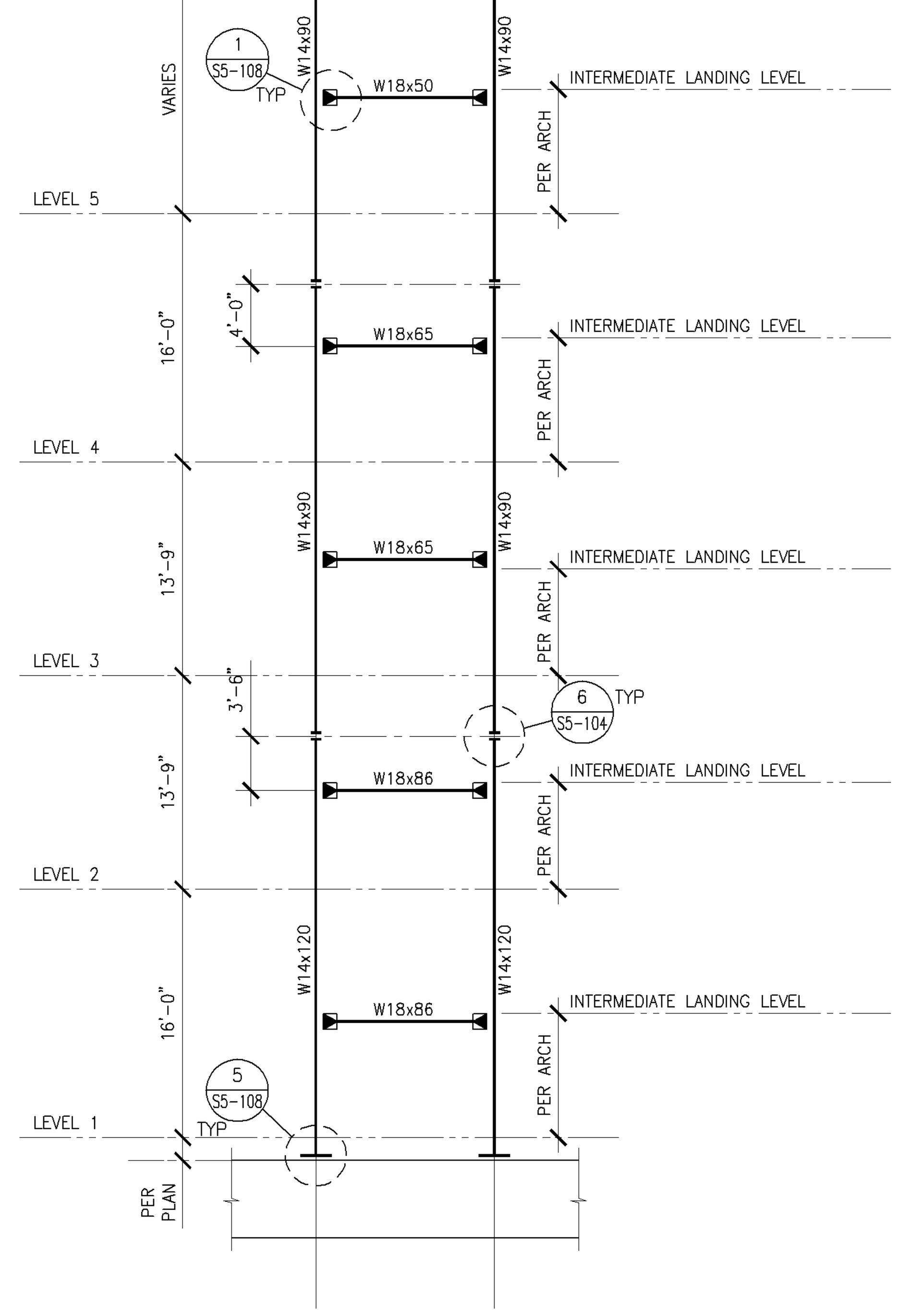


PENTHOUSE FRAME ELEVATION - GRID LINE D 1/8"=1'-0" 7

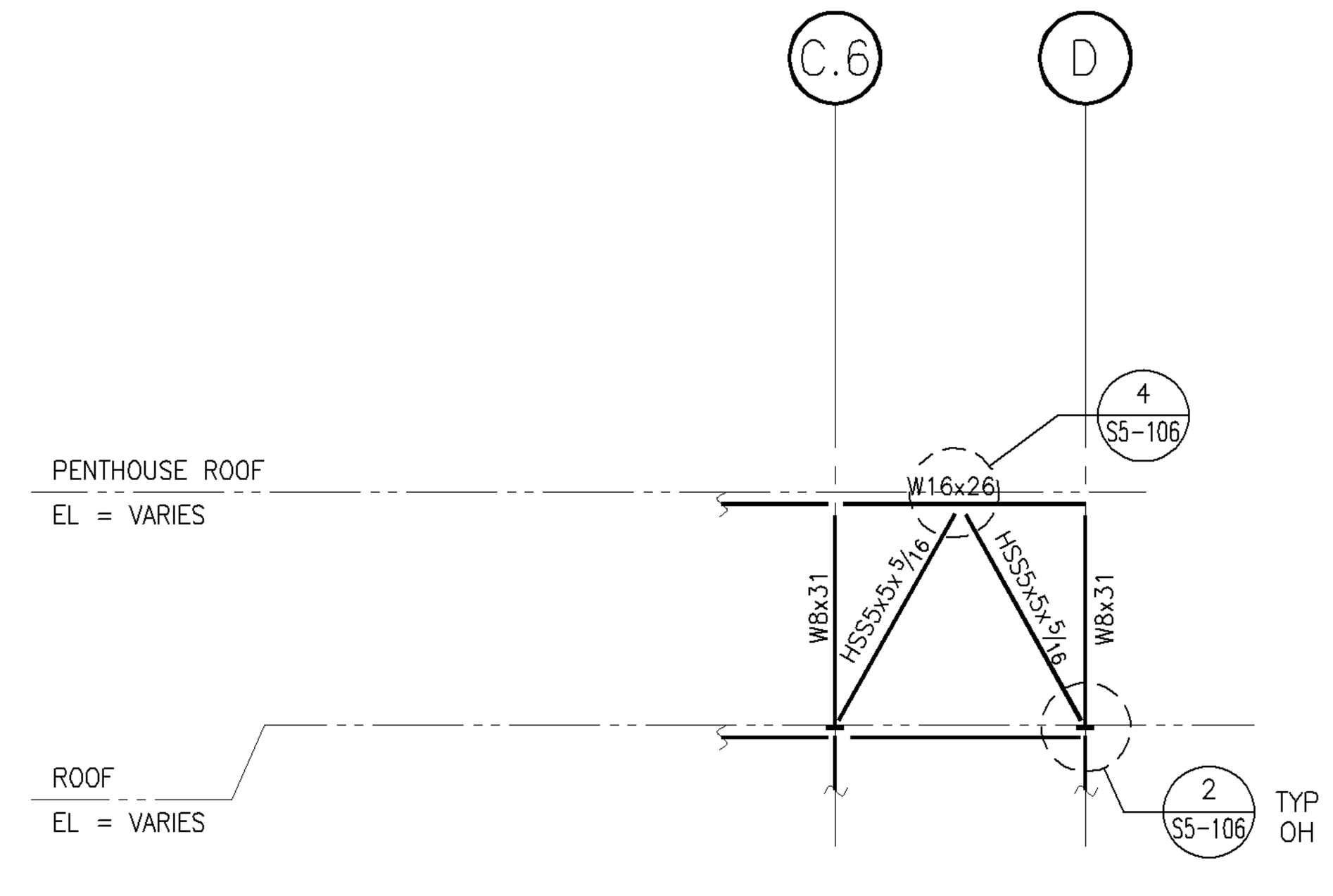


PENTHOUSE FRAME ELEVATION - GRID LINE D 1/8"=1'-0" 3

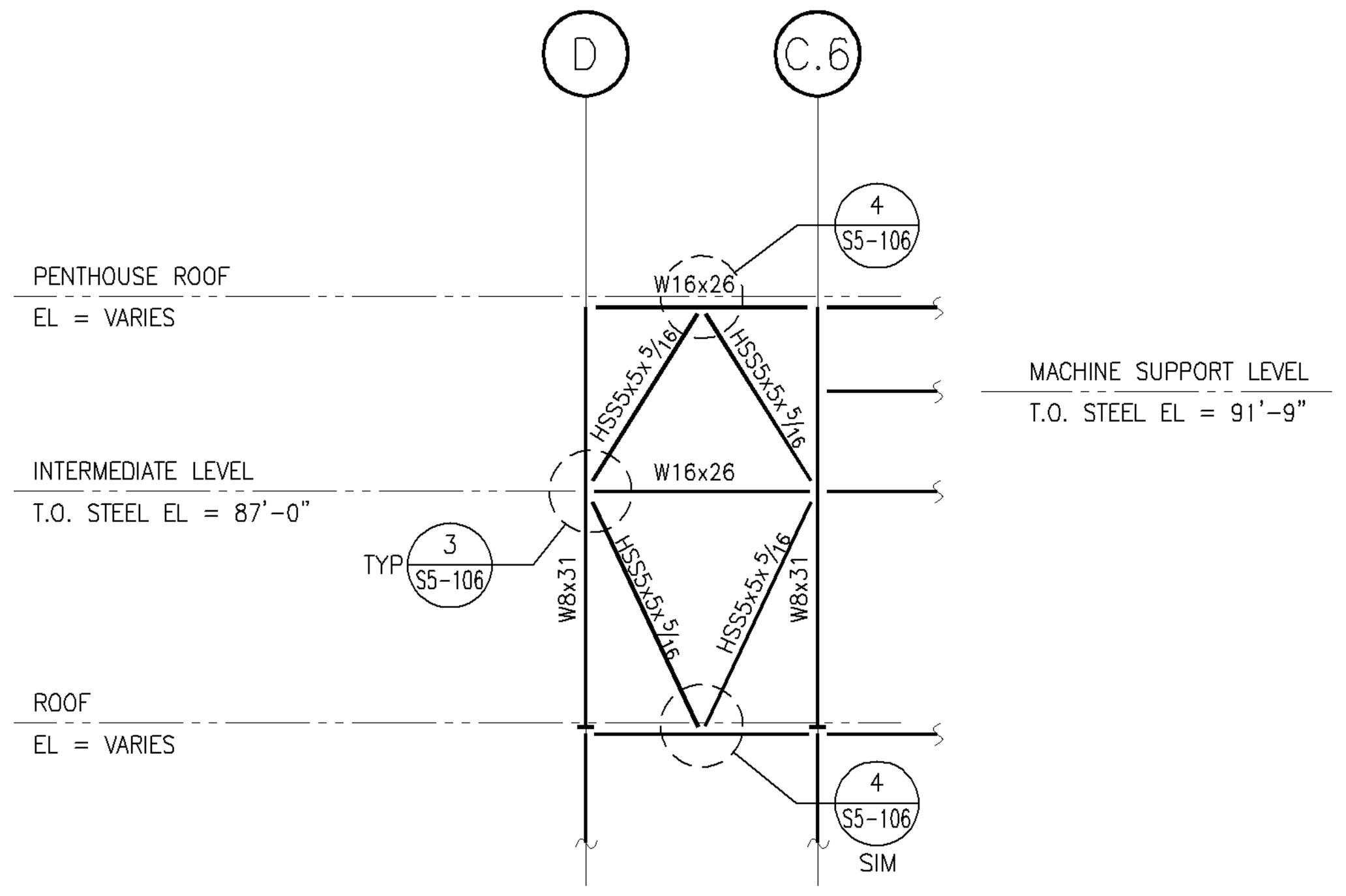
NOTE:
1. SEE 3/S5-108 FOR WELDING GENERAL NOTES FOR MOMENT FRAME CONNECTIONS.
2. EXTERIOR GLASS PANELS AT THIS LOCATION SHALL SPAN & ATTACH TO COLUMNS ONLY. PANELS SHALL NOT BE HUNG OFF OF MOMENT FRAME BEAMS. ADDITIONAL SUPPORT FRAMING REQUIRED FOR THE PANELS SHALL BE PROVIDED BY THE GLASS PANEL MFR PER THE REQUIREMENTS INDICATED IN THE GENERAL NOTES.



1/8"=1'-0" 11

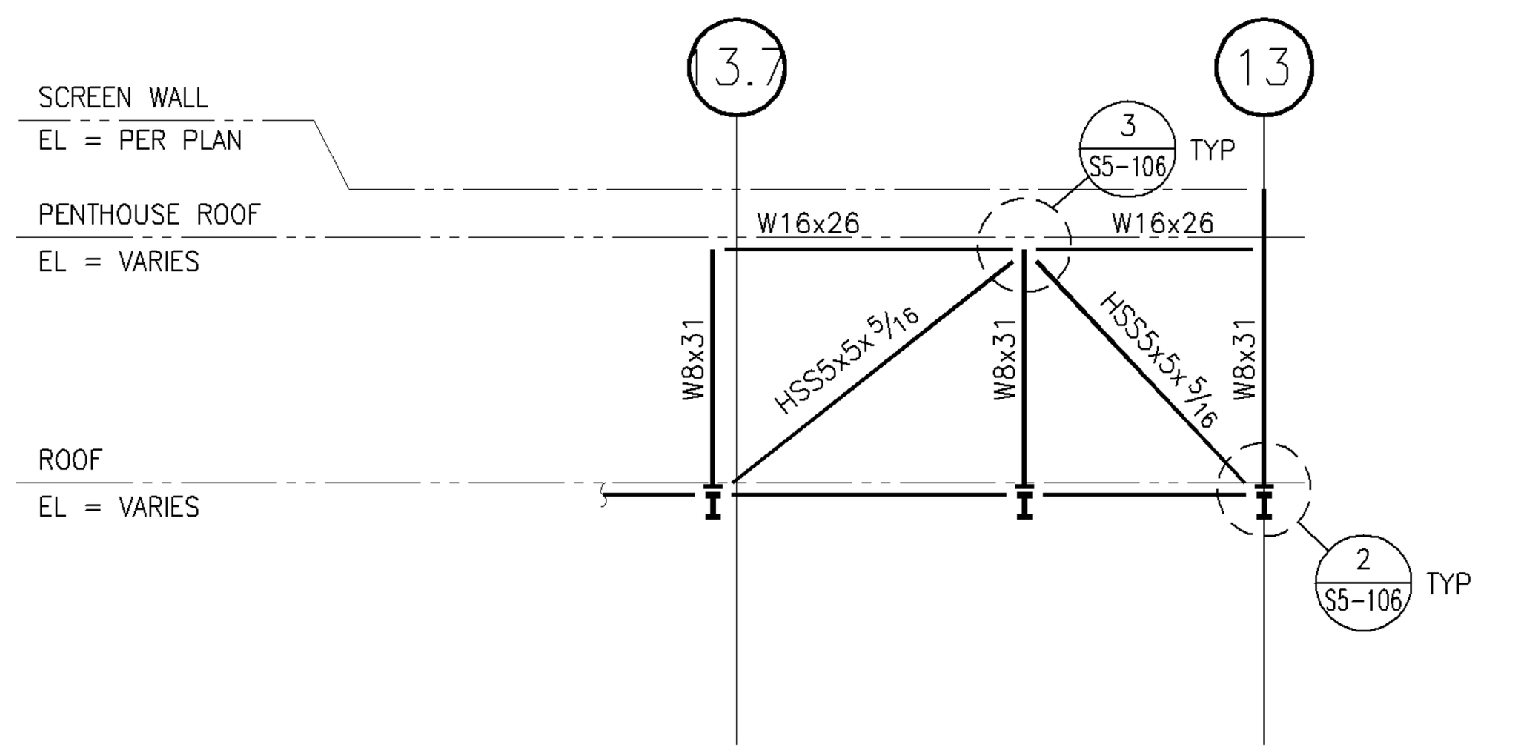


PENTHOUSE FRAME ELEVATION - GRID LINE 13 1/8"=1'-0" 8

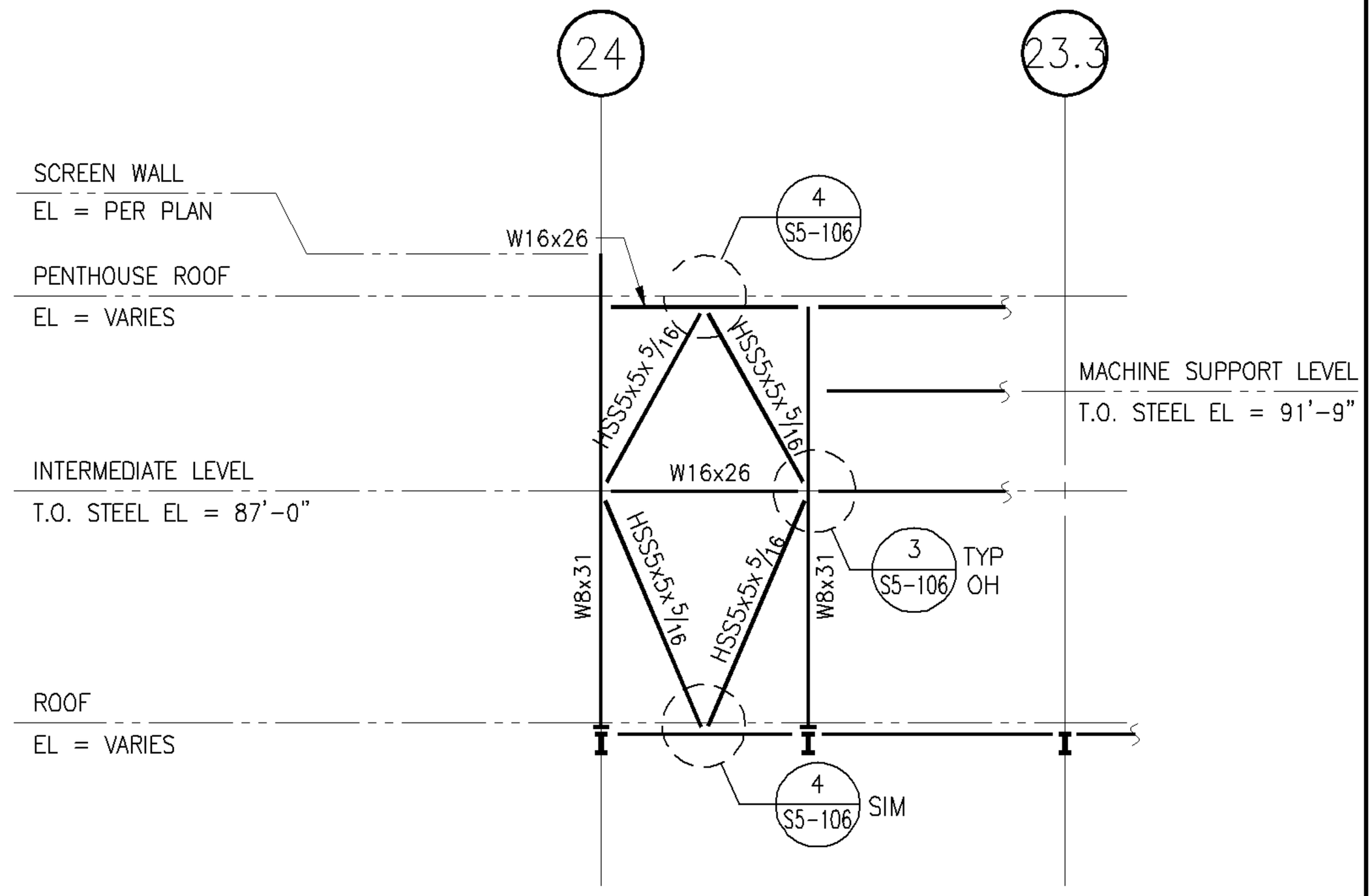


PENTHOUSE FRAME ELEVATION - GRID LINE 24 1/8"=1'-0" 4

MOMENT FRAME ELEVATION - GRID LINE 13 1/8"=1'-0" 1



PENTHOUSE FRAME ELEVATION - BETWEEN GRID LINES C & C.6 1/8"=1'-0" 9



PENTHOUSE FRAME ELEVATION - BETWEEN GRID LINES C & C.6 1/8"=1'-0" 5

NOTE:
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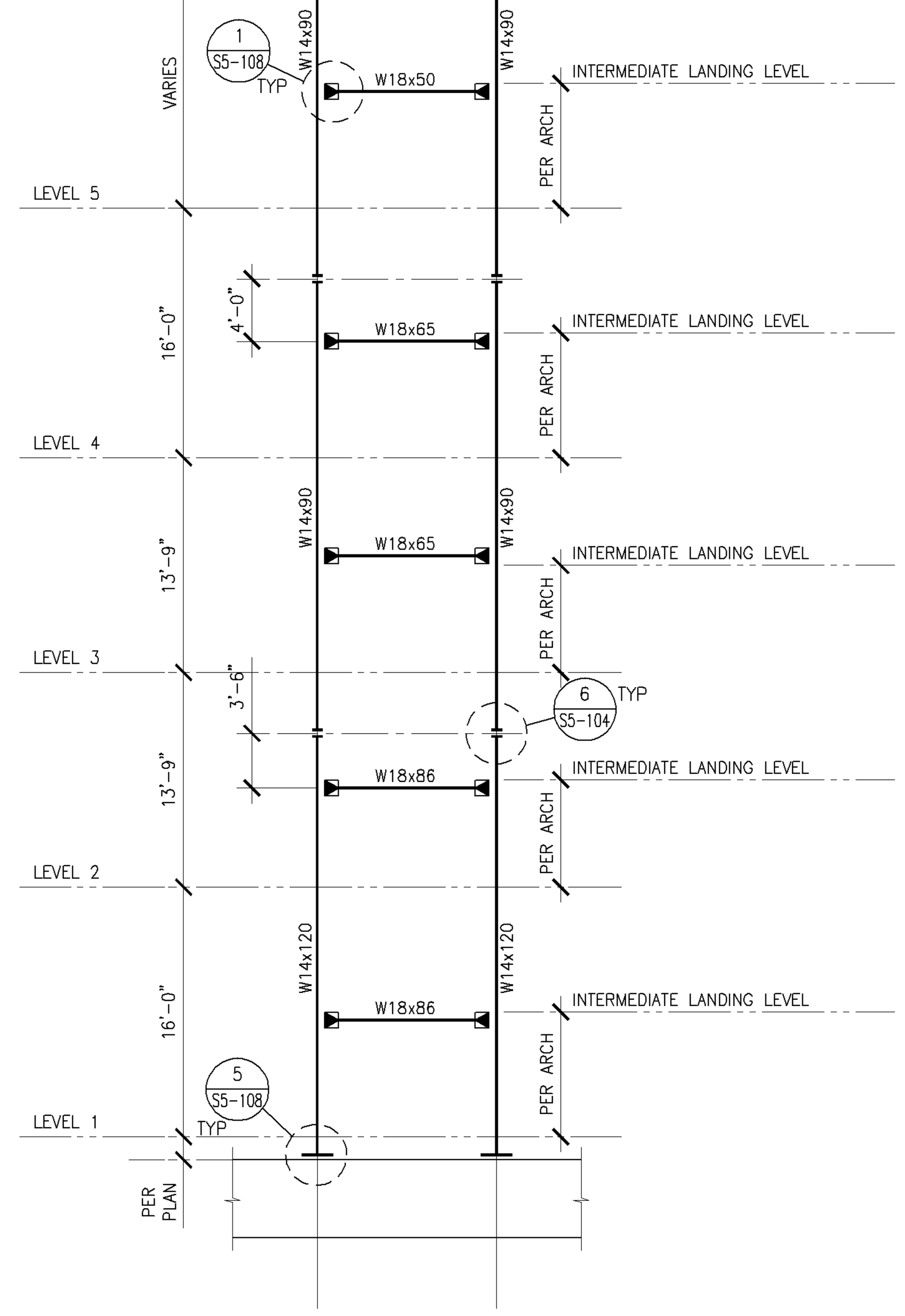
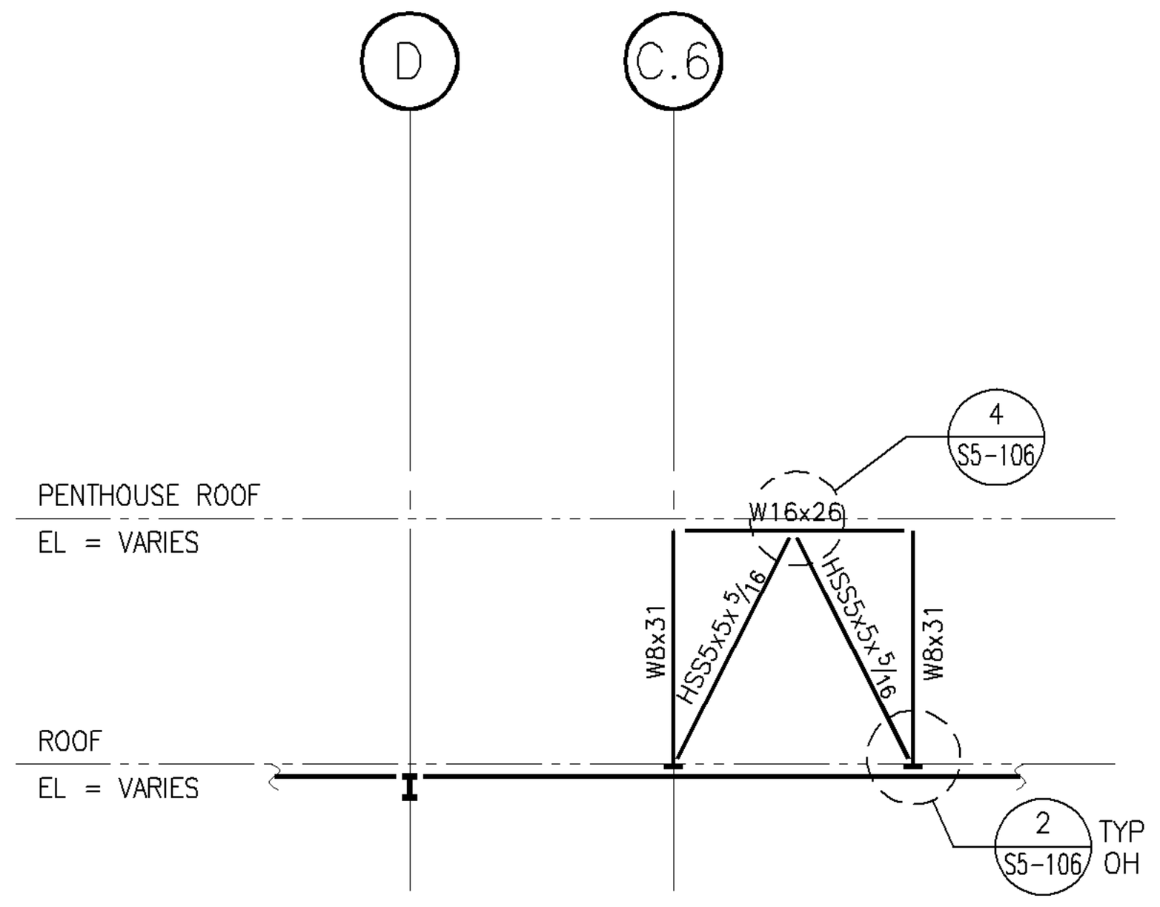
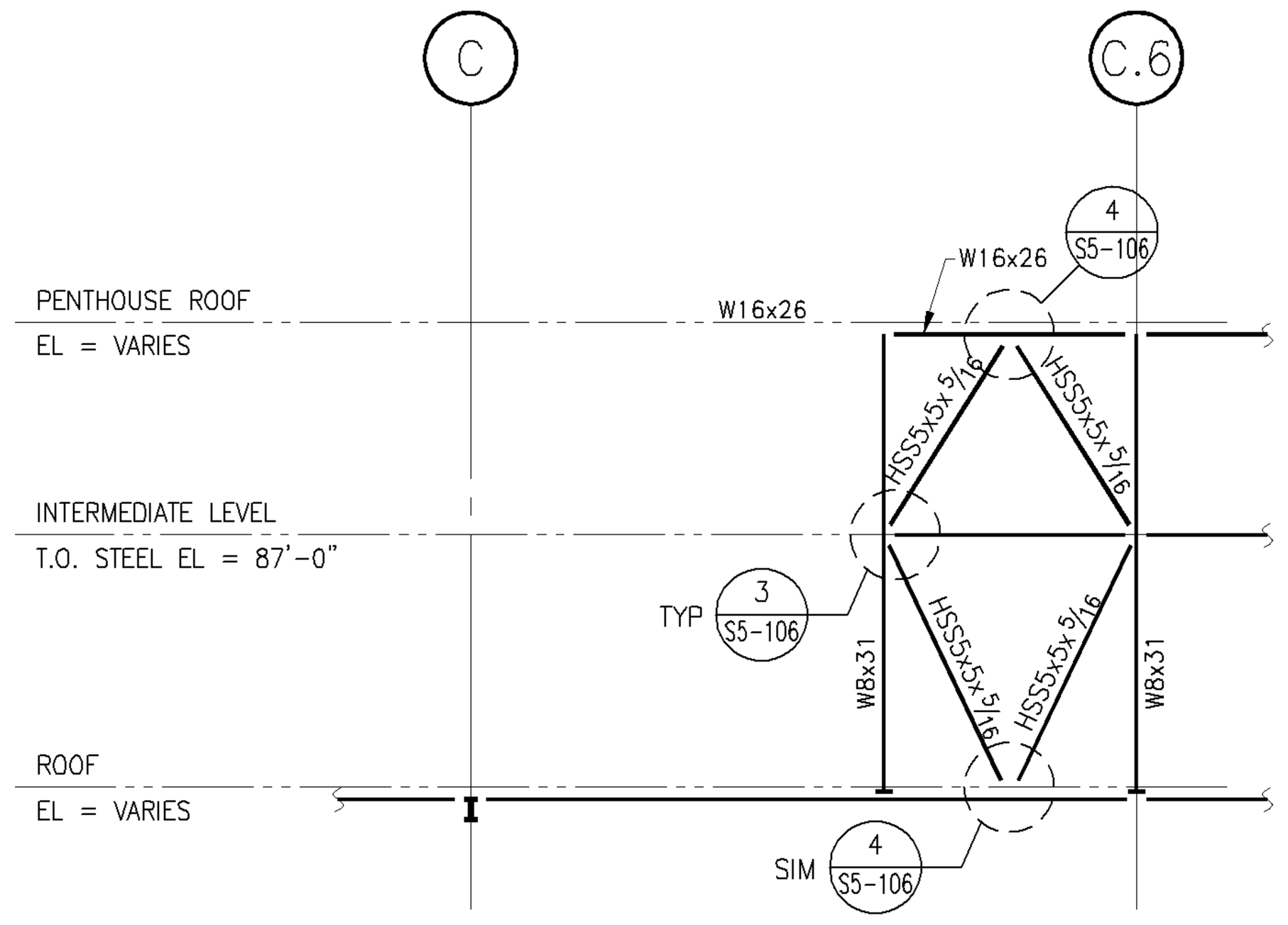




PENTHOUSE FRAME ELEVATION - GRID LINE 13.7 1/8"=1'-0" 10



PENTHOUSE FRAME ELEVATION - GRID LINE 23.3 1/8"=1'-0" 6

MOMENT FRAME ELEVATION - GRID LINE 24 1/8"=1'-0" 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

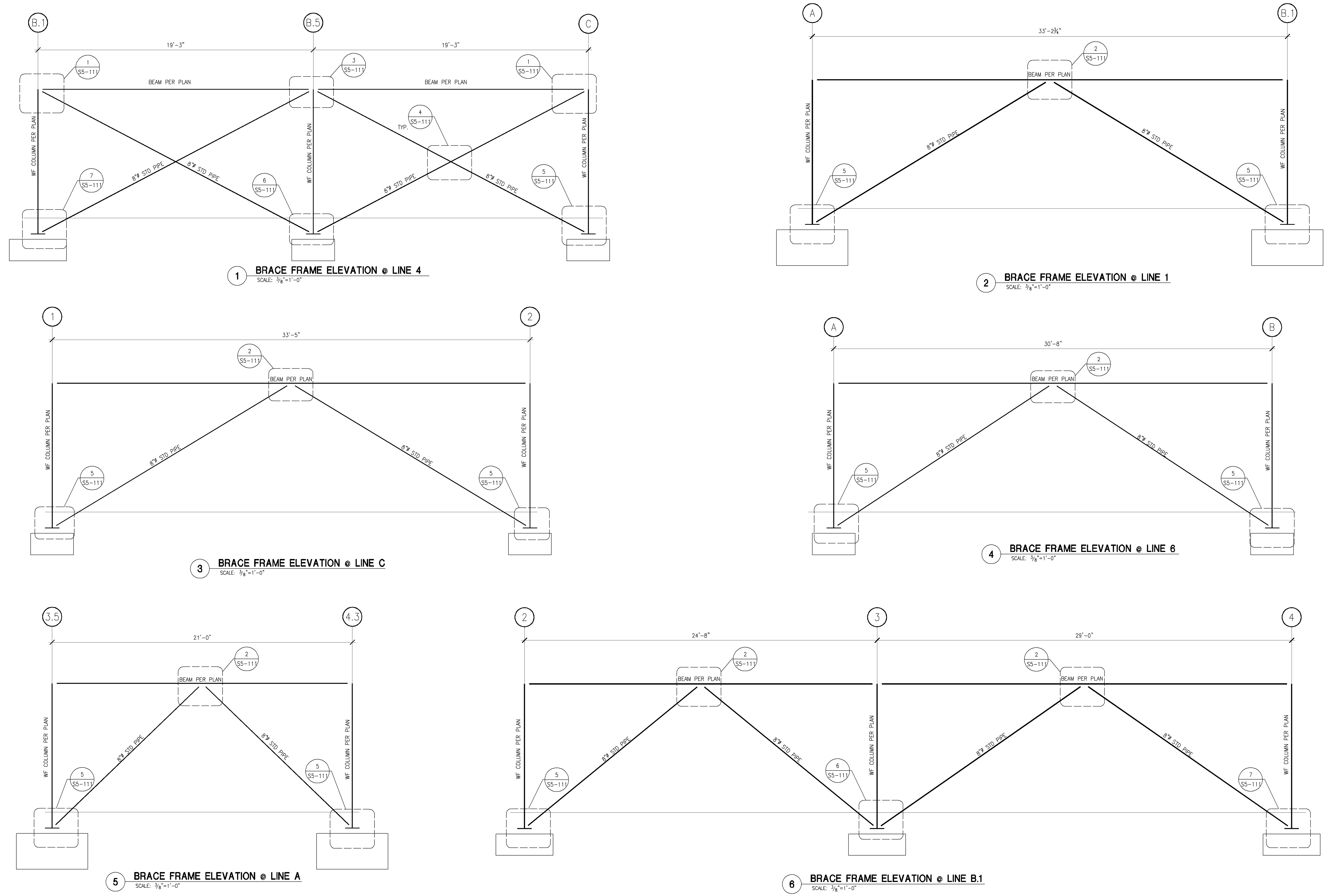
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

FRAME ELEVATIONS - BUILDING G

S3-112



OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CONSULTANTS

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CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
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2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
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CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
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REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
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2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

BRACED FRAME ELEVATIONS: CENTRAL PLANT

S3-121

File: W:\2006\1061030 - Guidant Corporate Campus, Temecula, CA\Sdraft\1061030_S3.121.dwg
Plotted: 3/21/07 at 1:11pm By: SGutierrez
XREFs: 1061030_bdr.dwg

COLUMN SCHEDULE

MARK / LEVEL	B-10	B-9	B-8	B-7	B-6	B-5	B-4	B-3	B-2	B-1	A.8-12	A.8-1	A.7-12	A.7-1	A-12	A-11	A-10	A-9	A-8	A-7	A-6.5	A-6	A-5	A-4	A-3	A-2	A-1
	NON-FRAME	NON-FRAME	FRAME	NON-FRAME	NON-FRAME	FRAME	NON-FRAME	NON-FRAME	NON-FRAME	FRAME	NON-FRAME	NON-FRAME	FRAME	FRAME	FRAME	NON-FRAME	FRAME	FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	FRAME	FRAME	NON-FRAME	FRAME
PARAPET																											
PENTHOUSE																											
ROOF																											
LEVEL 5	W14x90	W14x90	SEE FRAME ELEVATIONS	W14x61	W14x61	SEE FRAME ELEVATIONS	W14x90	W14x90	W14x90	SEE FRAME ELEVATIONS	W14x61 4'-6" TYP UNO	W14x61	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x61	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x61	W14x61	W14x61	W14x61	W14x61	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x61	SEE FRAME ELEVATIONS
LEVEL 4	W14x132	W14x132	SEE FRAME ELEVATIONS	W14x90	W14x90	SEE FRAME ELEVATIONS	W14x132	W14x132	W14x132	SEE FRAME ELEVATIONS	W14x61	W14x61	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x90	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x90	W14x90	W14x90	W14x90	W14x90	SEE FRAME ELEVATIONS 6/S5-104 TYP @ FRAME COL SPLICE	SEE FRAME ELEVATIONS	W14x90	SEE FRAME ELEVATIONS
LEVEL 3																										7/S5-104 TYP @ NON-FRAME COL SPLICE	
LEVEL 2																											
LEVEL 1	W14x257	W14x257	SEE FRAME ELEVATIONS	W14x120	W14x120	SEE FRAME ELEVATIONS	W14x257	W14x257	W14x176	SEE FRAME ELEVATIONS	W14x68	W14x68	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x120	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x120	W14x109	W14x109	W14x109	W14x120	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x120	SEE FRAME ELEVATIONS
COLUMN BASE PLATE: DETAIL	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	4/S5-104	SEE FRAME ELEVATIONS
COLUMN BASE PLATE: SIZE (WxL)	26"x26"	26"x26"		25"x25"	25"x25"		26"x26"	26"x26"	26"x26"		24"x24"	24"x24"				25"x25"			25"x25"	25"x25"	25"x25"	25"x25"	25"x25"			25"x25"	
COLUMN BASE PLATE: THICKNESS (GRADE)	3 1/4" 36 KSI	3 1/4" 36 KSI		2 1/2" 36 KSI	2 1/2" 36 KSI		3 1/4" 36 KSI	3 1/4" 36 KSI	3" 36 KSI		2" 36 KSI	2" 36 KSI				2 1/2" 36 KSI			2 1/2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2 1/2" 36 KSI			2 1/2" 36 KSI	

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
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ARCHITECT

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BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE I-7
TORRANCE, CA 90503

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER — No. S4399 — Exp. 12/31/07 — STRUCTURAL — STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

COLUMN SCHEDULE: BUILDING F

S3-201

COLUMN SCHEDULE

MARK / LEVEL			D-12	D-11	D-10	D-9	D-8	D-5	D-4	D-3	D-2	D-1	C.6-12	C.6-1
			NON-FRAME	NON-FRAME	FRAME	FRAME	NON-FRAME	NON-FRAME	FRAME	FRAME	NON-FRAME	NON-FRAME	FRAME	FRAME
PARAPET														
PENTHOUSE														
ROOF														
LEVEL 5			W14x61	W14x61	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x61	W14x61	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x61 4'-6" TYP UNO	W14x61	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
LEVEL 4			W14x90	W14x90	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x90	W14x90	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x90	W14x90	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
LEVEL 3														
LEVEL 2														
LEVEL 1			W14x109	W14x176	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x120	W14x120	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x176	W14x109	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
COLUMN BASE PLATE	DETAIL		4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
COLUMN BASE PLATE	SIZE (WxL)		25"x25"	26"x26"			25"x25"	25"x25"			26"x26"	25"x25"		
COLUMN BASE PLATE	THICKNESS (GRADE)		2" 36 KSI	3" 36 KSI			2 1/2" 36 KSI	2 1/2" 36 KSI			3" 36 KSI	2" 36 KSI		

MARK / LEVEL		C-12	C-11	C-10	C-9	C-8	C-6.5	C-5	C-4	C-3	C-2	C-1	B-12	B-11
		FRAME	NON-FRAME	NON-FRAME	NON-FRAME	FRAME	NON-FRAME	FRAME	NON-FRAME	NON-FRAME	NON-FRAME	FRAME	FRAME	NON-FRAME
PARAPET														
PENTHOUSE														
ROOF														
LEVEL 5		SEE FRAME ELEVATIONS	W14x90	W14x90	W14x90	SEE FRAME ELEVATIONS	W14x61	SEE FRAME ELEVATIONS	W14x90	W14x90	W14x90	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x90
LEVEL 4		SEE FRAME ELEVATIONS	W14x132	W14x132	W14x132	SEE FRAME ELEVATIONS	W14x90	SEE FRAME ELEVATIONS	W14x132	W14x132	W14x132	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x132
LEVEL 3											7/S5-104 TYP NON FRAME COL SPLICE		6/S5-104 TYP FRAME COL SPLICE	
LEVEL 2														
LEVEL 1		SEE FRAME ELEVATIONS	W14x176	W14x257	W14x257	SEE FRAME ELEVATIONS	W14x120	SEE FRAME ELEVATIONS	W14x257	W14x257	W14x176	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x176
COLUMN BASE PLATE	DETAIL	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	4/S5-104	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	4/S5-104
COLUMN BASE PLATE	SIZE (WxL)		26"x26"	26"x26"	26"x26"		25"x25"		26"x26"	26"x26"	26"x26"			26"x26"
COLUMN BASE PLATE	THICKNESS (GRADE)		3" 36 KSI	3 1/4" 36 KSI	3 1/4" 36 KSI		2 1/2" 36 KSI		3 1/4" 36 KSI	3 1/4" 36 KSI	3" 36 KSI			3" 36 KSI

OWNER/CLIENT

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ARCHITECT

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ESCONDIDO, CA 92029

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PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE I-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

COLUMN SCHEDULE: BUILDING F

S3-202

COLUMN SCHEDULE

MARK / LEVEL	B-23	B-22	B-21	B-20	B-19	B-18	B-17	B-16	B-15	B-14	B-13.7	B-13	A.7-24	A.7-13
	NON-FRAME	NON-FRAME	NON-FRAME	FRAME	FRAME	FRAME	FRAME	NON-FRAME	NON-FRAME	NON-FRAME	FRAME	FRAME	FRAME	FRAME
PARAPET														
PENTHOUSE														
ROOF	W14x61	W14x90	W14x90	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x90	W14x90	W14x61	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
LEVEL 5	W14x90	W14x90	W14x90	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x90	W14x90	W14x90	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
LEVEL 4														
LEVEL 3	W14x120	W14x176	W14x176	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x176	W14x176	W14x120	SEE FRAME ELEVATIONS; 6/S5-104 TYP FRAME COL SPLICE	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
LEVEL 2														
LEVEL 1														
COLUMN BASE PLATE – DETAIL	4/S5-104	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
COLUMN BASE PLATE – SIZE (WxL)	25"x25"	26"x26"	26"x26"					26"x26"	26"x26"	25"x25"				
COLUMN BASE PLATE – THICKNESS (GRADE)	2 1/2" 36 KSI	3" 36 KSI	3" 36 KSI					3" 36 KSI	3" 36 KSI	2 1/2" 36 KSI				

MARK / LEVEL	A-24	A-23	A-22	A-21	A-20	A-19	A-18.5	A-18	A-17	A-16	A-15	A-14	A-13
	FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	FRAME
PARAPET													
PENTHOUSE													
ROOF	SEE FRAME ELEVATIONS	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61; 4'-6" TYP UNO	SEE FRAME ELEVATIONS
LEVEL 5	SEE FRAME ELEVATIONS	W14x90	W14x90	W14x90	W14x61	W14x61	W14x61	W14x61	W14x61	W14x90	W14x90	W14x90	SEE FRAME ELEVATIONS
LEVEL 4													
LEVEL 3	SEE FRAME ELEVATIONS	W14x109	W14x109	W14x109	W14x74	W14x74	W14x74	W14x74	W14x74	W14x109	W14x109	W14x109; 7/S5-104 TYP NON-FRAME COL SPLICE	SEE FRAME ELEVATIONS
LEVEL 2													
LEVEL 1													
COLUMN BASE PLATE – DETAIL	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS
COLUMN BASE PLATE – SIZE (WxL)		25"x25"	25"x25"	25"x25"	25"x25"	25"x25"	25"x25"	25"x25"	25"x25"	25"x25"	25"x25"	25"x25"	
COLUMN BASE PLATE – THICKNESS (GRADE)		2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

COLUMN SCHEDULE: BUILDING G

S3-211

COLUMN SCHEDULE

MARK / LEVEL	D-24	D-23	D-22	D-21	D-20	D-17	D-16	D-15	D-14	D-13
	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME
PARAPET										
PENTHOUSE										
ROOF										
LEVEL 5	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61	W14x61
LEVEL 4	W14x90	W14x90	W14x90	W14x90	W14x90	W14x90	W14x90	W14x90	W14x90	W14x90
LEVEL 3										
LEVEL 2	W14x109	W14x176	W14x176	W14x176	W14x120	W14x120	W14x176	W14x176	W14x176	W14x109
LEVEL 1										
COLUMN BASE PLATE: DETAIL	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104
COLUMN BASE PLATE: SIZE (WxL)	25"x25"	26"x26"	26"x26"	26"x26"	25"x25"	25"x25"	26"x26"	26"x26"	26"x26"	25"x25"
COLUMN BASE PLATE: THICKNESS (GRADE)	2" 36 KSI	3" 36 KSI	3" 36 KSI	3" 36 KSI	2 1/2" 36 KSI	2 1/2" 36 KSI	3" 36 KSI	3" 36 KSI	3" 36 KSI	2" 36 KSI

MARK / LEVEL	C.6-24	C.6-13	C-24	C-23	C-22	C-21	C-20	C-19.1	C-17.9	C-17	C-16	C-15	C-14	C-13	B-24	B-23.3
	FRAME	FRAME	FRAME	NON-FRAME	NON-FRAME	NON-FRAME	FRAME	FRAME	FRAME	FRAME	NON-FRAME	NON-FRAME	NON-FRAME	FRAME	FRAME	FRAME
PARAPET																
PENTHOUSE																
ROOF																
LEVEL 5	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x90	W14x90	W14x90	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x90	W14x90 (4'-6" TYP UNO)	W14x90	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
LEVEL 4	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x132	W14x132	W14x132	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x132	W14x132	W14x132	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
LEVEL 3													7/S5-104 TYP @ NON FRAME COL SPLICE			6/S5-104 TYP @ FRAME COL SPLICE
LEVEL 2	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x176	W14x176	W14x176	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	W14x176	W14x176	W14x176	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
LEVEL 1																
COLUMN BASE PLATE: DETAIL	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	4/S5-104	4/S5-104	4/S5-104	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS	SEE FRAME ELEVATIONS
COLUMN BASE PLATE: SIZE (WxL)				26"x26"	26"x26"	26"x26"					26"x26"	26"x26"	26"x26"			
COLUMN BASE PLATE: THICKNESS (GRADE)				3" 36 KSI	3" 36 KSI	3" 36 KSI					3" 36 KSI	3" 36 KSI	3" 36 KSI			

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

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CIVIL ENGINEER
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REGISTRATION

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1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

COLUMN SCHEDULE: BUILDING G

S3-212

COLUMN SCHEDULE

MARK / LEVEL	C.6-12.5	C.4-12.5	D-12.8	D-12.2	C.3-12.8	C.3-12.2	B.5-12.8	B.5-12.6	B.5-12.4	B.5-12.2
	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME	NON-FRAME
PARAPET										
PENTHOUSE										
ROOF										
LEVEL 5										
LEVEL 4			W12x120	W12x120	W12x120	W12x120	W12x120	W12x120	W12x120	W12x120
LEVEL 3								4'-6" TYP UNO		
LEVEL 2			W12x136	W12x136 (7/S5-104 TYP @ NON FRAME COL SPLICE)	W12x136	W12x136	W12x136	W12x136	W12x136	W12x136
LEVEL 1	W12x72	W12x72								
COLUMN BASE PLATE: DETAIL	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104	4/S5-104
COLUMN BASE PLATE: SIZE (WxL)	24"x24"	24"x24"	24"x24"	24"x24"	24"x24"	24"x24"	24"x24"	24"x24"	24"x24"	24"x24"
COLUMN BASE PLATE: THICKNESS (GRADE)	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI	2" 36 KSI

OWNER/CLIENT

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ARCHITECT

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515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER
ALDRIN J. ORUE
No. S4399
Exp. 12/31/07
STRUCTURAL
STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

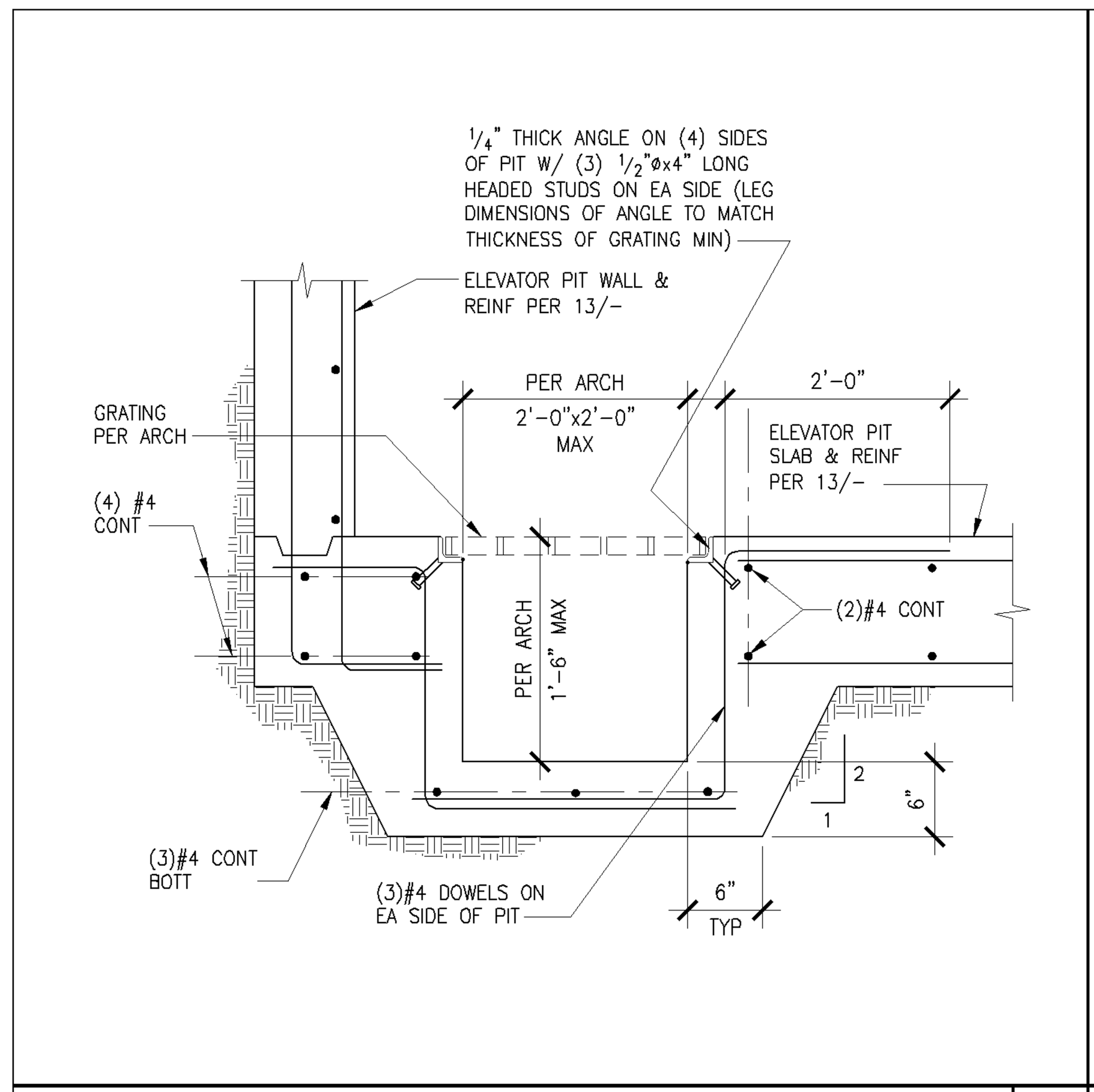
KEY PLAN

SHEET TITLE

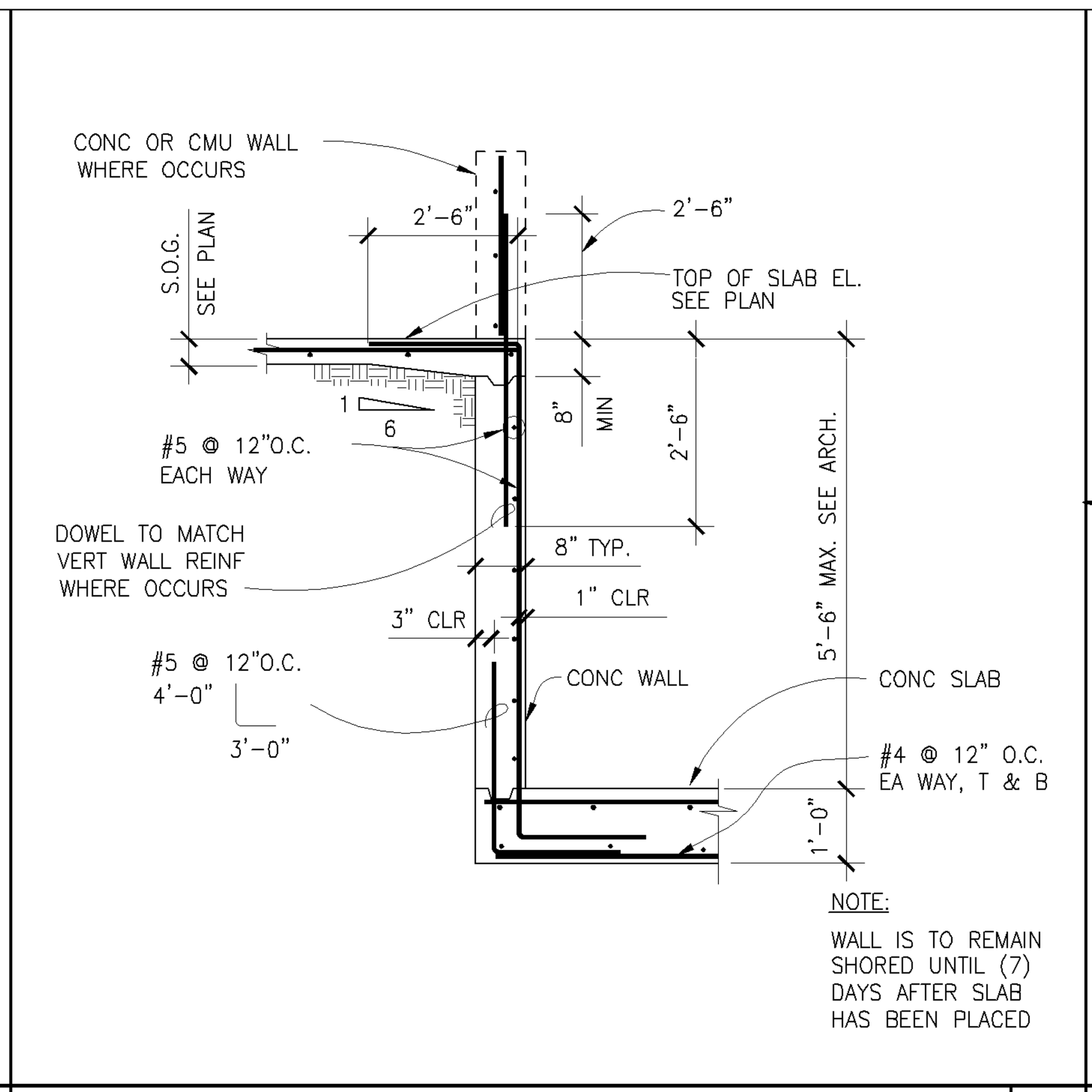
COLUMN SCHEDULE: LOBBY

S3-213

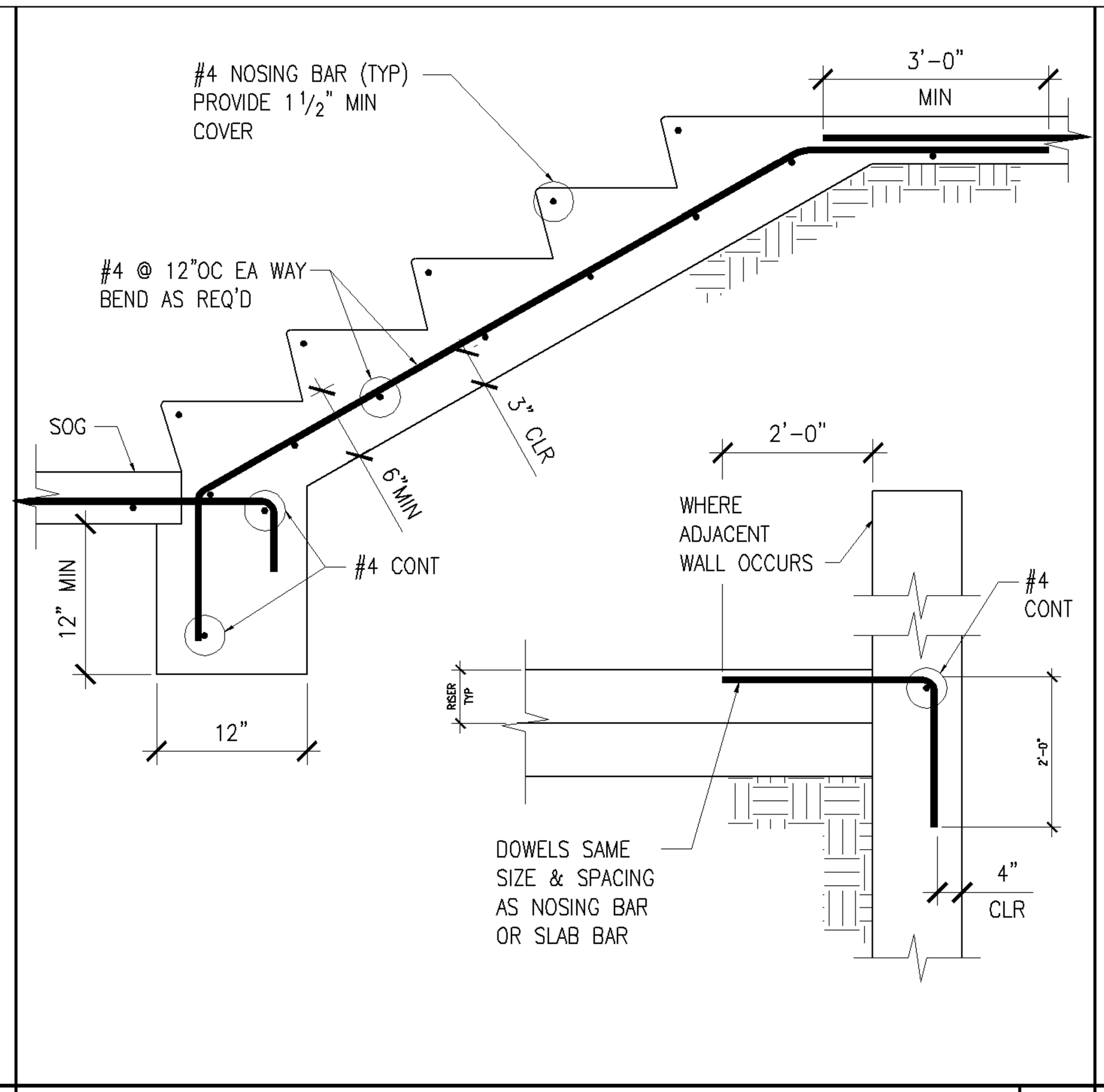
File: W:\2006\1061030 - Guidant Corporate Campus, Temecula, CA\Sdraft\1061030_S3.213.dwg
Plotted: 3/21/07 at 1:12pm By: SGutierrez
XREFs: 1061030_bdr.dwg



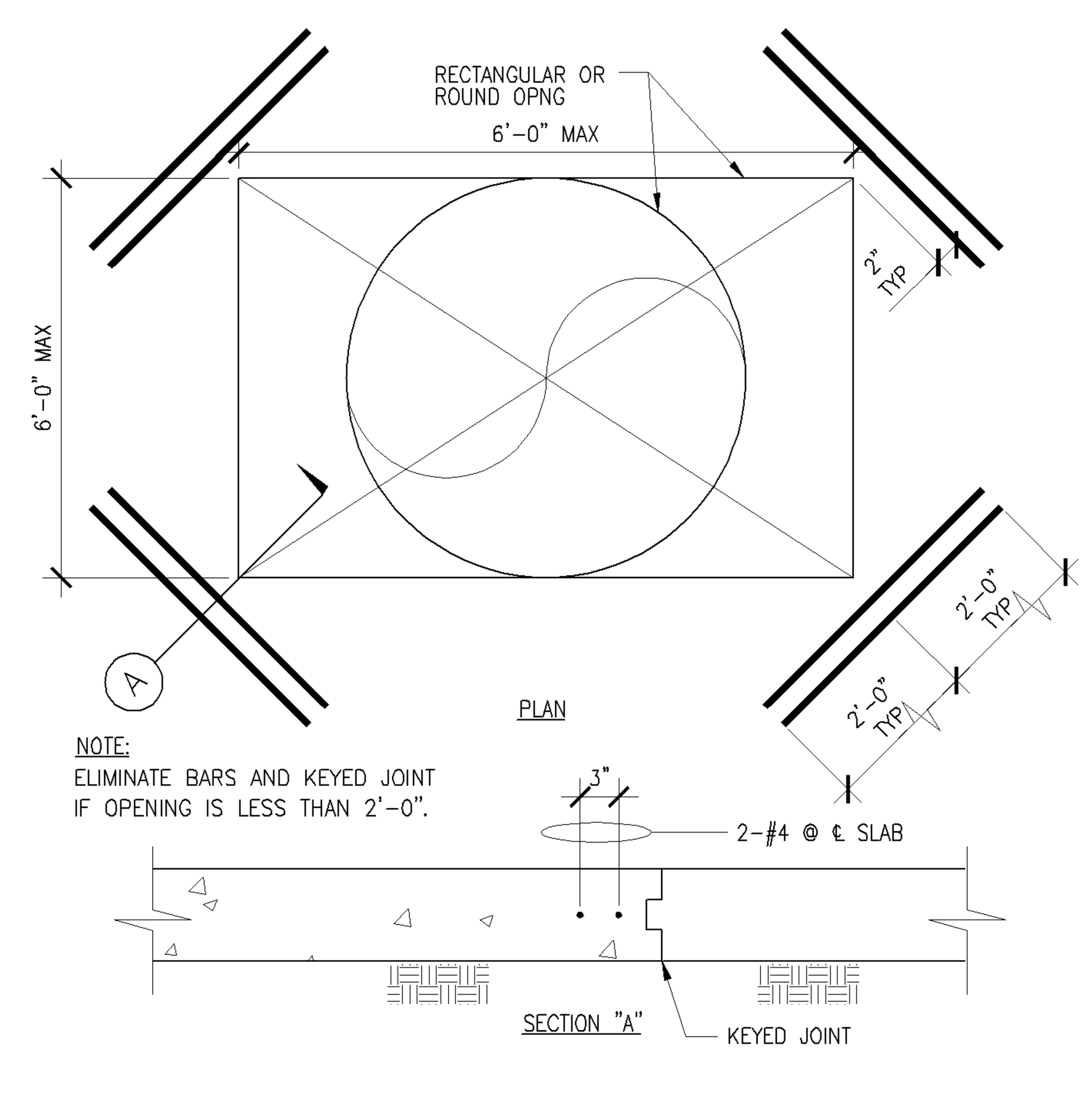
ELEVATOR PIT DETAIL AT SUMP 1"=1'-0" 17



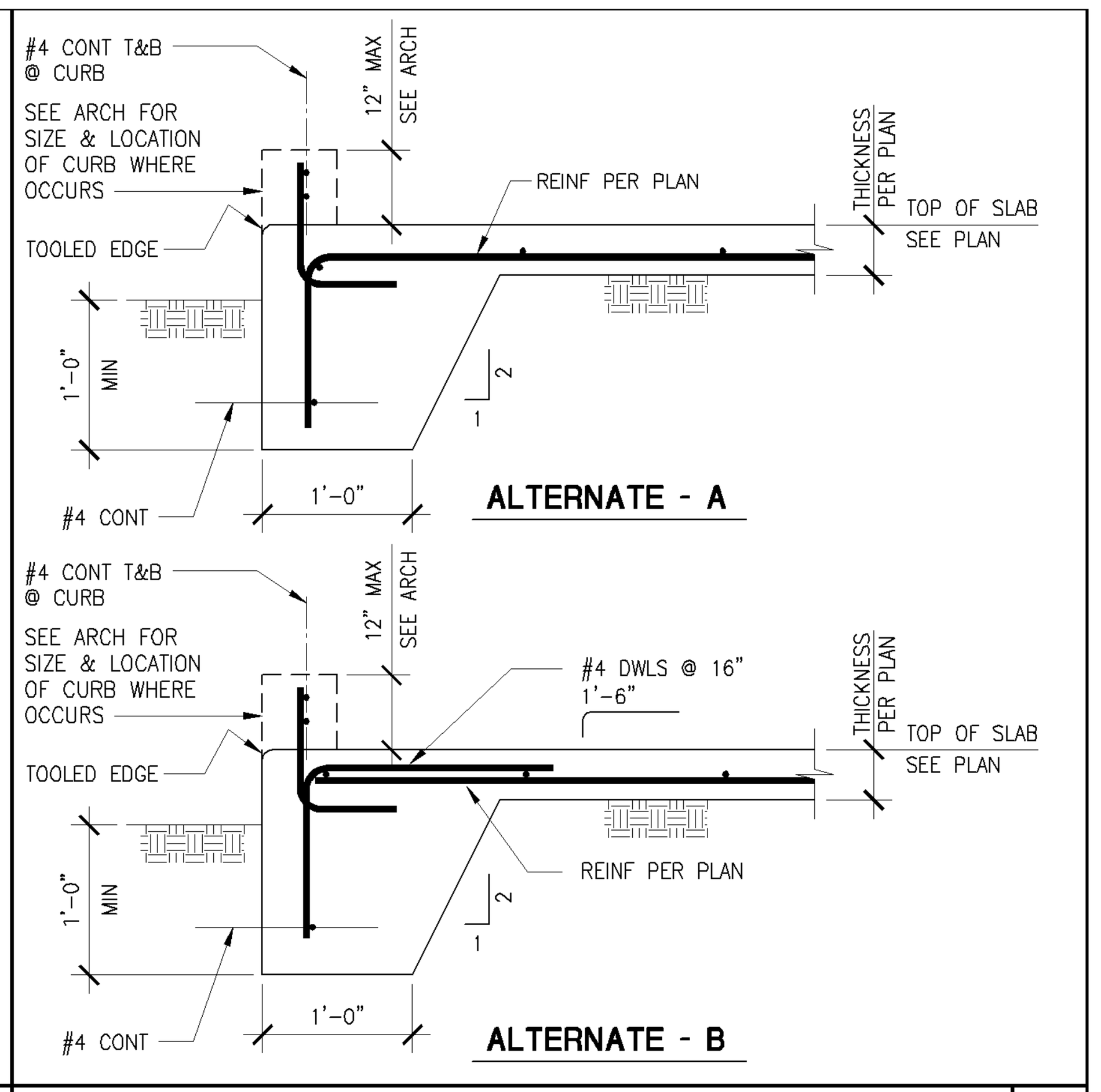
ELEVATOR PIT DETAIL 1/2"=1'-0" 13



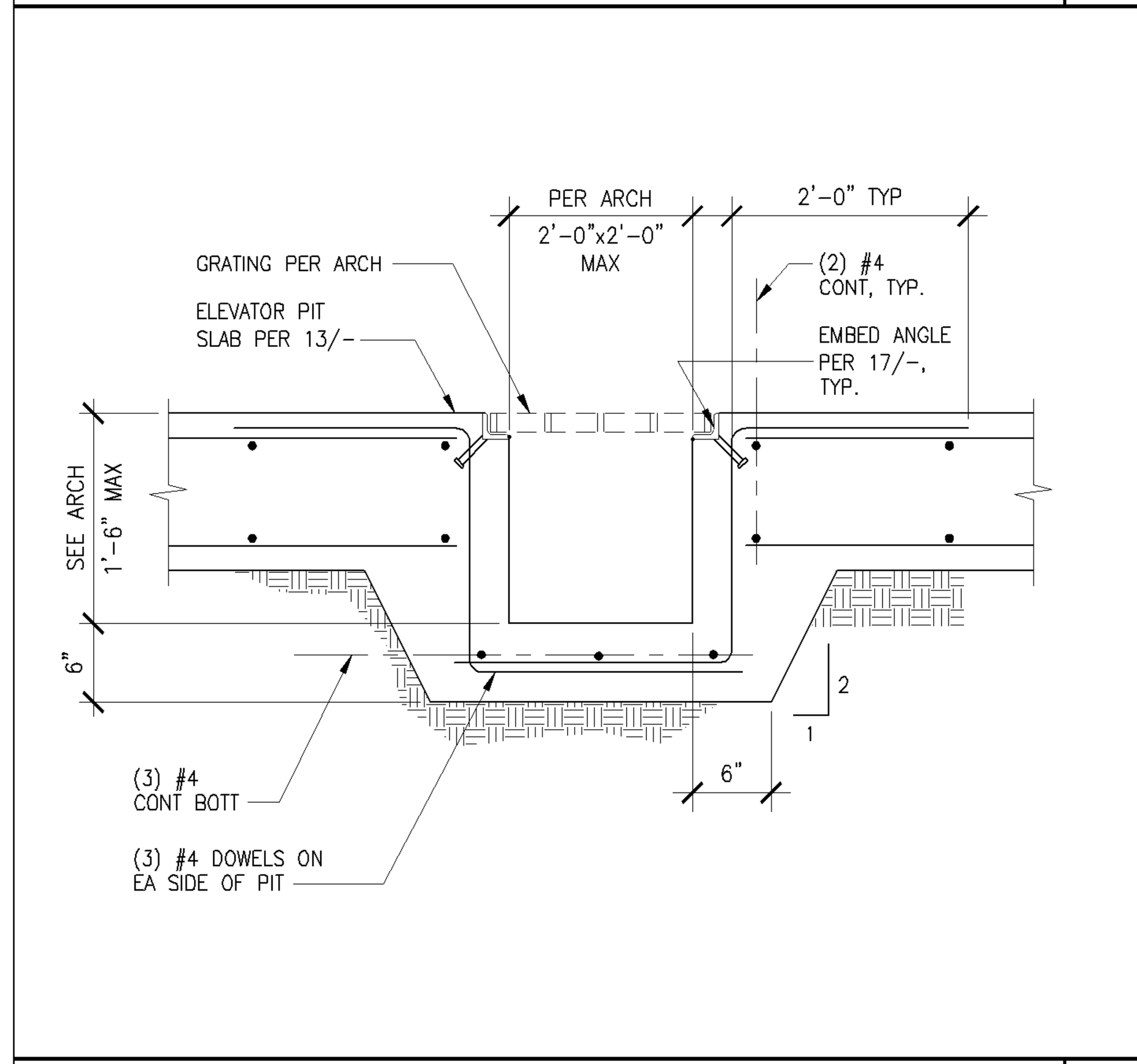
CONCRETE STAIRS ON GRADE DETAIL 1/2"=1'-0" 9



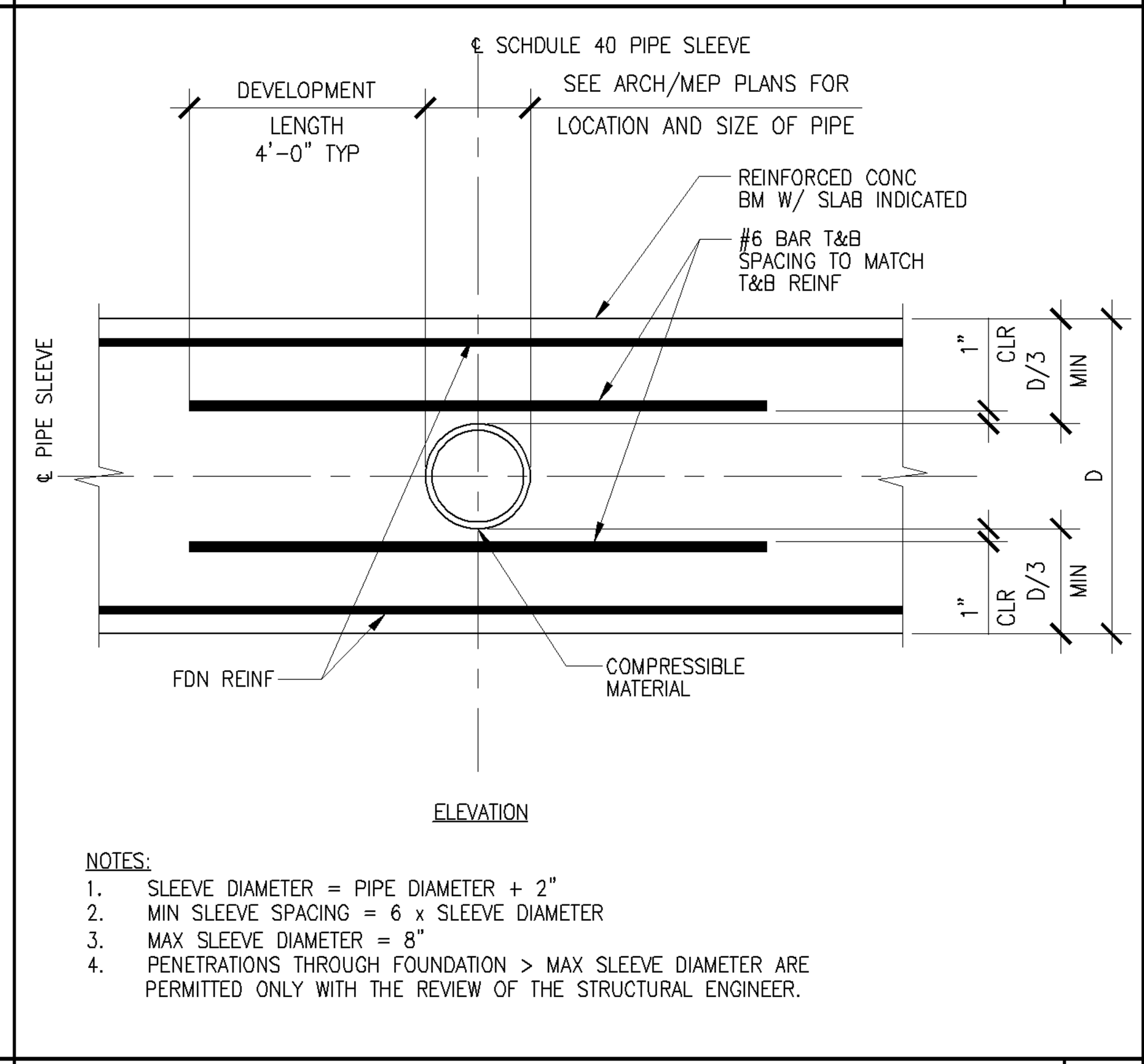
OPENING IN SLAB ON GRADE 1"=1'-0" 5



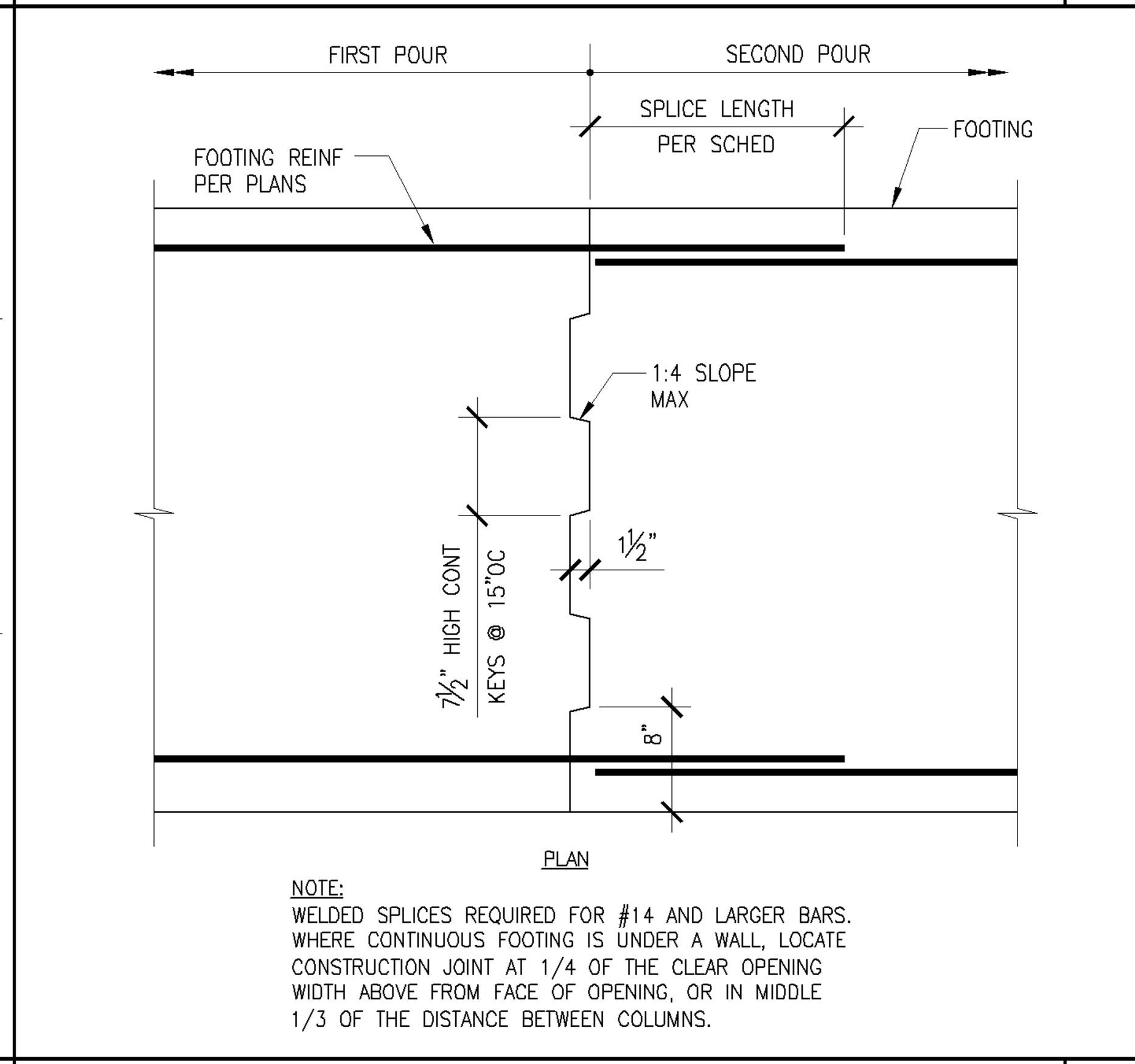
EDGE OF SLAB ON GRADE 1"=1'-0" 1



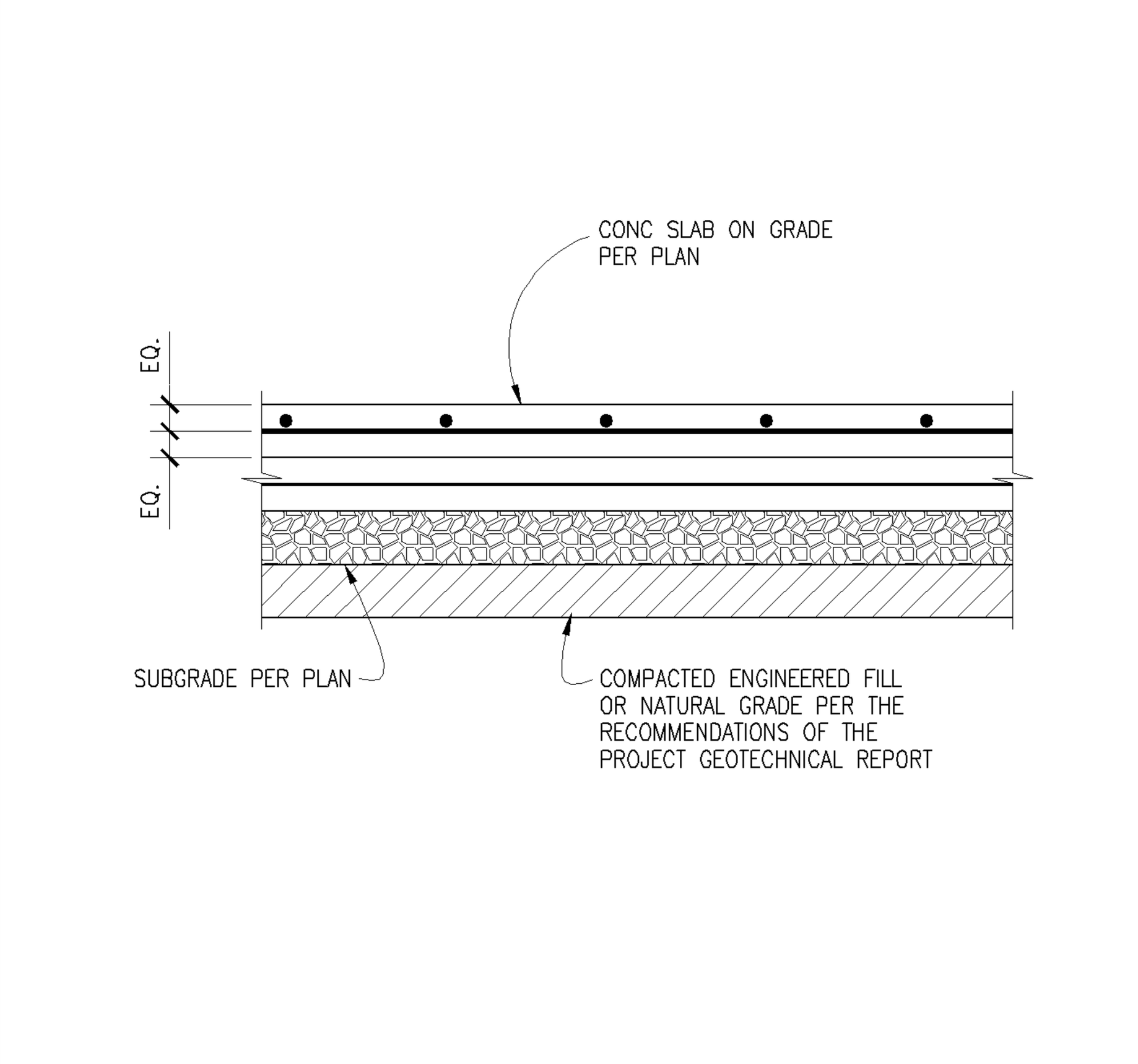
SUMP PIT DETAIL 1"=1'-0" 18



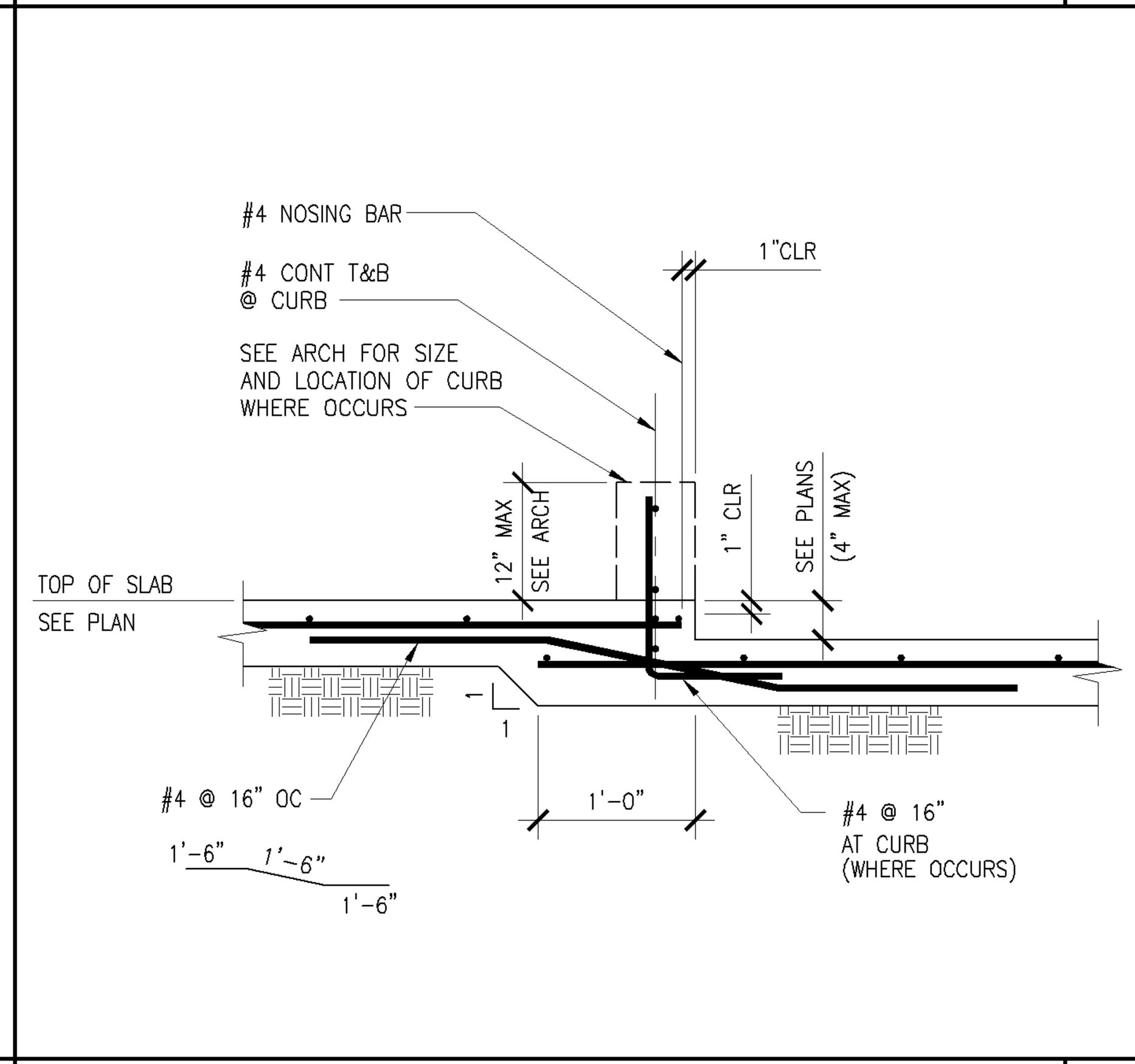
PIPE SLEEVE THRU CONCRETE FOUNDATION 1"=1'-0" 14



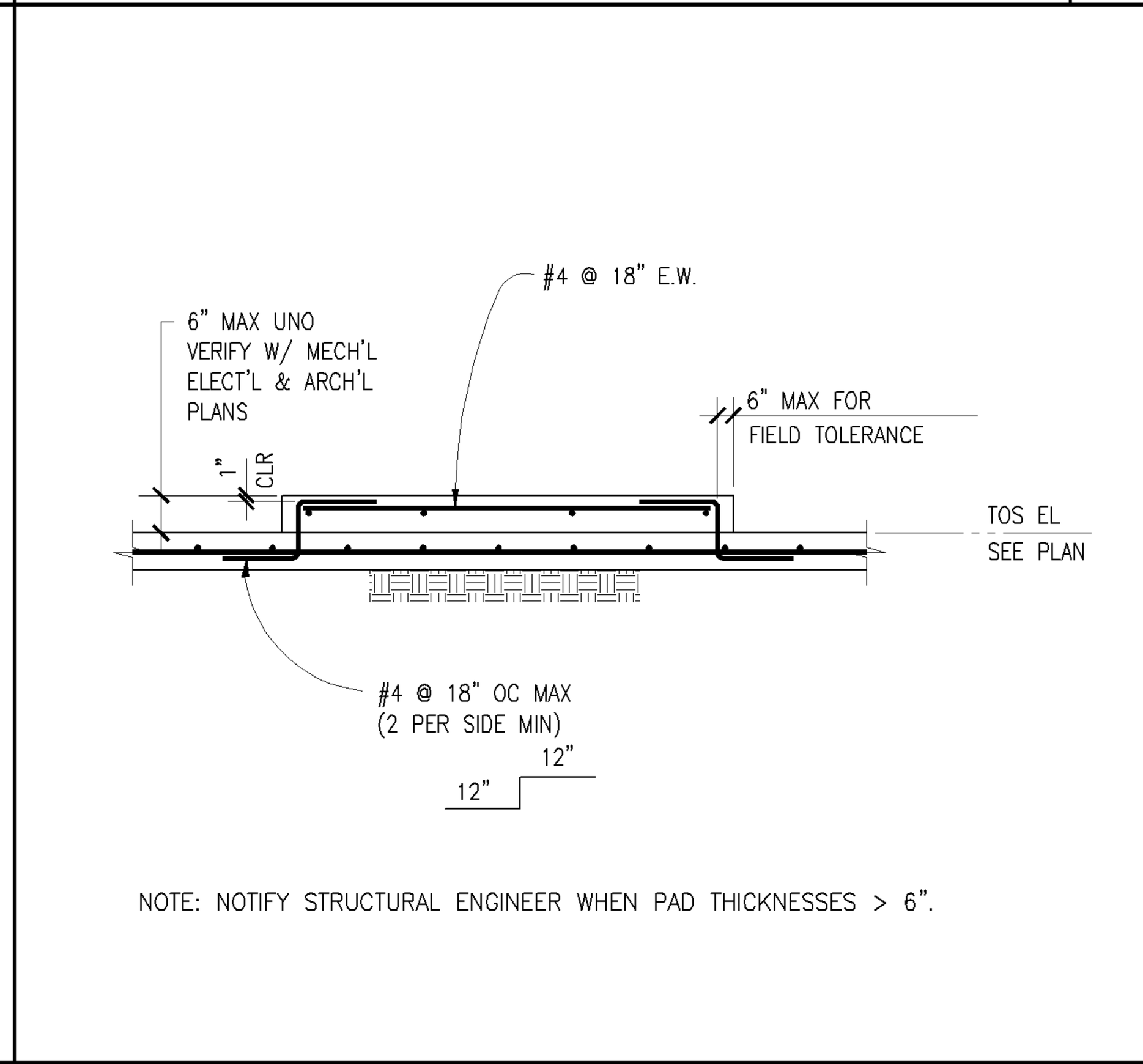
FOOTING CONSTRUCTION JOINT 1"=1'-0" 10



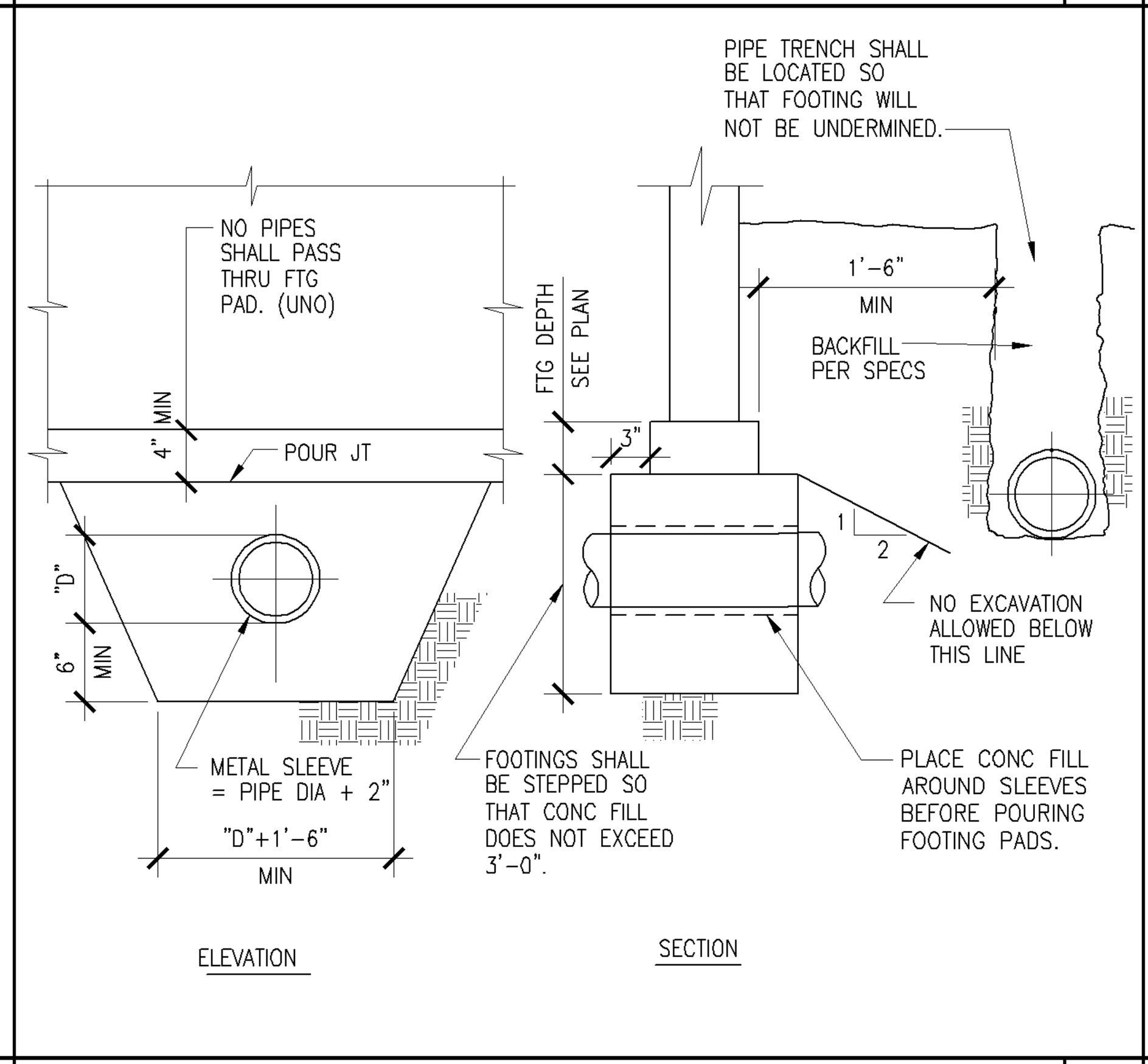
TYPICAL INTERIOR SLAB ON GRADE 1"=1'-0" 6



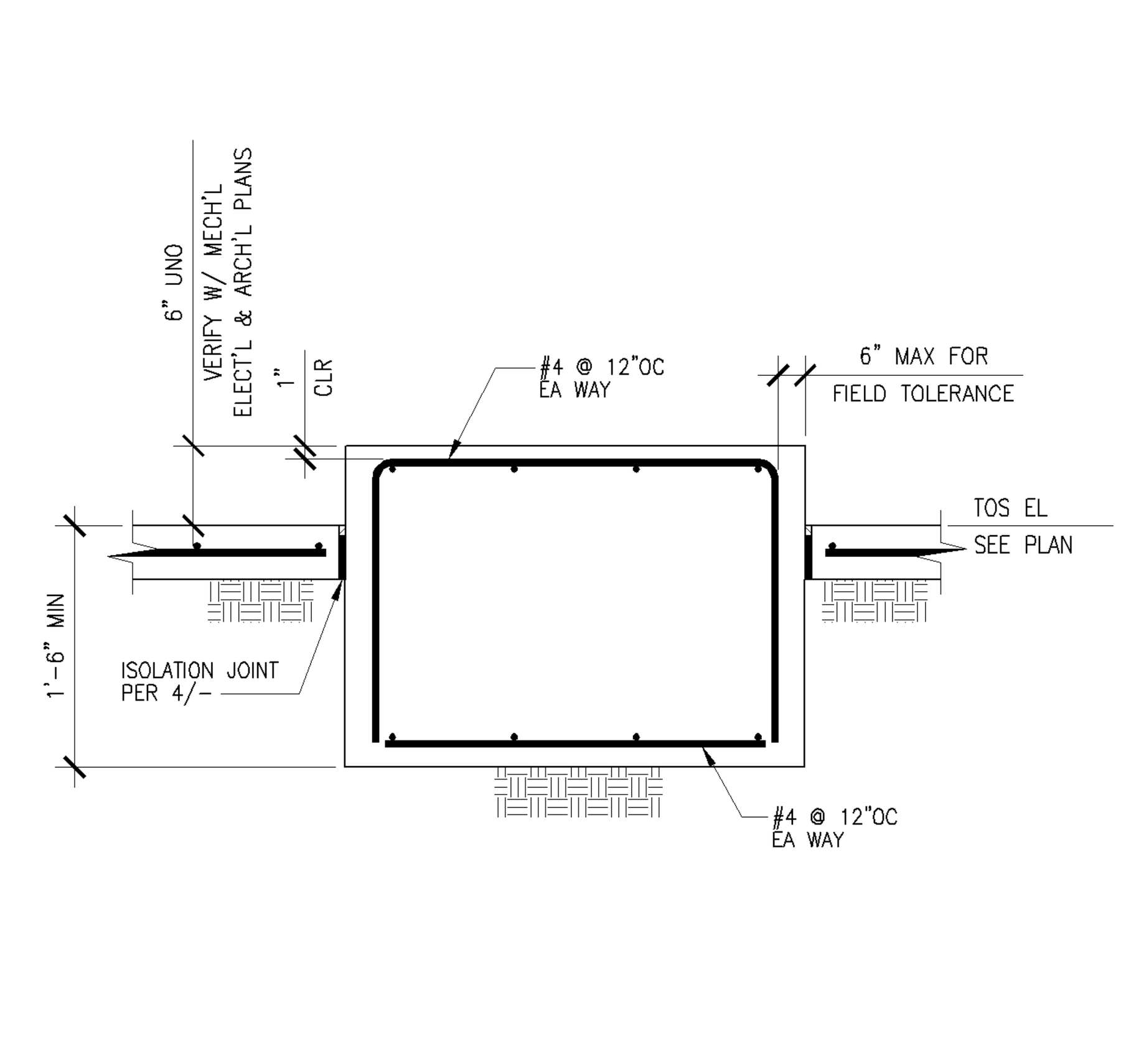
DEPRESSION IN SLAB ON GRADE 1"=1'-0" 2



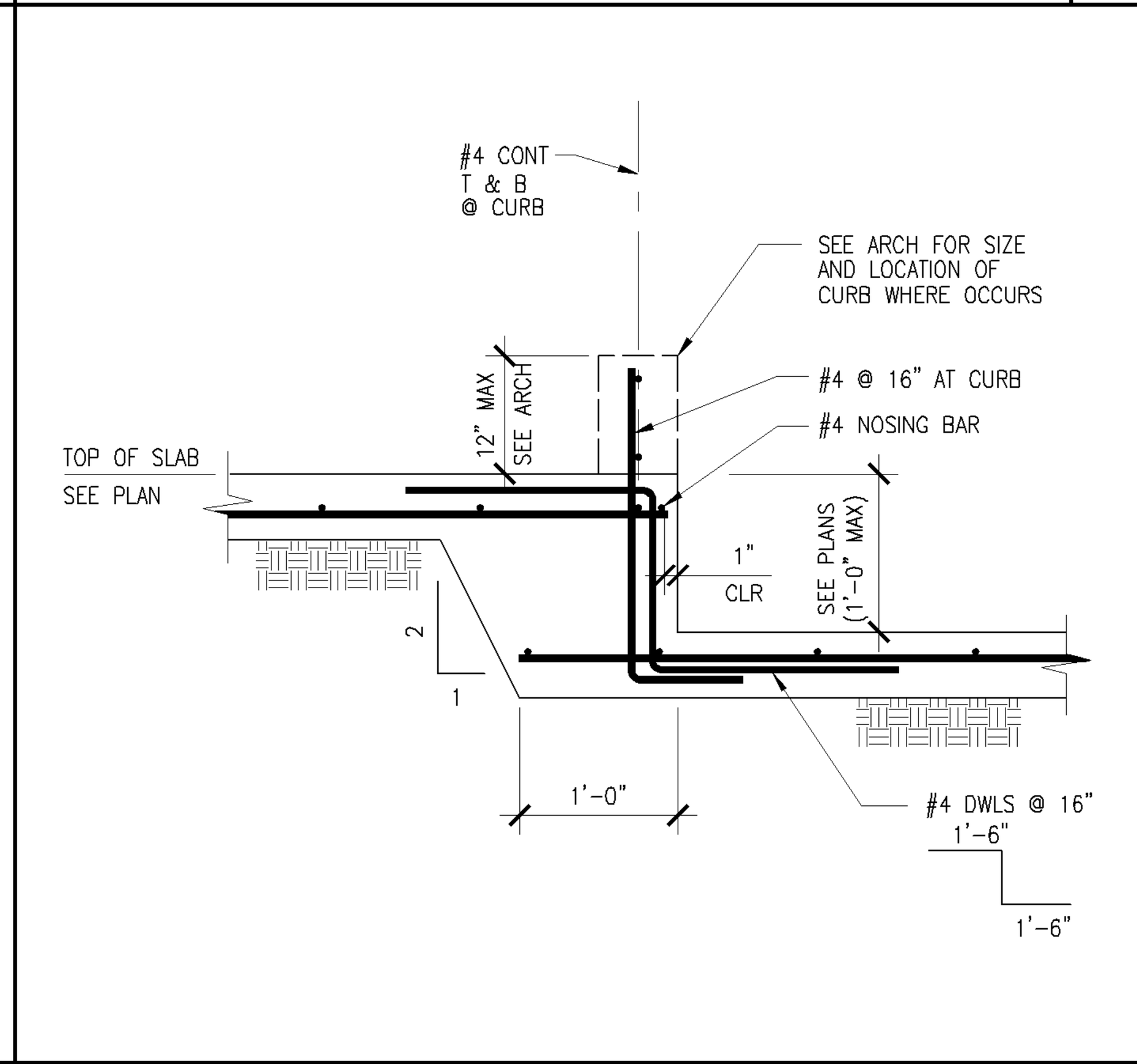
MECHANICAL PAD ON GRADE 1"=1'-0" 15



PIPE SLEEVE AND TRENCH FOOTING DETAIL NTS 11

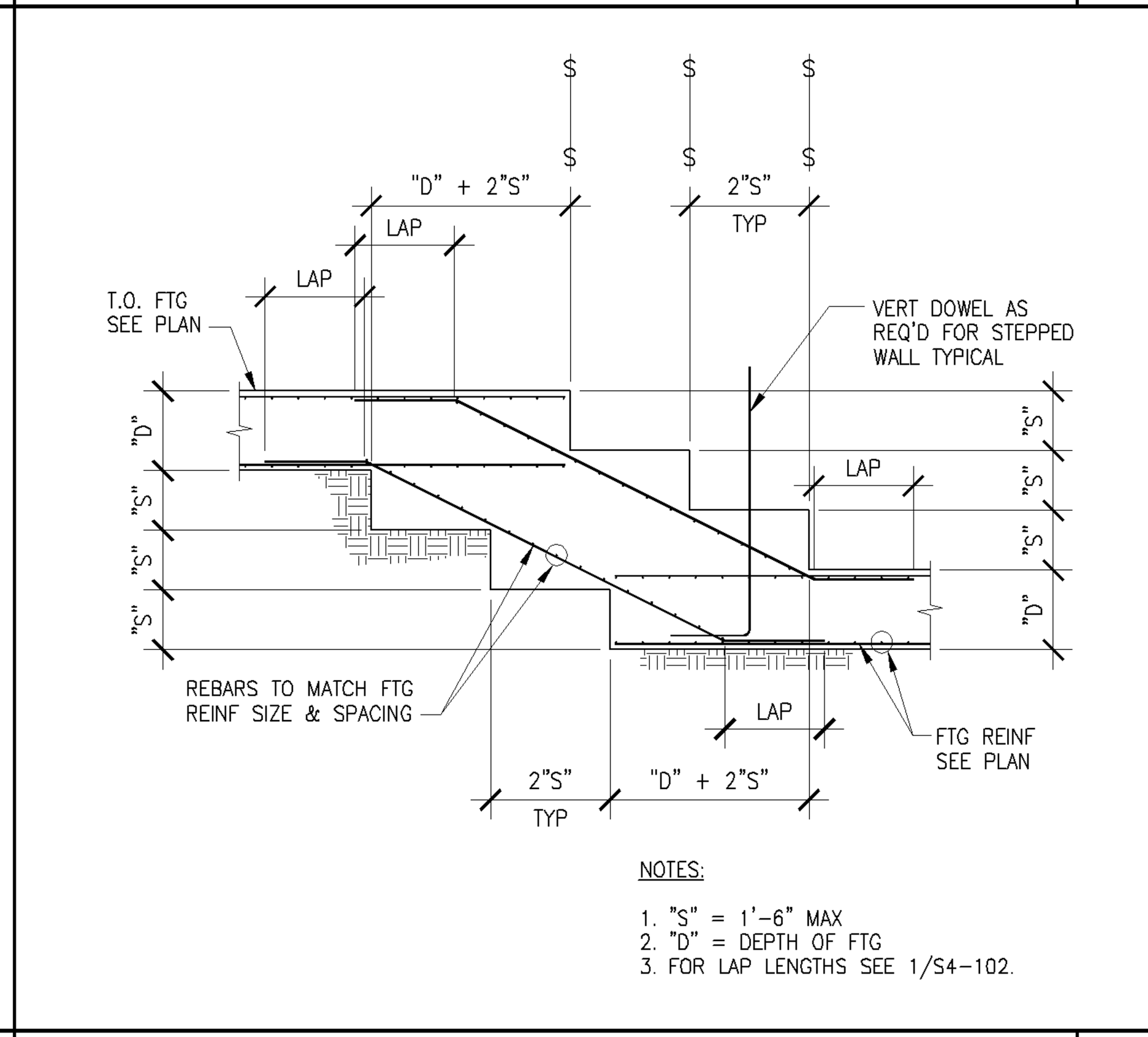


ISOLATED MECHANICAL PAD ON GRADE NTS 7

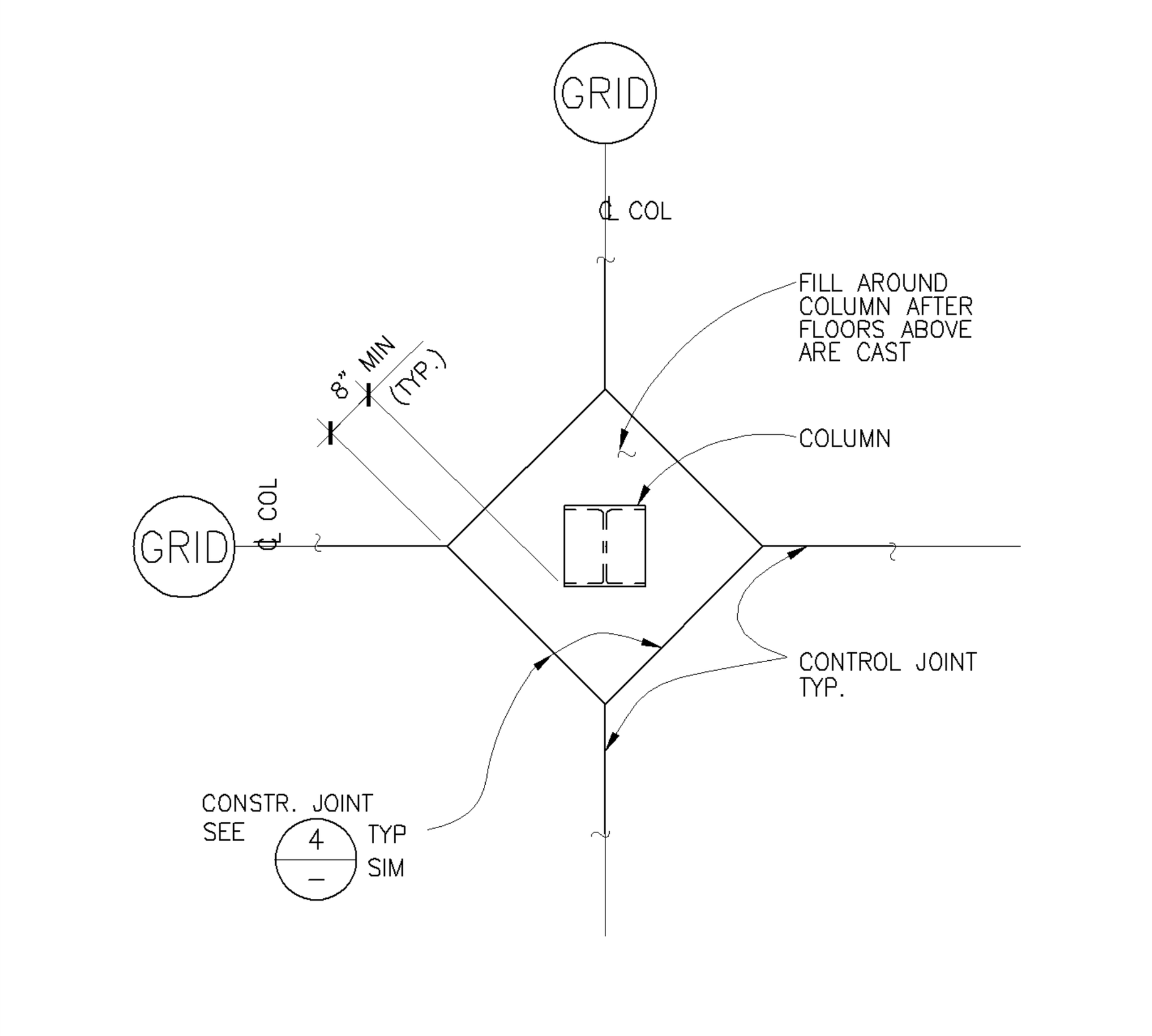


STEP IN SLAB ON GRADE 1"=1'-0" 3

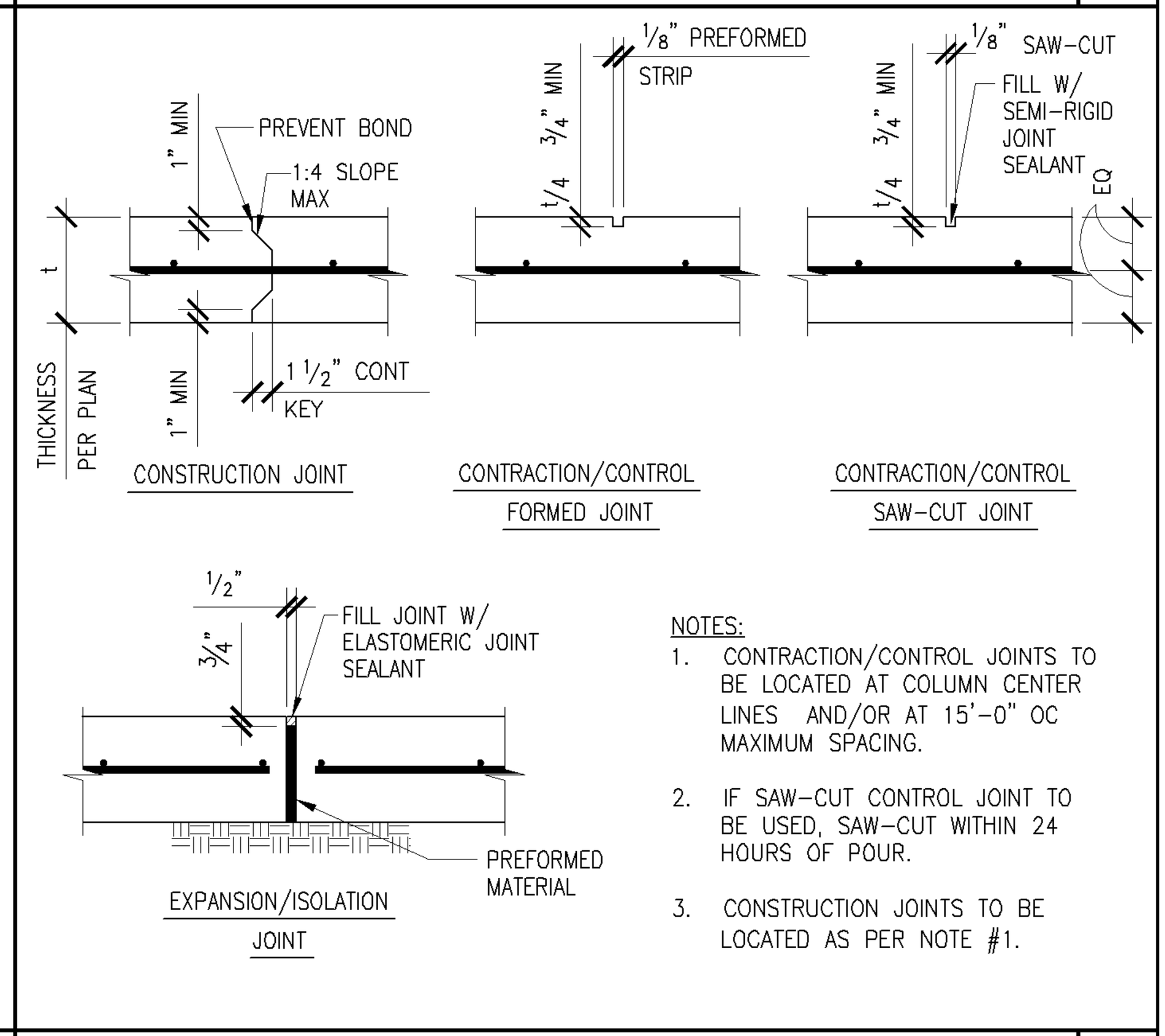
NOT USED 16



STEPPED FOOTING DETAIL 1/4"=1'-0" 12



ISOLATION AT COLUMN AND S.O.G. 1"=1'-0" 8



INTERIOR SLAB ON GRADE JOINTS NTS 4

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

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515 SOUTH FLOWER ST., 8TH FLOOR
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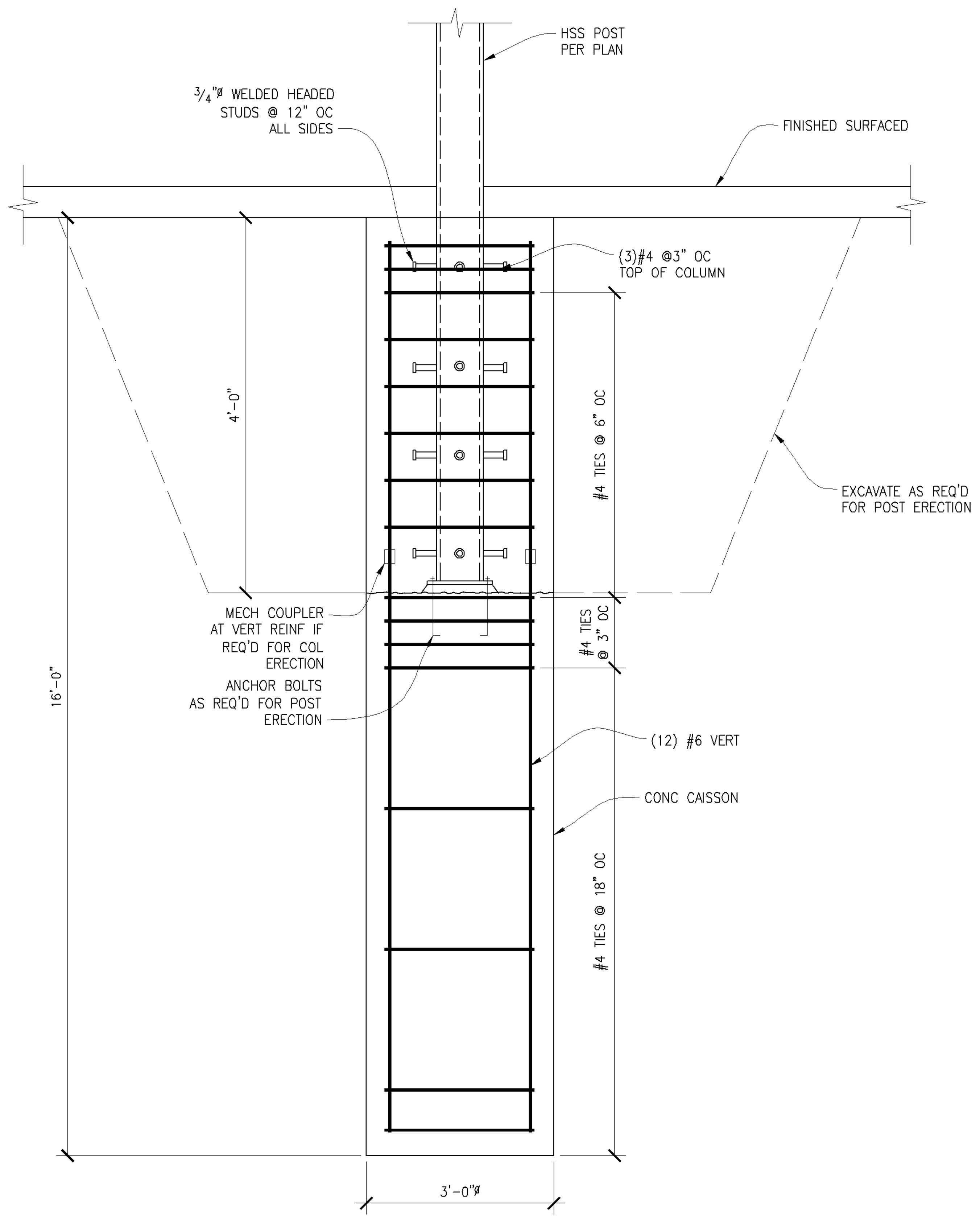
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

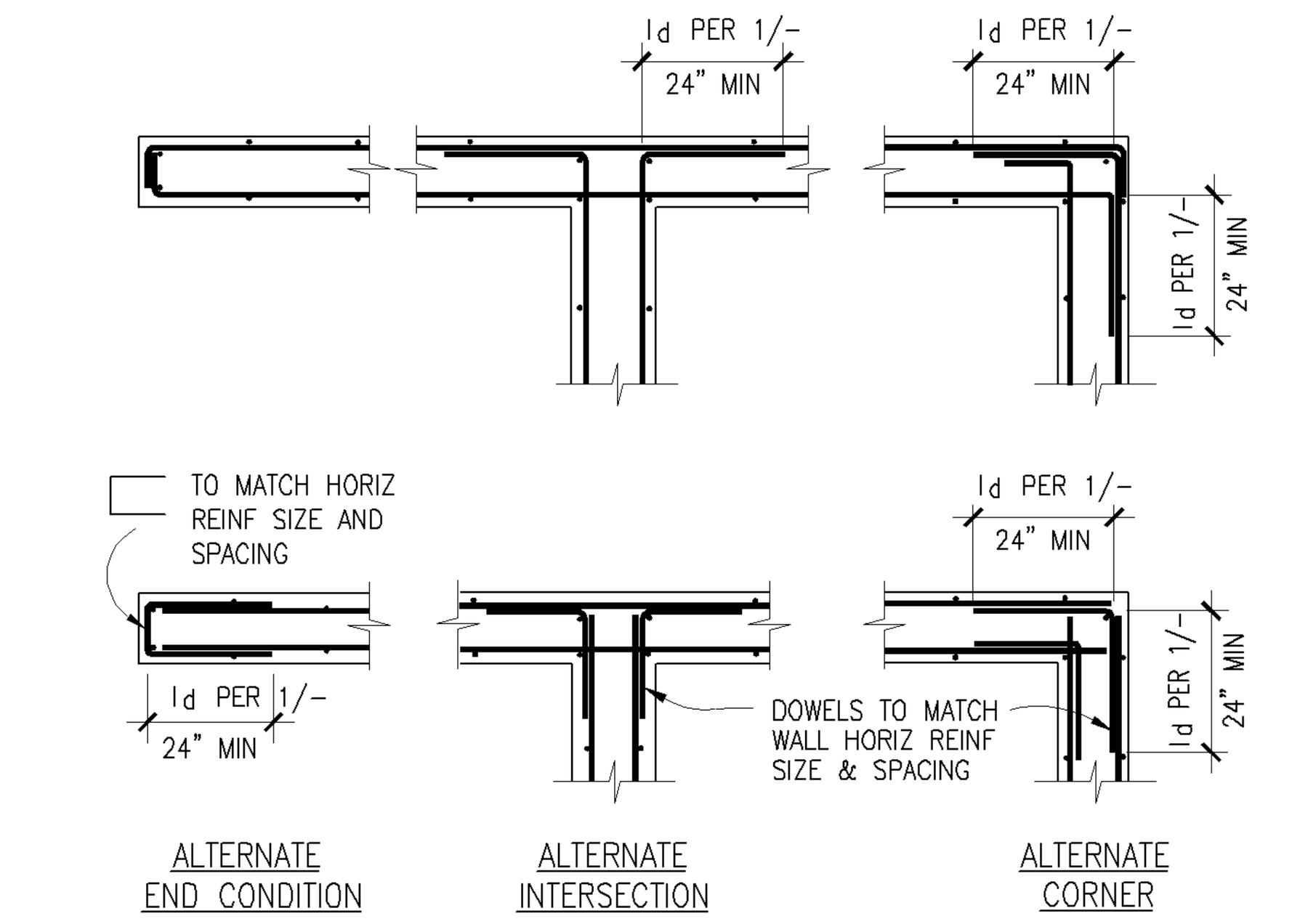
TYPICAL CONCRETE DETAILS

S4-101

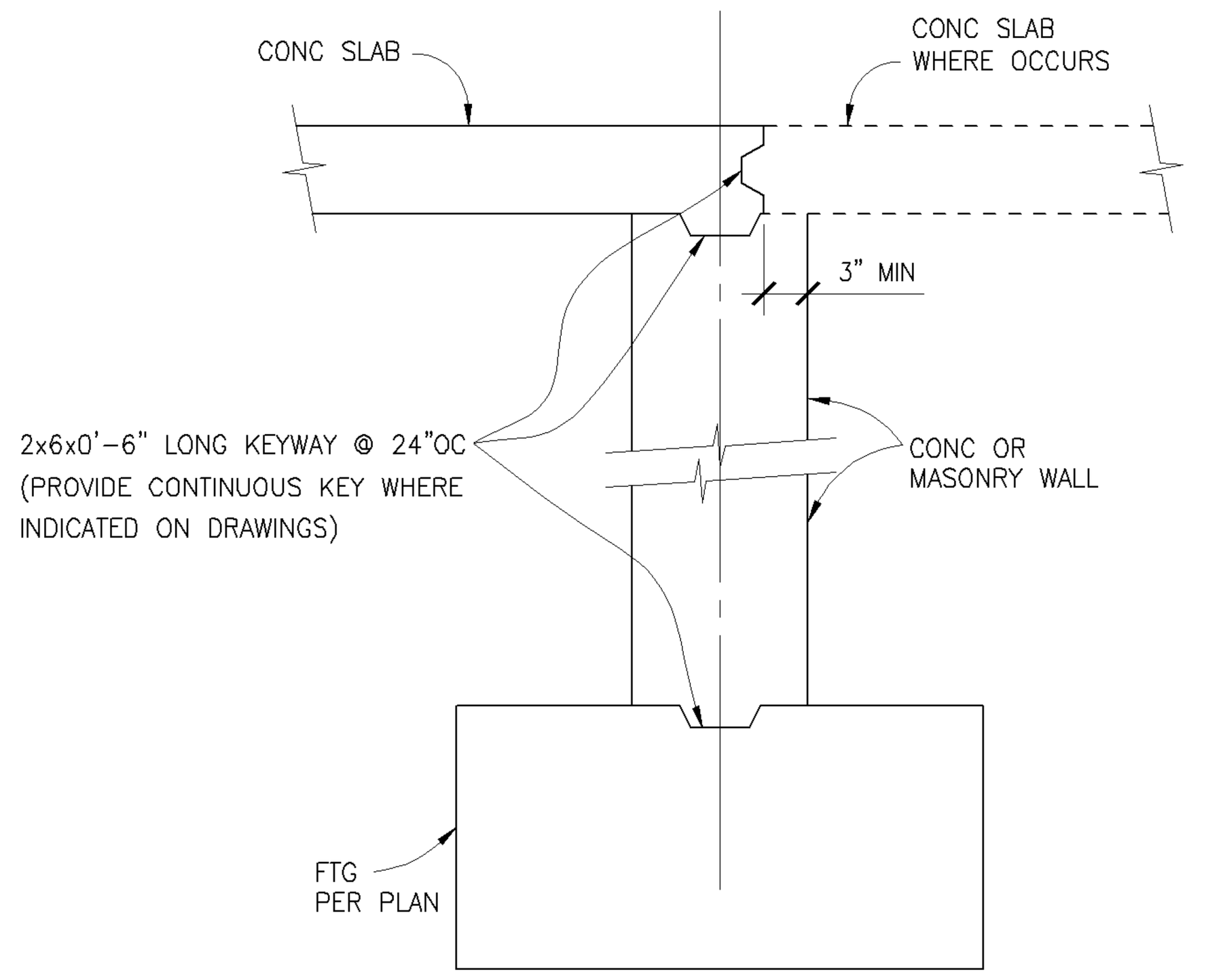


CAISSON DETAIL
1"=1'-0" 8

NOTE:
REINFORCEMENT SHOWN ON SPECIFICALLY REFERENCED DETAILS TAKE PRECEDENCE OVER REINFORCEMENT SHOWN HERE.



PLAN - CONC WALL DETAIL W/ DBL LAYER REINF
4



TYP CONC KEYWAYS AT COLD JOINTS
1"=1'-0" 5

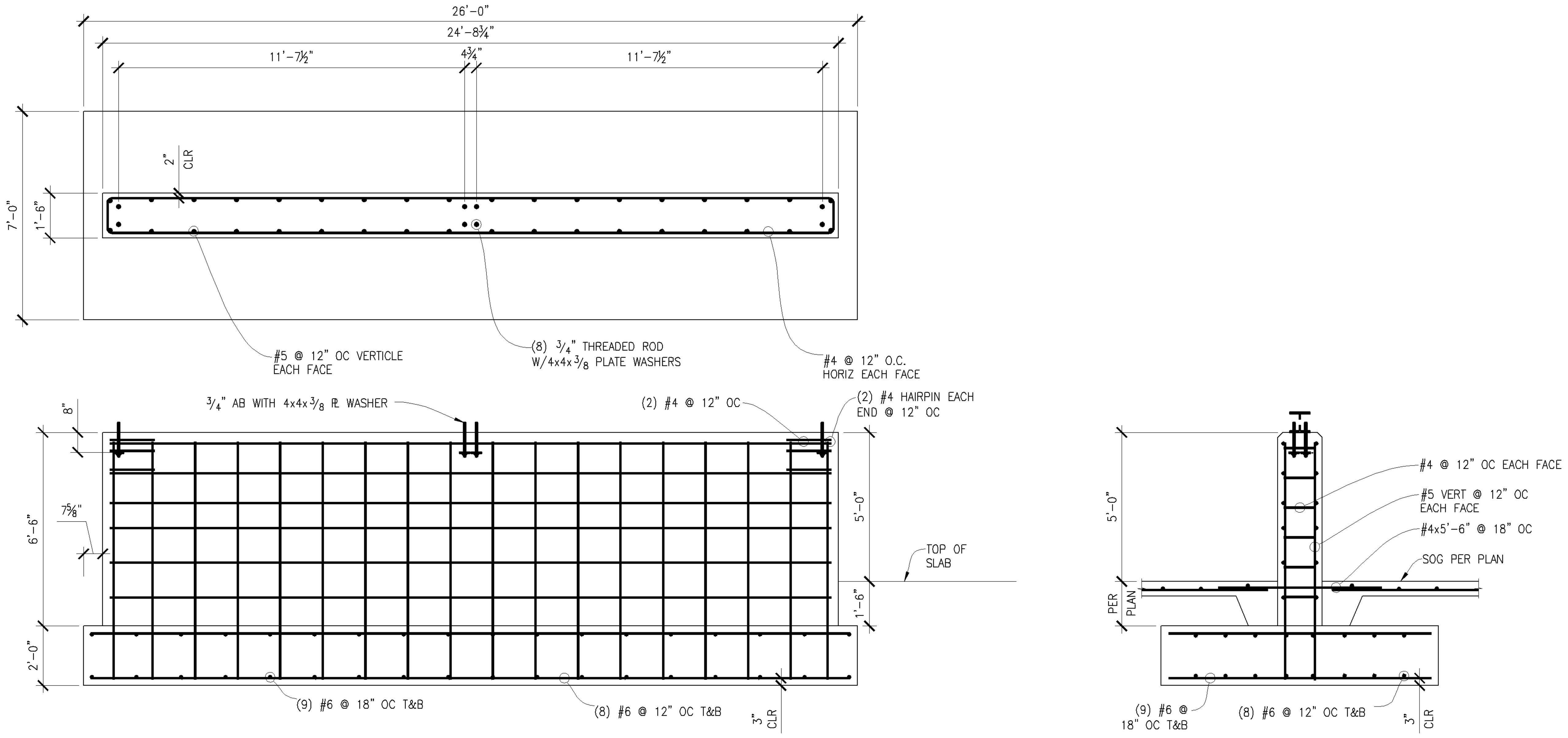
BAR SIZE	"TOP" REINFORCEMENT		"OTHER" REINFORCEMENT	
	DEVELOPMENT LENGTH (l_d)	LAP SPLICE LENGTH (l_s)	DEVELOPMENT LENGTH (l_d)	LAP SPLICE LENGTH (l_s)
#3	1'-10"	2'-4"	1'-5"	1'-10"
#4	2'-5"	3'-2"	1'-10"	2'-5"
#5	3'-0"	3'-11"	2'-4"	3'-0"
#6	3'-7"	4'-8"	2'-9"	3'-7"
#7	5'-3"	6'-9"	4'-0"	5'-3"
#8	6'-0"	7'-9"	4'-7"	6'-0"
#9	6'-9"	8'-9"	5'-2"	6'-9"
#10	7'-7"	9'-10"	5'-10"	7'-7"
#11	8'-5"	11'-0"	6'-6"	8'-5"

1. USE SPLICE LENGTH (l_s) INDICATED ON TABLE FOR ALL DOWEL LENGTHS NOT SHOWN ON PLANS UNLESS OTHERWISE PERMITTED BY ENGINEER.
2. DEVELOPMENT LENGTHS (l_d) AND SPLICE LENGTHS (l_s) SHOWN IN THE SCHEDULE ABOVE ARE IN ACCORDANCE WITH ACI 318-95 AND CORRESPOND TO GRADE 60 BARS EMBEDDED IN NORMAL WEIGHT CONCRETE (f'c=3000 PSI). FOR CONCRETE WITH f'c=4000 psi MULTIPLY VALUES IN TABLE BY 0.86. FOR LT WT AGGREGATE CONCRETE, MULTIPLY VALUES IN TABLE BY 1.3.
3. THIS SCHEDULE SHALL BE USED WHERE DEVELOPMENT AND SPLICE LENGTHS ARE NOT SPECIFICALLY DETAILED AND DIMENSIONED ON THE DRAWINGS.
4. THE SCHEDULE ABOVE IS APPLICABLE FOR THE FOLLOWING CONDITIONS:
   a) CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN d_b, CLEAR COVER NOT LESS THAN d_b, AND STIRRUPS OR TIES THROUGHOUT l_d NOT LESS THAN THE CODE MINIMUM.

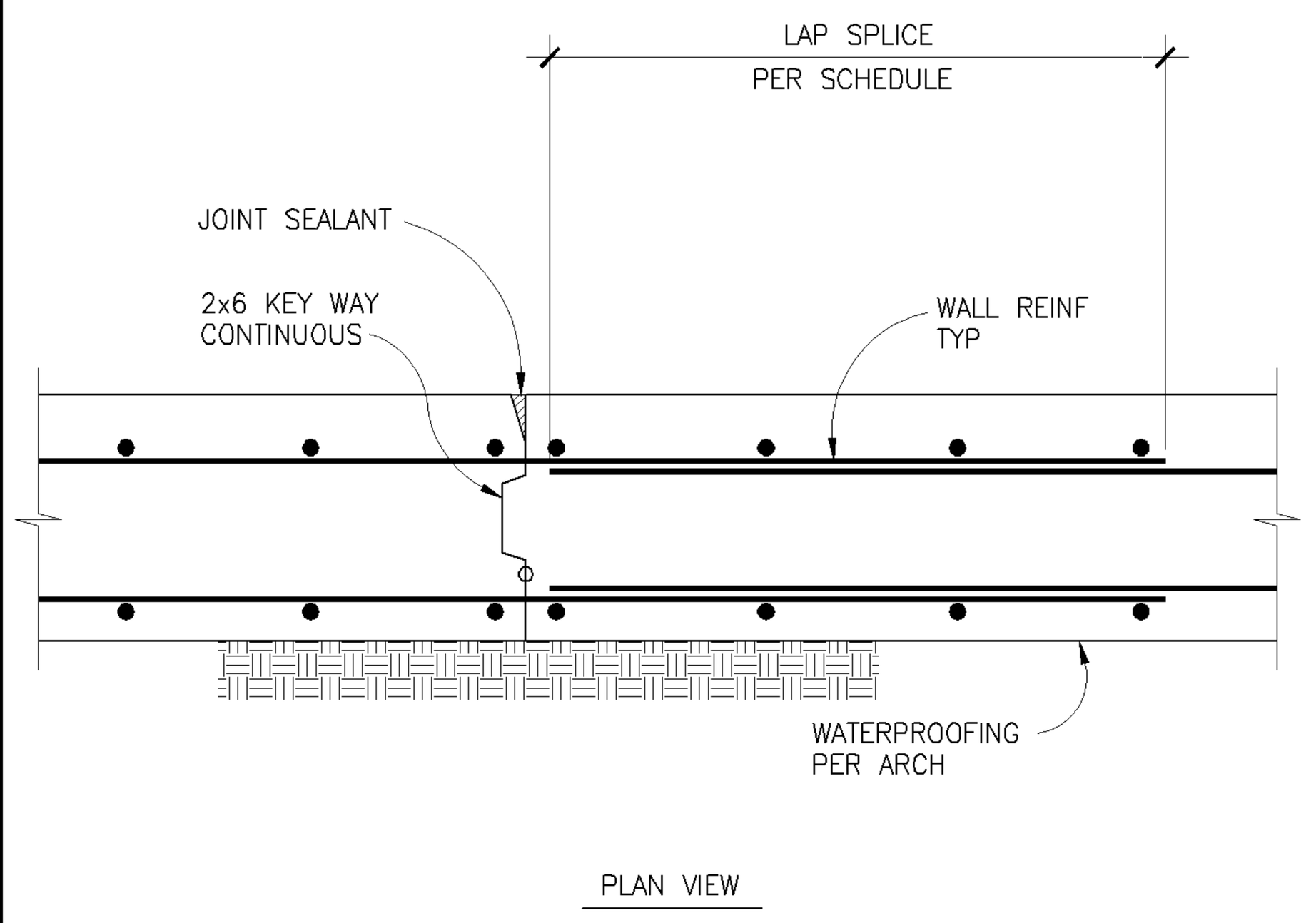
   OR

   b) CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN $2d_b$ AND CLEAR COVER NOT LESS THAN d_b.
5. "TOP" REINFORCEMENT IS DEFINED AS HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12" OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT OR SPLICE LENGTH. "OTHER" REINFORCEMENT INCLUDES ALL VERTICAL BARS AND HORIZONTAL BARS WITH LESS THAN 12" OF CONCRETE CAST BELOW BAR.
6. WHERE BARS OF DIFFERENT SIZE ARE LAP SPLICED, SPLICE LENGTH SHALL BE THE LARGER OF DEVELOPMENT LENGTH OF LARGER BAR, OR SPLICE LENGTH OF SMALLER BAR.

REINF DEVELOPMENT/LAP SPLICE SCHEDULE
NTS 1

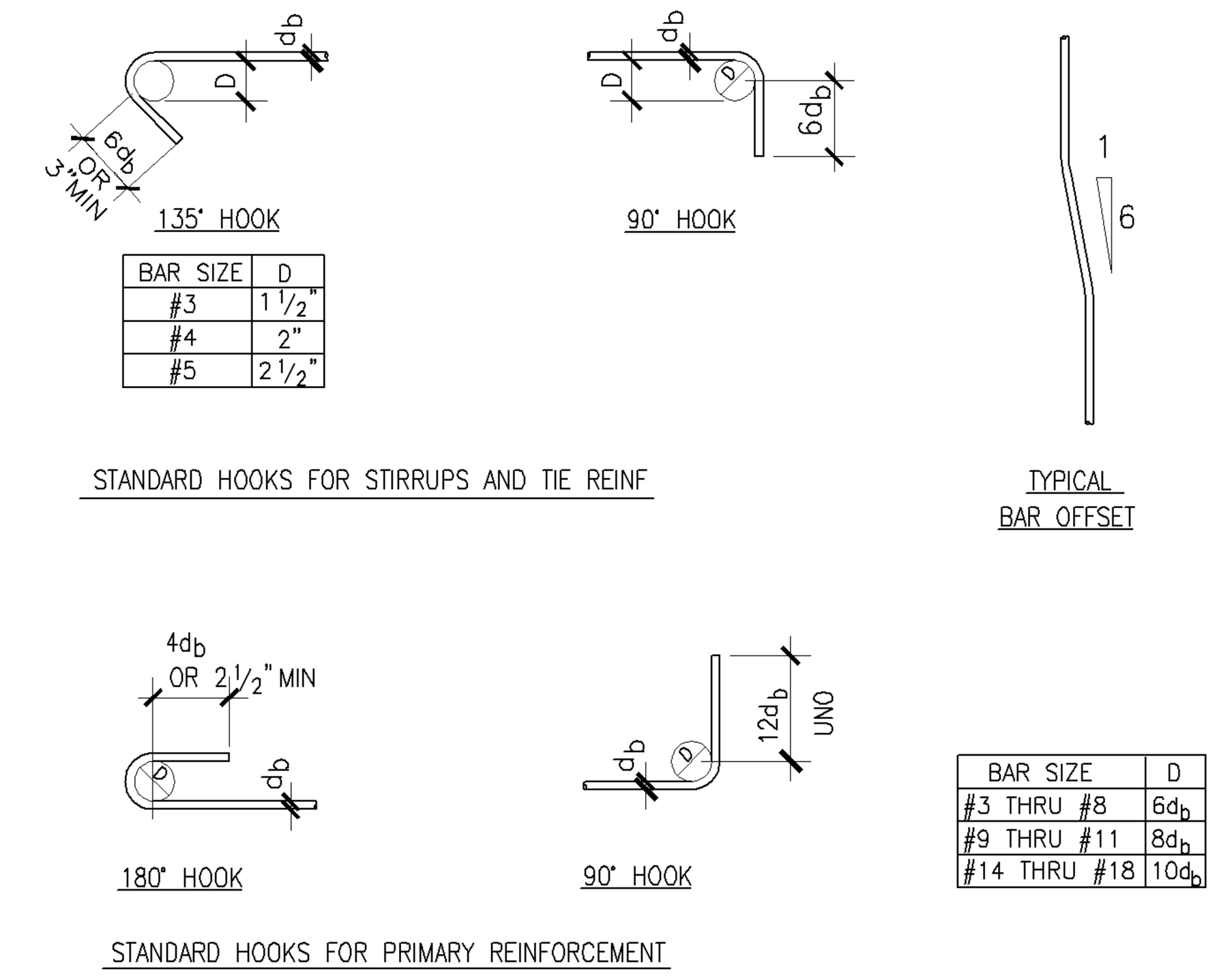


COOLING TOWER SUPPORT DETAIL
3/8"=1'-0" 9

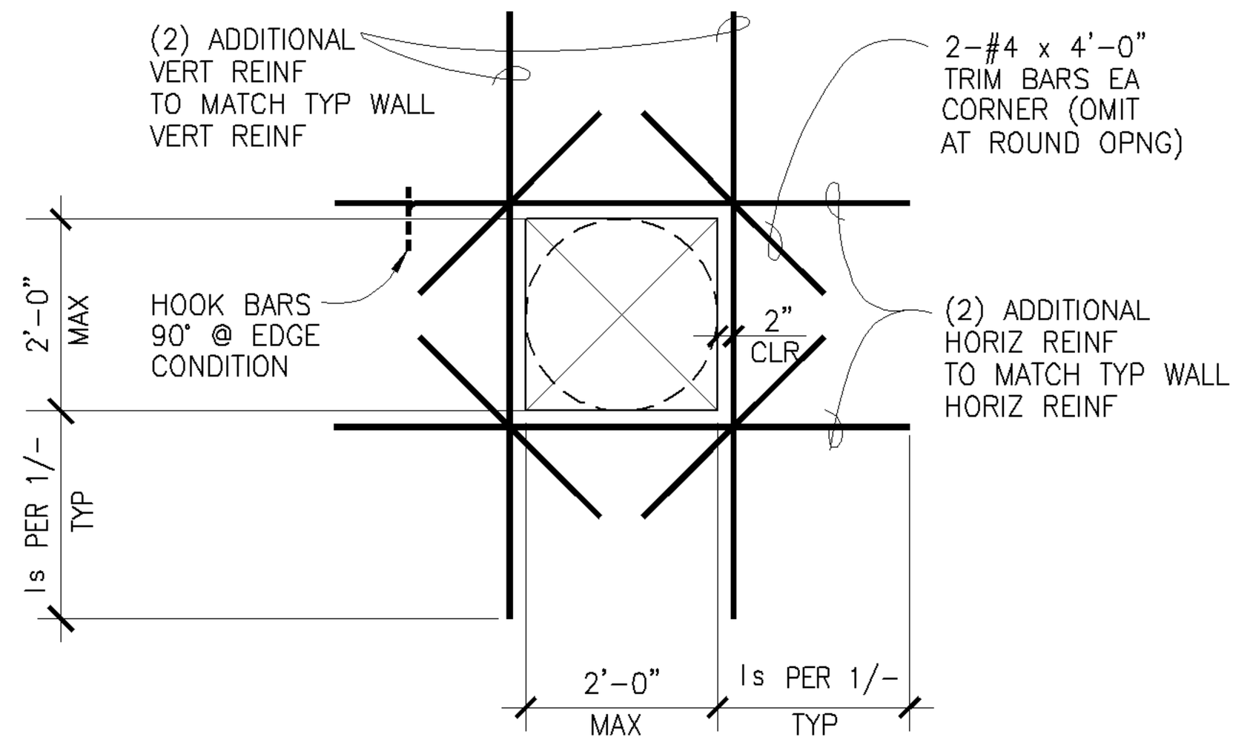


1"=1'-0"

CONCRETE WALL CONSTRUCTION/CONTROL JOINT
6

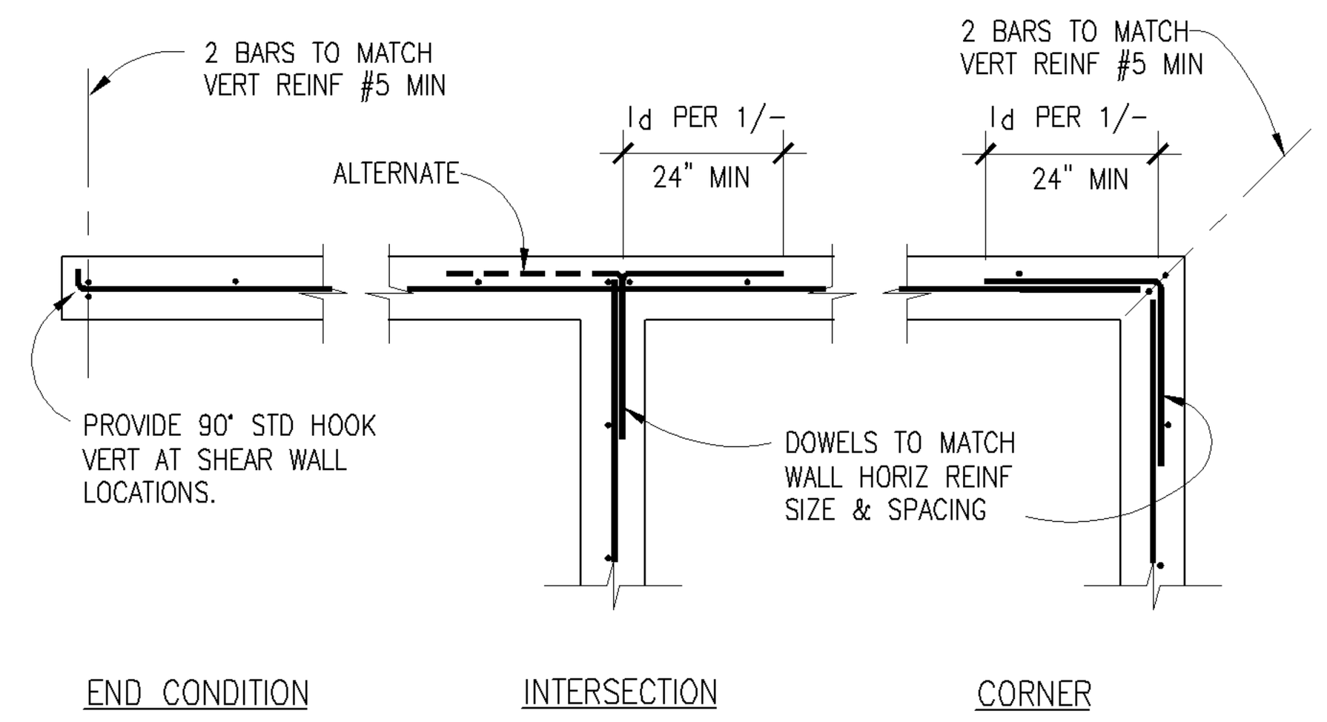


REINFORCING BAR BENDING DETAILS
1"=1'-0" 2



NOTES:
1. SEE WALL ELEVATIONS OR PLANS FOR REINFORCEMENT AT LARGER OPENINGS.
2. CLUSTERS OF SMALL HOLES WHOSE OVERALL MEASUREMENT EXCEED 1'-0" SHOULD BE REINFORCED AS ONE OPENING.
3. NO OPENING SHALL BE LOCATED CLOSER THAN 2'-0" FROM A CORNER OR EDGE OF A WALL UNLESS APPROVED BY THE STRUCTURAL ENGINEER.
4. NOTIFY STRUCTURAL ENGINEER WHEN OPENINGS EXCEED MAX DIMENSION OR OPENINGS ARE SPACED CLOSER THAN 6'-0" APART.

TYP CONC WALL OPENING DETAIL
1/2"=1'-0" 7



NOTE:
REINFORCEMENT SHOWN ON SPECIFICALLY REFERENCED DETAILS TAKE PRECEDENCE OVER REINFORCEMENT SHOWN HERE.

1"=1'-0"

PLAN - CONC WALL DETAIL W/ SINGLE LAYER REINF
3

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

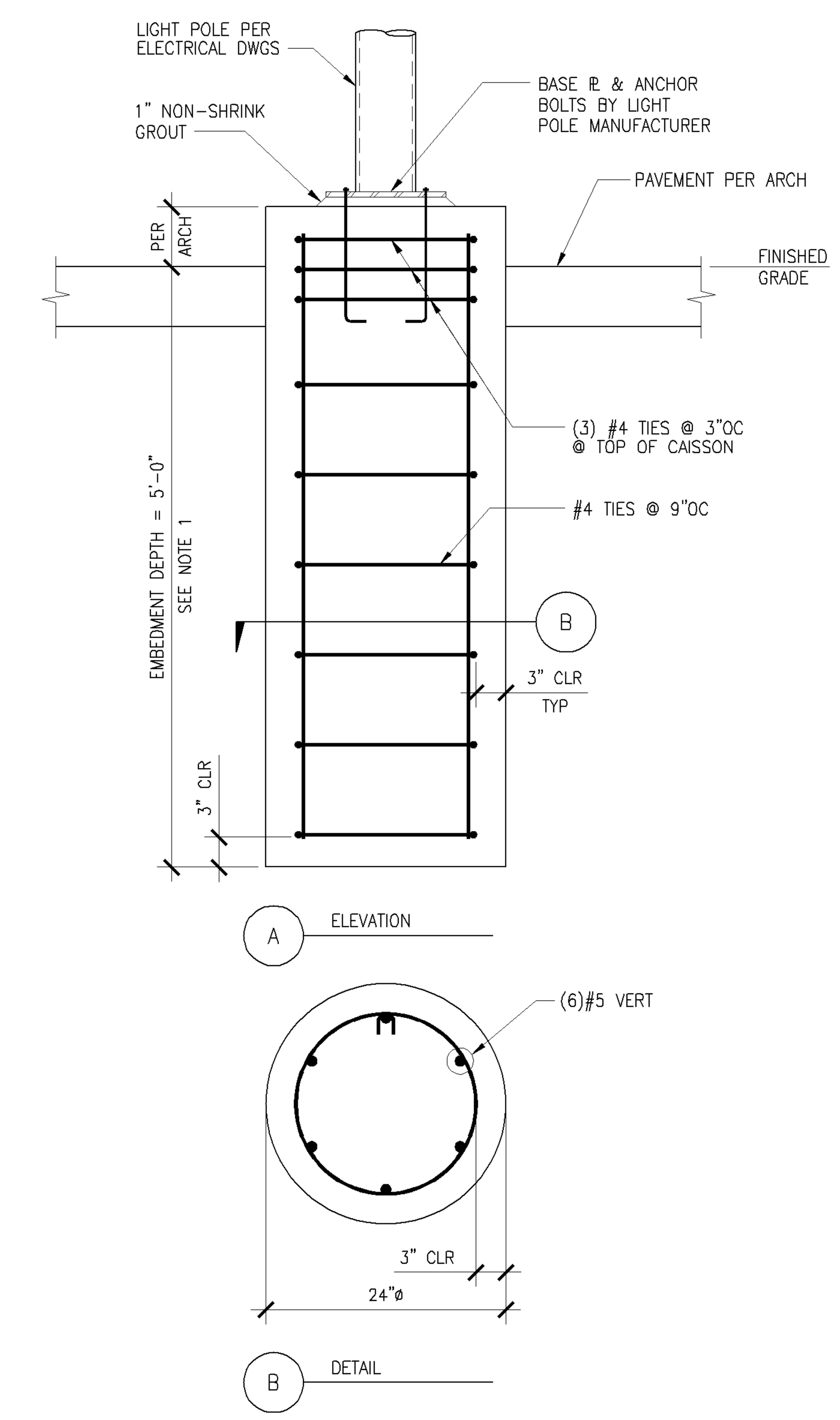
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

TYPICAL CONCRETE DETAILS

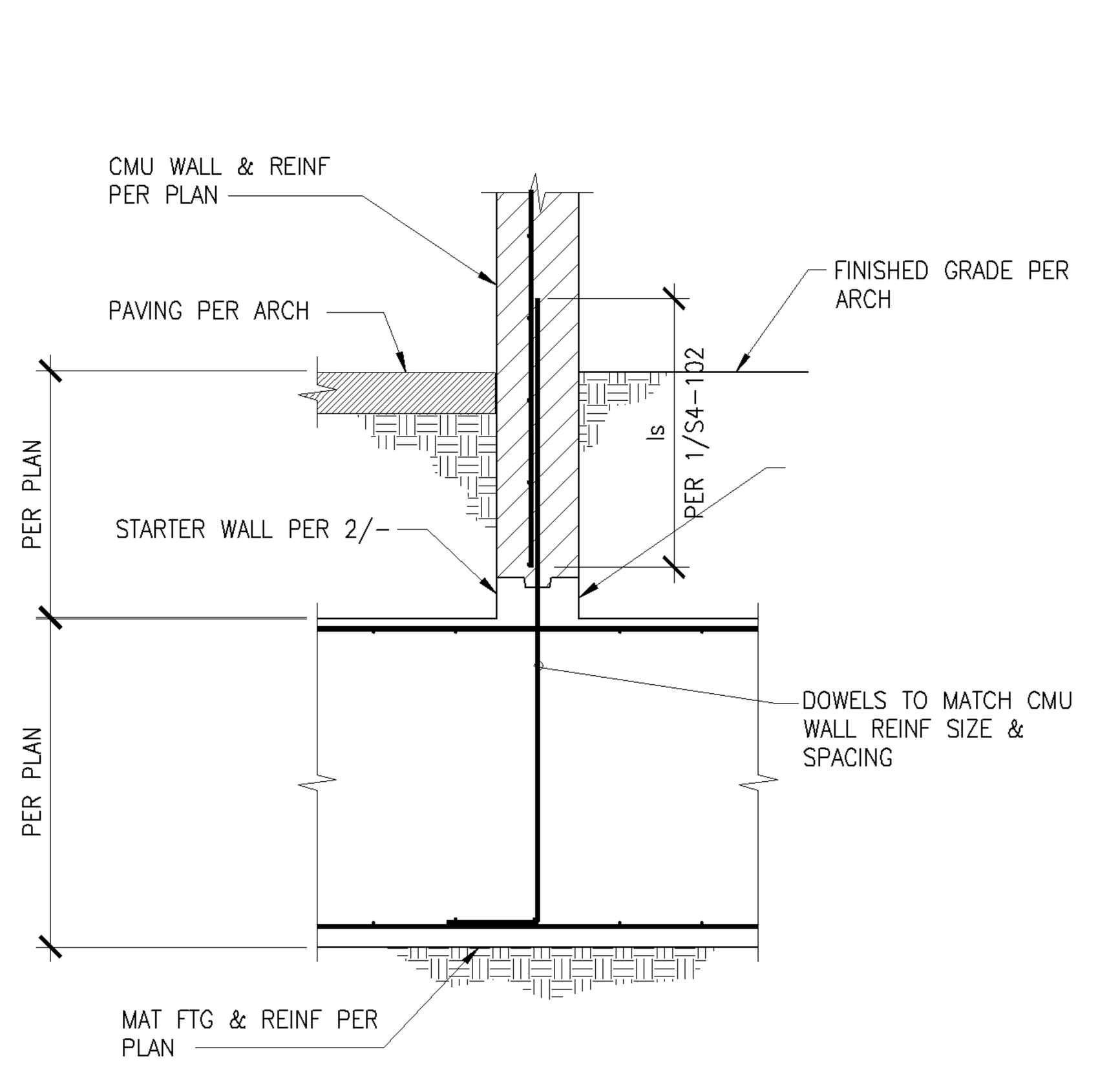
S4-102



NOTES:

1. IF ADJACENT FINISHED GRADE IS NOT PAVED, INCREASE EMBEDMENT DEPTH SHOWN BY 1'-0"
2. CAISSON TO BE INSTALLED INTO COMPACTED FILL PER SOILS REPORT.
3. CAISSON CONCRETE SHALL HAVE f'c=4000 PSI.
4. MAXIMUM HEIGHT OF LIGHT POLE SHALL BE 21'-0" ABOVE FINISHED GRADE.

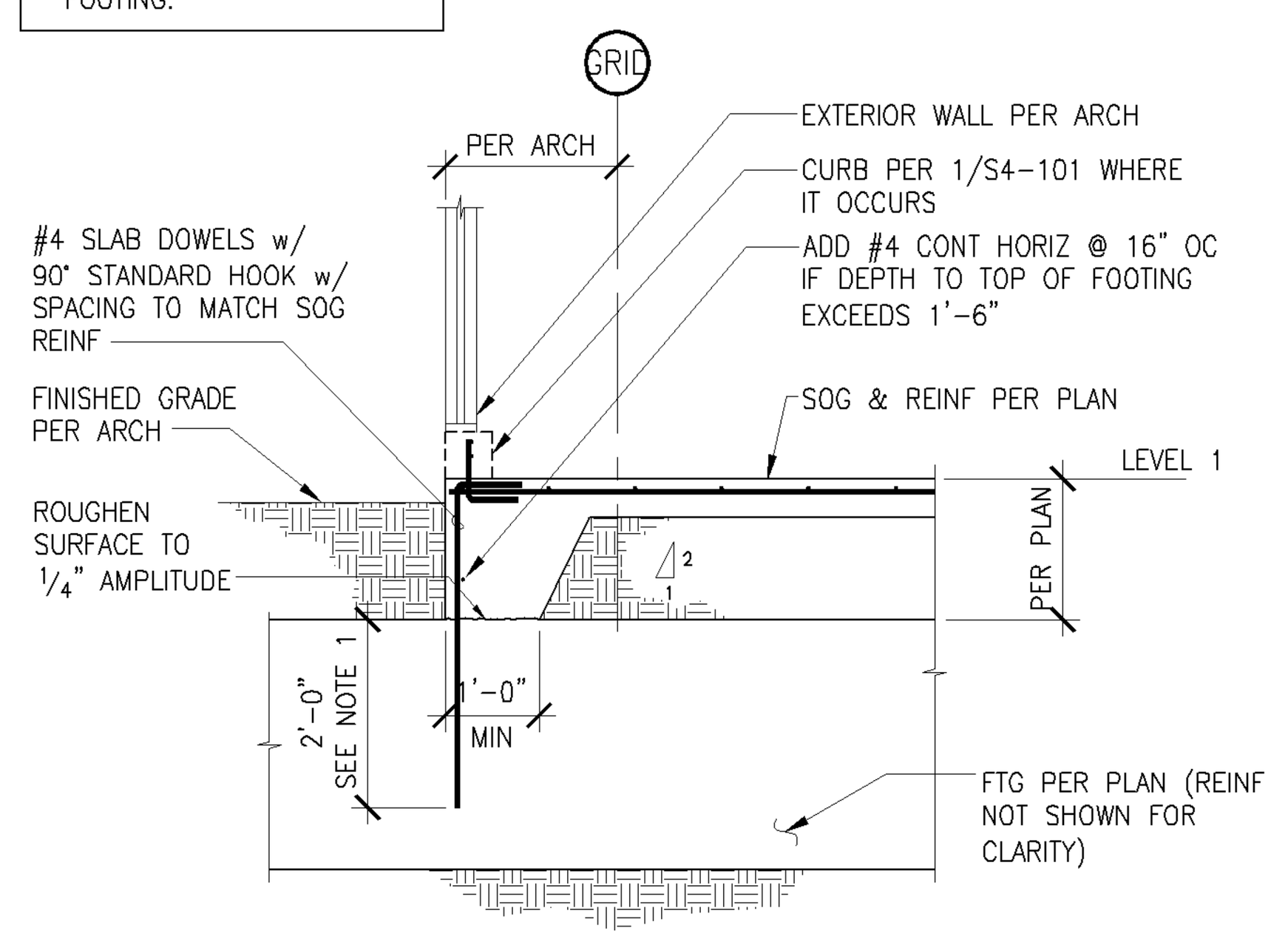
CAISSON FOOTING @ LIGHT POLES 1"=1'-0" 10



CMU WALL OVER MAT FTG 1/2"=1'-0" 6

NOTES:

1. IF FOOTING DEPTH IS 2'-0" OR LESS EMBED DOWEL TO 3" CLEAR FROM BOTTOM OF FOOTING.



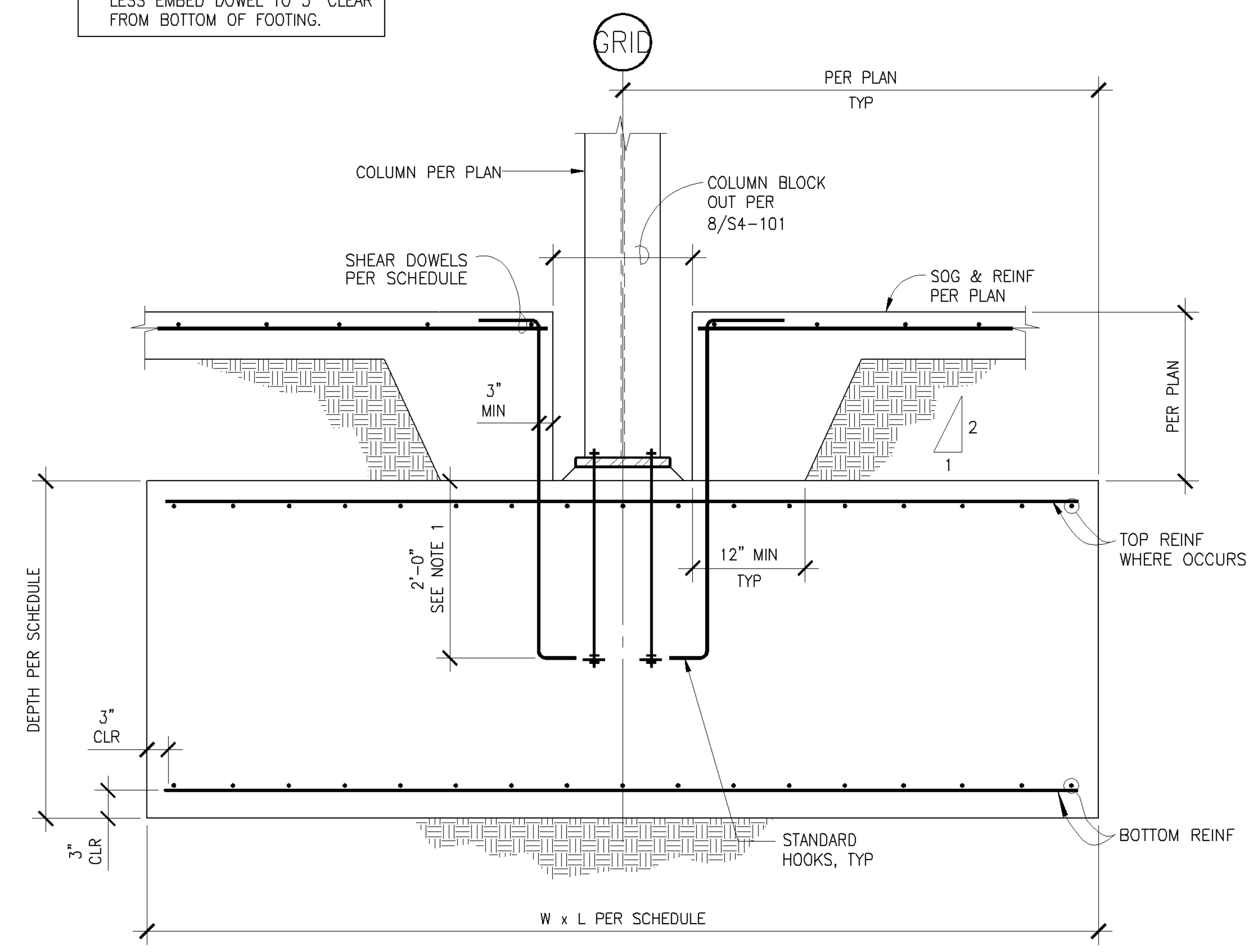
EDGE OF SLAB ON GRADE OVER FOOTING 1/2"=1'-0" 7

GRAVITY COLUMN FOOTING SCHEDULE					
MARK	SIZE			REINFORCEMENT	SHEAR DOWELS
	LENGTH	WIDTH	DEPTH		
A	21'-0"	21'-0"	3'-6"	#4 @ 18"OC TOP EA WAY #11 @ 12"OC BOTT EA WAY	#4 @ 16"OC
B	19'-0"	19'-0"	3'-6"	#4 @ 18"OC TOP EA WAY #10 @ 12"OC BOTT EA WAY	#4 @ 16"OC
C	17'-6"	17'-6"	3'-0"	#10 @ 12"OC BOTT, EA WAY	#4 @ 16"OC
D	15'-6"	15'-6"	3'-0"	#10 @ 12"OC BOTT, EA WAY	#4 @ 16"OC
E	11'-6"	11'-6"	2'-0"	#9 @ 12"OC BOTT, EA WAY	#4 @ 16"OC
F	9'-6"	9'-6"	2'-0"	#9 @ 12"OC BOTT, EA WAY	#4 @ 16"OC
G	6'-0"	6'-0"	2'-0"	#7 @ 12"OC BOTT, EA WAY	#4 @ 16"OC
H	PER PLAN	PER PLAN	2'-3"	#8 @ 12"OC T&B, EA WAY	#4 @ 16"OC
J	2'-0"	2'-0"	1'-3"	#6 @ 12"OC BOTT, EA WAY	#4 @ 16"OC
K	3'-0"	3'-0"	1'-6"	#6 @ 12"OC BOTT, EA WAY	#4 @ 16"OC

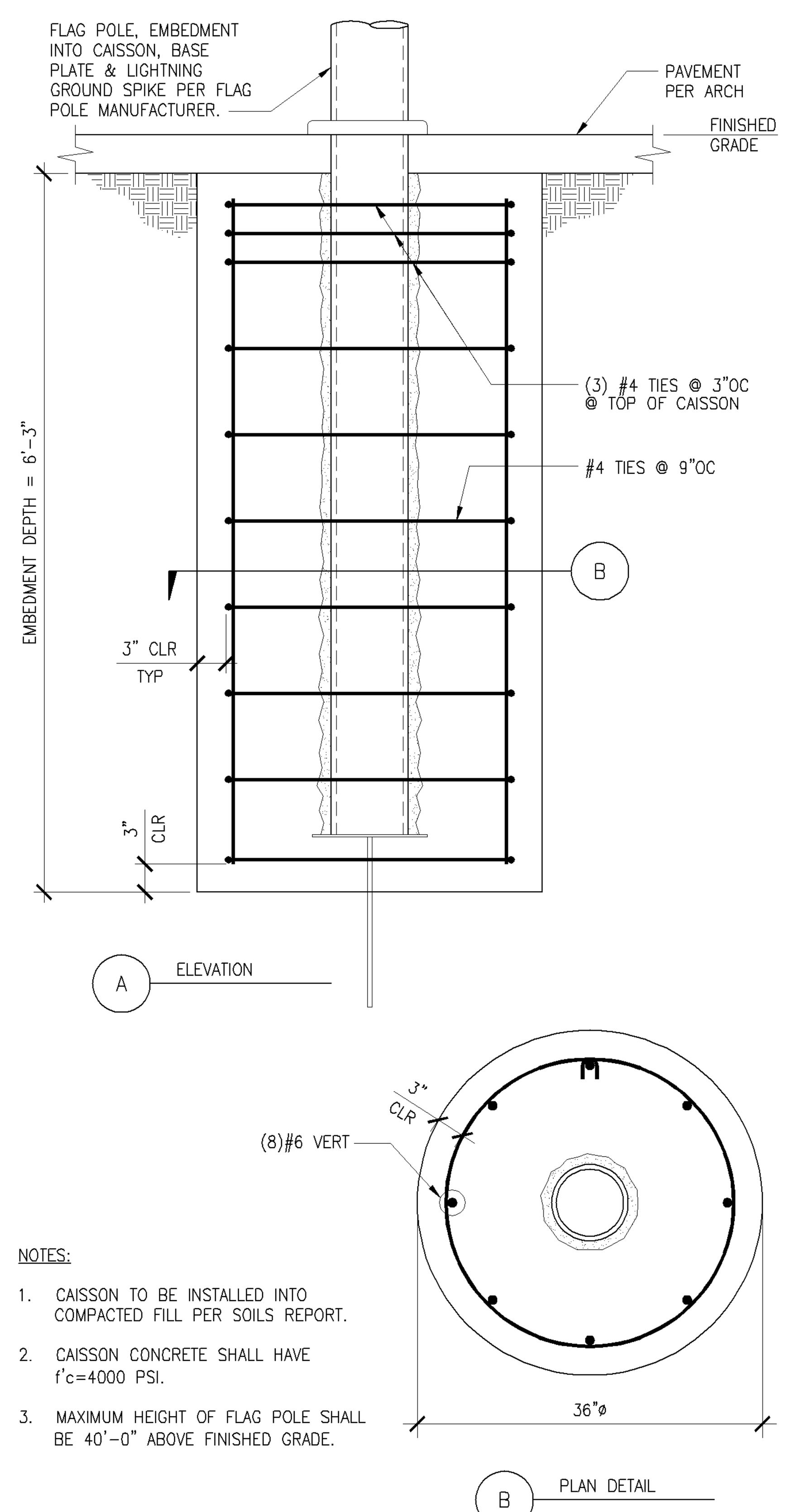
FRAME COLUMN FOOTING SCHEDULE					
MARK	SIZE			REINFORCEMENT	SHEAR DOWELS
	LENGTH	WIDTH	DEPTH		
1A	SEE DETAIL 1/S4-104				#4 @ 16"OC
1B	SEE DETAIL 2/S4-104				#4 @ 16"OC
2	SEE DETAIL 1/S4-105				#4 @ 16"OC
3	SEE DETAIL 2/S4-105				#4 @ 16"OC
4A	SEE DETAIL 1/S4-106				#4 @ 16"OC
4B	SEE DETAIL 2/S4-106				#4 @ 16"OC
5	SEE DETAIL 1/S4-107				#4 @ 16"OC
6	3'-0"	3'-0"	1'-6"	(4) #5 T&B, EA WAY	#4 @ 16"OC
7	4'-0"	4'-0"	2'-0"	(5) #5 T&B, EA WAY	#4 @ 16"OC
8	5'-0"	5'-0"	2'-6"	(6) #5 T&B, EA WAY	#4 @ 16"OC

NOTES:

1. IF FOOTING DEPTH IS 2'-0" OR LESS EMBED DOWEL TO 3" CLEAR FROM BOTTOM OF FOOTING.



FOOTING SCHEDULE NTS 1



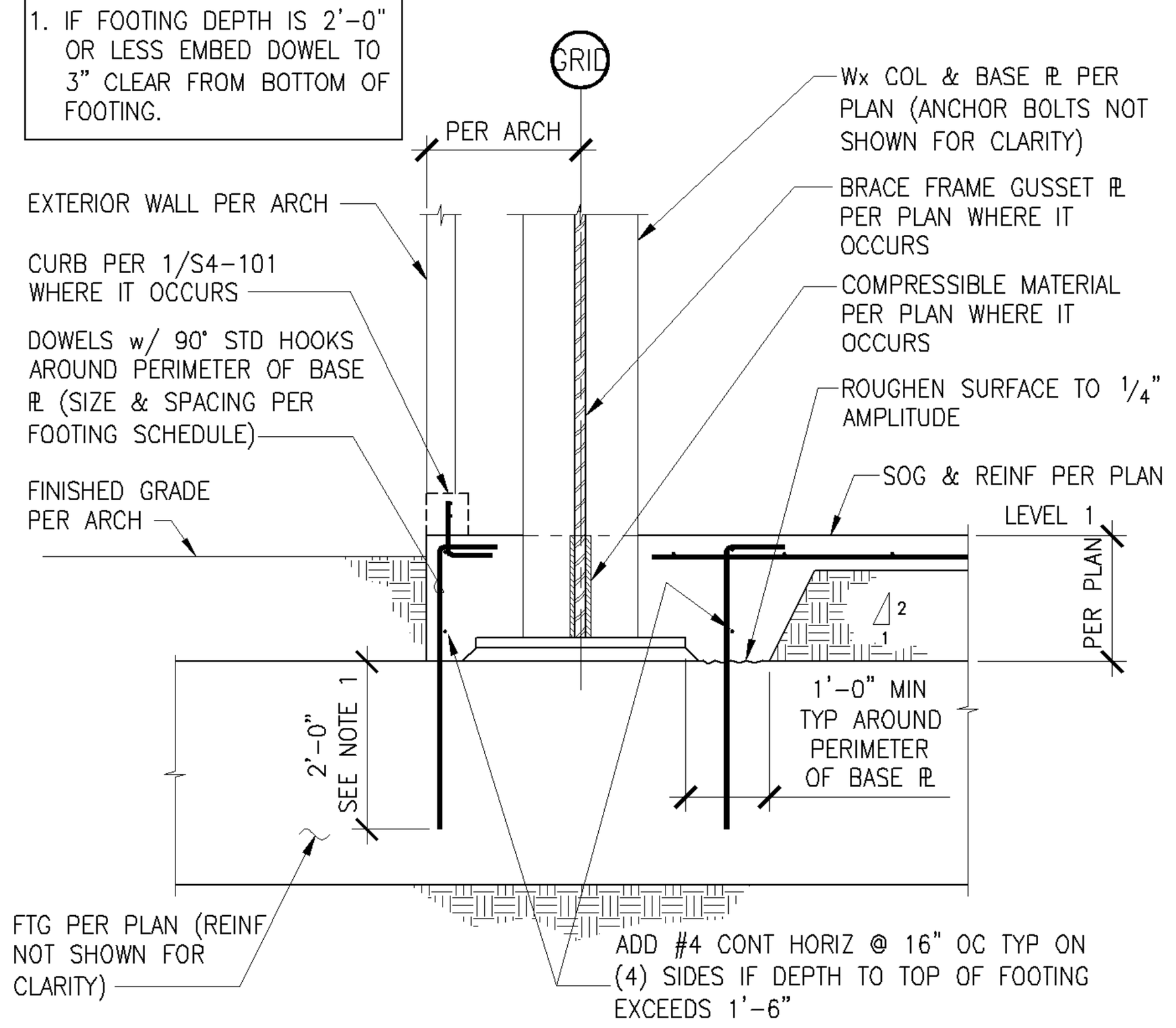
NOTES:

1. CAISSON TO BE INSTALLED INTO COMPACTED FILL PER SOILS REPORT.
2. CAISSON CONCRETE SHALL HAVE f'c=4000 PSI.
3. MAXIMUM HEIGHT OF FLAG POLE SHALL BE 40'-0" ABOVE FINISHED GRADE.

CAISSON FOOTING @ FLAG POLE 1"=1'-0" 11

NOTES:

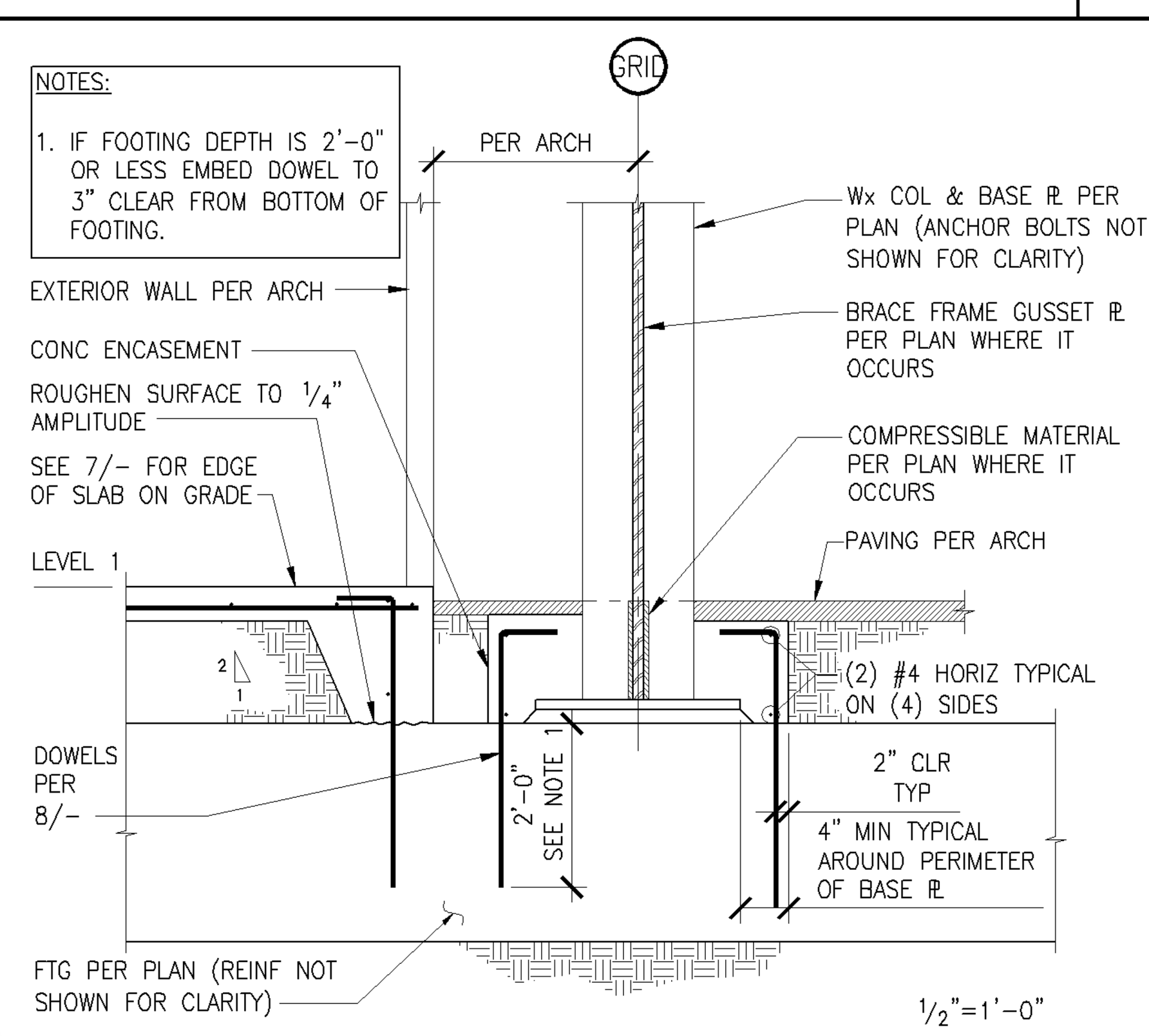
1. IF FOOTING DEPTH IS 2'-0" OR LESS EMBED DOWEL TO 3" CLEAR FROM BOTTOM OF FOOTING.



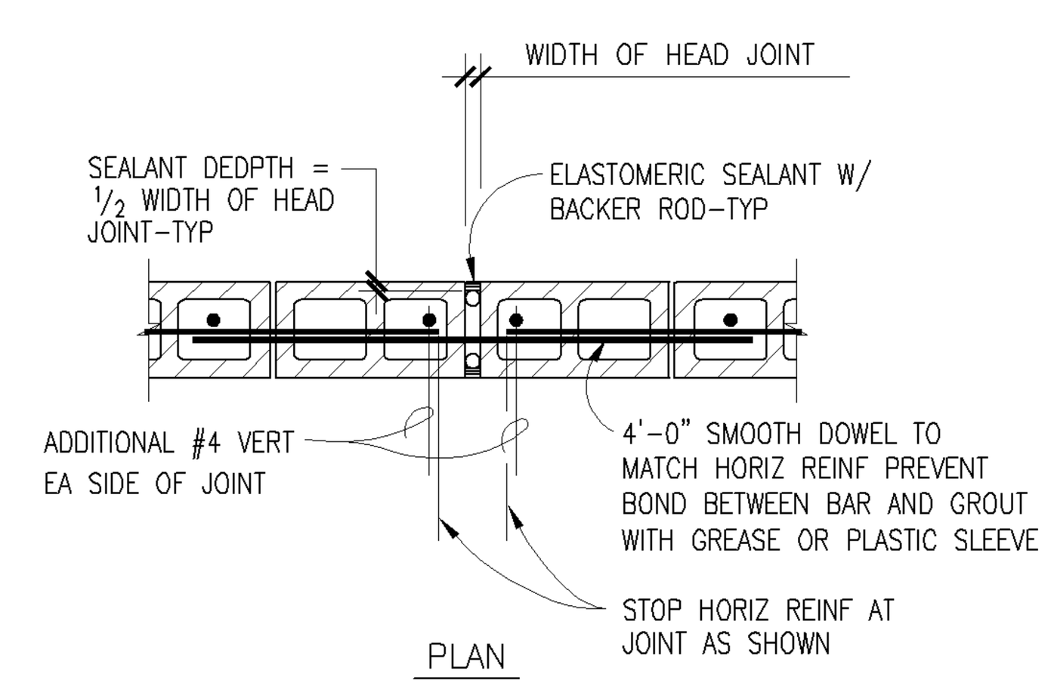
EDGE OF SLAB ON GRADE @ COLUMN 1/2"=1'-0" 8

NOTES:

1. IF FOOTING DEPTH IS 2'-0" OR LESS EMBED DOWEL TO 3" CLEAR FROM BOTTOM OF FOOTING.



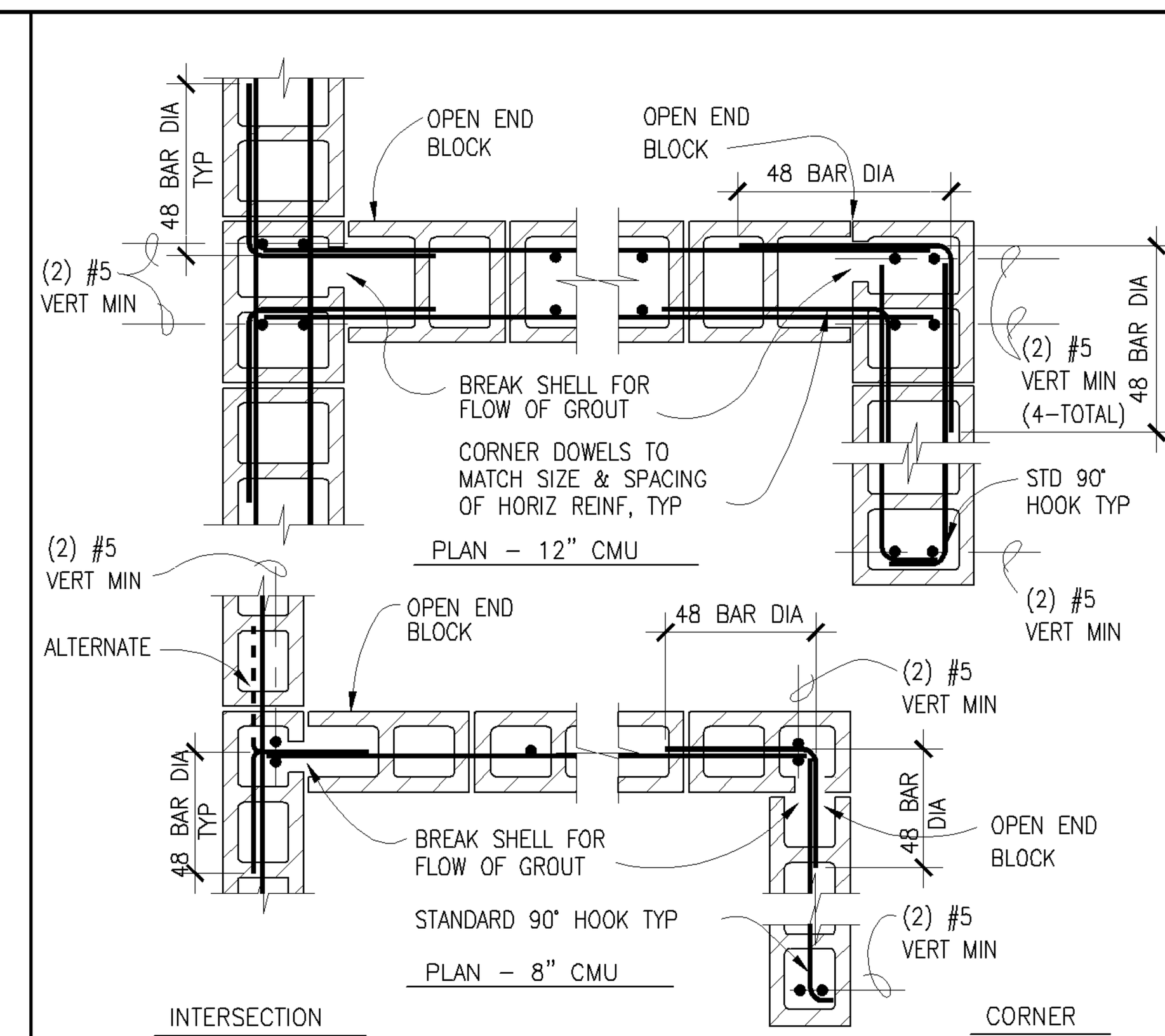
COLUMN BASE BEYOND EDGE OF SLAB ON GRADE 1/2"=1'-0" 9



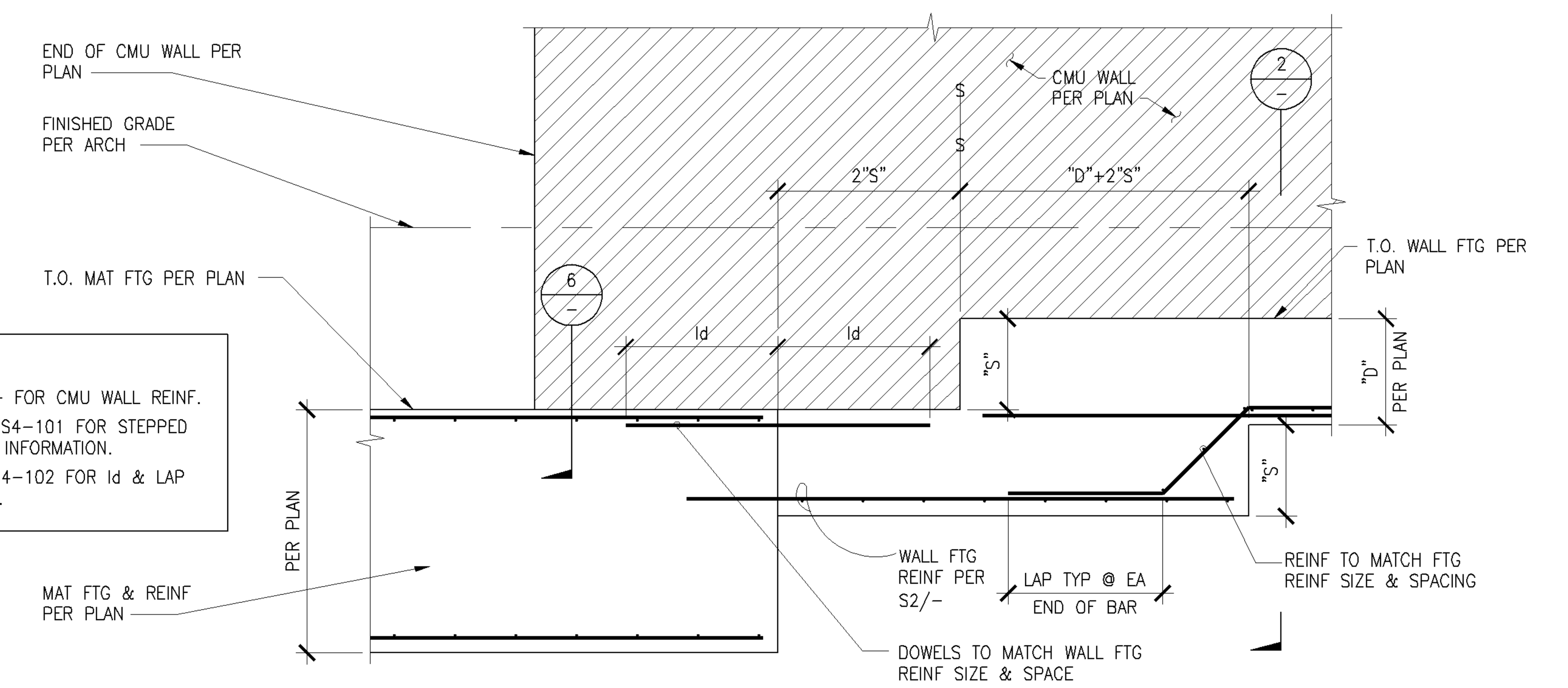
NOTES :

1. PROVIDE JOINTS AT INTERVALS NOT TO EXCEED 20 FEET UNO.
2. CONCRETE BLOCK SHOWN - BRICK SIMILAR.
3. THIS DETAIL APPLIES ONLY TO SITE FENCE/RETAINING WALLS, NOT APPLICABLE TO WALLS TIED IN TO BUILDING STRUCTURE.

CONTROL JOINT @ CMU SITE WALLS NTS 4



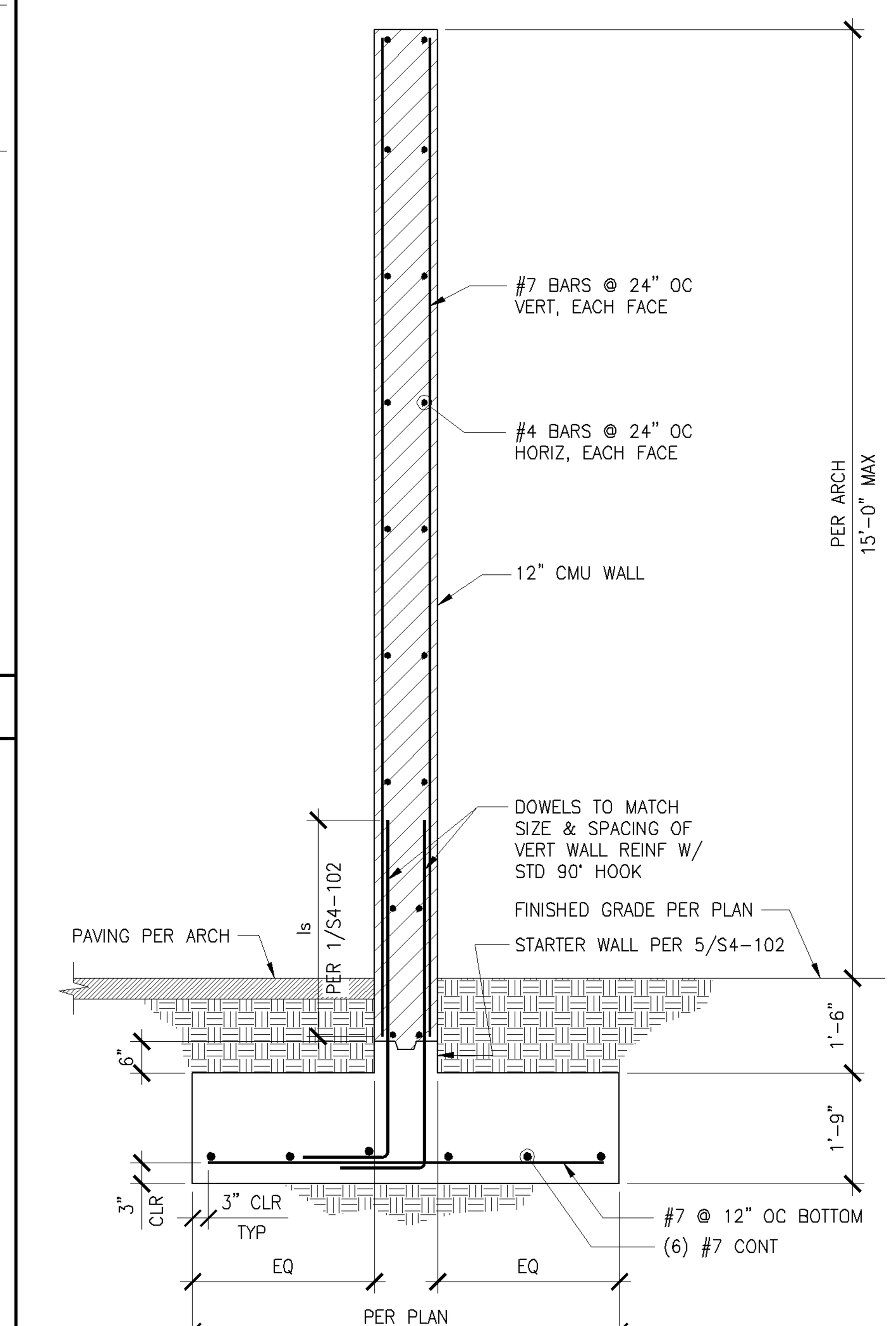
CMU WALL INTERSECTION & CORNER DETAIL 1"=1'-0" 3



NOTES:

1. SEE 2/- FOR CMU WALL REINF.
2. SEE 12/S4-101 FOR STEPPED FOOTING INFORMATION.
3. SEE 1/S4-102 FOR ld AND LAP LENGTHS.

SECTION @ STEPPED CMU WALL FTG 1/2"=1'-0" 5



CMU FENCE WALL DETAIL NTS 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
4	11-29-07	BULLETIN 012 MODS.
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

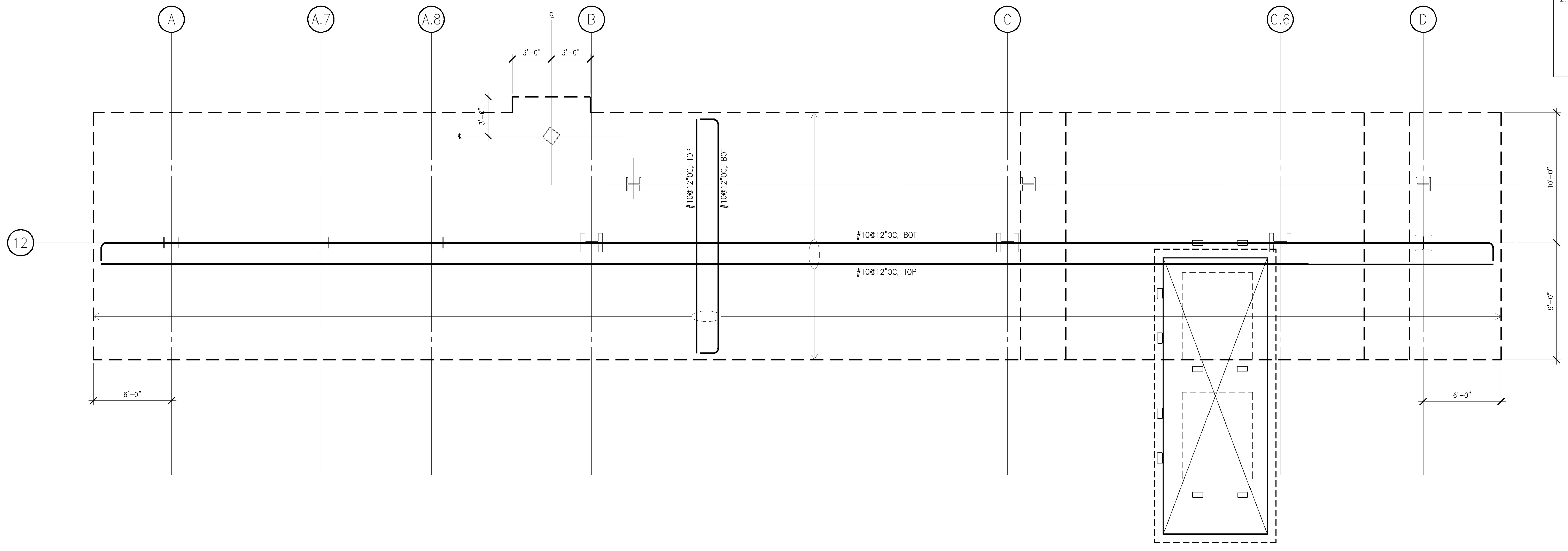
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

CONCRETE DETAILS

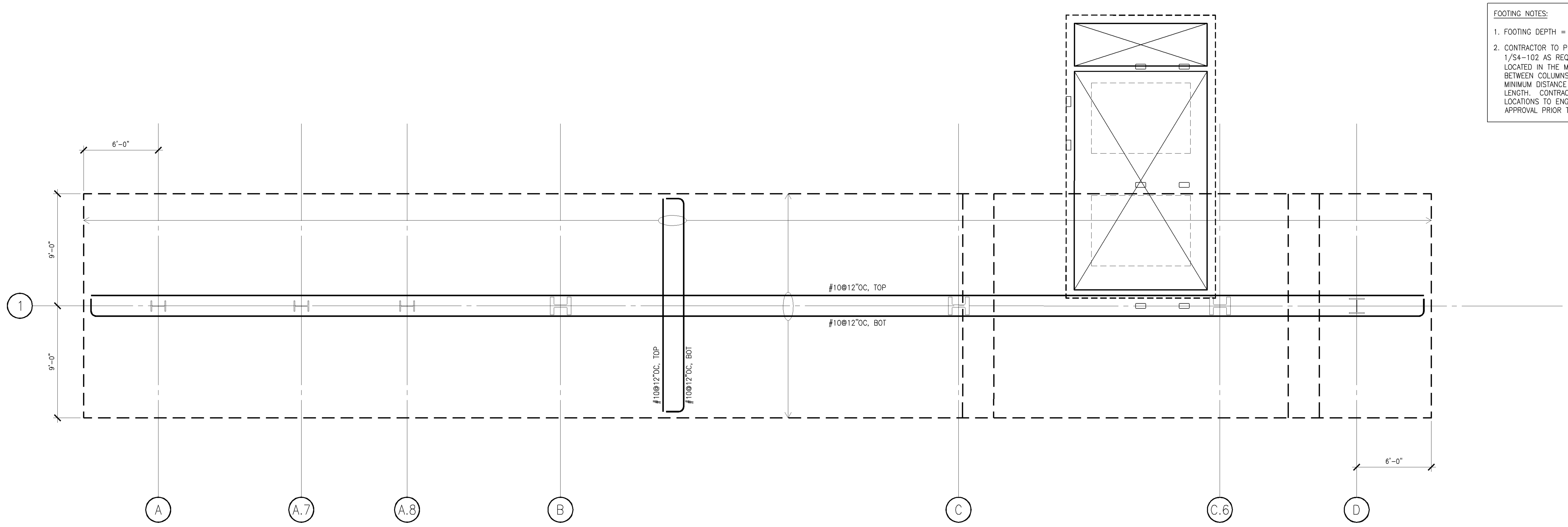
S4-103



FOOTING NOTES:

1. FOOTING DEPTH = 4'-0" TYPICAL U.N.O.
2. CONTRACTOR TO PROVIDE REBAR SPLICES PER 1/S4-102 AS REQUIRED. SPLICES SHALL BE LOCATED IN THE MIDDLE 1/3 OF THE DISTANCE BETWEEN COLUMNS & SHALL BE STAGGERED A MINIMUM DISTANCE EQUAL TO THE SPLICE LENGTH. CONTRACTOR SHALL PROVIDE SPLICE LOCATIONS TO ENGINEER FOR REVIEW & APPROVAL PRIOR TO CONSTRUCTION.

PARTIAL FOUNDATION PLAN - FOOTING TYPE 1A 1/4"=1'-0" **1**



FOOTING NOTES:

1. FOOTING DEPTH = 4'-0" TYPICAL U.N.O.
2. CONTRACTOR TO PROVIDE REBAR SPLICES PER 1/S4-102 AS REQUIRED. SPLICES SHALL BE LOCATED IN THE MIDDLE 1/3 OF THE DISTANCE BETWEEN COLUMNS & SHALL BE STAGGERED A MINIMUM DISTANCE EQUAL TO THE SPLICE LENGTH. CONTRACTOR SHALL PROVIDE SPLICE LOCATIONS TO ENGINEER FOR REVIEW & APPROVAL PRIOR TO CONSTRUCTION.

PARTIAL FOUNDATION PLAN - FOOTING TYPE 1B 1/4"=1'-0" **2**

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

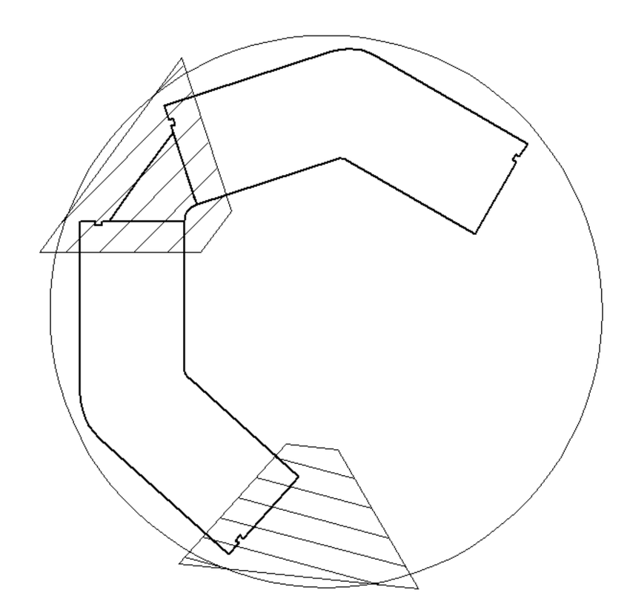
REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN



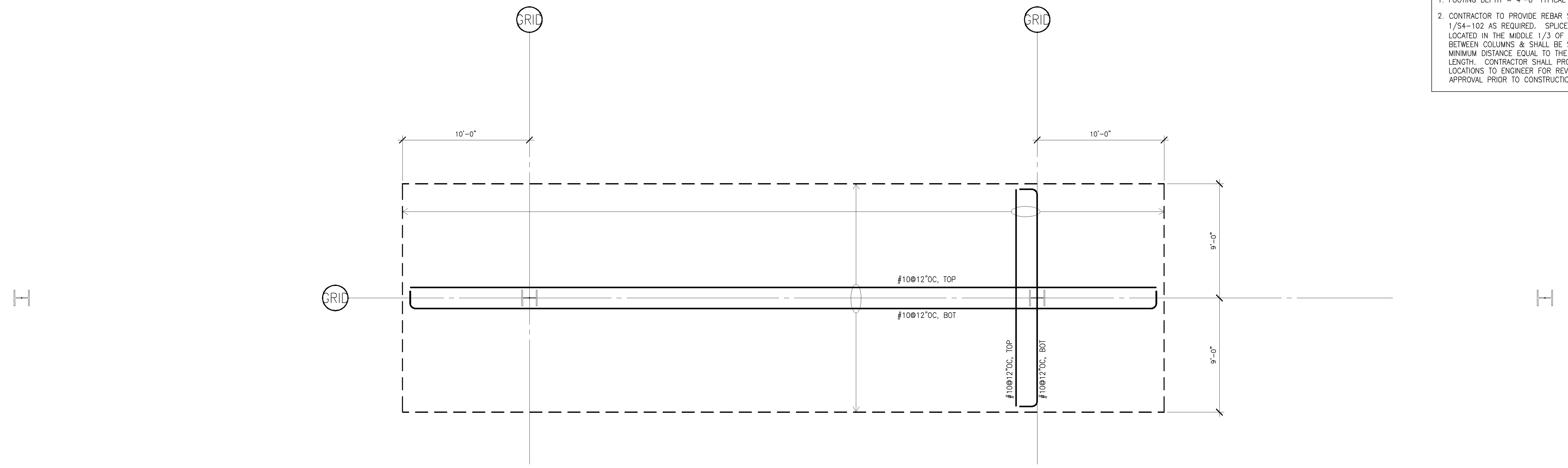
SHEET TITLE

PARTIAL FOUNDATION PLANS - FOOTING TYPE 1A & 1B

S4-104

FOOTING NOTES:

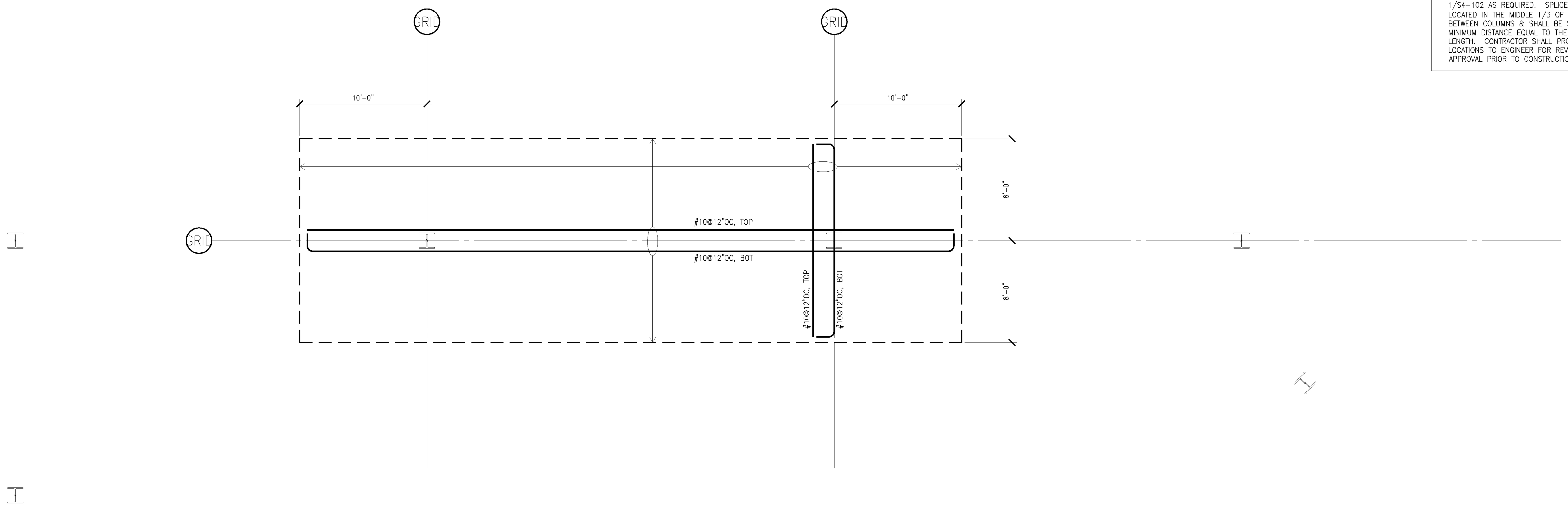
1. FOOTING DEPTH = 4'-0" TYPICAL U.N.O.
2. CONTRACTOR TO PROVIDE REBAR SPLICES PER 1/S4-102 AS REQUIRED. SPLICES SHALL BE LOCATED IN THE MIDDLE 1/3 OF THE DISTANCE BETWEEN COLUMNS & SHALL BE STAGGERED A MINIMUM DISTANCE EQUAL TO THE SPLICE LENGTH. CONTRACTOR SHALL PROVIDE SPLICE LOCATIONS TO ENGINEER FOR REVIEW & APPROVAL PRIOR TO CONSTRUCTION.



PARTIAL FOUNDATION PLAN - FOOTING TYPE 2 | 1/4"=1'-0" | 1

FOOTING NOTES:

1. FOOTING DEPTH = 4'-0" TYPICAL U.N.O.
2. CONTRACTOR TO PROVIDE REBAR SPLICES PER 1/S4-102 AS REQUIRED. SPLICES SHALL BE LOCATED IN THE MIDDLE 1/3 OF THE DISTANCE BETWEEN COLUMNS & SHALL BE STAGGERED A MINIMUM DISTANCE EQUAL TO THE SPLICE LENGTH. CONTRACTOR SHALL PROVIDE SPLICE LOCATIONS TO ENGINEER FOR REVIEW & APPROVAL PRIOR TO CONSTRUCTION.



PARTIAL FOUNDATION PLAN - FOOTING TYPE 3 | 1/4"=1'-0" | 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
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CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
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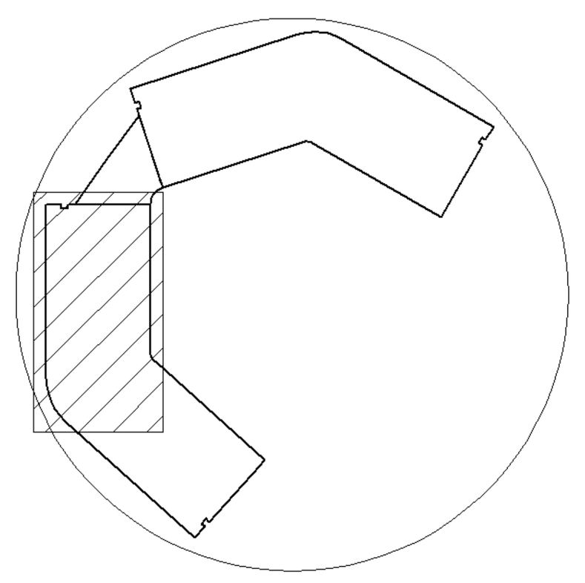
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

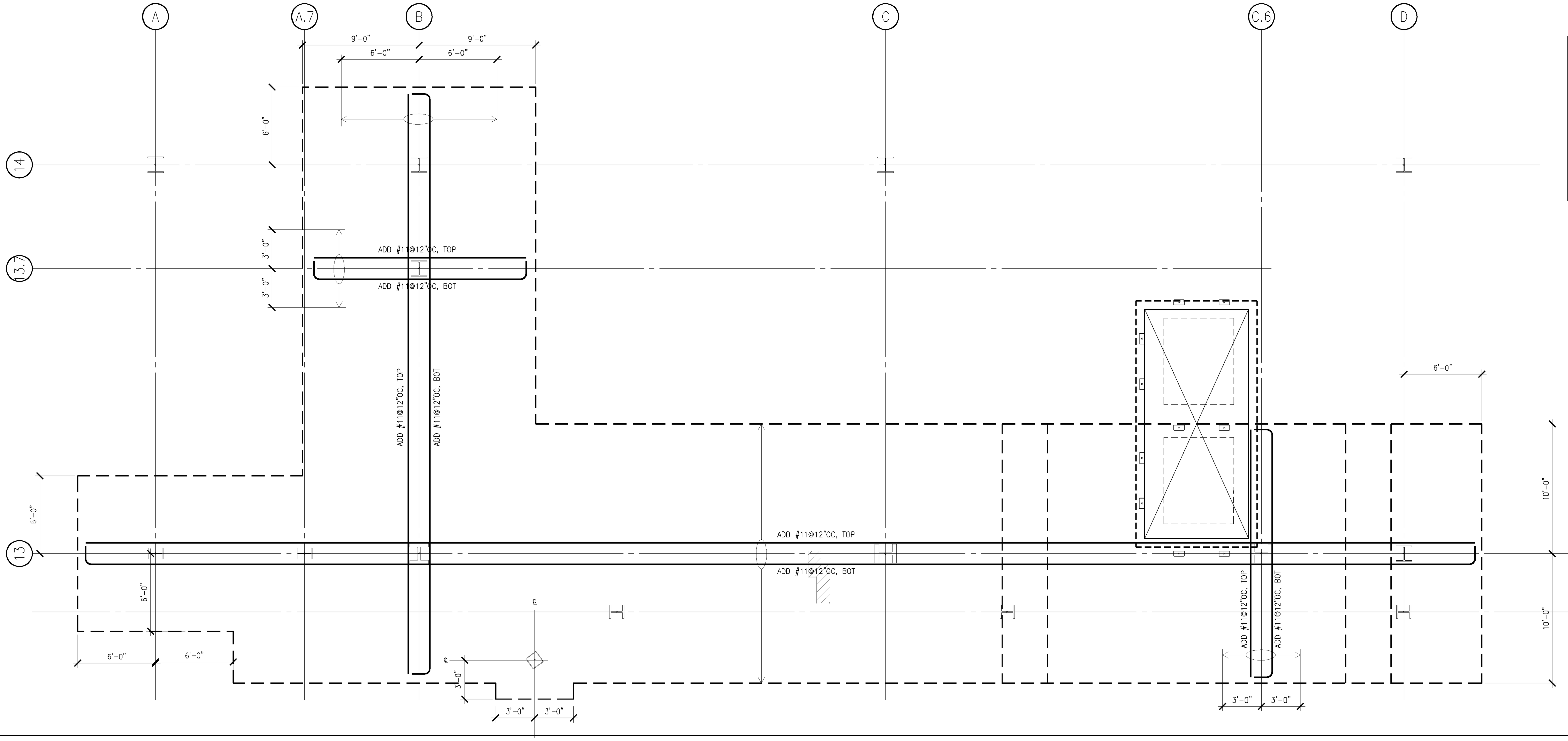
KEY PLAN



SHEET TITLE

PARTIAL FOUNDATION PLANS - FOOTING TYPE 2 & 3

S4-105

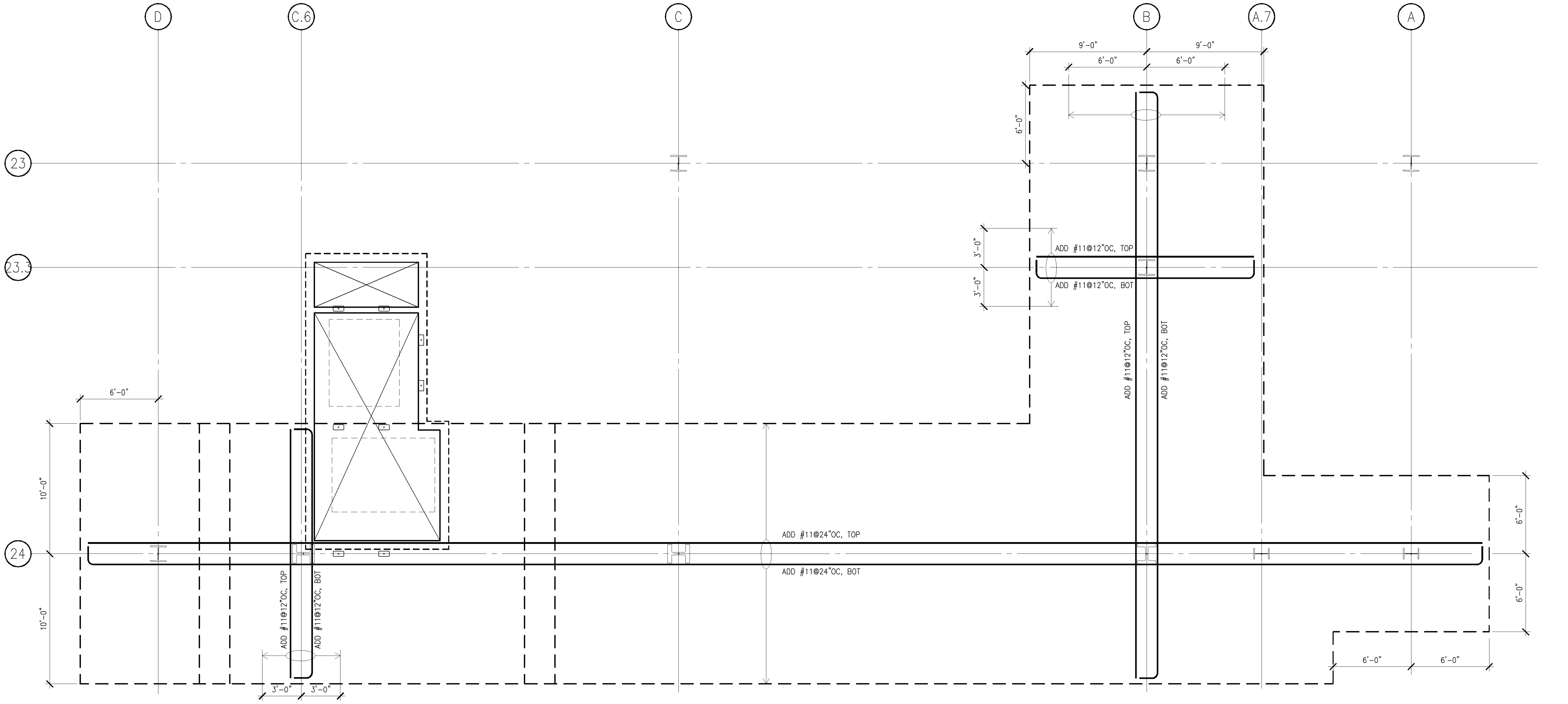


PARTIAL FOUNDATION PLAN - FOOTING TYPE 4A

$^{1}/_{4}$"=1'-0" 1

FOOTING NOTES:

1. FOOTING REINFORCEMENT IS #11@12" OC, TOP & BOTTOM IN BOTH DIRECTIONS WITH ADDED REINFORCEMENT AS SHOWN ON PLAN.
2. FOOTING DEPTH = 4'-0" TYPICAL U.N.O.
3. CONTRACTOR TO PROVIDE REBAR SPLICES PER 1/S4-102 AS REQUIRED. SPLICES SHALL BE LOCATED IN THE MIDDLE 1/3 OF THE DISTANCE BETWEEN COLUMNS & SHALL BE STAGGERED A MINIMUM DISTANCE EQUAL TO THE SPLICE LENGTH. CONTRACTOR SHALL PROVIDE SPLICE LOCATIONS TO ENGINEER FOR REVIEW & APPROVAL PRIOR TO CONSTRUCTION.



PARTIAL FOUNDATION PLAN - FOOTING TYPE 4B

$^{1}/_{4}$"=1'-0" 2

FOOTING NOTES:

1. FOOTING REINFORCEMENT IS #11@12" OC, TOP & BOTTOM IN BOTH DIRECTIONS WITH ADDED REINFORCEMENT AS SHOWN ON PLAN.
2. FOOTING DEPTH = 4'-0" TYPICAL U.N.O.
3. CONTRACTOR TO PROVIDE REBAR SPLICES PER 1/S4-102 AS REQUIRED. SPLICES SHALL BE LOCATED IN THE MIDDLE 1/3 OF THE DISTANCE BETWEEN COLUMNS & SHALL BE STAGGERED A MINIMUM DISTANCE EQUAL TO THE SPLICE LENGTH. CONTRACTOR SHALL PROVIDE SPLICE LOCATIONS TO ENGINEER FOR REVIEW & APPROVAL PRIOR TO CONSTRUCTION.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

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CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

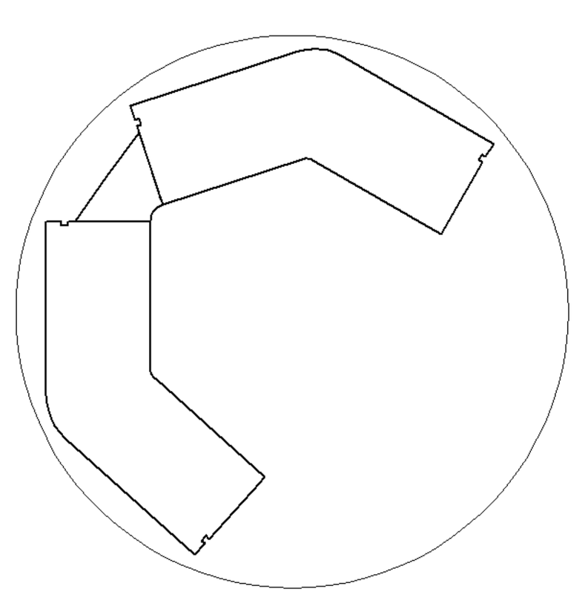
REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN



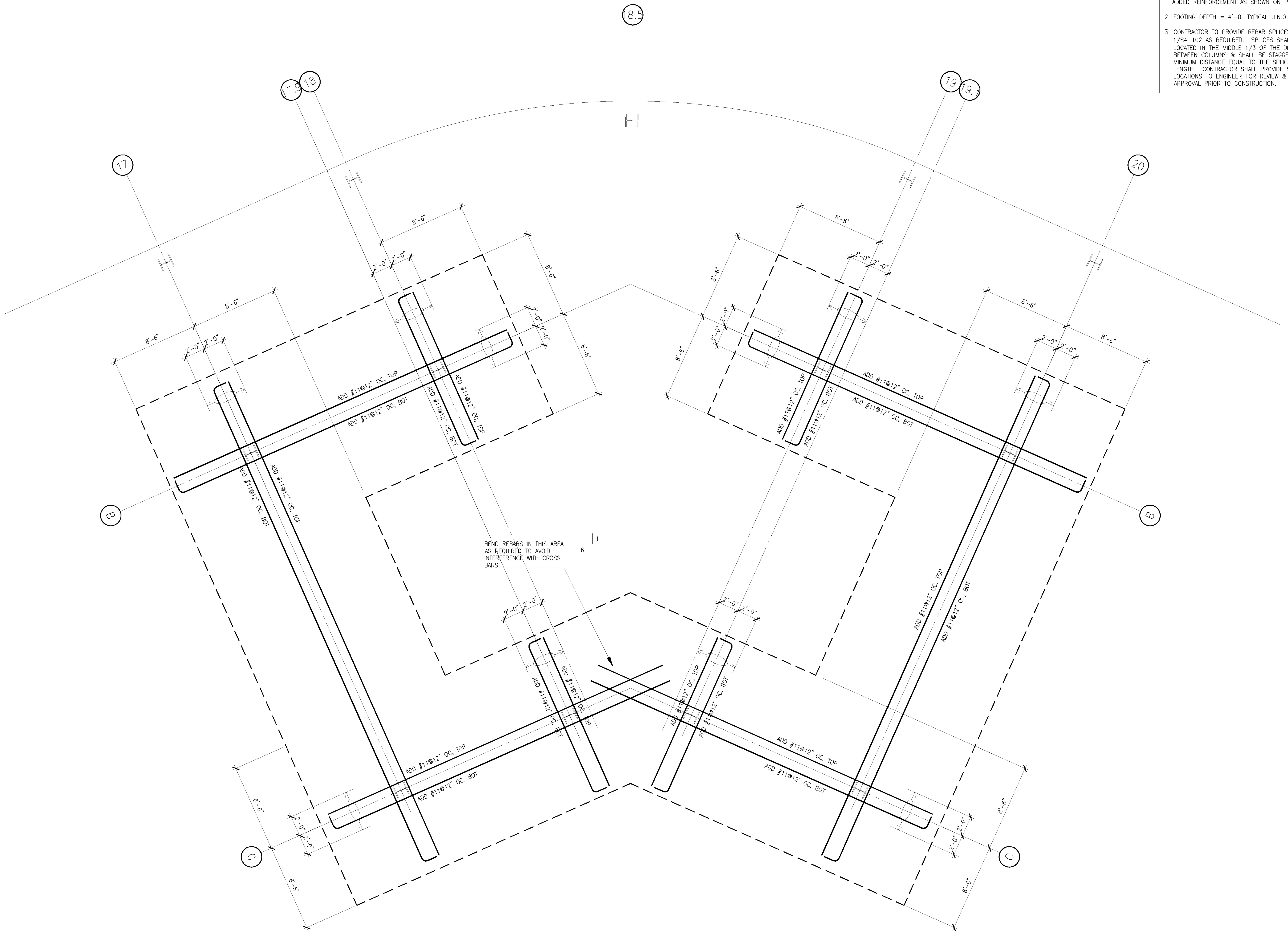
SHEET TITLE

PARTIAL FOUNDATION PLANS - FOOTING TYPE 4A & 4B

S4-106

FOOTING NOTES:

1. FOOTING REINFORCEMENT IS #11@12" OC, TOP & BOTTOM IN BOTH DIRECTIONS WITH ADDED REINFORCEMENT AS SHOWN ON PLAN.
2. FOOTING DEPTH = 4'-0" TYPICAL U.N.O.
3. CONTRACTOR TO PROVIDE REBAR SPLICES PER 1/S4-102 AS REQUIRED. SPLICES SHALL BE LOCATED IN THE MIDDLE 1/3 OF THE DISTANCE BETWEEN COLUMNS & SHALL BE STAGGERED A MINIMUM DISTANCE EQUAL TO THE SPLICE LENGTH. CONTRACTOR SHALL PROVIDE SPLICE LOCATIONS TO ENGINEER FOR REVIEW & APPROVAL PRIOR TO CONSTRUCTION.



PARTIAL FOUNDATION PLAN - TYPE 5

1/4"=1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
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CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

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LANDSCAPE ARCHITECT
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TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

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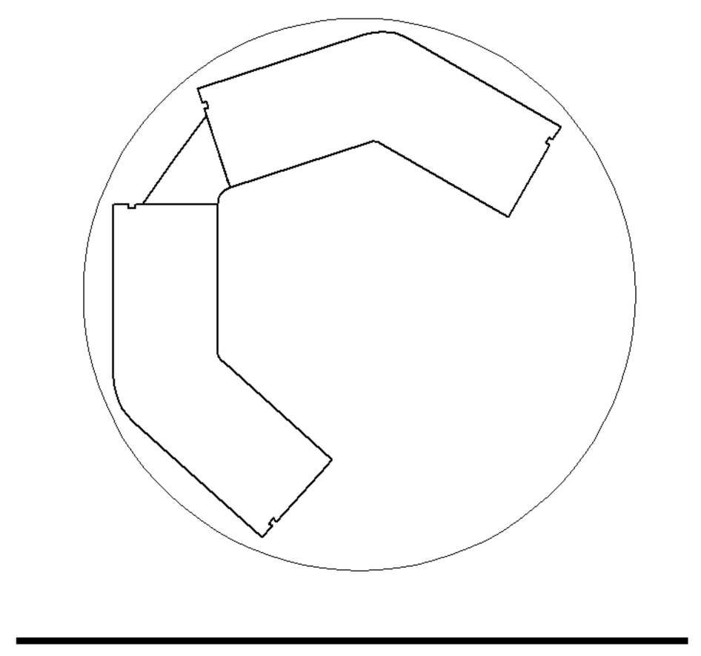
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

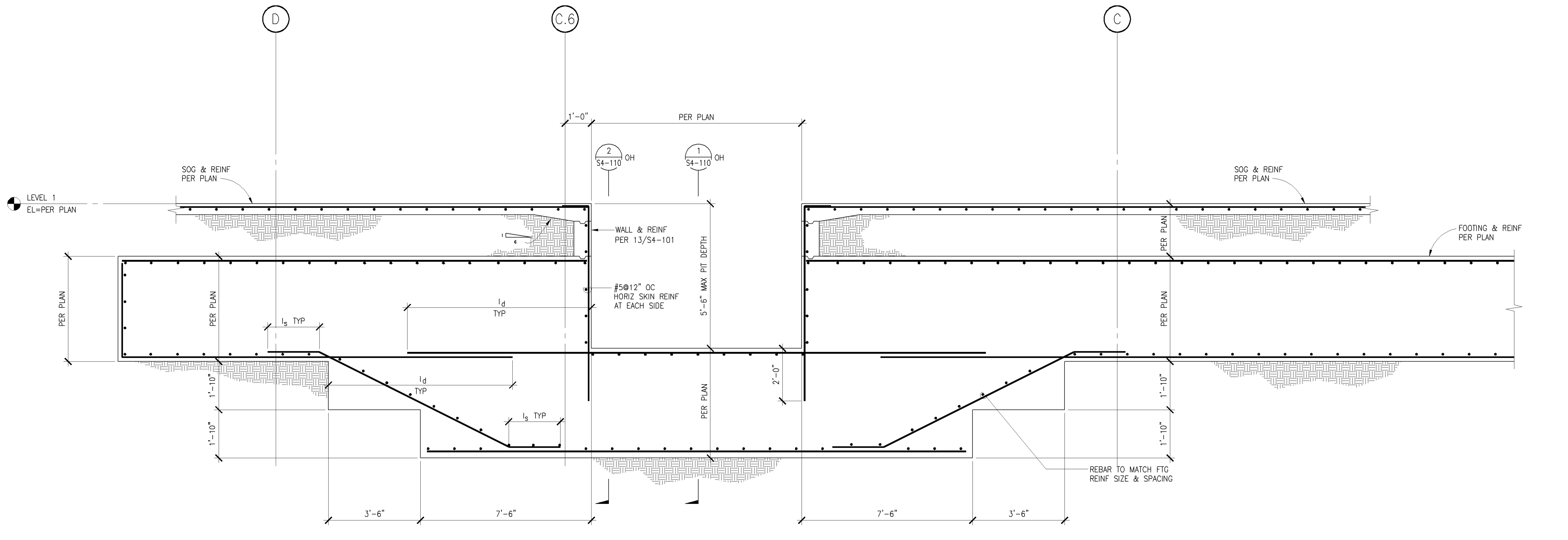
KEY PLAN



SHEET TITLE

PARTIAL FOUNDATION PLAN - FOOTING TYPE 5

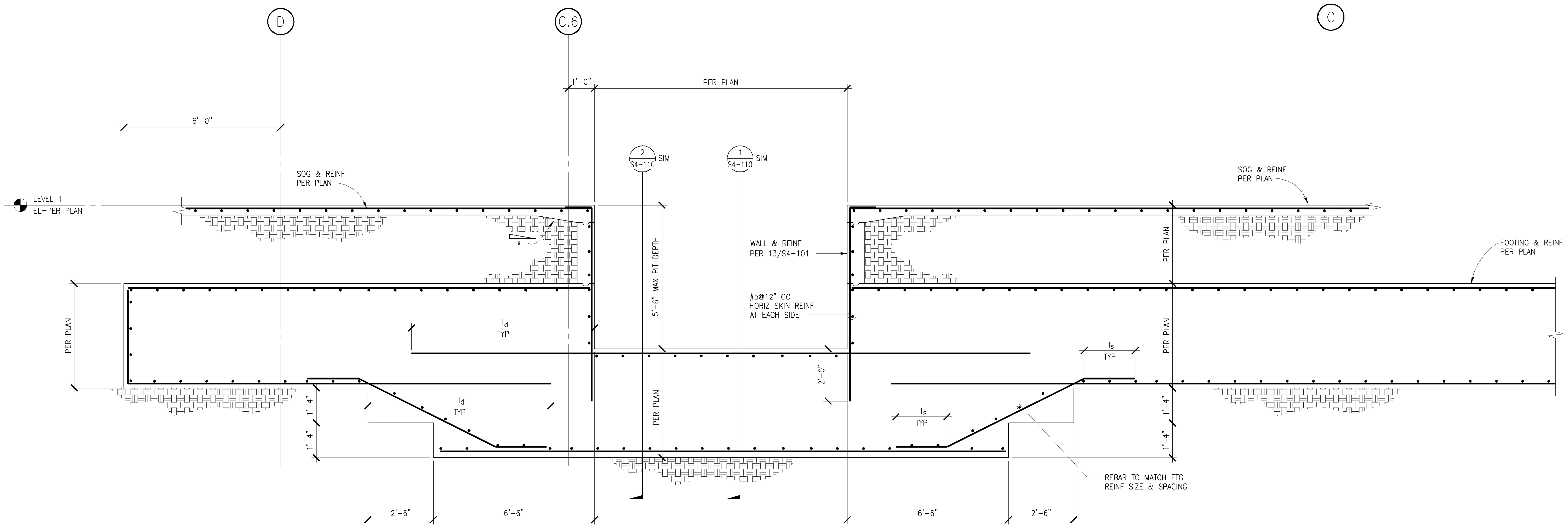
S4-107



NOTE:
1. ld & ls PER 1/S4-102.

SECTION THRU FOOTING TYPE 1A

SCALE: 1/2"=1'-0" 1



NOTE:
1. ld & ls PER 1/S4-102.

SECTION THRU FOOTING TYPE 1B

SCALE: 1/2"=1'-0" 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

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LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

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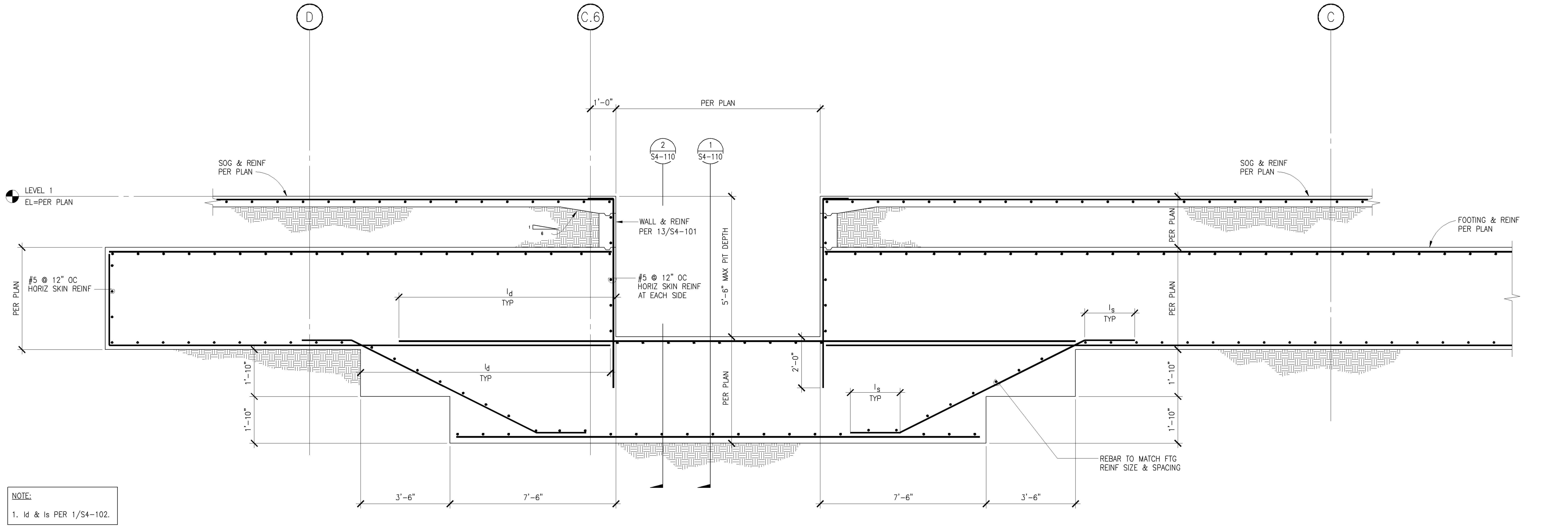
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE
FOUNDATION SECTIONS

S4-108

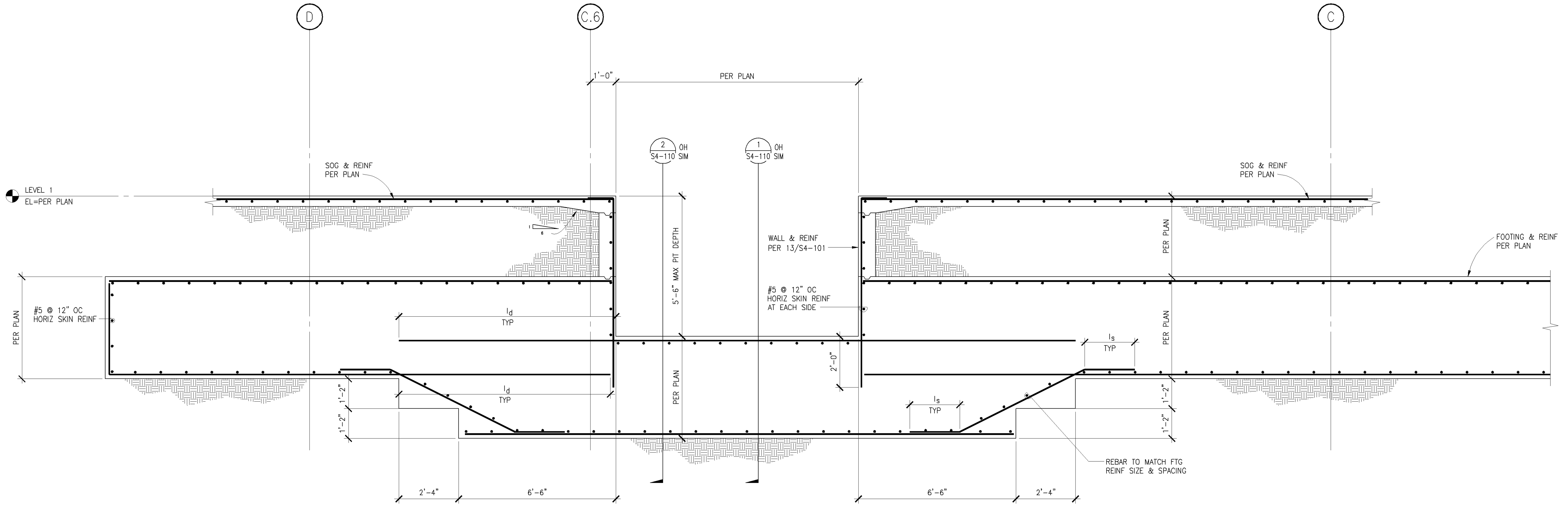


NOTE:

1. ld & ls PER 1/S4-102.

SECTION THRU FOOTING TYPE 4A

SCALE: 1/2"=1'-0" 1



NOTE:

1. ld & ls PER 1/S4-102.

SECTION THRU FOOTING TYPE 4B

SCALE: 1/2"=1'-0" 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

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LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER ALDRIN J. ... No. S4399 Exp. 12/31/07 STRUCTURAL STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
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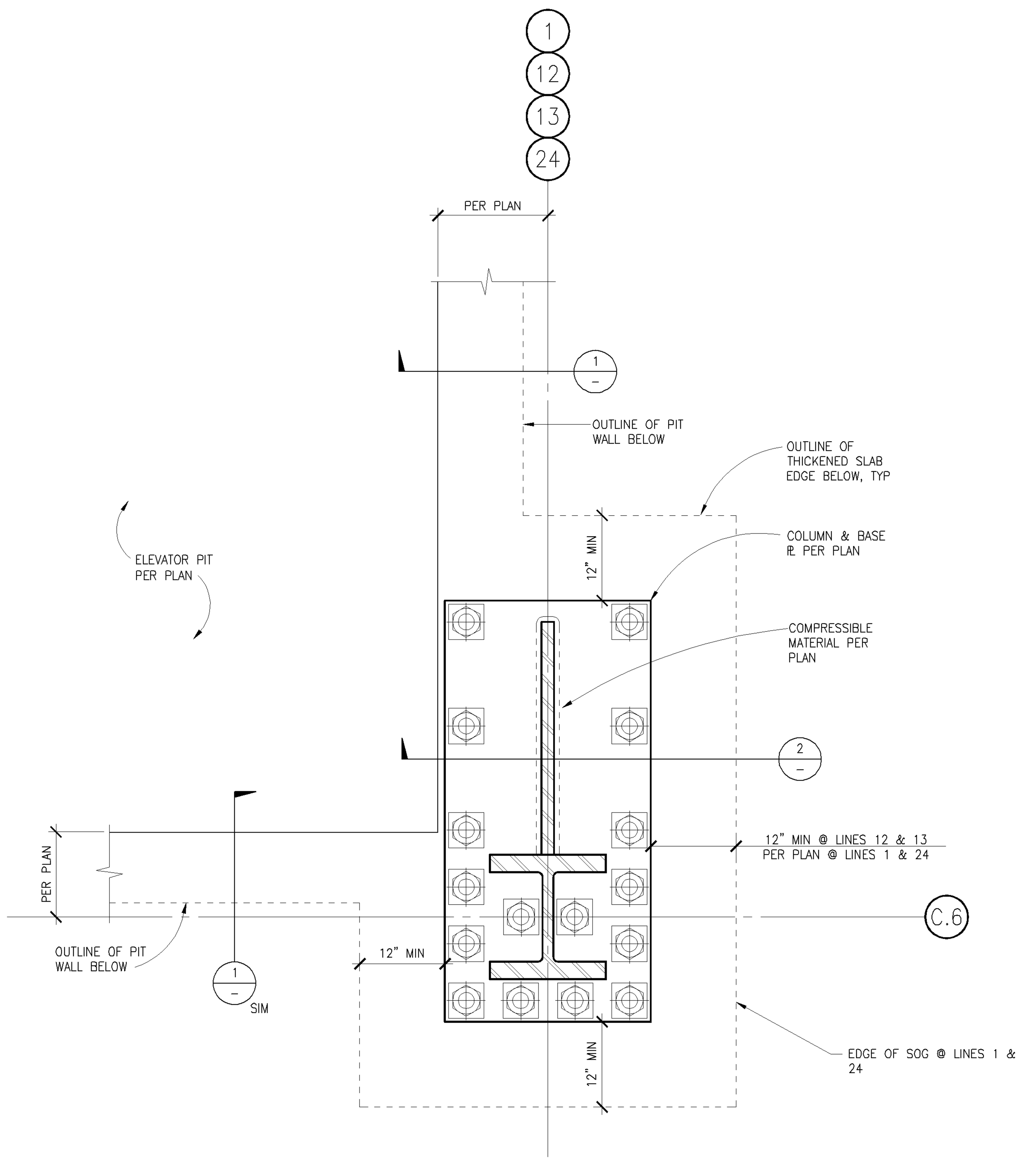
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

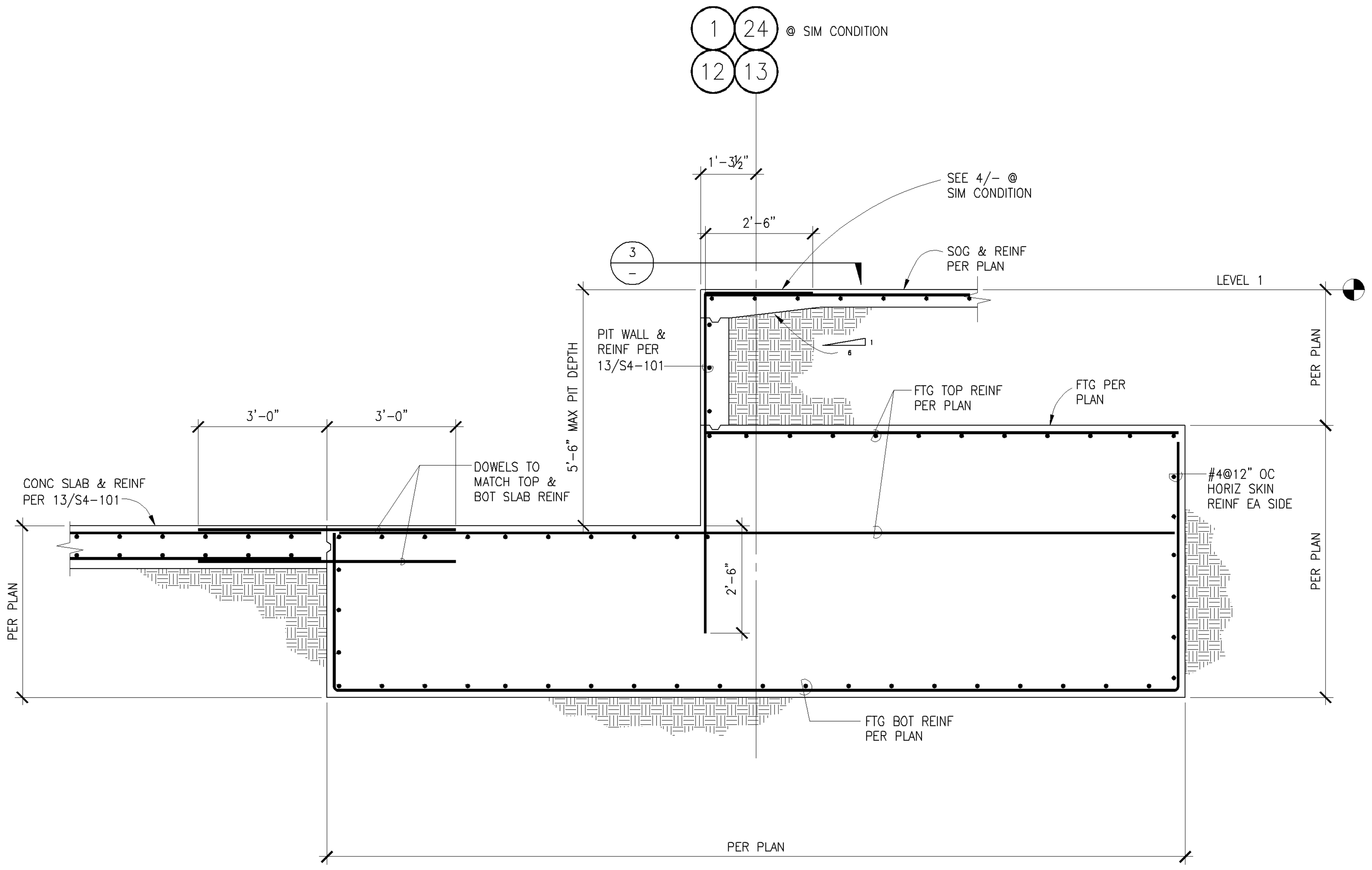
SHEET TITLE

FOUNDATION SECTIONS

S4-109

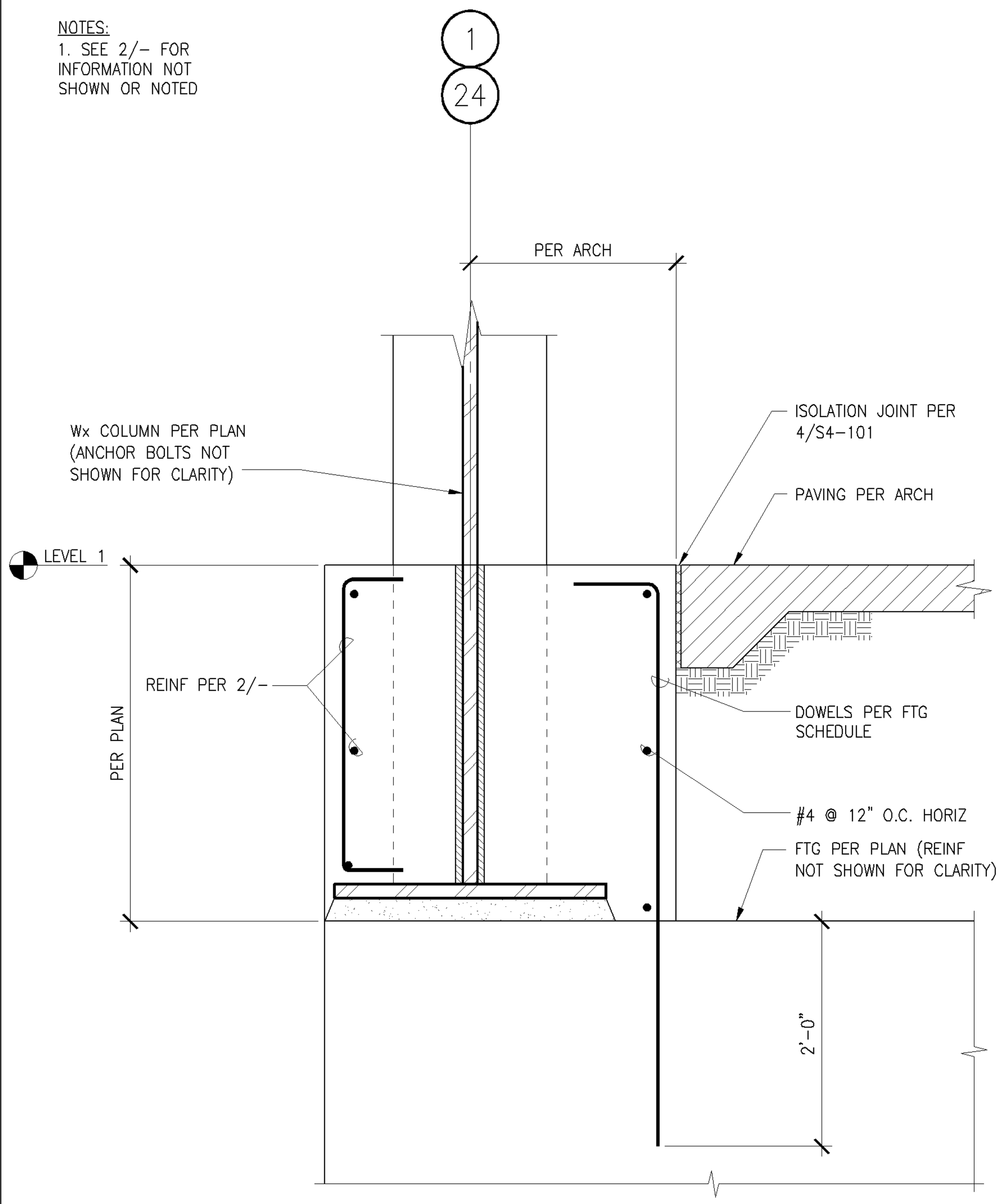


PLAN - DETAIL SCALE: 1"=1'-0" 3



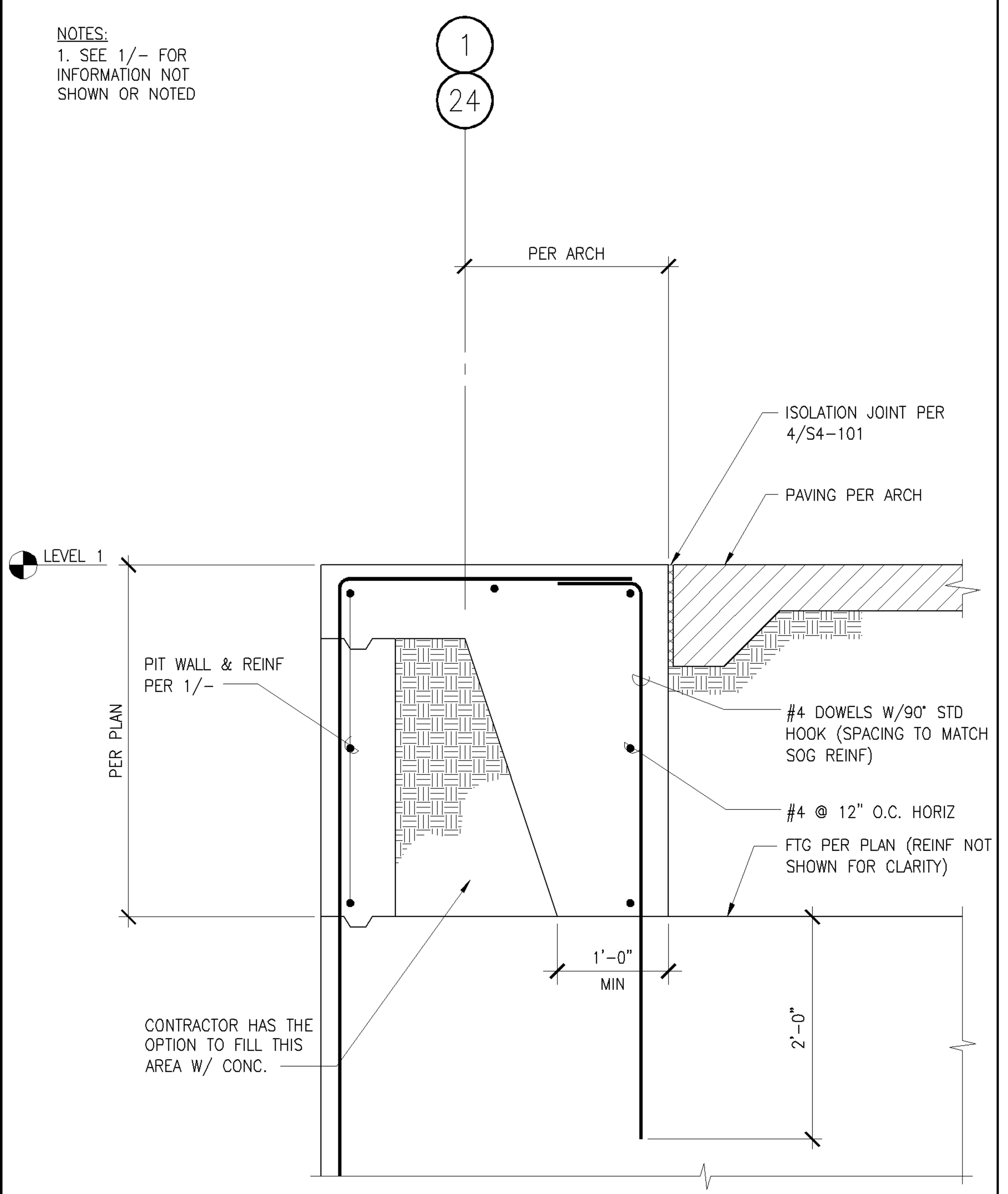
SECTION THRU FOOTING TYPES 4A AND 4B SCALE: ½"=1'-0" 1

NOTES:
1. SEE 2/- FOR INFORMATION NOT SHOWN OR NOTED



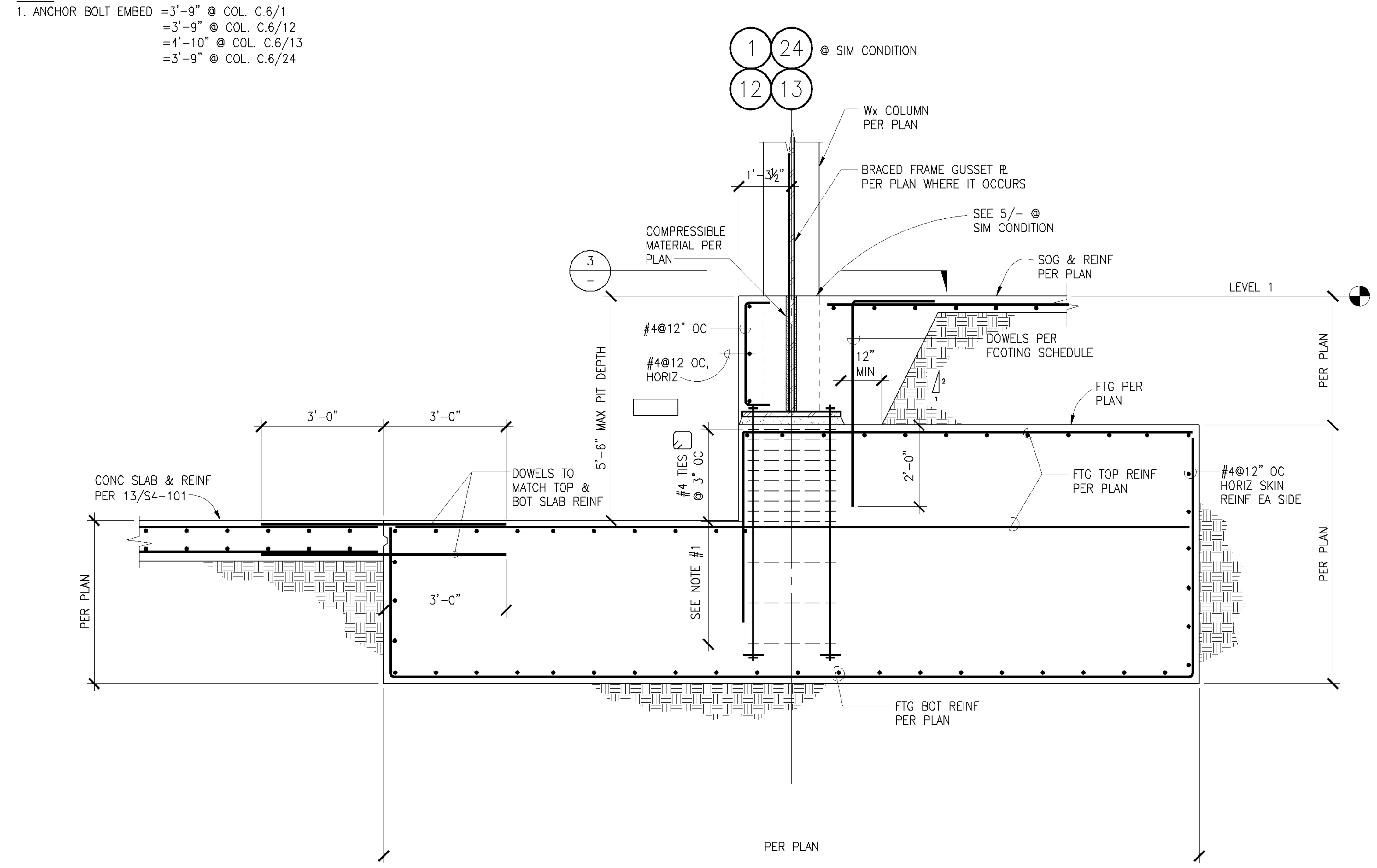
DETAIL SCALE: 1"=1'-0" 5

NOTES:
1. SEE 1/- FOR INFORMATION NOT SHOWN OR NOTED



DETAIL SCALE: 1"=1'-0" 4

NOTES:
1. ANCHOR BOLT EMBED =3'-9" @ COL. C.6/1
=3'-9" @ COL. C.6/12
=4'-10" @ COL. C.6/13
=3'-9" @ COL. C.6/24



SECTION THRU FOOTING TYPE 4A AND 4B SCALE: ½"=1'-0" 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER — ALDRIN J. DUE — No. S4399 — Exp. 12/31/07 — STRUCTURAL — STATE OF CALIFORNIA

ISSUE

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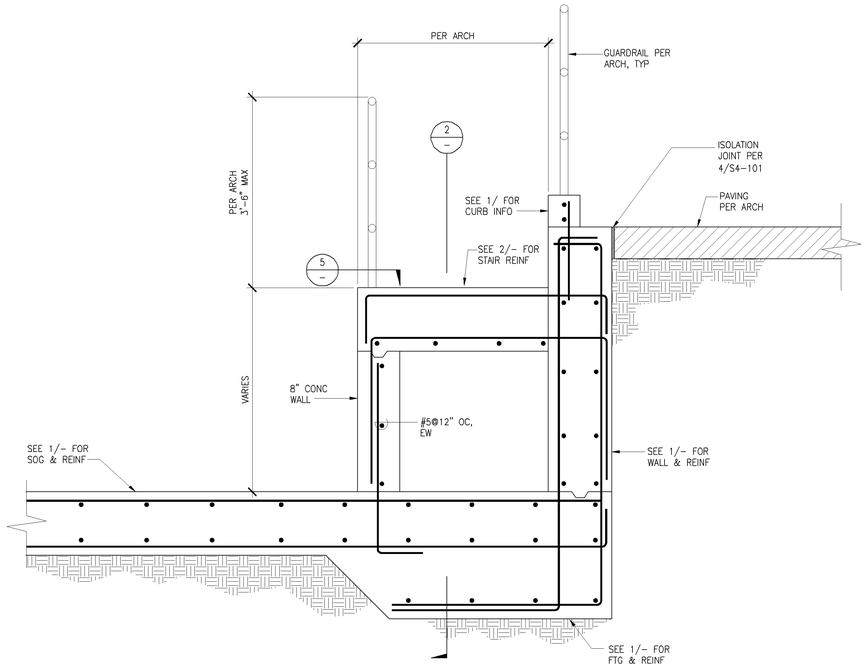
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

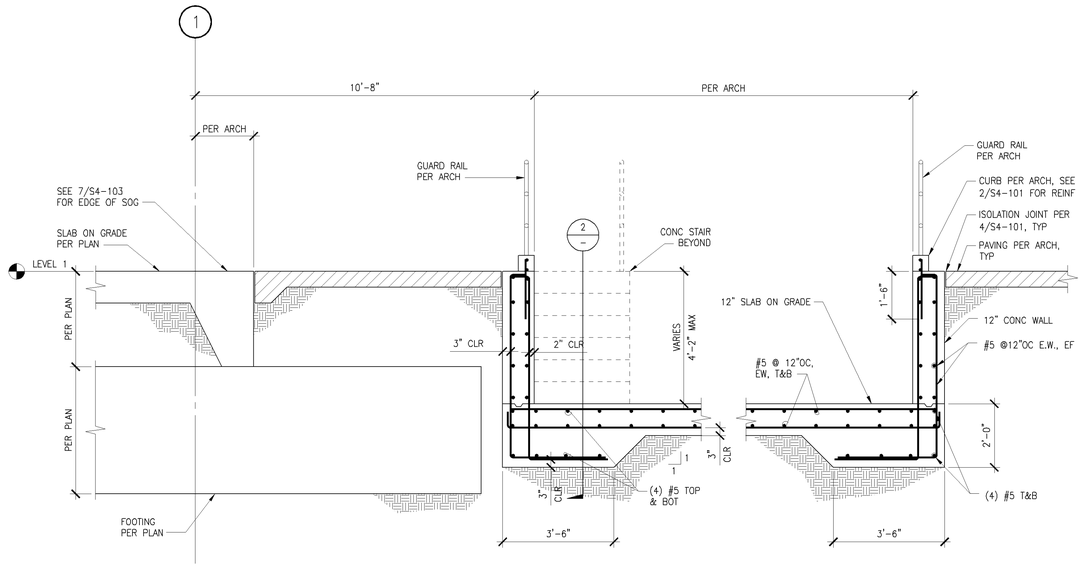
SHEET TITLE
FOUNDATION SECTIONS AND DETAILS

S4-110



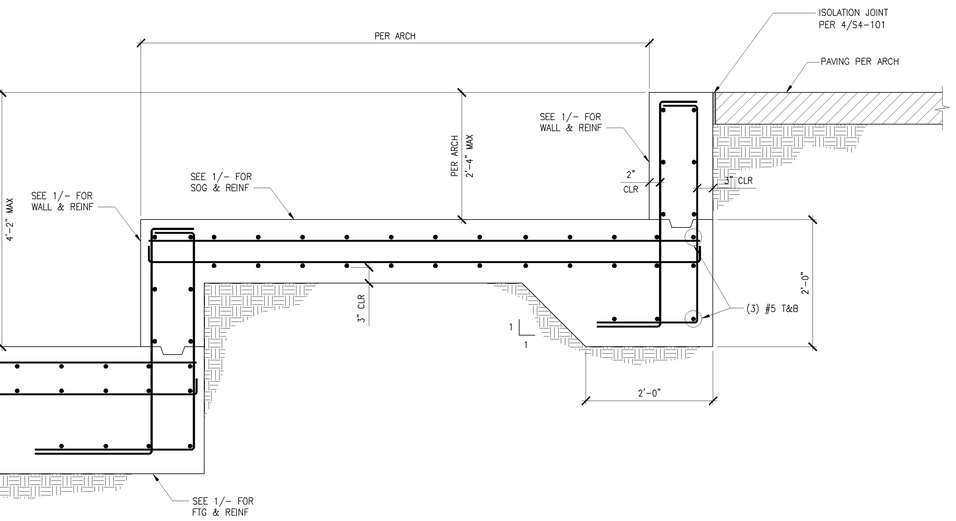
SECTION AT CONCRETE STAIR

SCALE: 1"=1'-0" | 3



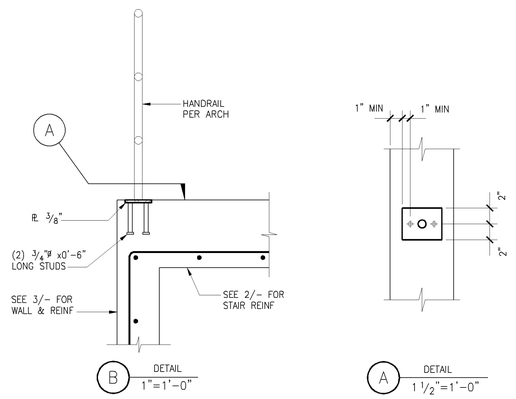
SECTION THRU LOADING DOCK

SCALE: 1/2"=1'-0" | 1



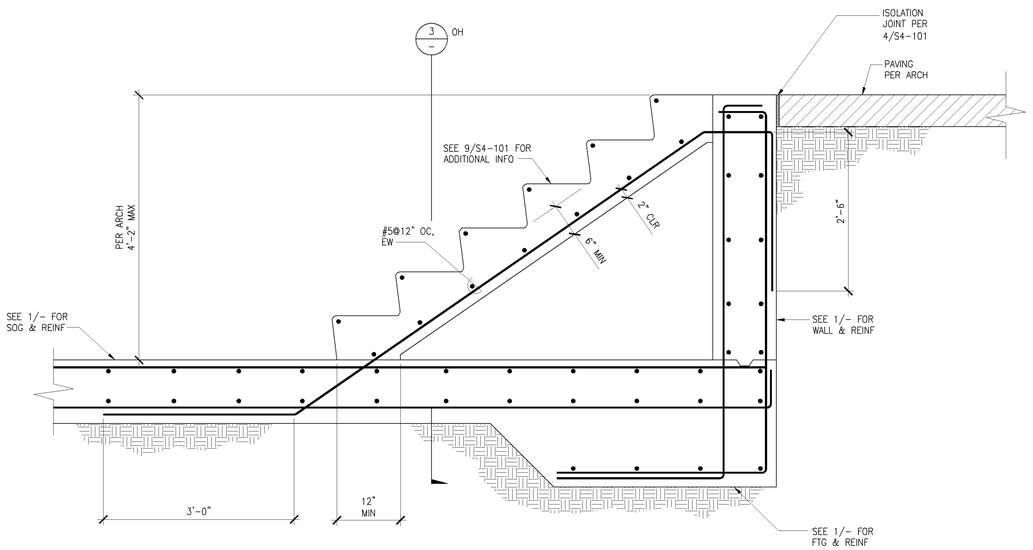
SECTION AT DOCK LEVELER

SCALE: 1"=1'-0" | 4



HANDRAIL BASEPLATE AT CONCRETE STAIRS

SCALE: AS NOTED | 5



SECTION AT CONCRETE STAIR

SCALE: 1"=1'-0" | 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3774
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER ADRIN V. DAVE No. S4399 Exp. 12/31/07 STRUCTURAL STATE OF CALIFORNIA

ISSUE

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PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

CONCRETE SECTIONS AND DETAILS

S4-111

File: W:\2005\1061030 - Guidant Corporate Campus, Temecula, CA\Sdraft\1061030_S4_111.dwg
Plotted: 03/22/07 @ 1:39pm By: SGutierrez
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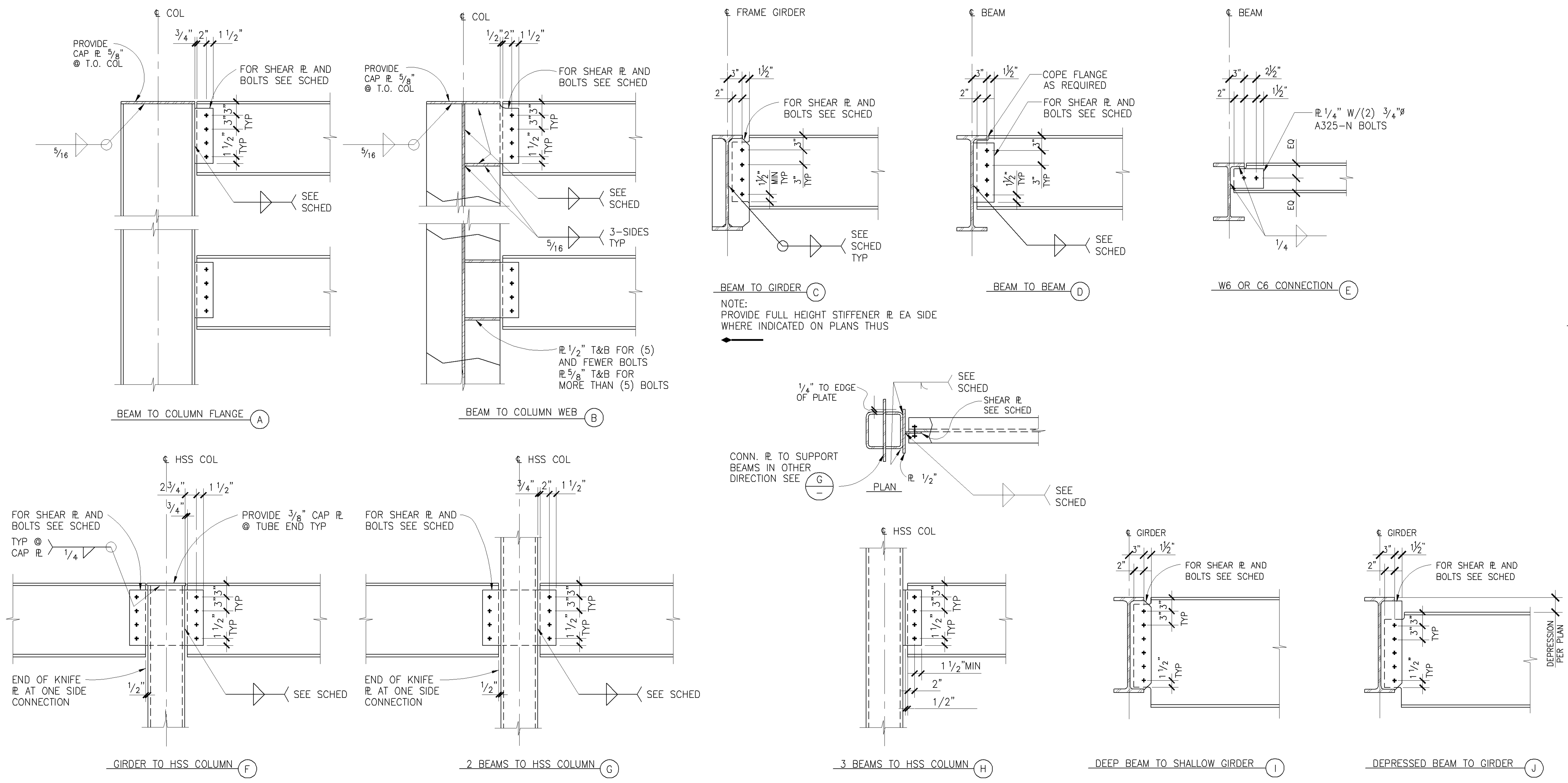
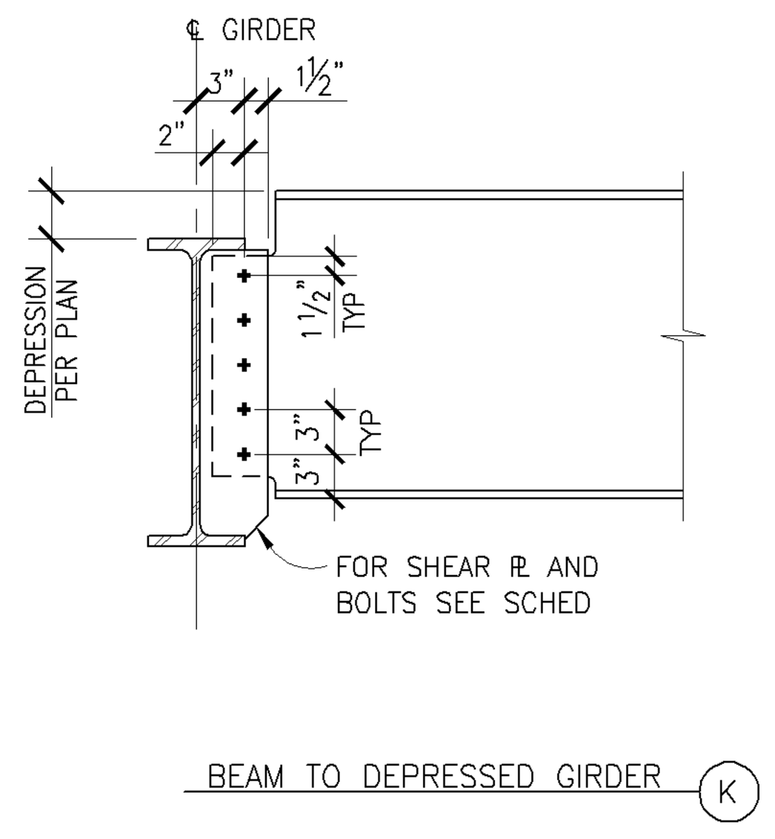


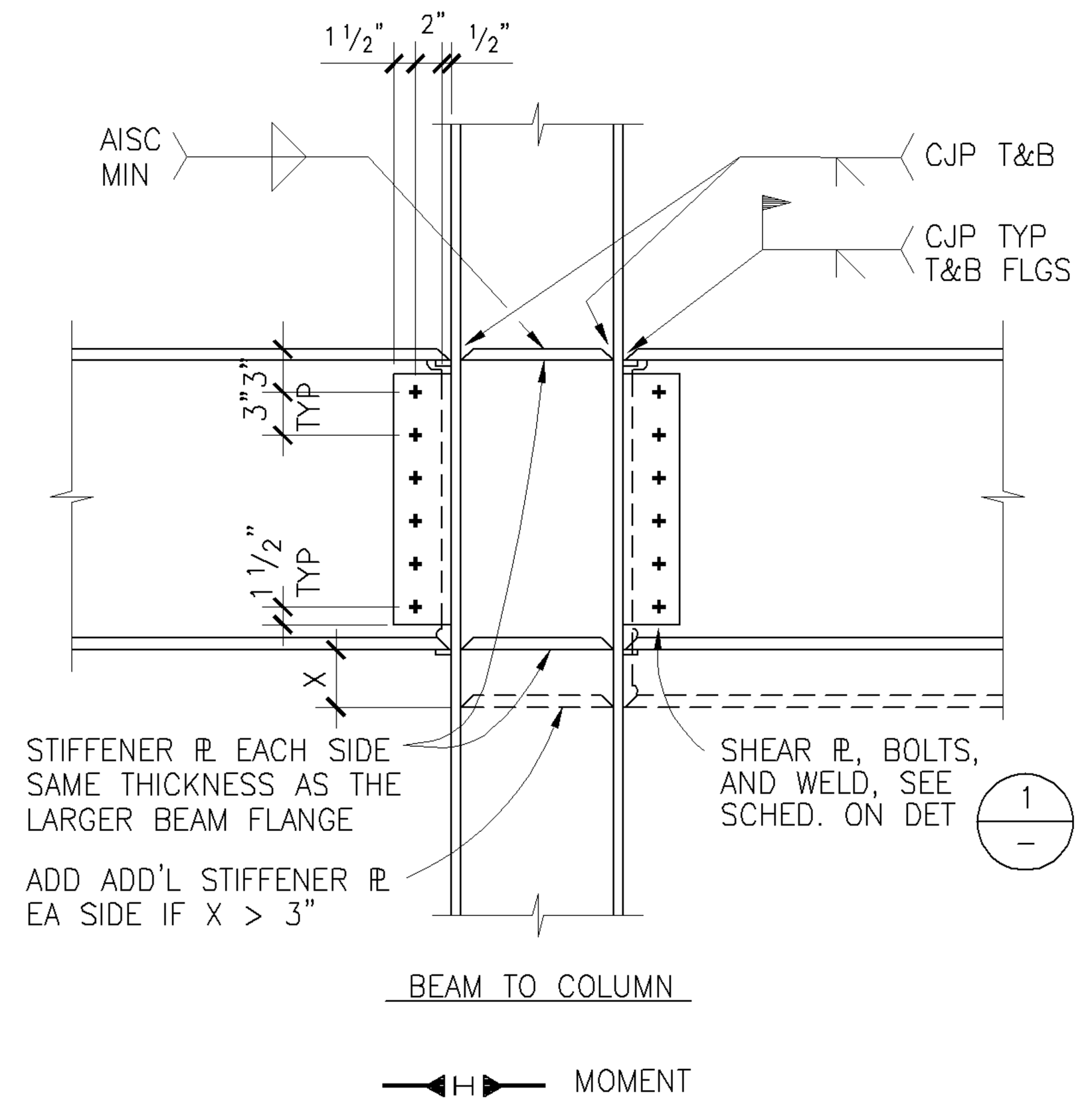


STANDARD BOLTED CONNECTION SCHEDULE				
BEAM SIZE	NO.& SIZE OF BOLTS REQUIRED	PLATE THICKNESS	WELD SIZE	CONN. CAPACITY (SING.SHEAR)
W8 C8	(2) 7/8"Ø	1/4"	1/4"	11.1K
W10 C10	(2) 7/8"Ø	1/4"	1/4"	11.1K
W12 C12	(3) 7/8"Ø	1/4"	1/4"	22.1K
W14	(3) 7/8"Ø	1/4"	1/4"	22.1K
W16	(4) 7/8"Ø	3/8"	1/4"	35.4K
W18	(5) 7/8"Ø	3/8"	1/4"	49.1K
W21	(6) 7/8"Ø	3/8"	1/4"	62.7K
W24	(7) 7/8"Ø	3/8"	5/16"	76.4K
W27	(7) 7/8"Ø	3/8"	5/16"	76.4K
W30	(8) 7/8"Ø*	1/2"	3/8"	83.9K
W33	(9) 7/8"Ø*	1/2"	3/8"	91.2K
W36	(9) 1 1/8"Ø*	1/2"	3/8"	176K

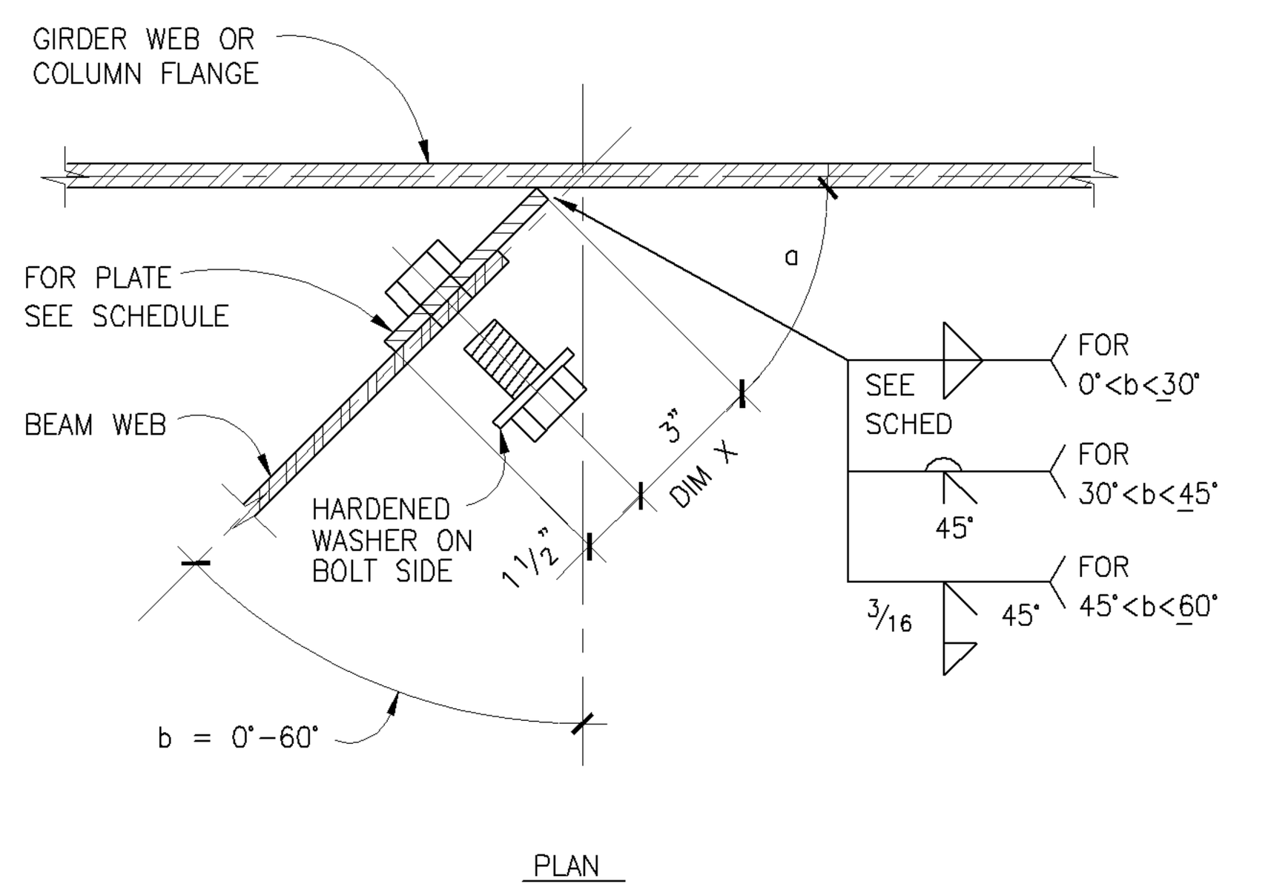
CONNECTION NOTES:

1. ALL BOLTS TO BE ASTM A325-N AND FULLY PRETENSIONED PER AISC STANDARDS.
2. CONNECTION PLATES TO HAVE AISC STANDARD ROUND HOLES UNLESS NOTED OTHERWISE.
3. ALL CONNECTION PLATE MATERIAL TO BE A36.
4. * INDICATES USE HORIZONTAL SHORT SLOTTED HOLES IN THE PLATE AT COLUMN CONNECTION ONLY.
5. [#] SHOWN ON PLANS INDICATES NUMBER OF BOLTS REQUIRED. IF NOT SHOWN ON PLANS USE NUMBER OF BOLTS SHOWN ON SCHEDULE.
6. PROVIDE 1"Ø BOLTS WHERE INDICATED ON PLANS THUS

STANDARD BOLTED BEAM CONNECTION DETAILS 1"=1'-0" 1



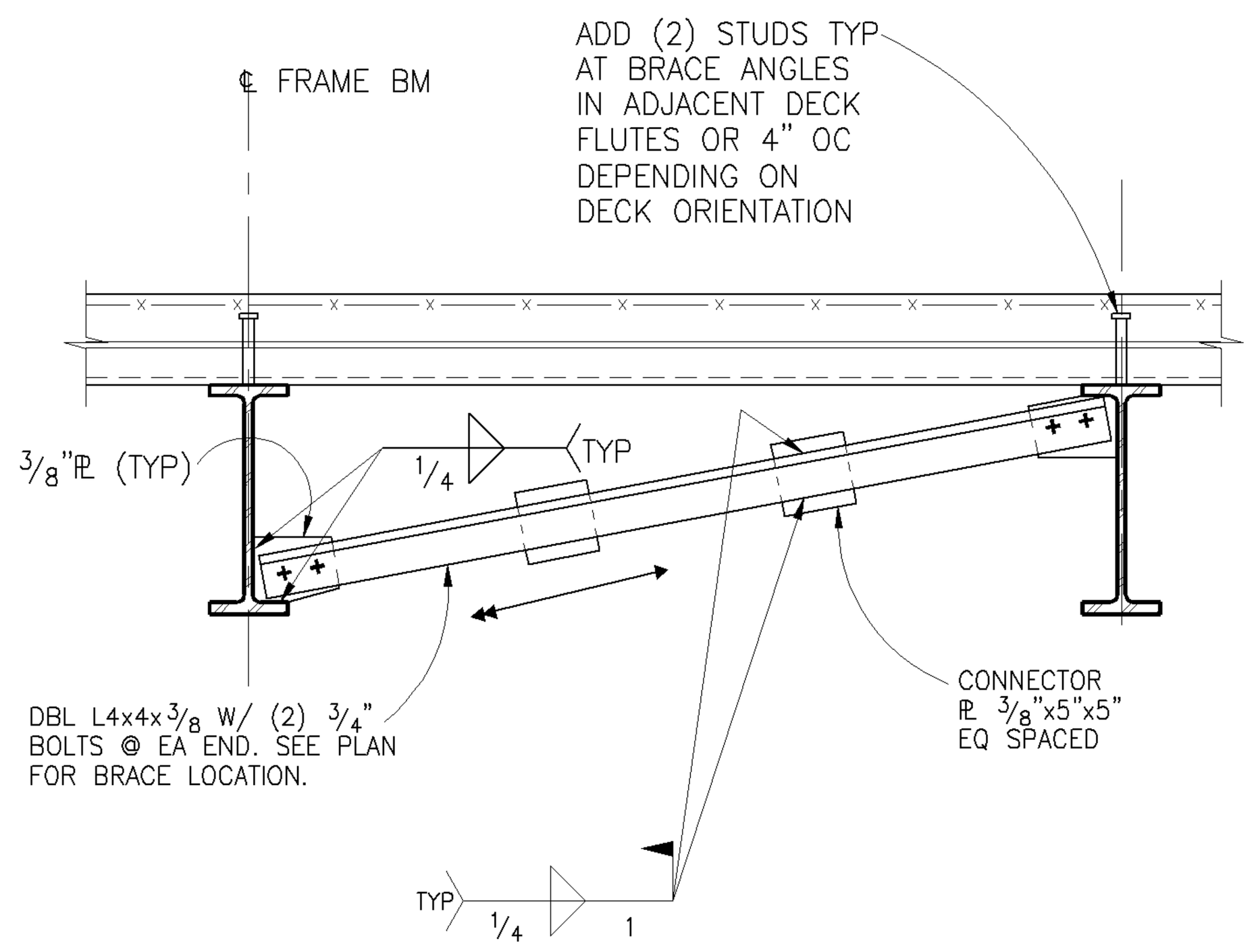
BEAM TO COLUMN FLANGE MOMENT CONNECTION 1"=1'-0" 7



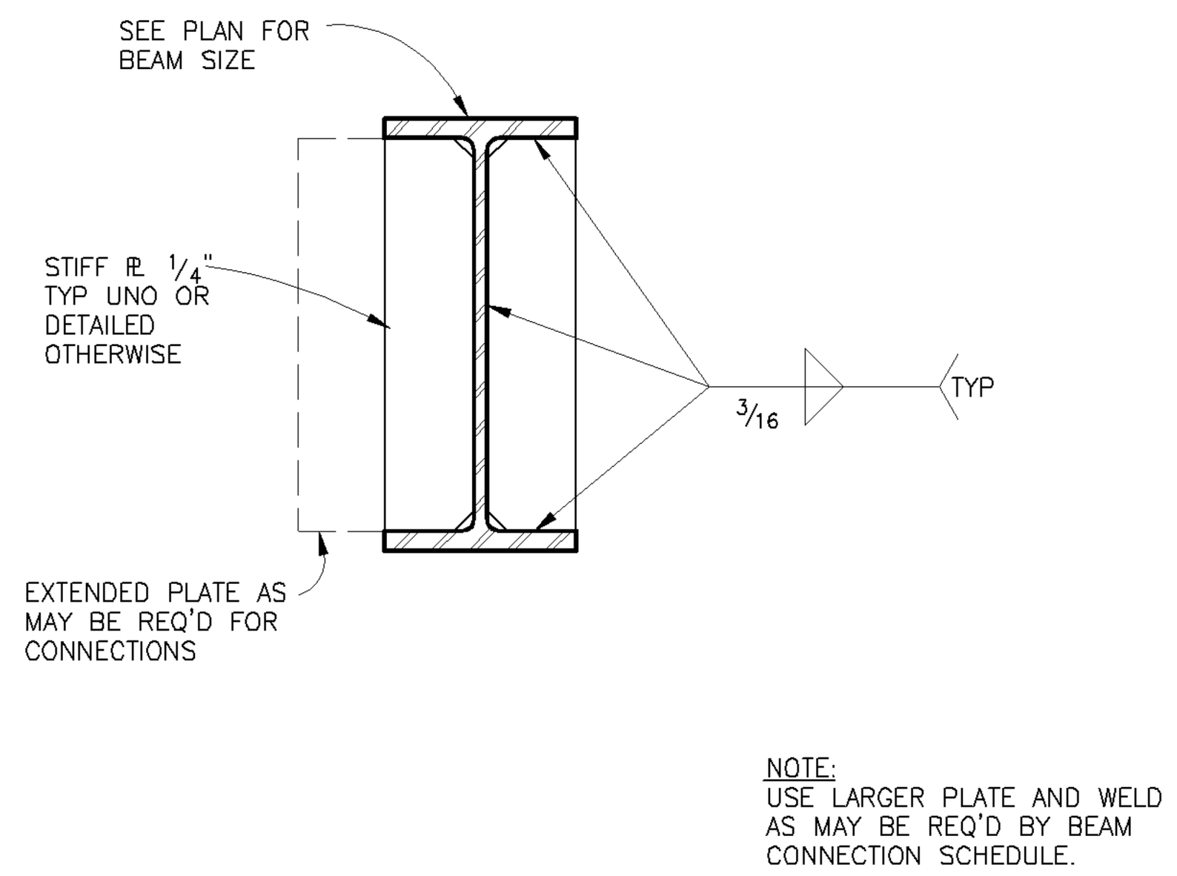
SKEWED BEAM CONNECTION SCHEDULE					
BEAM SIZE	NO.& SIZE OF BOLTS REQUIRED	PLATE THICKNESS	CONN CAPACITY (SING.SHEAR)	WELD FOR 0°<b<10°	WELD FOR 10°<b<30°
W8	(2) 7/8"Ø	3/8"	11.1K	S / S+1/16	S / S+1/8
W10	(2) 7/8"Ø W/ 4"GA	3/8"	13.7K		
W12	(3) 7/8"Ø	3/8"	22.1K		
W14	(3) 7/8"Ø	3/8"	22.1K		
W16	(4) 7/8"Ø	3/8"	35.4K		
W18	(4) 7/8"Ø	3/8"	35.4K		
W21	(5) 7/8"Ø	3/8"	49.1K		
W24	(6) 7/8"Ø	3/8"	62.7K		45°
W27	(7) 7/8"Ø	3/8"	76.4K		
W30	(8) 7/8"Ø	1/2"	83.9K		
W33	(9) 7/8"Ø*	1/2"	91.2K	S / S+1/8	
W36	(9) 7/8"Ø*	1/2"	91.2K		

NOTES:
1. FILLET WELD SIZE S = PLATE THICKNESS.
2. FOR BEVEL WELDS, BEVEL END OF CONNECTION PLATE TO MAINTAIN a AT 45°.
3. DO NOT INCREASE DIM X WITHOUT ENGINEER'S APPROVAL.
4. * USE HORIZ SHORT SLOTTED HOLES IN PLATE AT COL CONN ONLY.
5. SEE DET 1/- FOR ADDITIONAL NOTES.

SKEWED BEAM CONN (FOR SKEW ANGLE=0°<b<60°) 1"=1'-0" 6

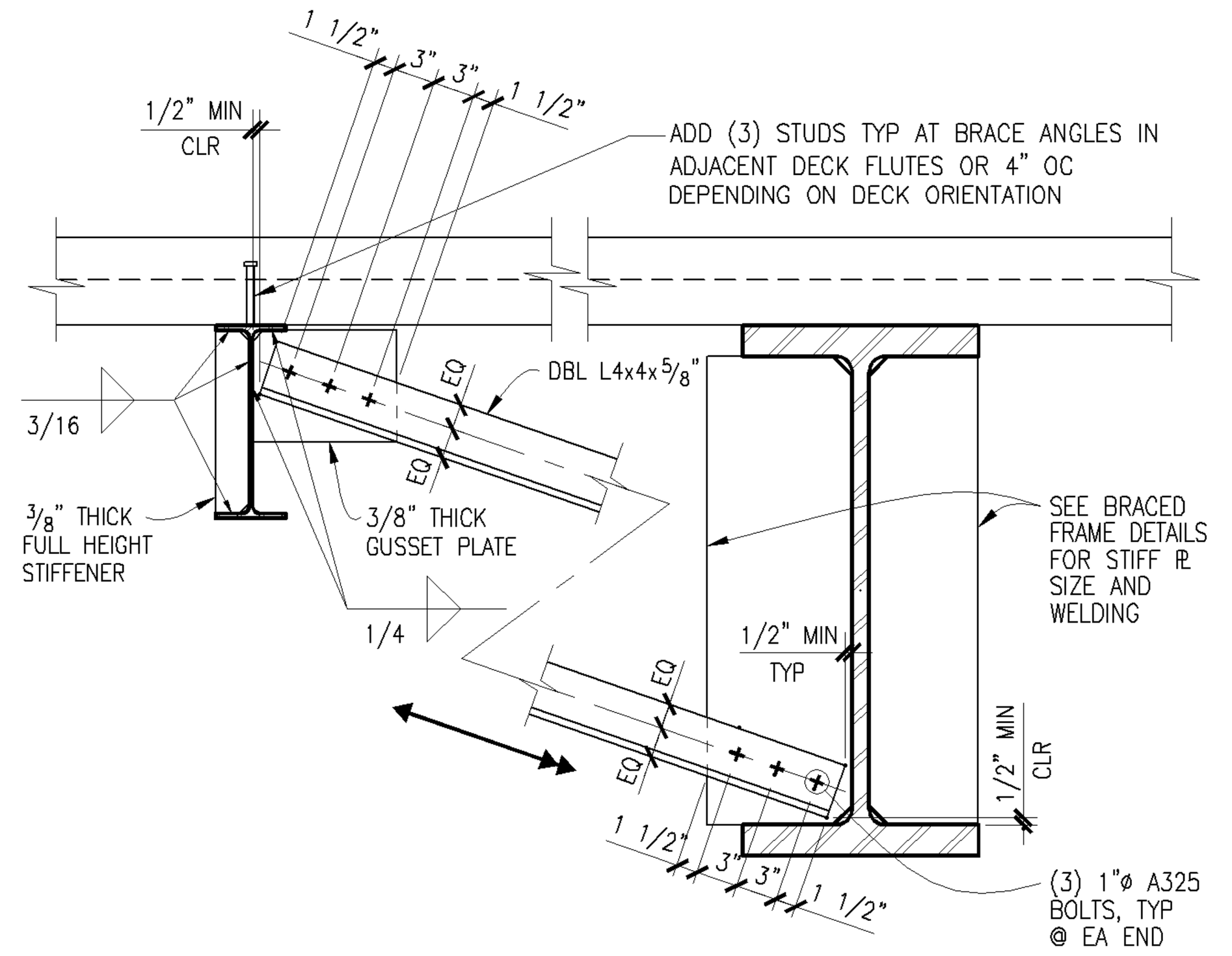


LATERAL BRACE DETAIL 1"=1'-0" 4

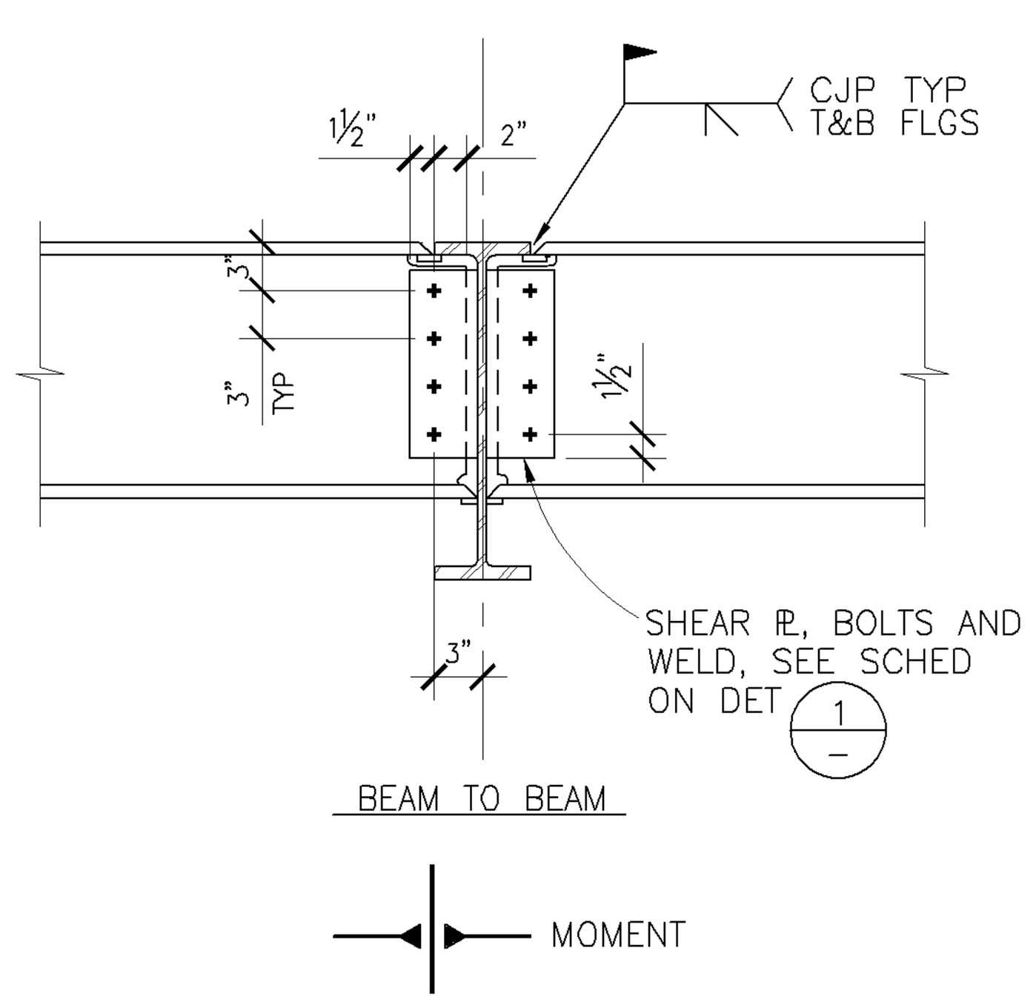


NOTE:
USE LARGER PLATE AND WELD AS MAY BE REQ'D BY BEAM CONNECTION SCHEDULE.

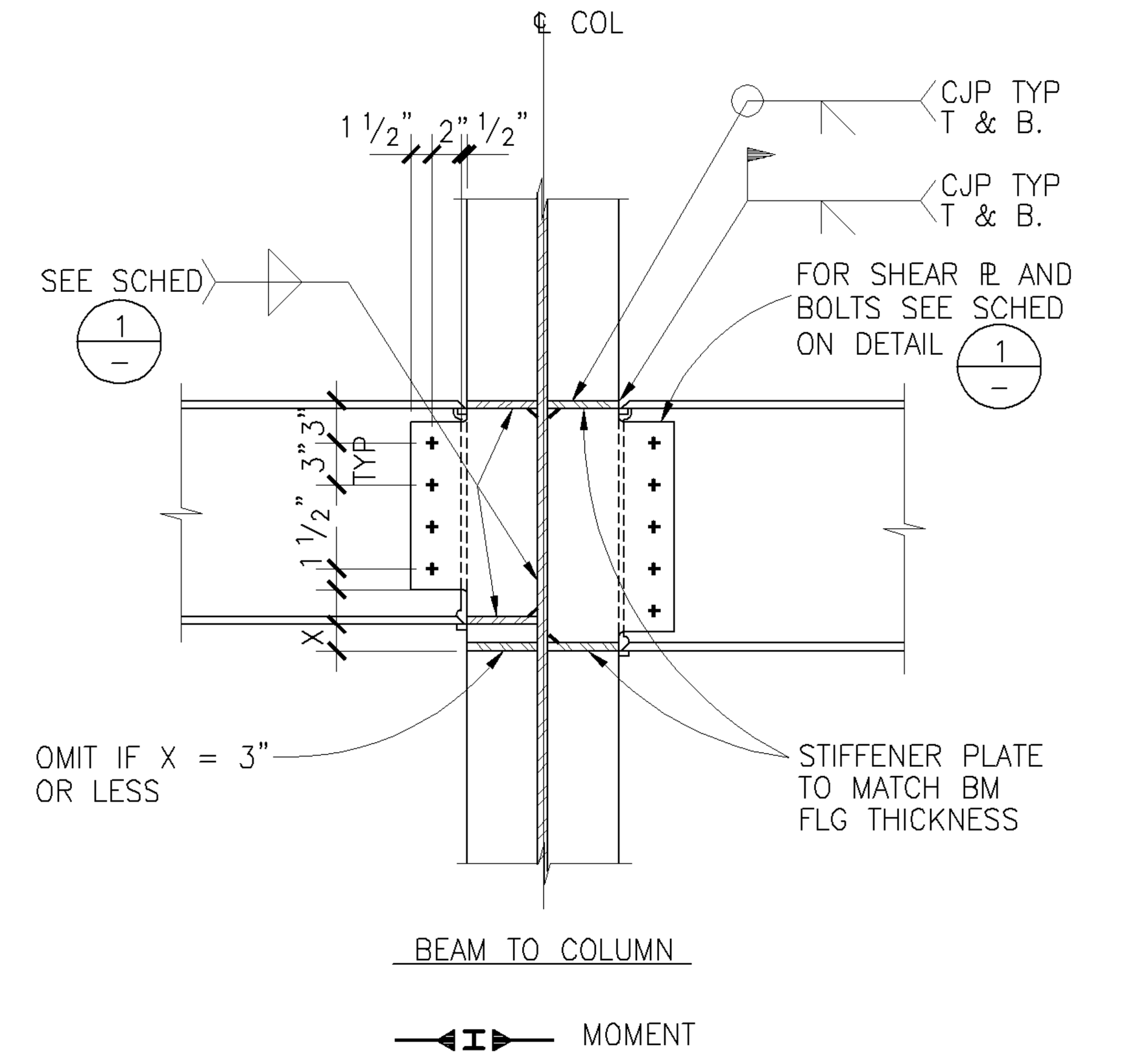
TYPICAL STIFFENER PLATES 1"=1'-0" 2



LATERAL BRACE AT CHEVRON BRACE INTERSECTION 1"=1'-0" 8



BM TO BM MOMENT CONNECTION 1"=1'-0" 5



BM TO COLUMN WEB MOMENT CONNECTION 1"=1'-0" 3

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
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CIVIL ENGINEER
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1	10-23-06	BLDG & SAFETY SUBMITTAL

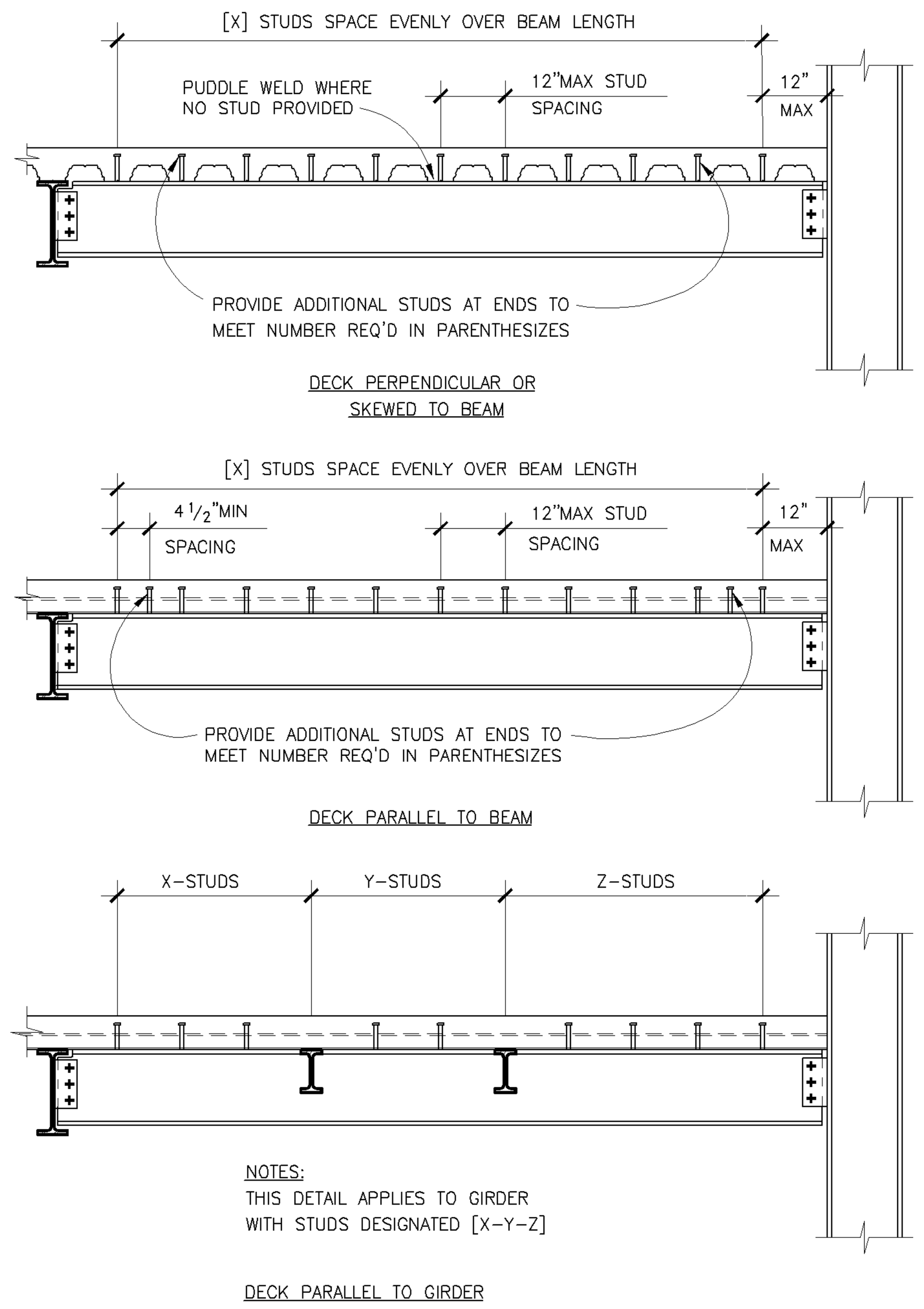
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

TYPICAL STEEL DETAILS

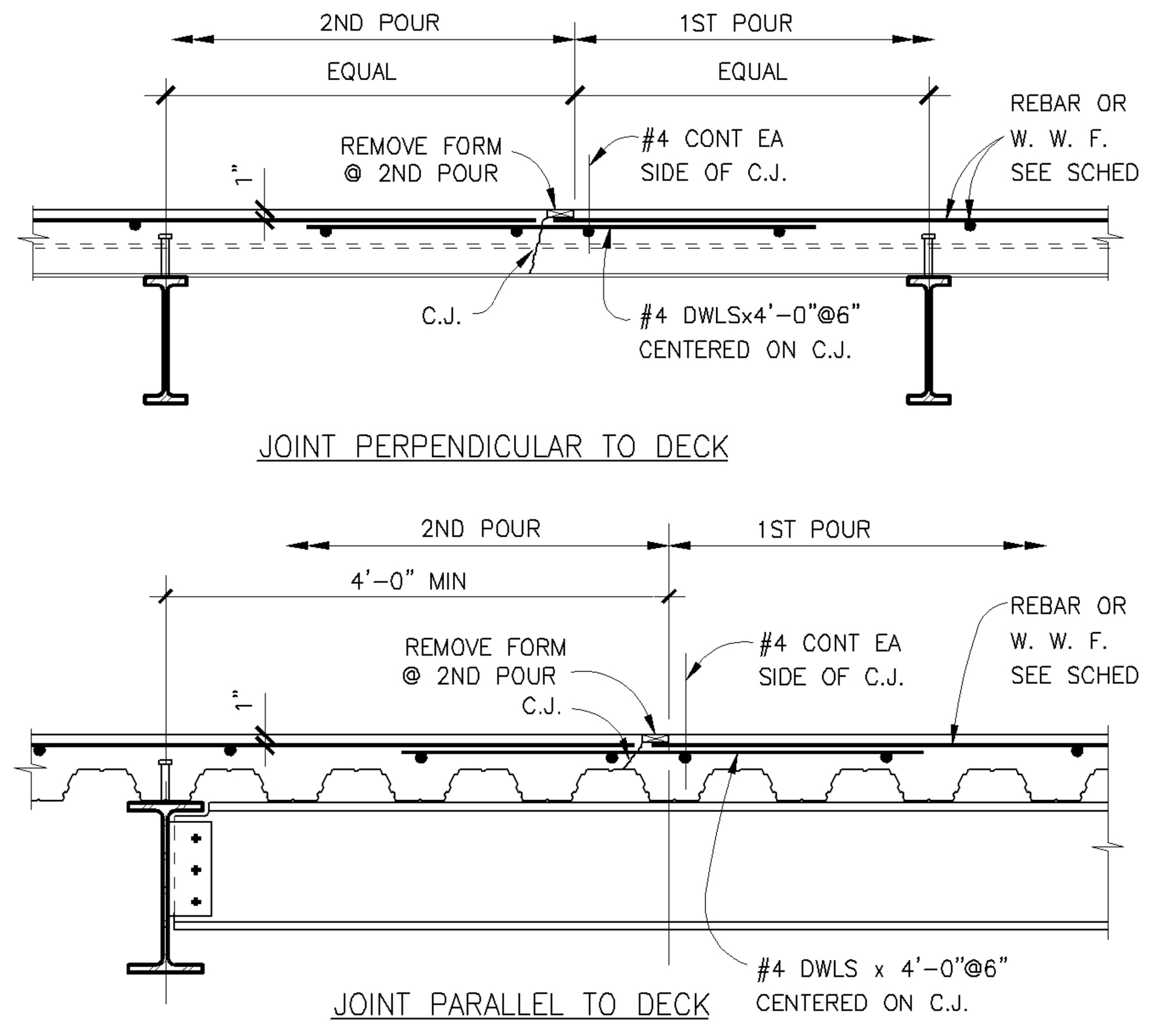
S5-101



NOTES:

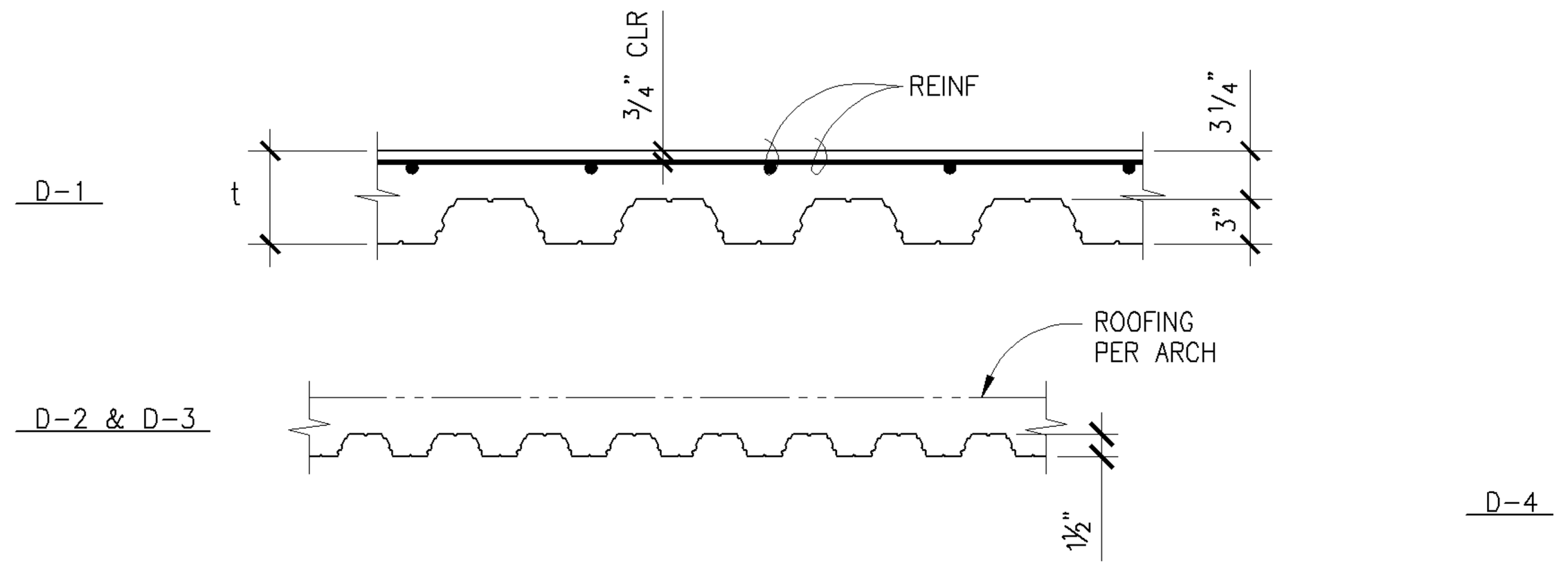
1. THE MINIMUM NUMBER OF STUDS REQUIRED IS SHOWN AS [X] ON FRAMING PLANS OR 12"O.C. MAXIMUM SPACING IF NOT CALLED OUT ON PLANS.
2. IF TWO STUDS ARE REQUIRED IN ONE FLUTE THE TRANSVERSE SPACING SHALL BE 3" MINIMUM. SEE DET 5/-.
3. UNLESS NOTED [0] ALL BEAMS AND GIRDERS THAT SUPPORT STRUCTURAL CONCRETE SHALL HAVE 3/4"ØX5" LONG HEADED SHEAR STUDS @ 12" O.C. (MAX.)

COMPOSITE BEAM STUD SPACING 1"=1'-0" 10



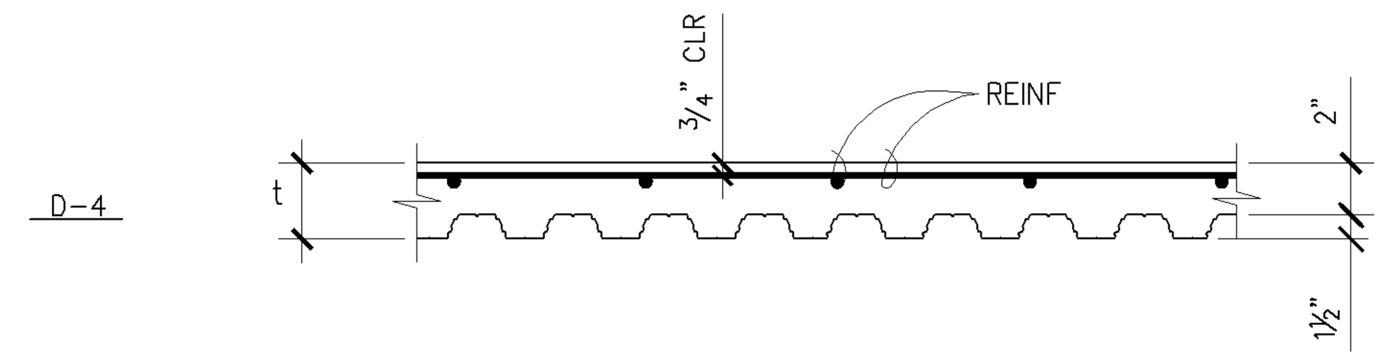
CONSTRUCTION JOINT FOR SLAB 1"=1'-0" 6

MARK	STEEL DECK TYPE	GAGE	TOTAL THICKNESS (t)	CONCRETE TYPE	REINFORCEMENT	STEEL DECK WELDING PATTERN				MAXIMUM UNSHORED SPAN		
						END [3]	INTERMEDIATE [3]	SEAM	MARGINAL	SINGLE SPAN	DOUBLE SPAN	TRIPLE SPAN
D1	VERCO W3 FORMLOK	20	6 1/4"	f'c = 4000 psi LIGHT WEIGHT	#4 @ 18" OC, EW	3/4"Ø PUDDLE [4] WELD @ 12"OC	3/4"Ø PUDDLE [4] WELD @ 12"OC	BUTTON PUNCH @ 24" OC	3/4"Ø PUDDLE WELD @ 12"OC	9'-6"	10'-6"	11'-6"
D2	VERCO PLB-36	20	1 1/2"	–	–	3/4"Ø PUDDLE [4] WELD @ 12"OC	3/4"Ø PUDDLE [4] WELD @ 12"OC	VSC @ 24"OC	3/4"Ø PUDDLE WELD @ 12"OC	–	–	–
D3	VERCO HSB-36	18	1 1/2"	–	–	3/4"Ø PUDDLE [4] WELD @ 12"OC	3/4"Ø PUDDLE [4] WELD @ 12"OC	BUTTON PUNCH @ 24"OC	3/4"Ø PUDDLE WELD @ 12"OC	–	–	–
D4	B FORMLOCK	16	3 1/2"	f'c = 4000 psi LIGHT WEIGHT	#4 @ 18" OC, EW	3/4"Ø PUDDLE [4] WELD @ 12"OC	3/4"Ø PUDDLE [4] WELD @ 12"OC	BUTTON PUNCH @ 24"OC	3/4"Ø PUDDLE WELD @ 12"OC	7'-0"	7'-6"	8'-0"

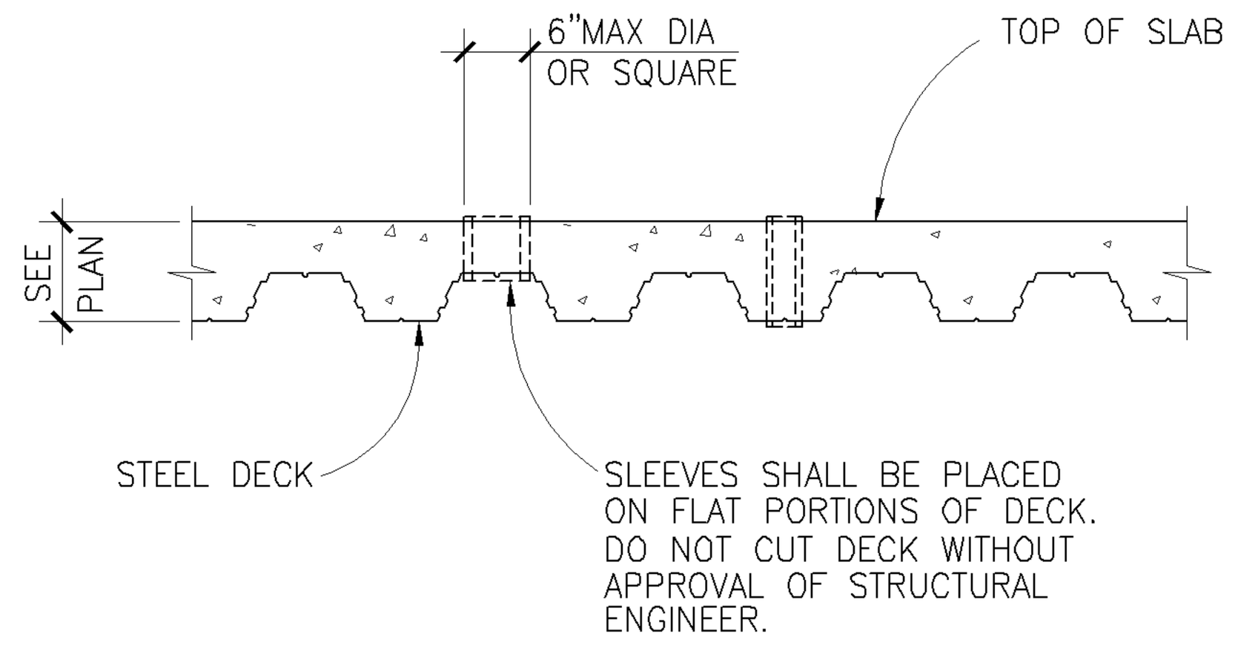


NOTES:

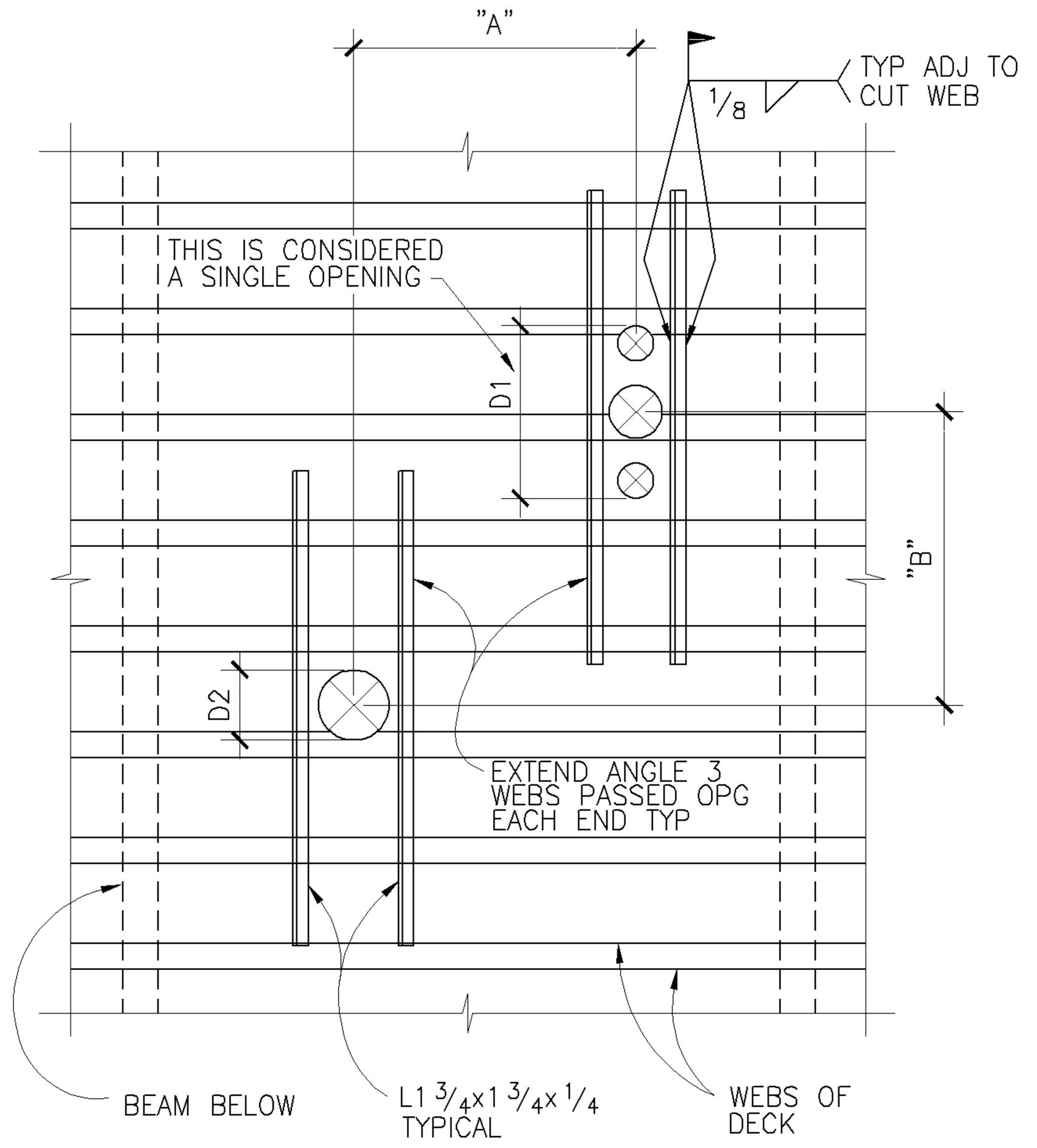
1. SEE GENERAL NOTES ON S1-101 FOR STEEL DECK PROPERTIES.
2. SEE DET 11/- FOR DECK BEARING.
3. NO PUDDLE WELDS REQUIRED WHERE STUDS OCCUR.
4. PROVIDE MIN OF (4) WELDS PER SHEET.
5. SEE ARCH FOR FINISH REQUIREMENTS.



S T E E L   D E C K   S C H E D U L E 1"=1'-0" 1



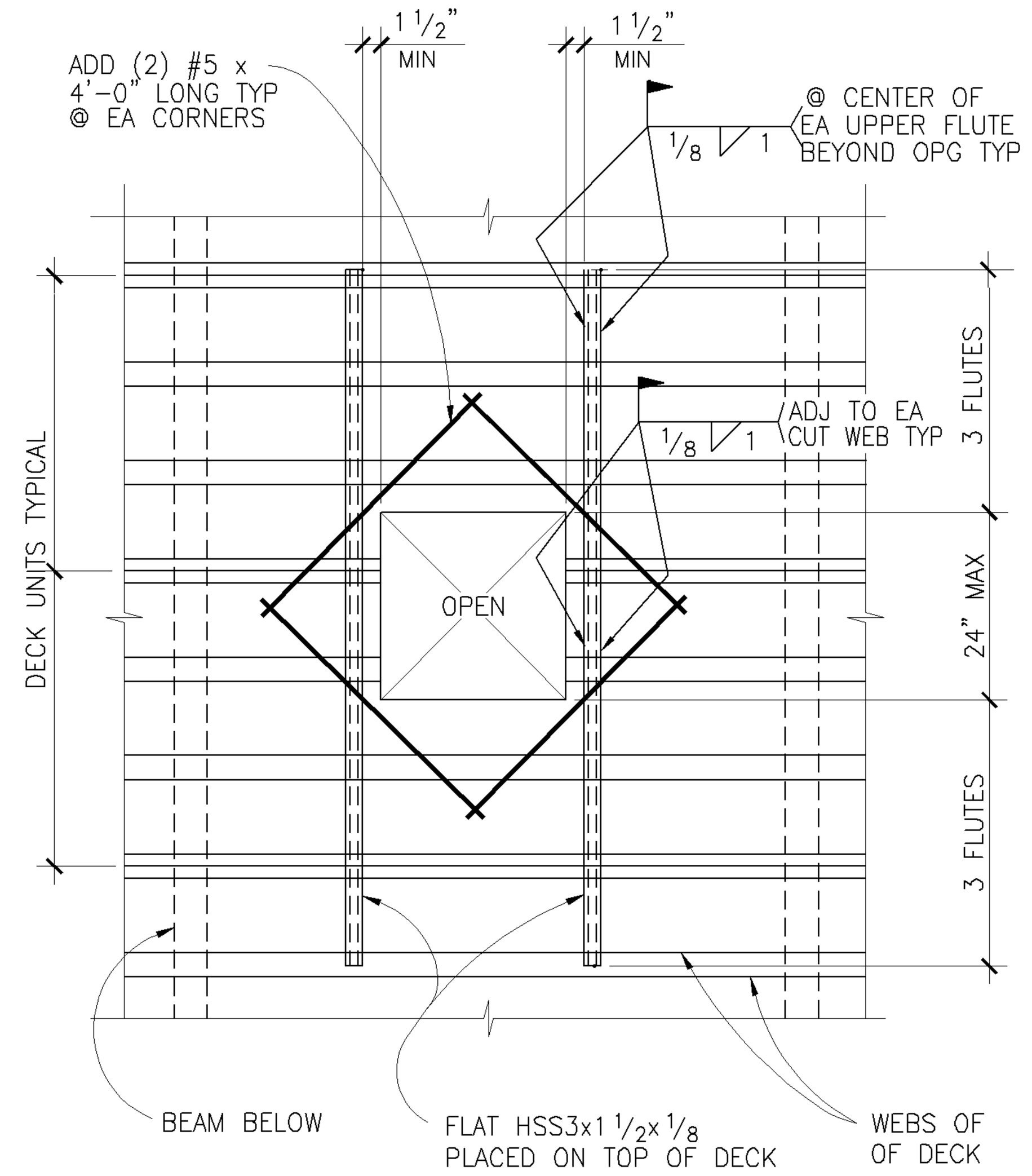
TYPICAL DECK SLEEVE 1"=1'-0" 7



NOTES:

1. HOLES LESS THAN 6" IN DIAMETER AND CUTTING NO MORE THAN 1 WEB NEED NO REINFORCING.
2. DO NOT CUT MORE THAN 2 ADJACENT WEBS.
3. ANGLES SHALL BE PLACED ON TOP OF DECK. ALTERNATELY THEY CAN BE PLACED ON BOTTOM OF DECK.
4. IF DIMENSIONS "A" AND "B" ARE LESS THAN 4D1, 4D2, OR 32" WHICHEVER IS LARGER, THE OPENING GROUP SHALL BE CONSIDERED AS A SINGLE HOLE, AND MUST BE REINFORCED AS REQUIRED FOR THE LARGER OPENING.
5. IF DIMENSION "A" IS GREATER THAN 4D1, 4D2, OR 32" WHICHEVER IS LARGER, THEN THERE IS NO RESTRICTION ON DIMENSION "B".
6. IF DIMENSION "B" IS GREATER THAN 4D1, 4D2, OR 32" WHICHEVER IS LARGER, THEN THERE IS NO RESTRICTION ON DIMENSION "A".
7. WHEN MAXIMUM DIMENSION OF AN OPENING OR OPENING GROUP EXCEEDS 12" USE DET 2/-

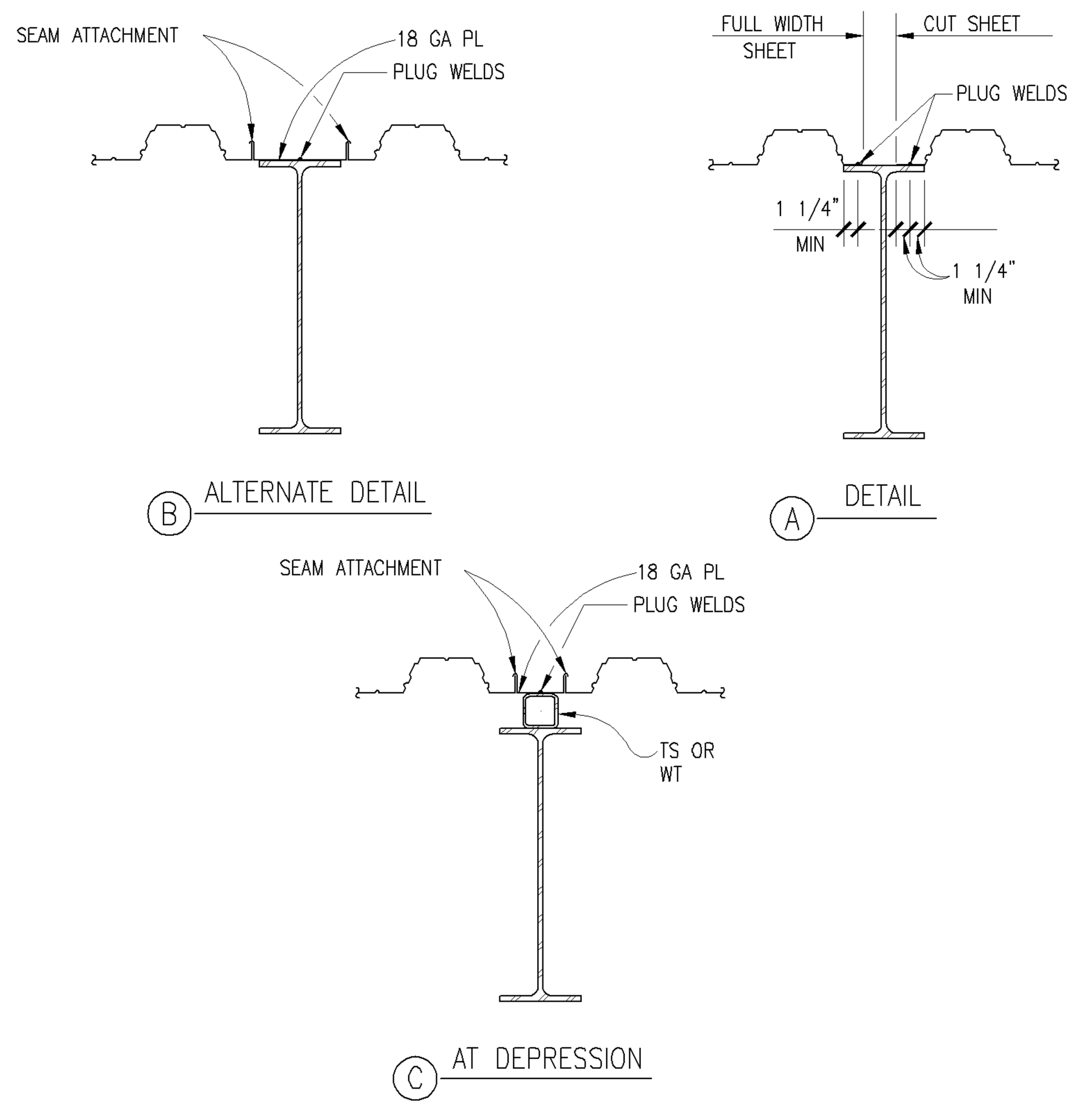
CONCRETE OVER METAL DECK SLEEVE DETAIL 1"=1'-0" 4



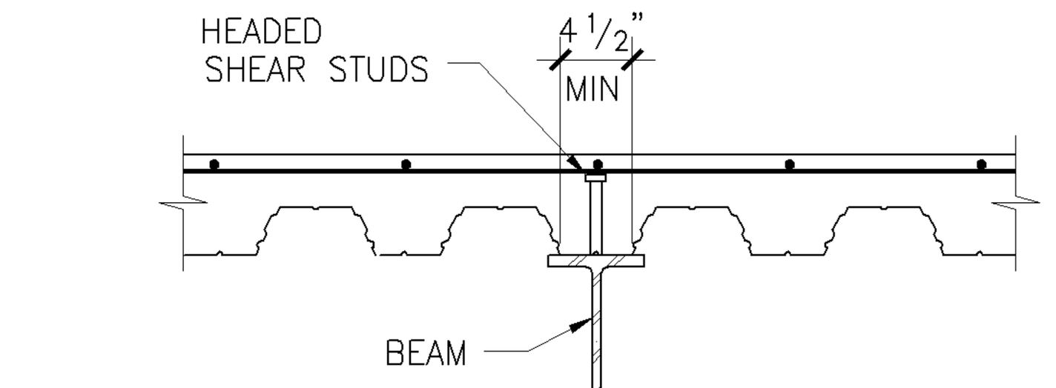
NOTE:

1. IF THE OPENING OR GROUP OF OPENING OCCURS IN ONE DECKING UNIT, THE OPENING OR OPENING GROUP MAY BE CUT PRIOR TO POURING OF CONCRETE.
2. IF, AS SHOWN IN THE DETAIL ABOVE, THE OPENING OR OPENING GROUP CUTS THROUGH TWO DECKING UNITS, THE DECKING SHALL NOT BE CUT UNTIL CONCRETE HAS BEEN PLACED AND CURED FOR A MIN OF 7 DAYS. AT THE TIME OF POURING, SUITABLE SLEEVES OR BULKHEADS SHALL BE PLACED AROUND THE OPENING.
3. ADD REBAR AT CORNERS OF OPENING.
4. WHEN THE MAXIMUM DIMENSION OF AN OPENING OR OPENING GROUP EXCEEDS 24", SEE PLANS FOR OPENING FRAMING.

CONCRETE OVER METAL DECK BLOCK OUT DETAIL 1"=1'-0" 2

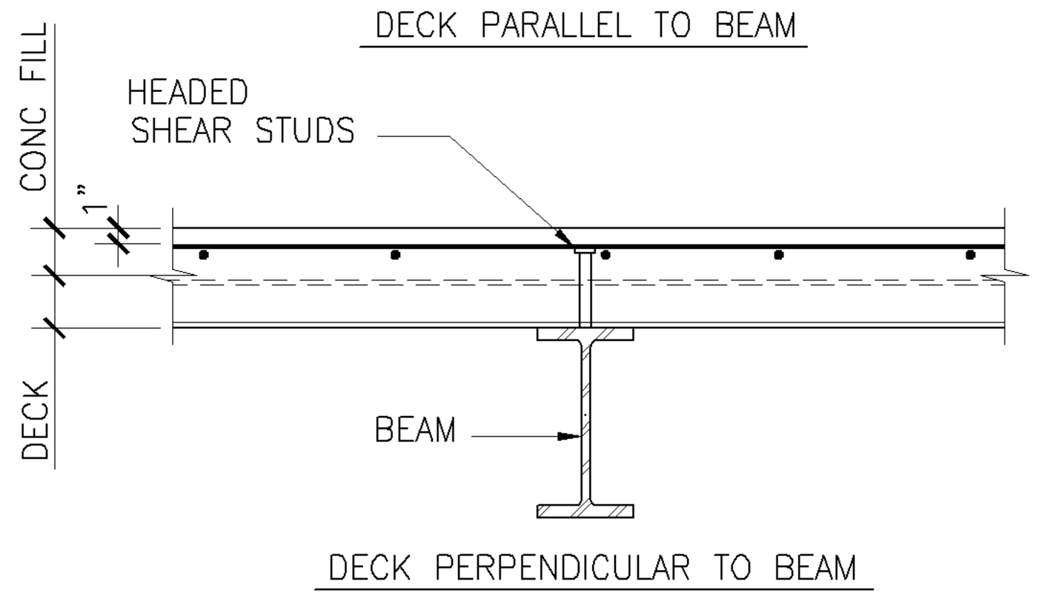


STEEL DECK ALIGNMENT/BEARING DETAIL 1"=1'-0" 11



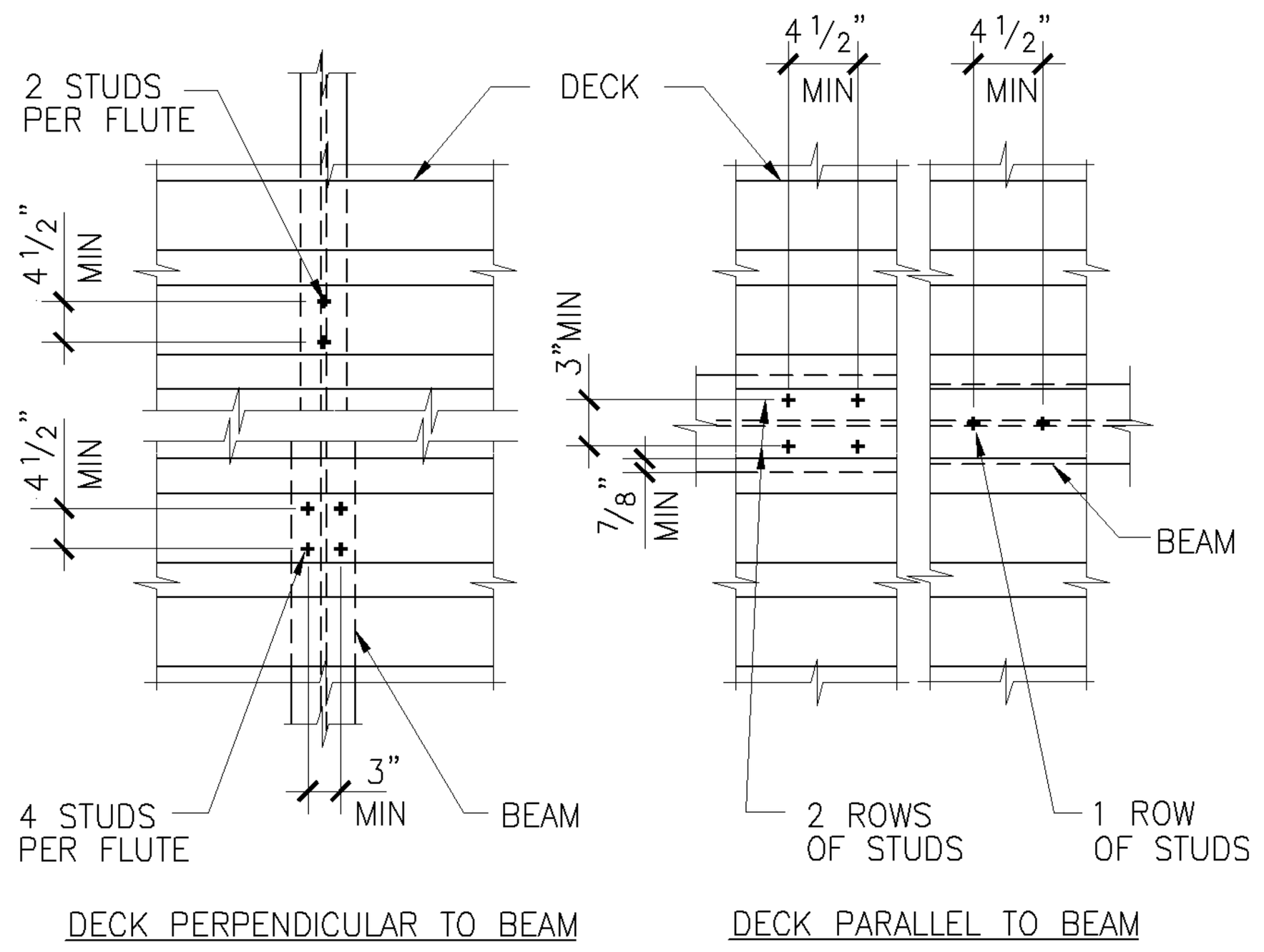
NOTE:
DECK RIB VALLEY TO BE CENTERED OVER BEAM WHERE POSSIBLE OR DECK MUST BE SPLIT FULL LENGTH OF BEAM TO PROVIDE CONCRETE HAUNCH WITH MIN. WIDTH OF 4 1/2".

DECK PARALLEL TO BEAM

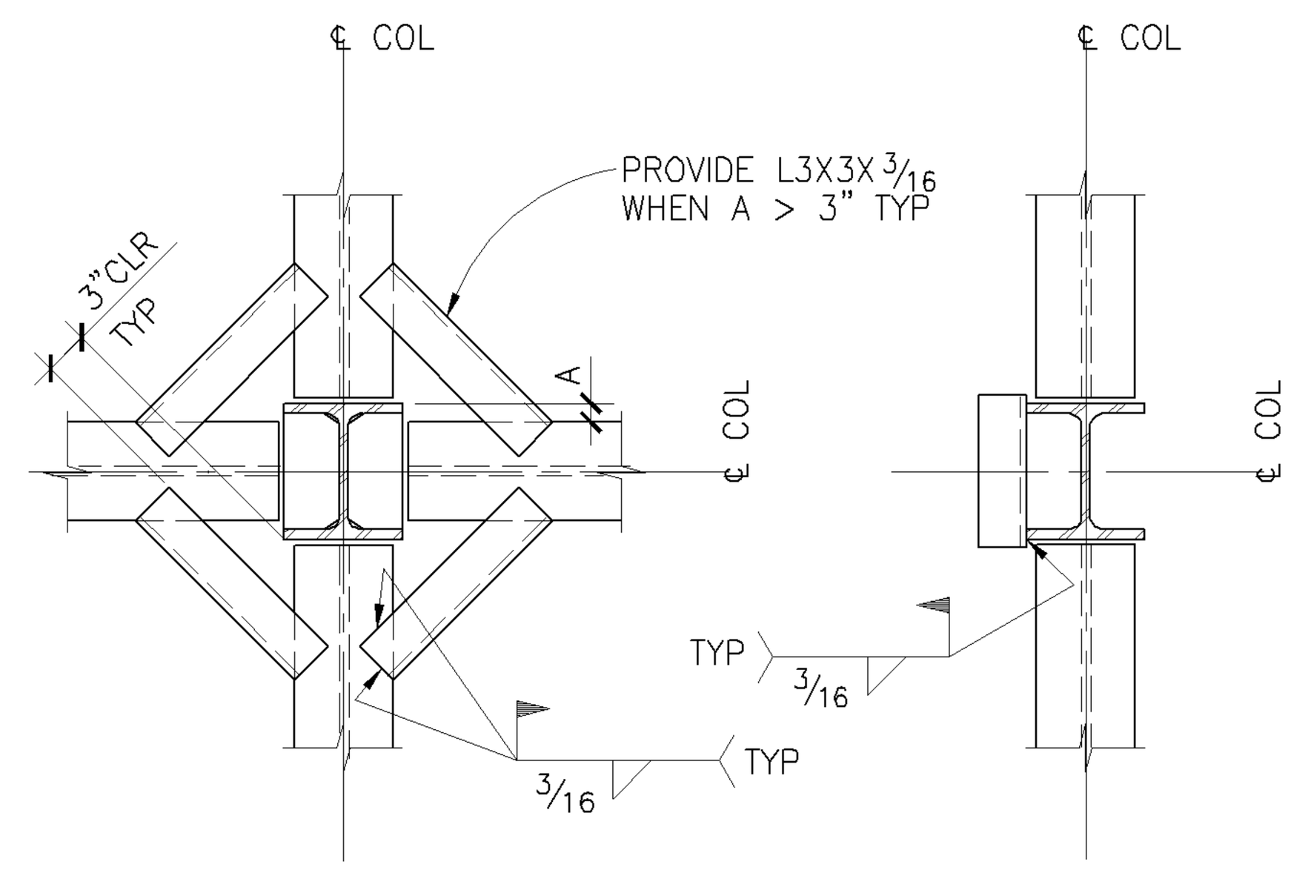


COMPOSITE BEAM TO COMPOSITE SLAB 1"=1'-0" 8

NOT USED 9



PLAN - STUD SPACING 1"=1'-0" 5



NOTE:
DECK SUPPLIER SHALL PROVIDE REQUIRED DECK SUPPORTS AND CLOSURES AROUND COLUMN AS REQUIRED.

METAL DECK SUPPORT AT COLUMN 1"=1'-0" 3

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE I-7
TORRANCE, CA 90503

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER ADRIN J. ... No. S4399 Exp. 12/31/07 STRUCTURAL STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

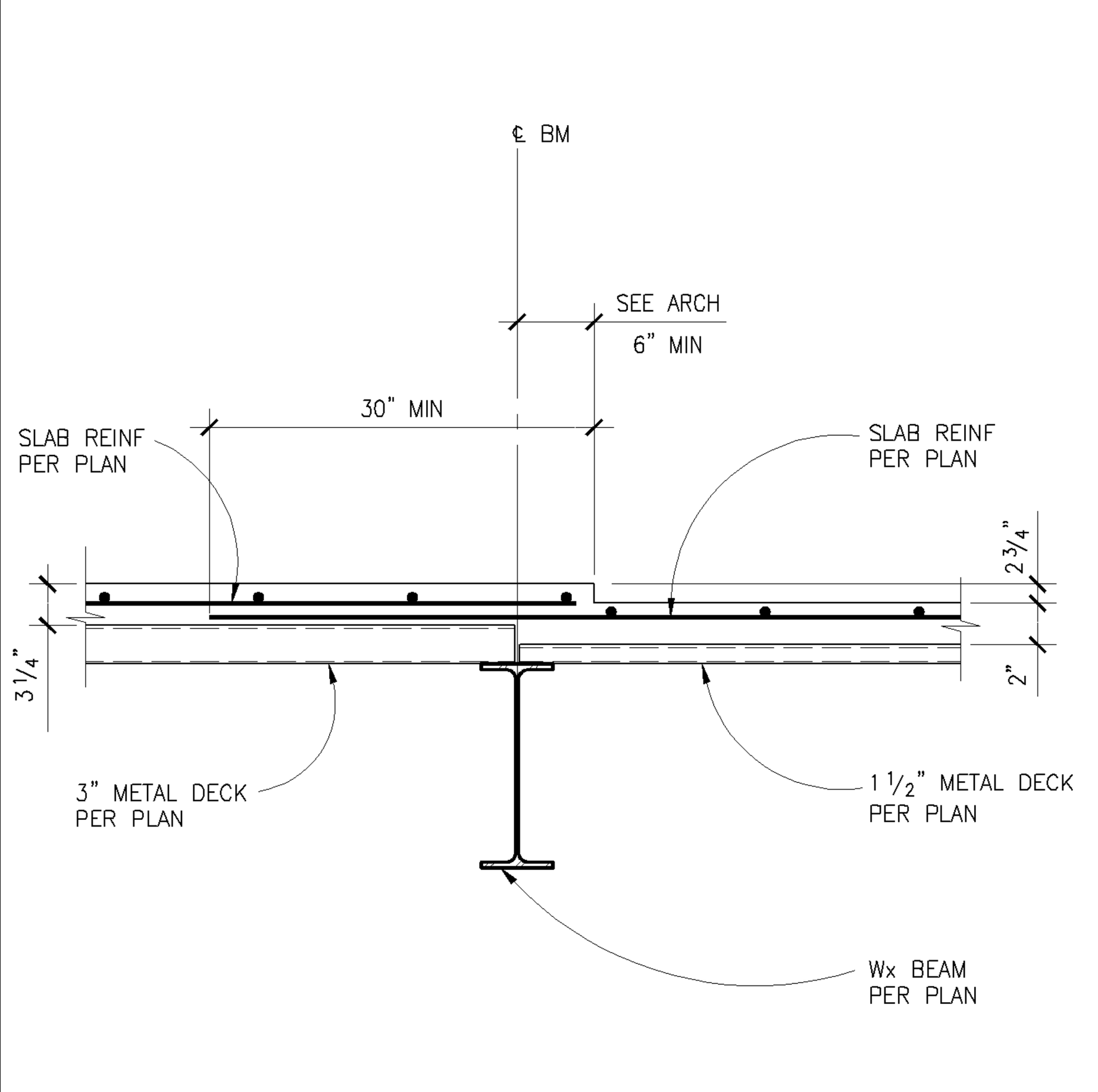
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

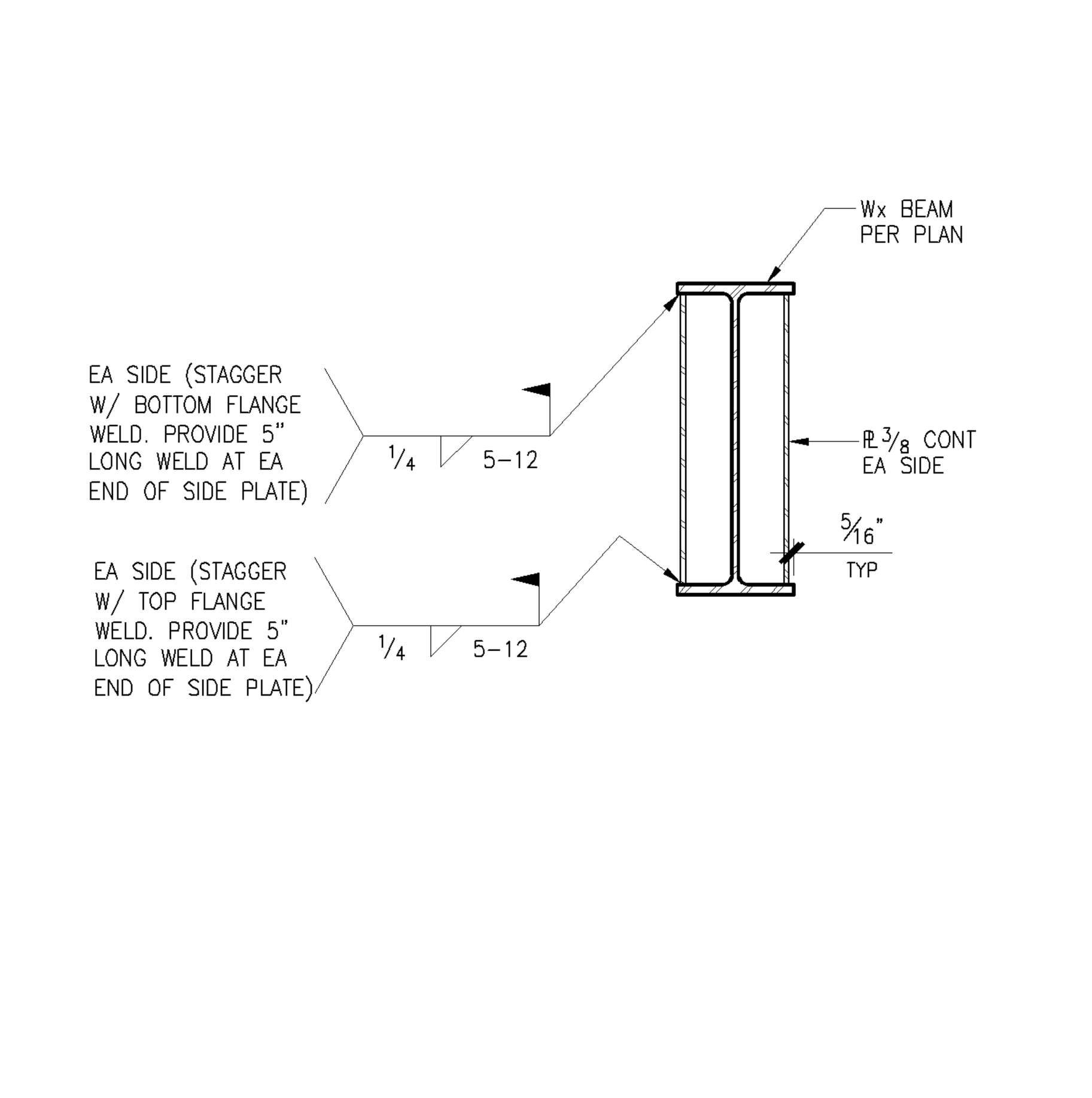
SHEET TITLE

TYPICAL STEEL DETAILS

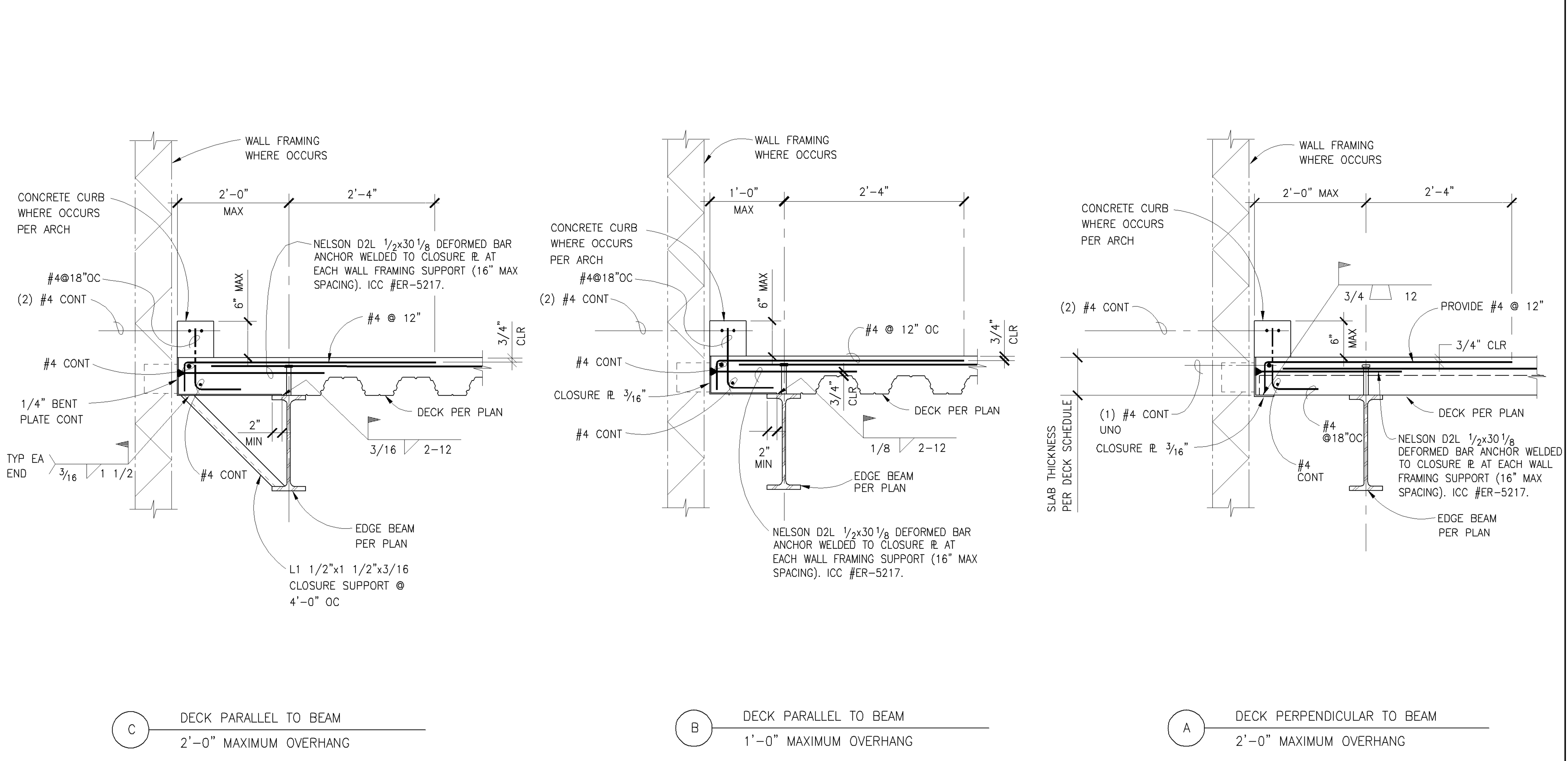
S5-102



2 3/4" DEPRESSION AT PERPENDICULAR BEAM 1"=1'-0" 12

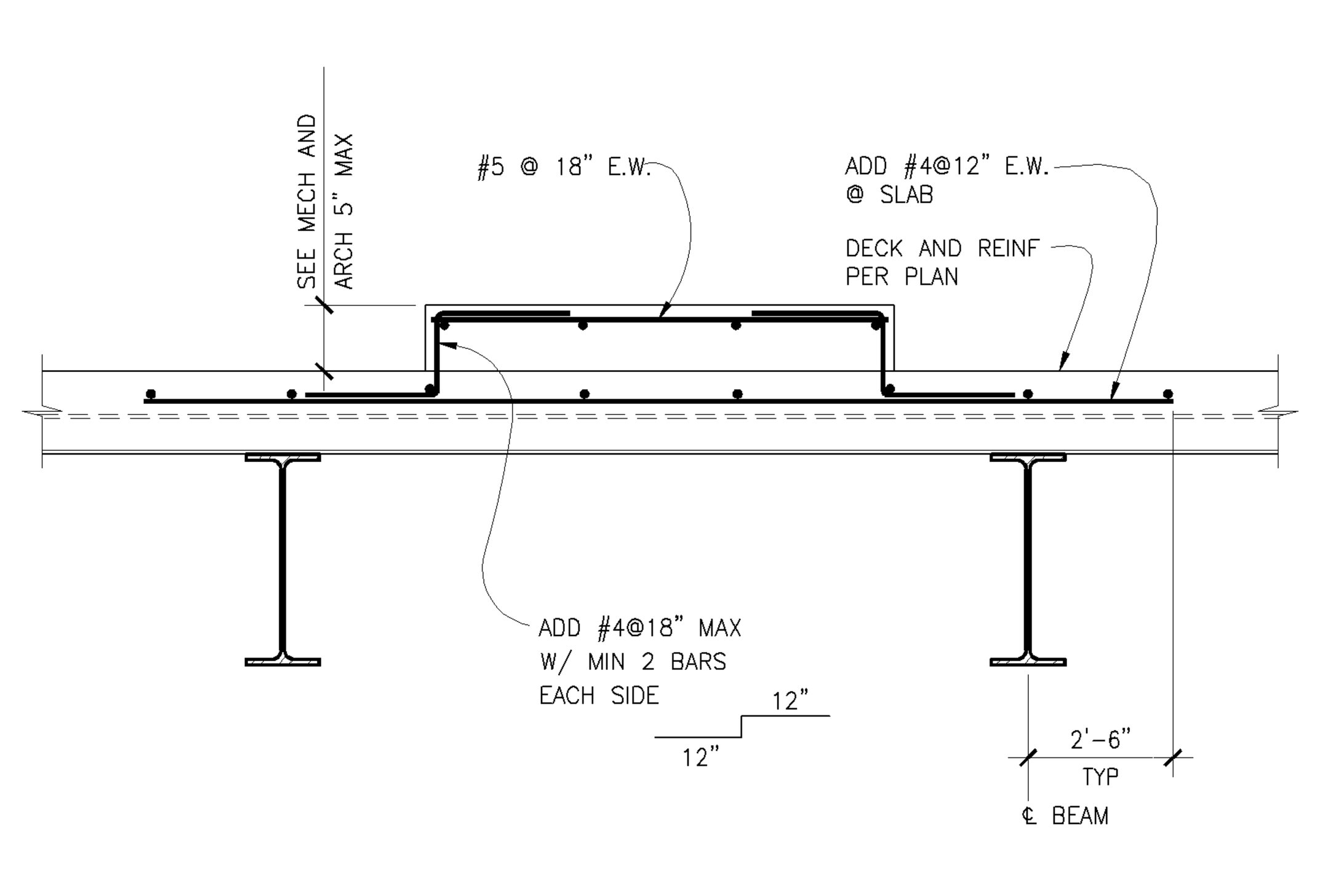


SIDE PLATE DETAIL 3/4"=1'-0" 8



NOTES:

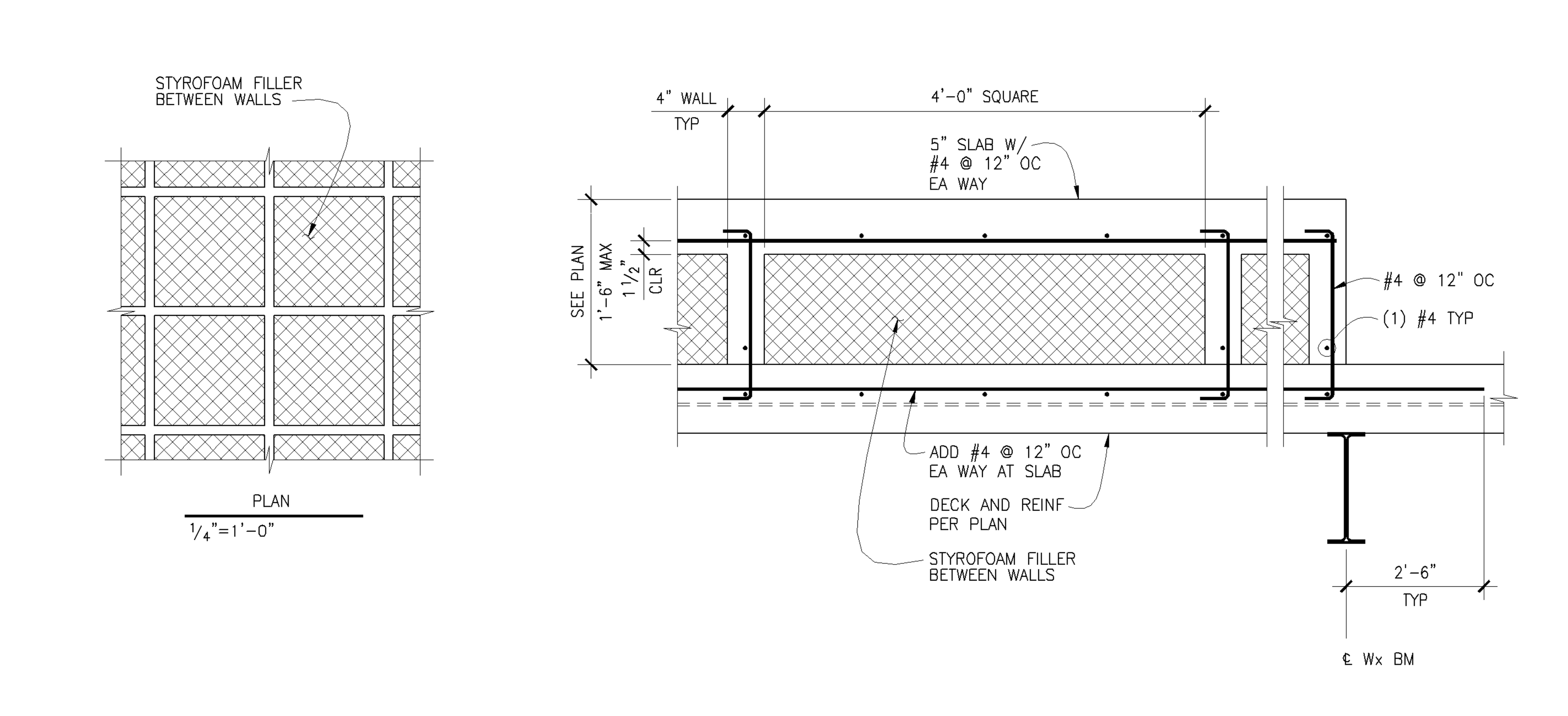
1. WELDED REBAR SHALL BE A706 MATERIAL
2. COORDINATE STUD CONNECTIONS TO ALIGN W/ WELDED DOWELS.
3. SEE SHEET S5-107 FOR TYPICAL EXTERIOR SLAB EDGE SECTIONS.



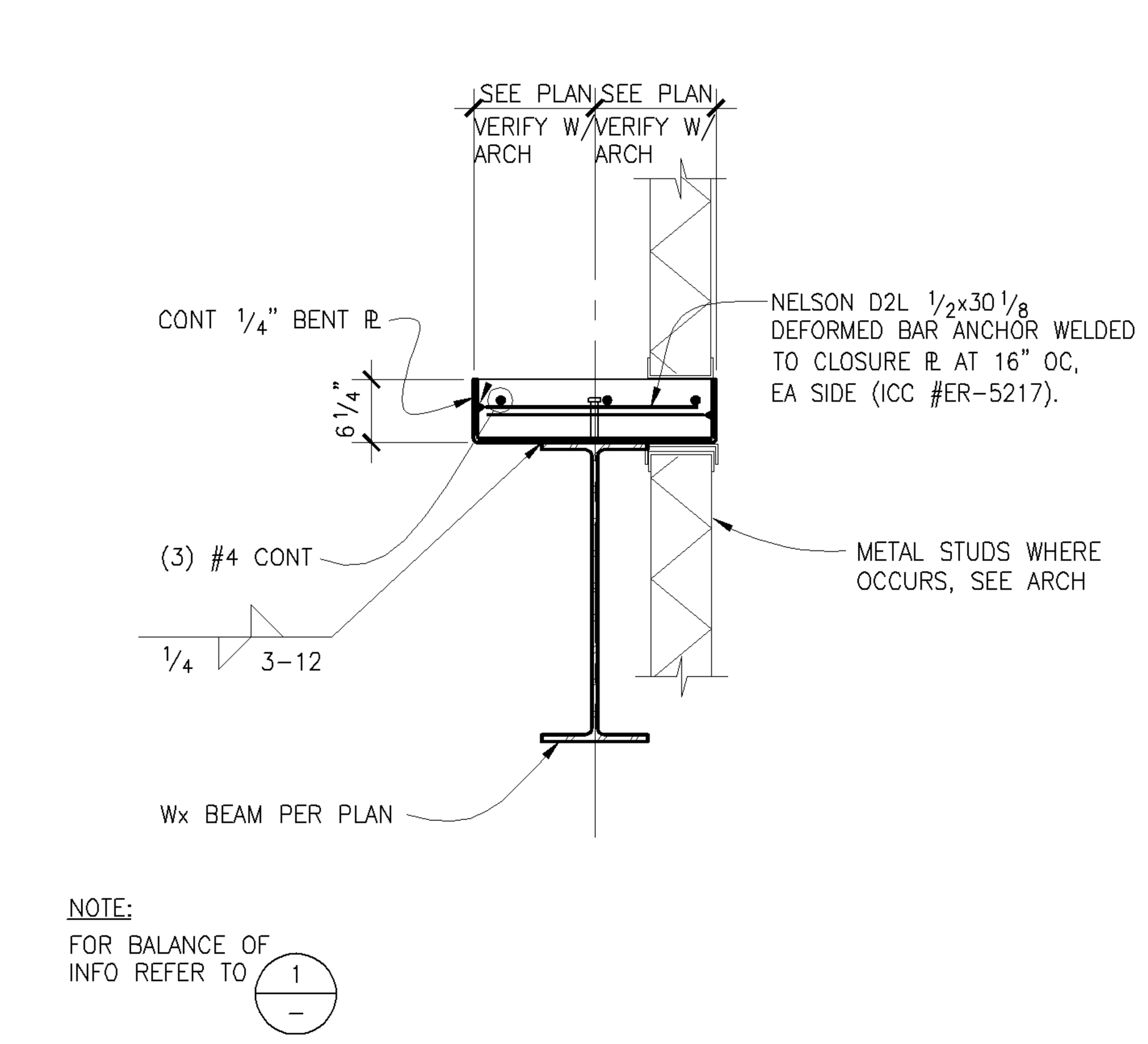
NOTE: NOTIFY STRUCTURAL ENGINEER WHEN PAD THICKNESSES > 5".

MECHANICAL PAD ON DECK 1"=1'-0" 9

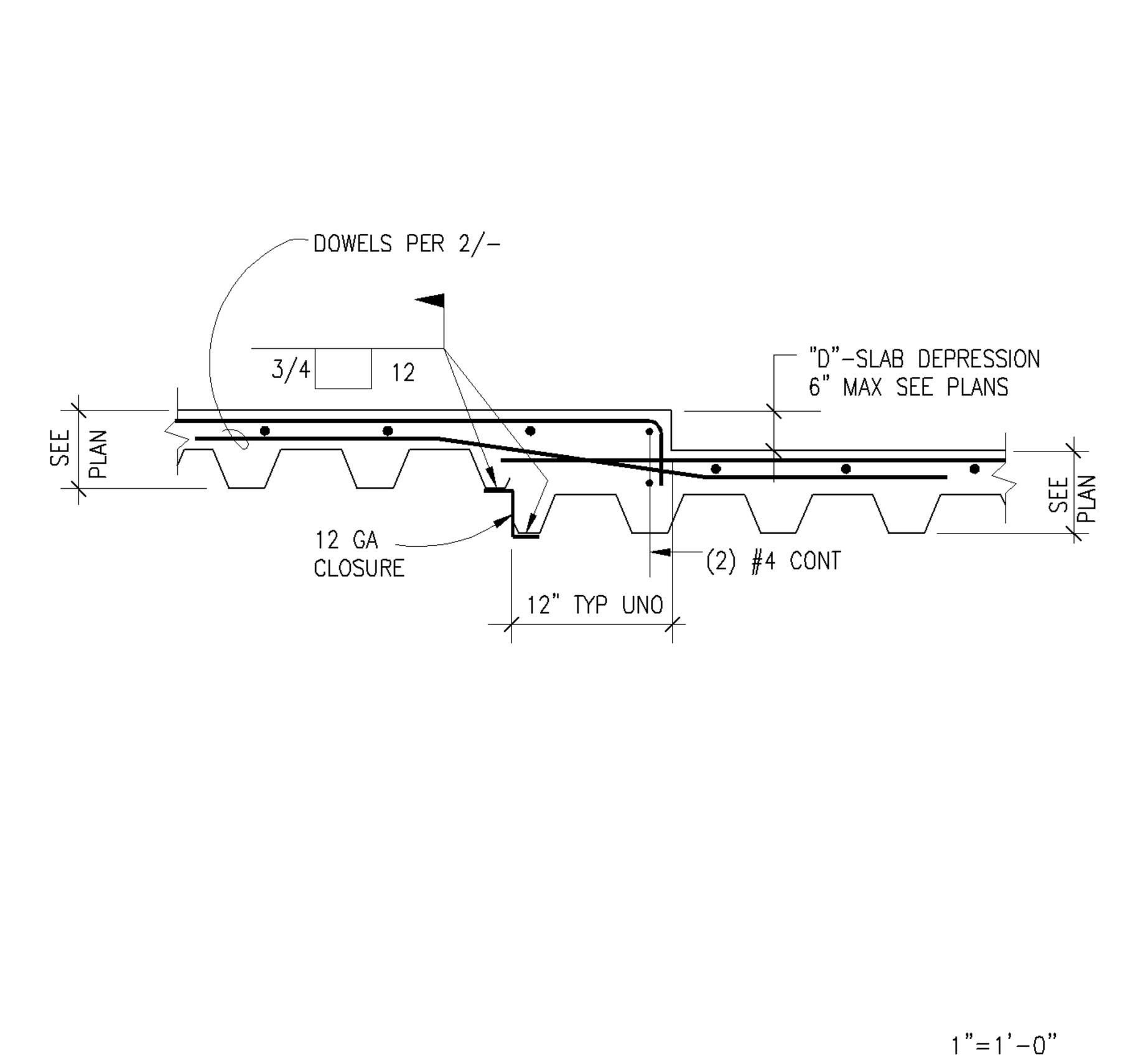
TYPICAL EDGE OF STEEL DECK SUPPORT DETAILS 1"=1'-0" 1



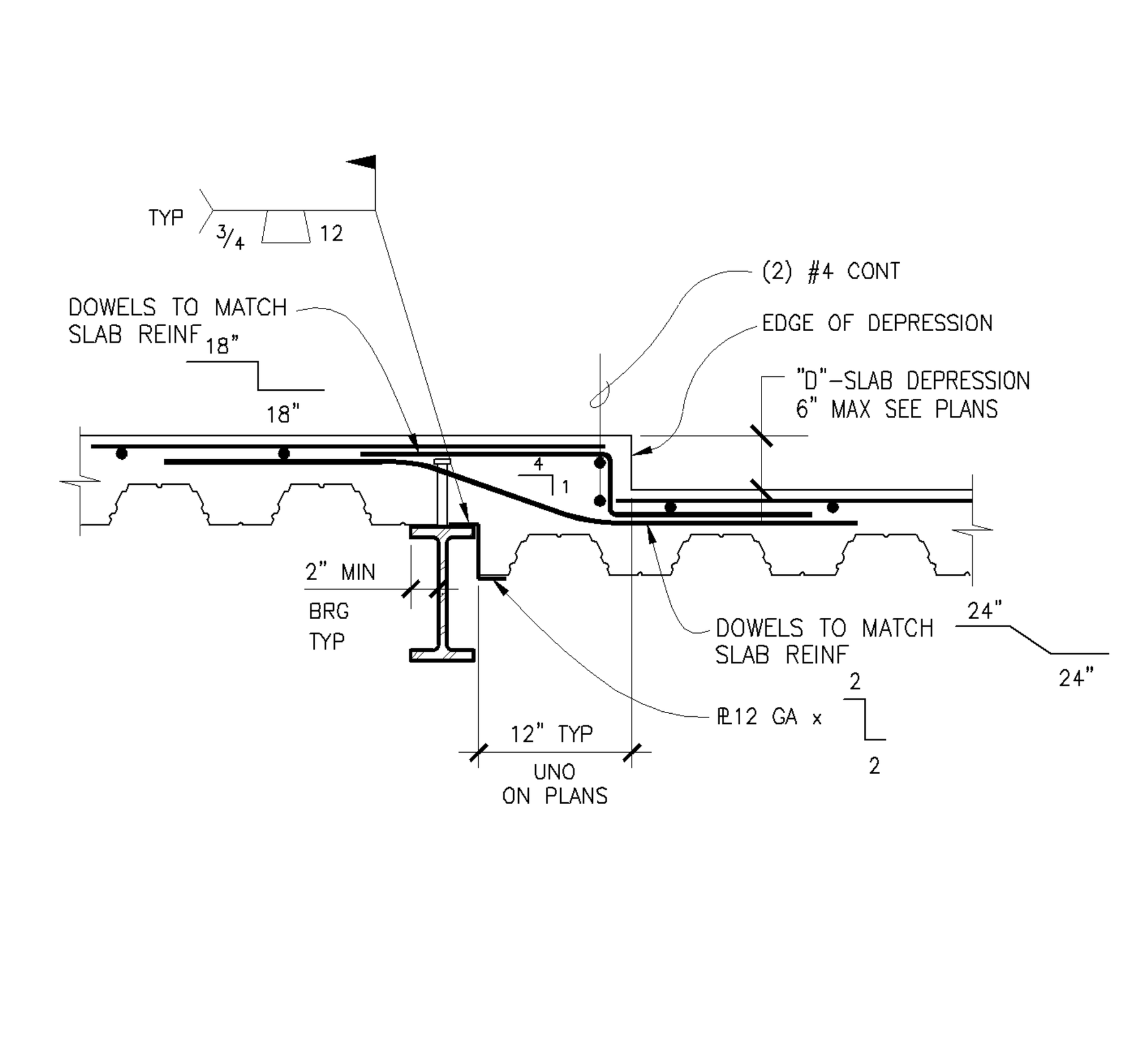
BUILT-UP SLAB ON DECK 1"=1'-0" 10



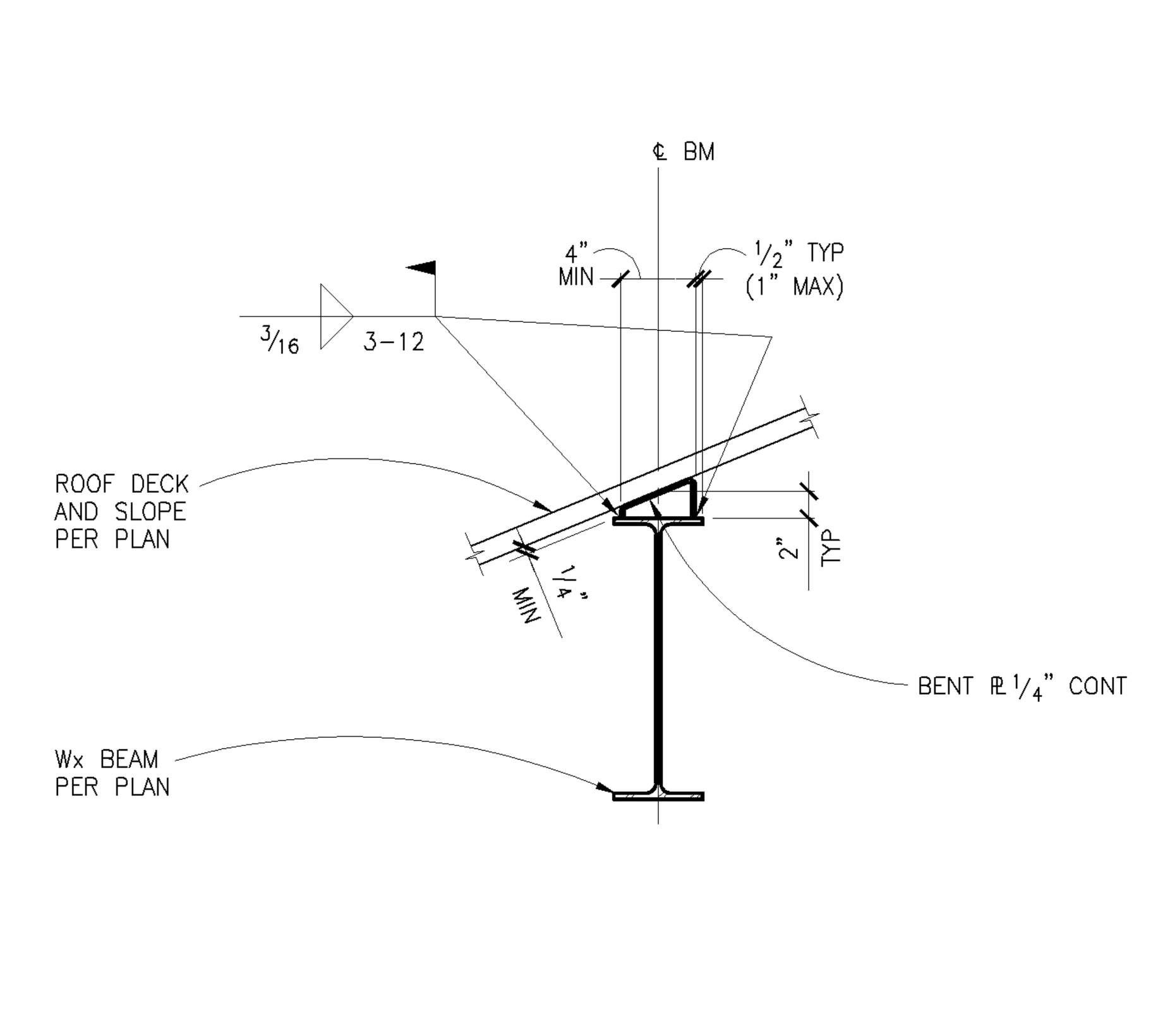
TYPICAL DOUBLE EDGE OF DECK DETAIL 3/4"=1'-0" 6



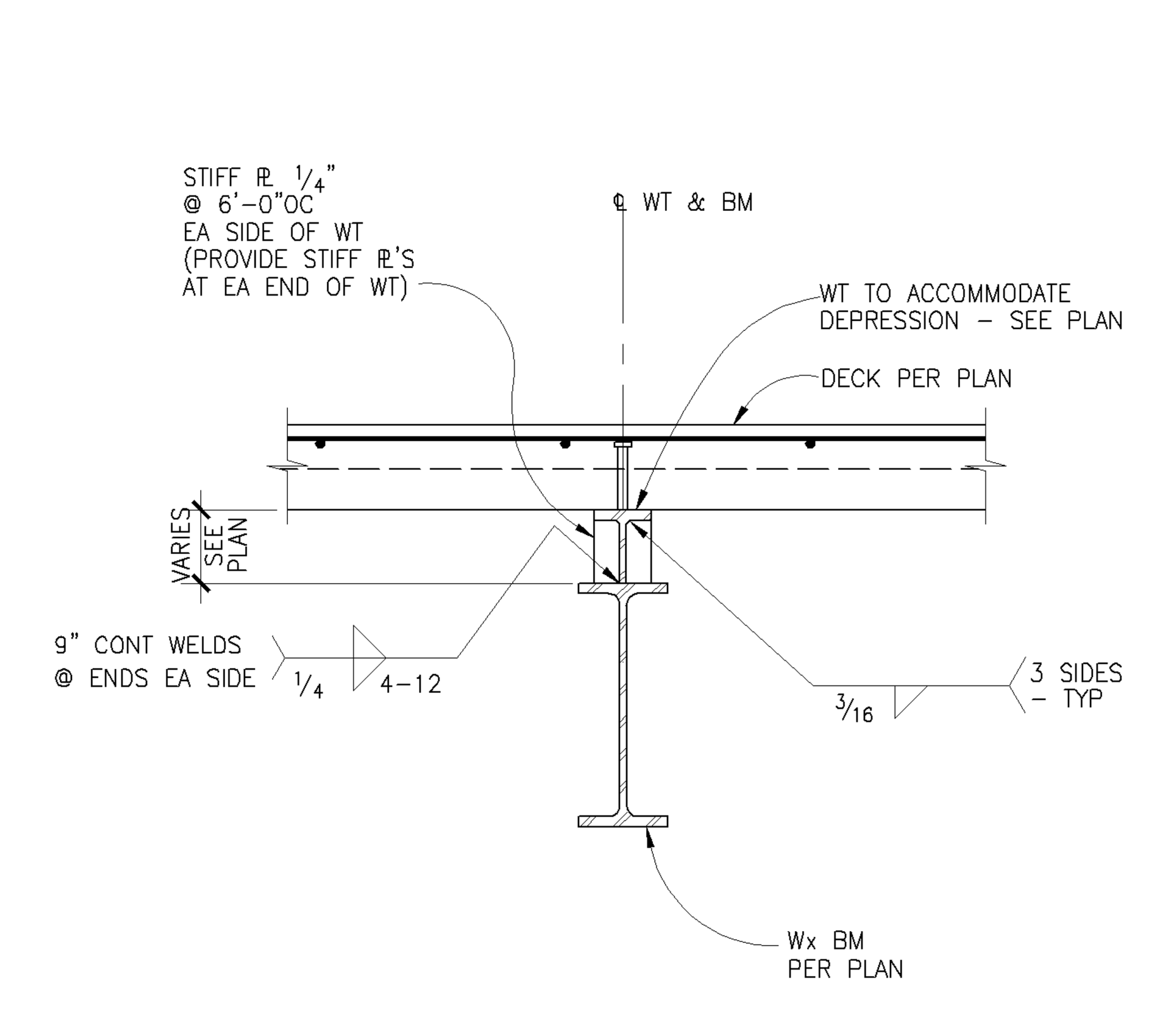
TYP SLAB DEPRESSION DETAIL (UP TO 5" MAX) 4



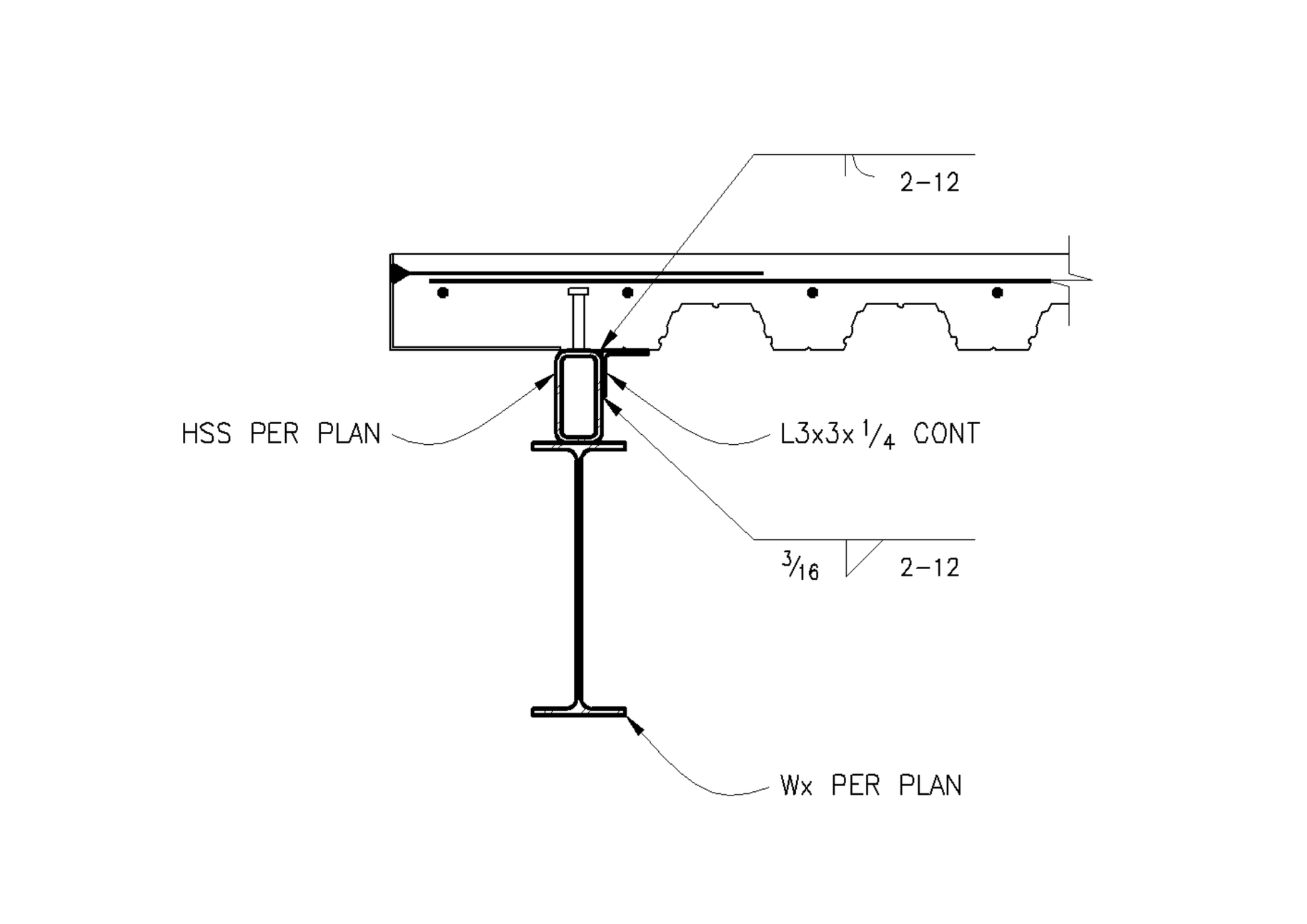
SLAB DEPRESSION DECK PARALLEL TO BEAM 1"=1'-0" 2



SLOPING DECK SUPPORT DETAIL 3/4"=1'-0" 11

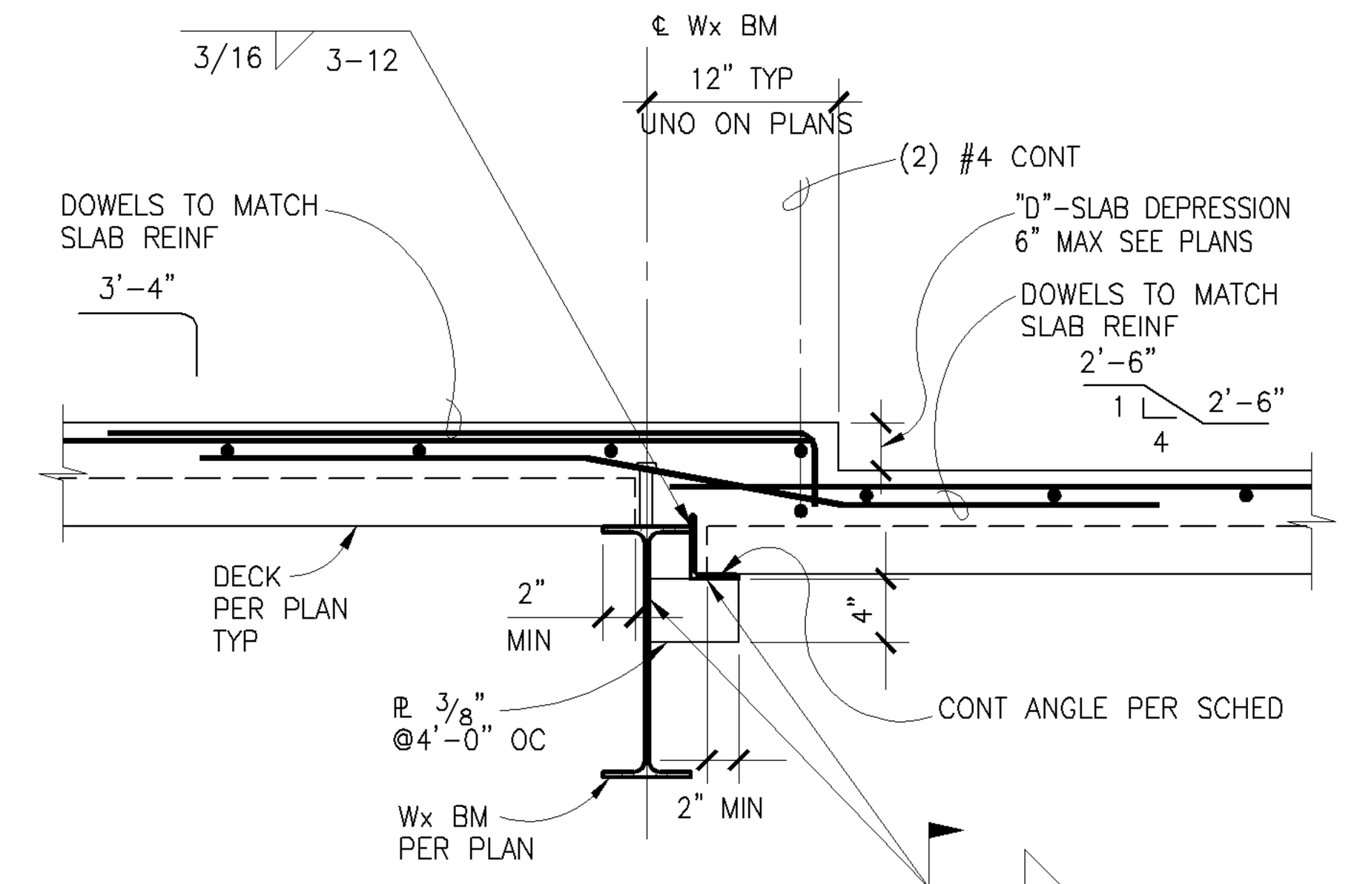


DEPRESSED BEAM WITH WT 1"=1'-0" 7



NOTE: SEE DETAILS 1/– AND 10/– FOR BALANCE OF INFORMATION.

DEPRESSED BEAM WITH HSS 1"=1'-0" 5



DEPRESSION "D"	ANGLE SIZE
UP TO 1 1/2"	L3x3x5/16
1 1/2"<D≤3"	L4x3x5/16 (LLV)
3"<D≤5"	L6x3 1/2x5/16(LLV)
D=6"	L7x4x3/8 (LLV)

SLAB DEPRESSION DECK PERPENDICULAR TO BEAM 3

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

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PROJECT NO: 60004775
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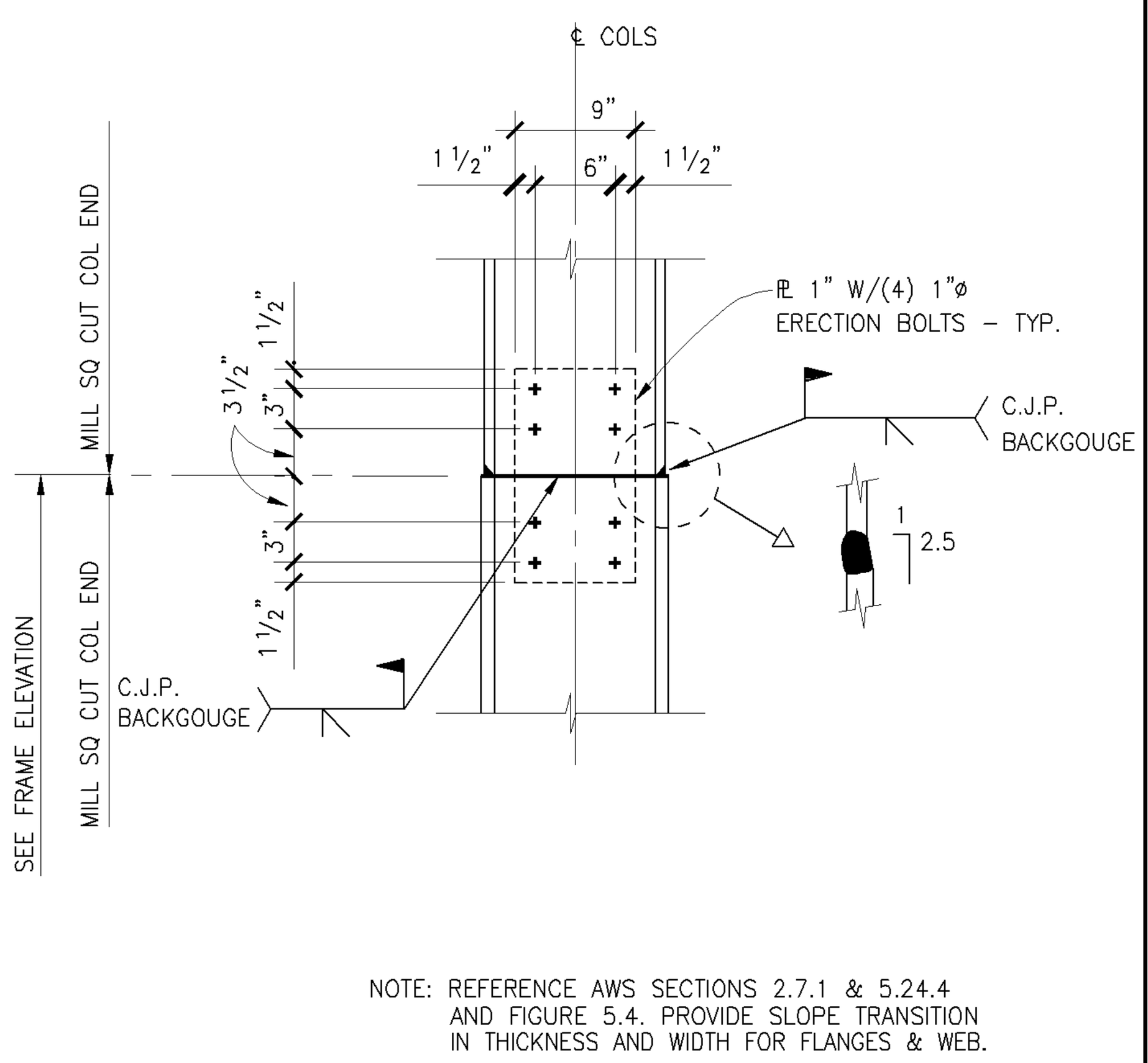
KEY PLAN

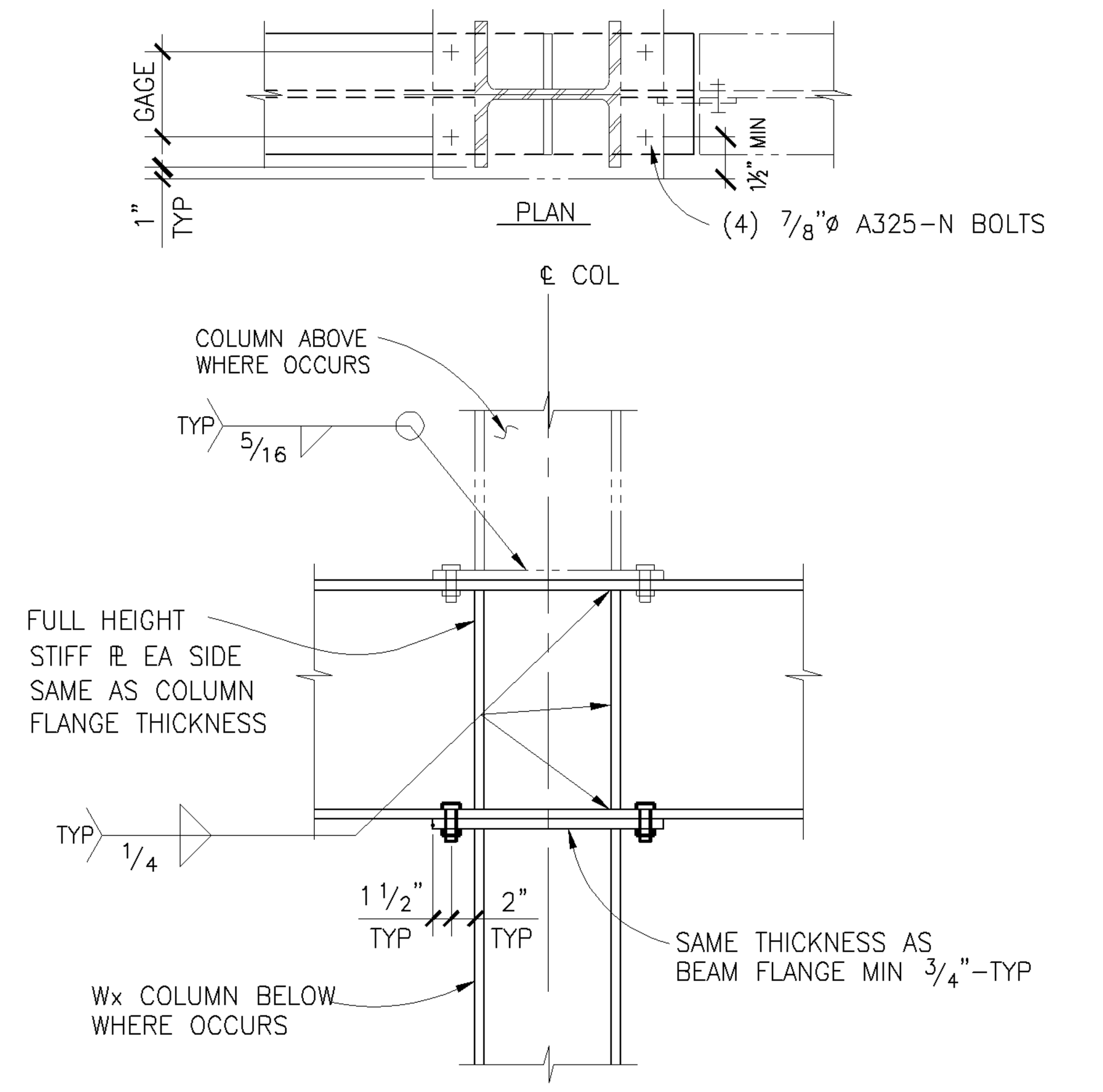
SHEET TITLE

TYPICAL STEEL DETAILS

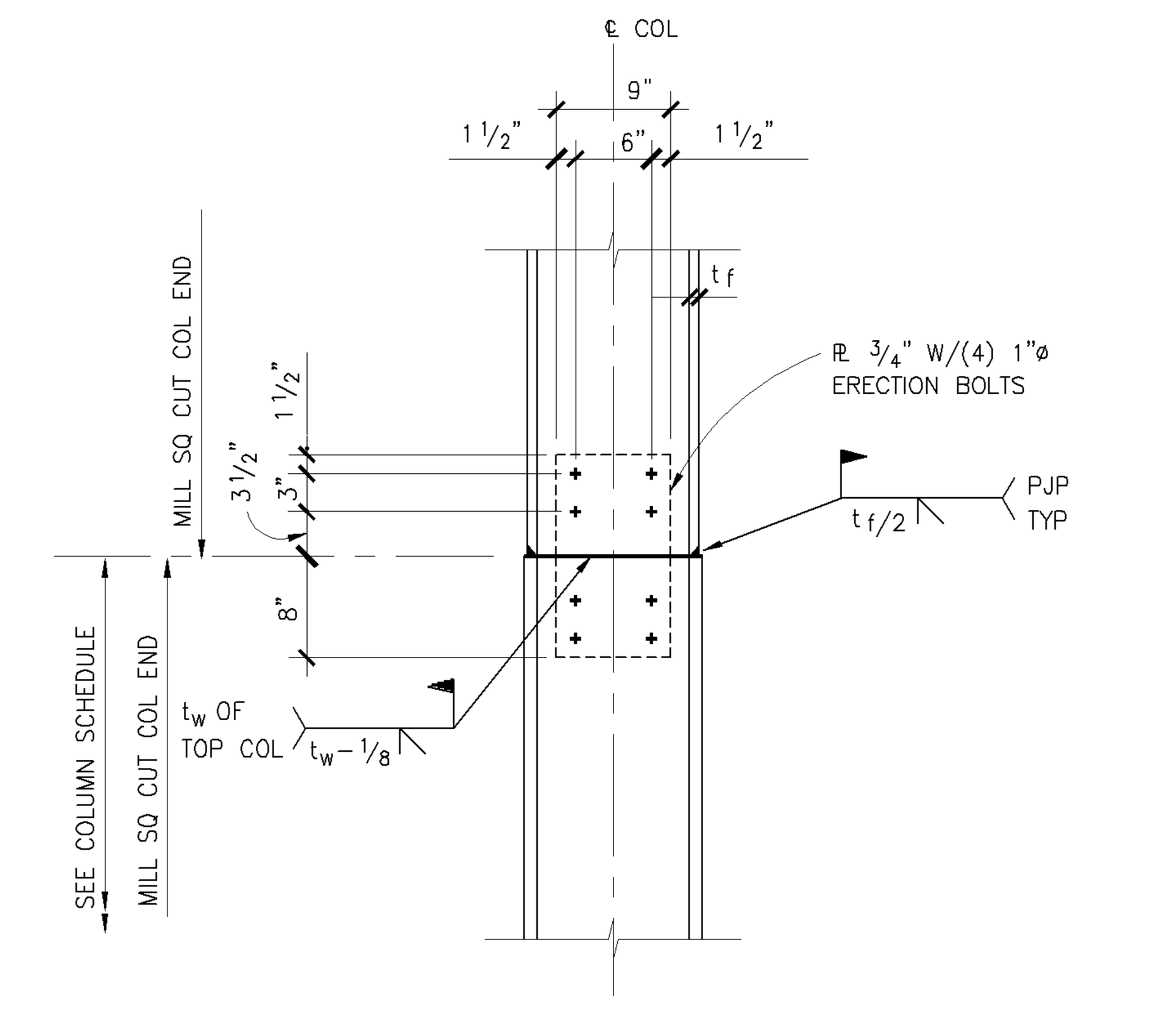
S5-103



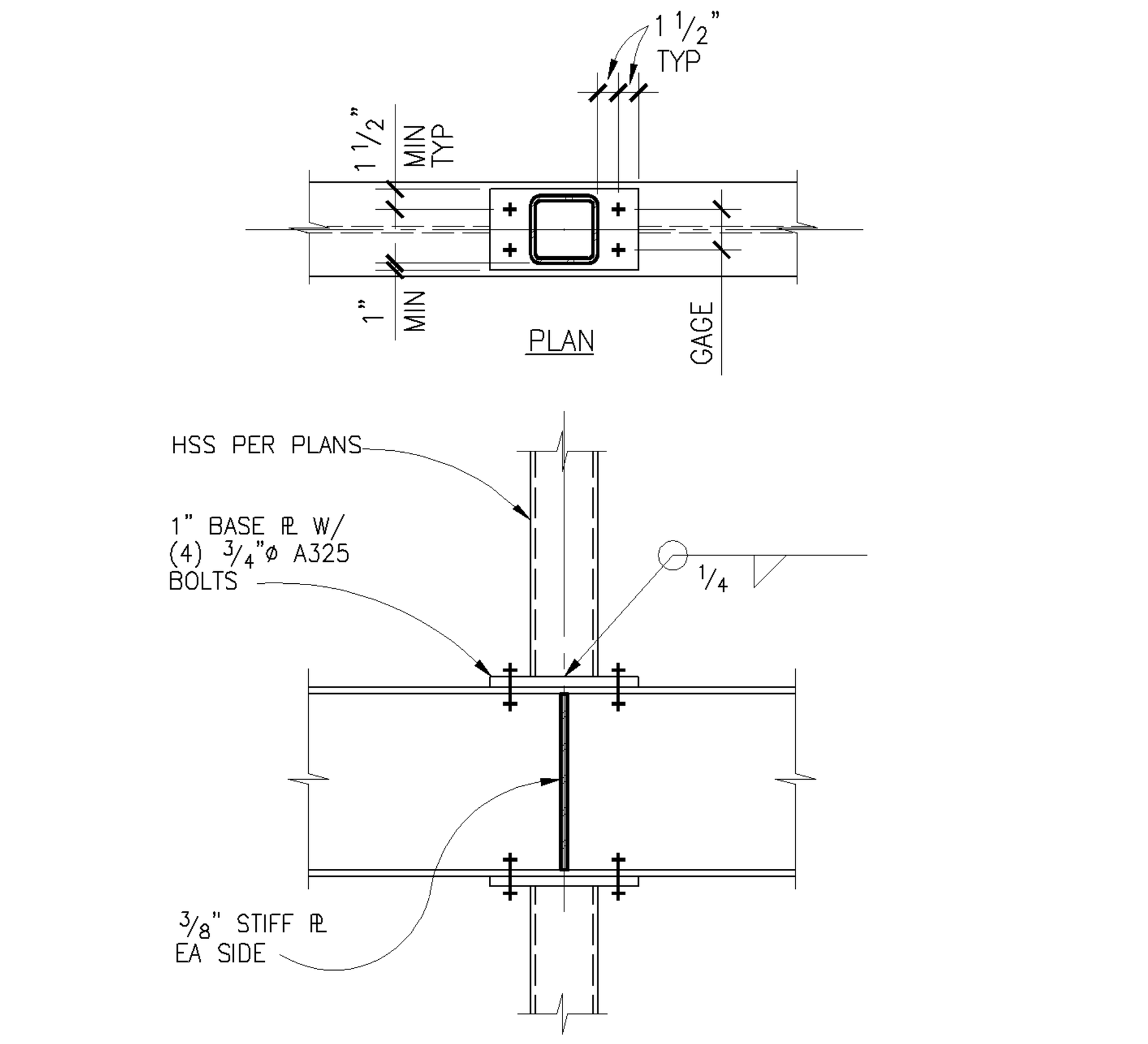
FRAME COLUMN SPLICE 1"=1'-0" 6



Wx COLUMN ABOVE & BELOW BEAM 1"=1'-0" 2

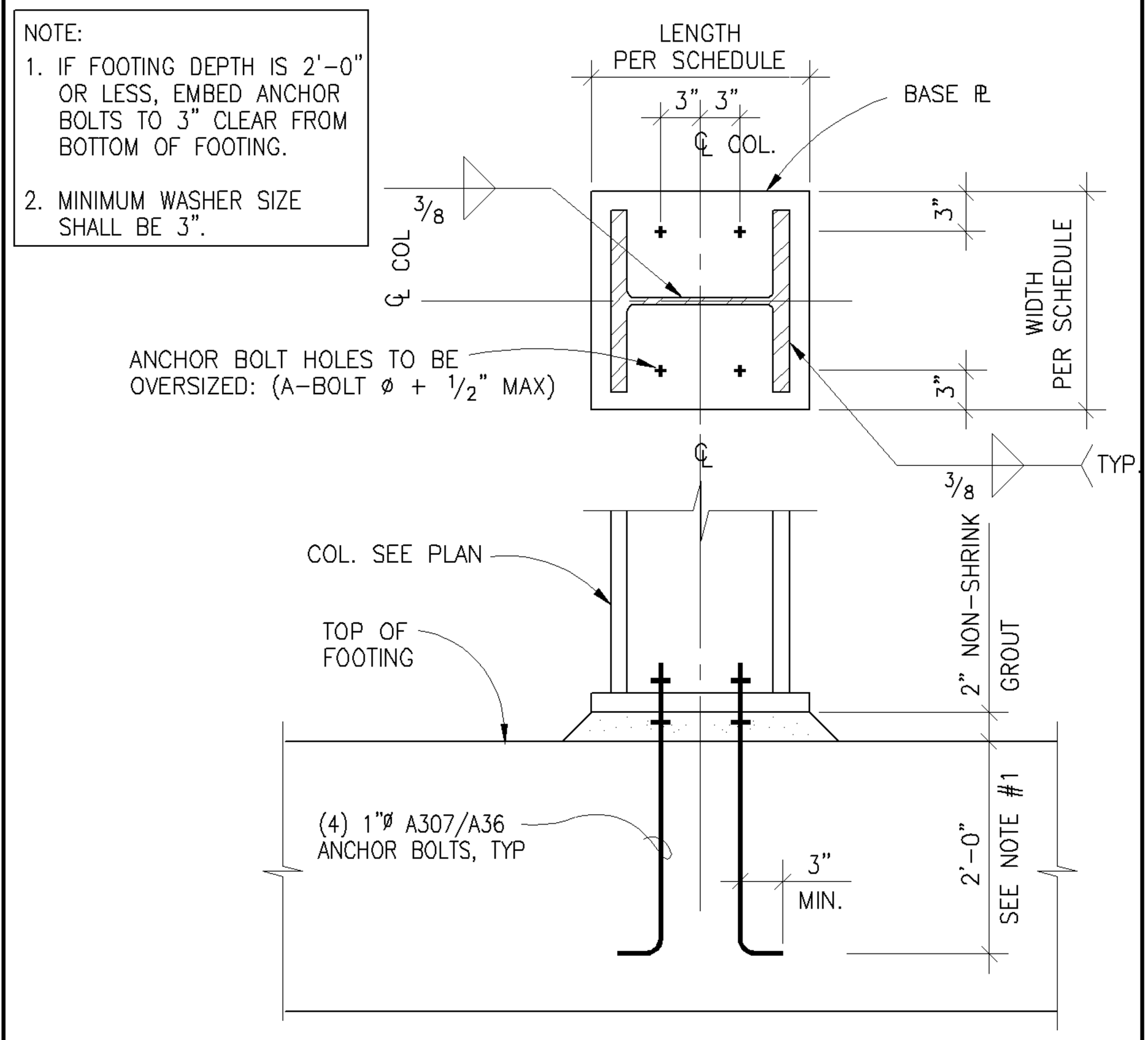


NON-FRAME COLUMN SPLICE 1"=1'-0" 7



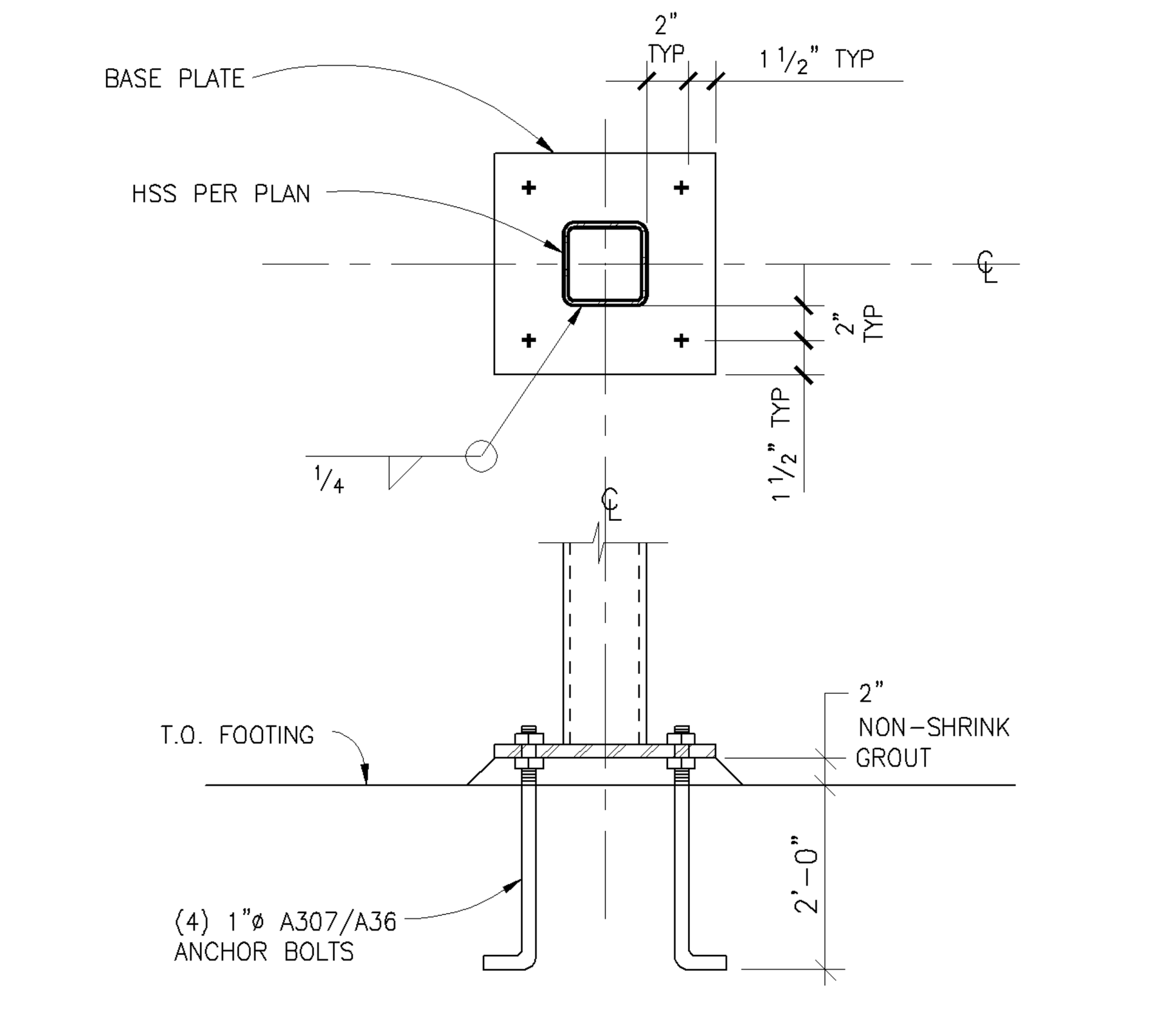
HSS COLUMN ABOVE AND BELOW BEAM 1"=1'-0" 3

NOT USED 8

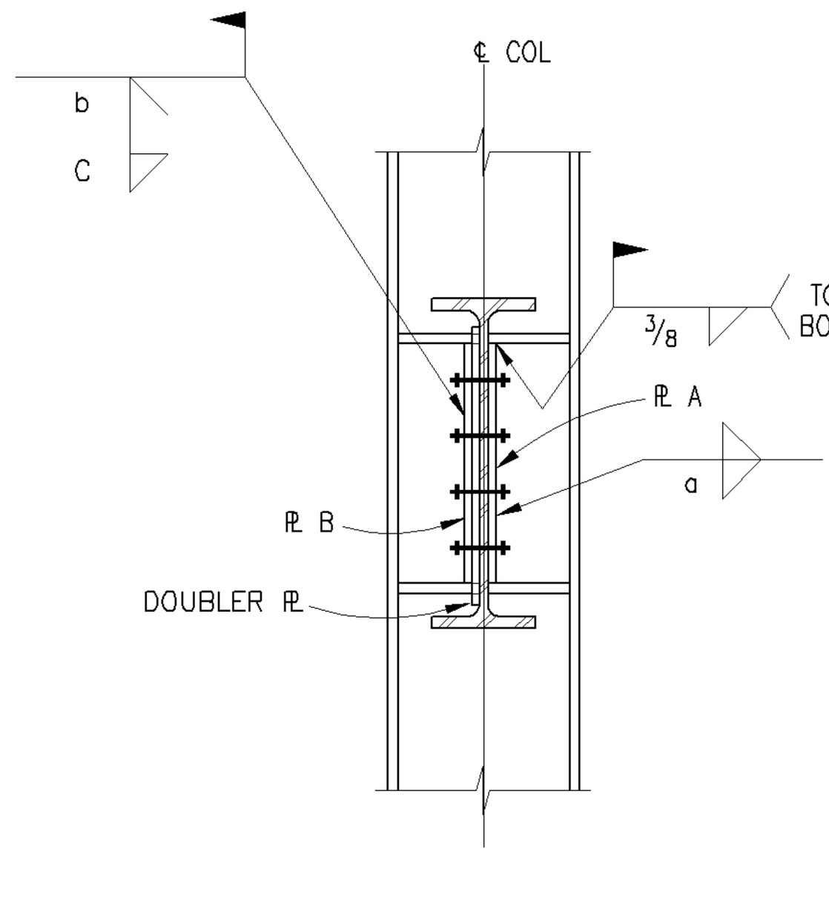


NON-FRAME COLUMN BASE PLATE DETAIL 1"=1'-0" 4

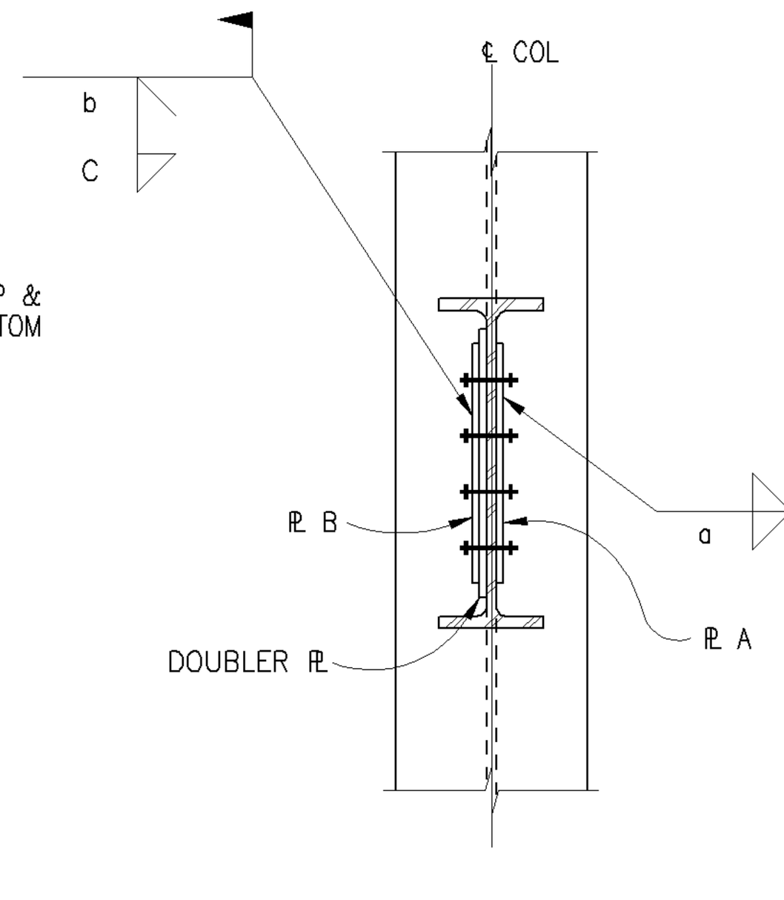
NOT USED 9



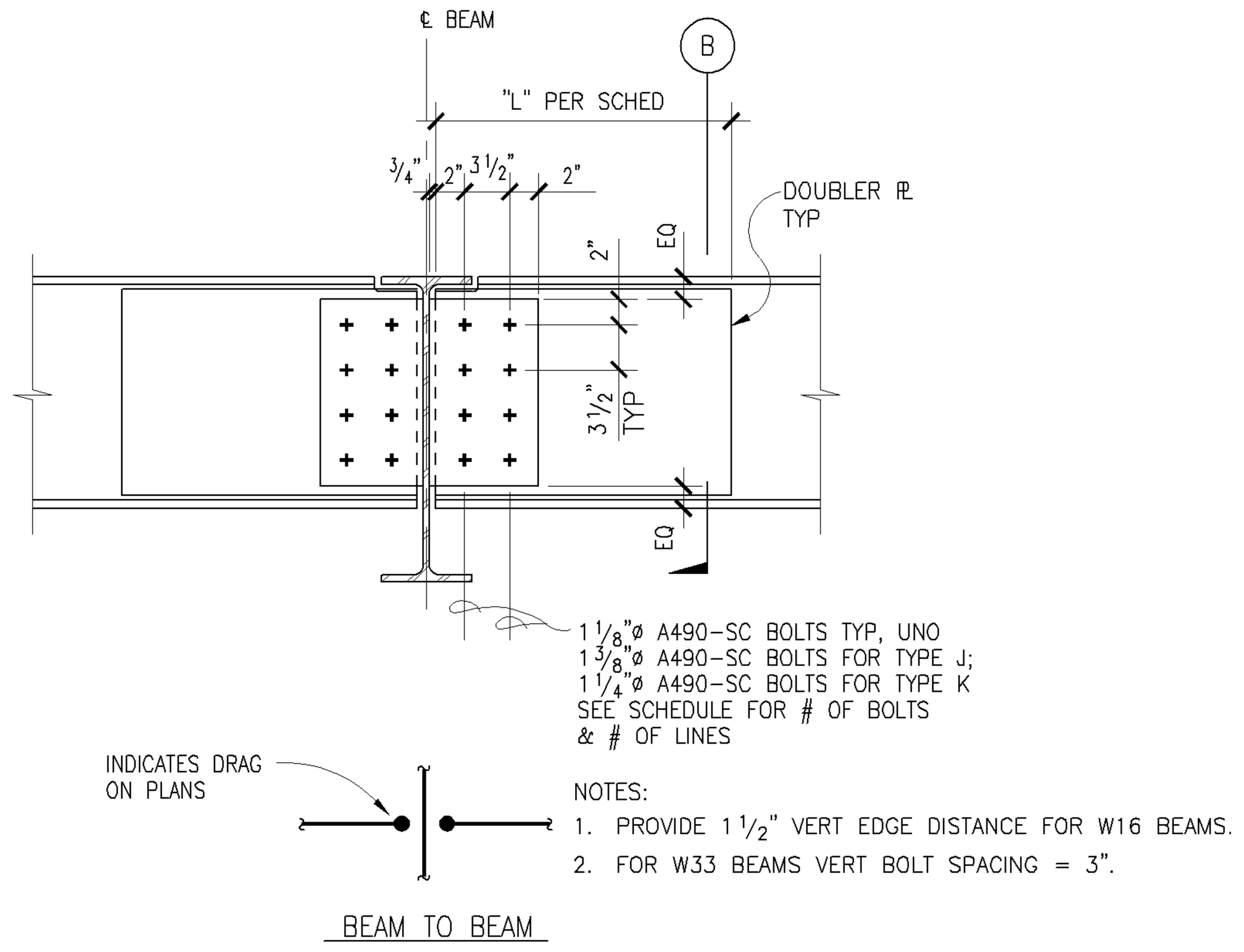
HSS COLUM BASE PLATE DETAIL 1 1/2"=1'-0" 5



F SECTION



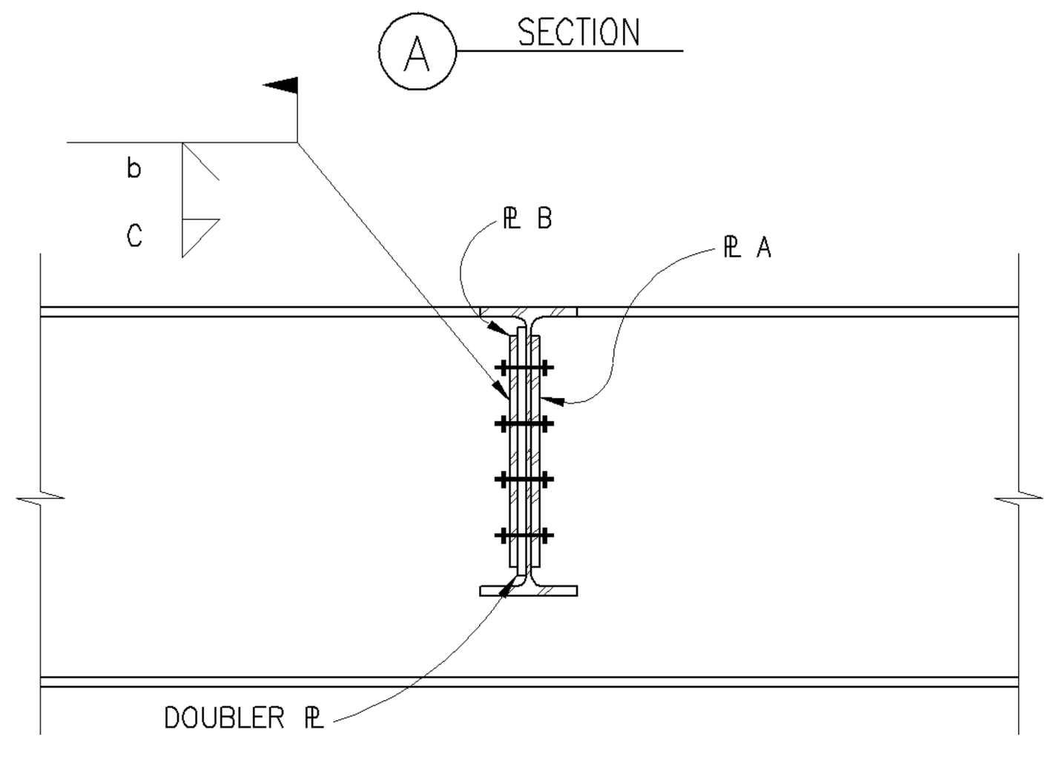
E SECTION



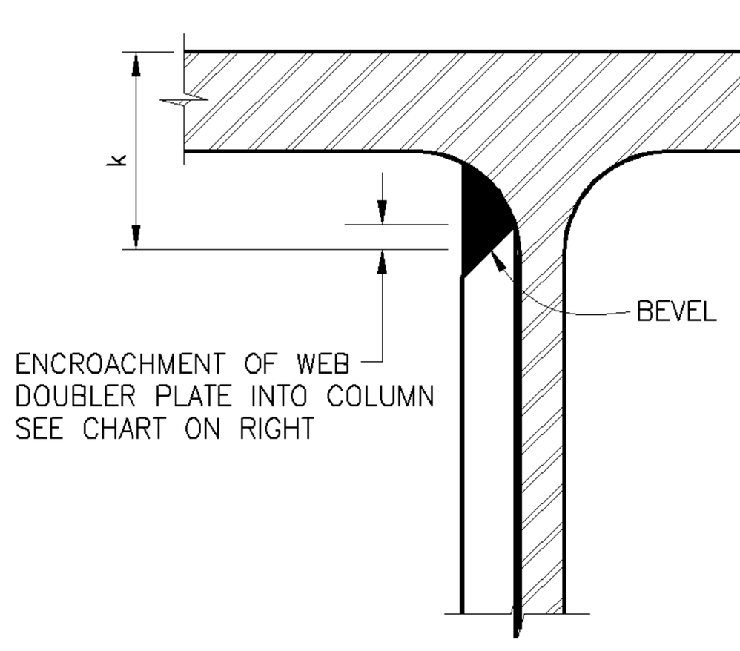
NOTES:
1. PROVIDE 1 1/2" VERT EDGE DISTANCE FOR W16 BEAMS.
2. FOR W33 BEAMS VERT BOLT SPACING = 3".

BEAM TO BEAM

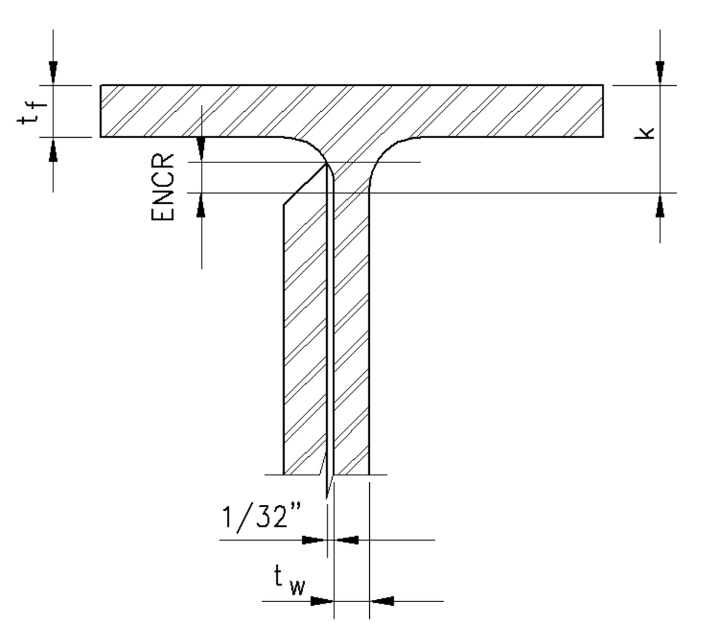
A SECTION



B SECTION

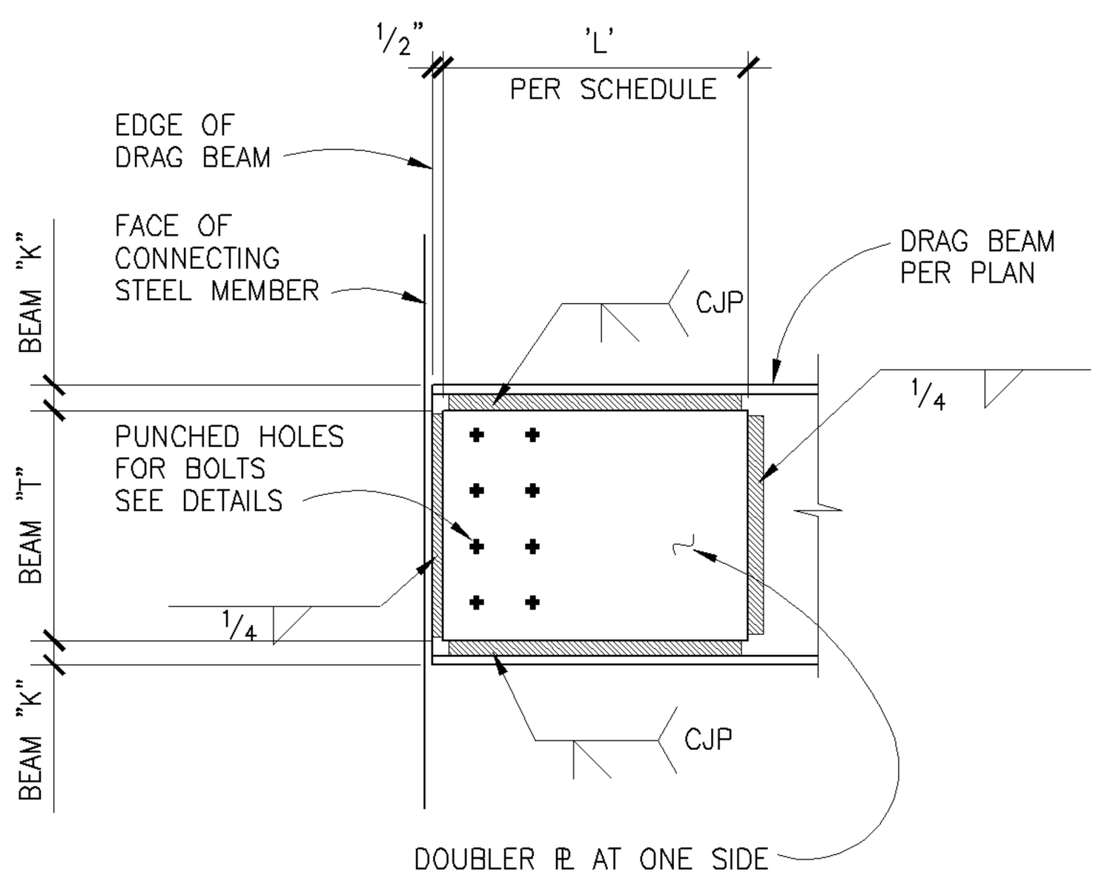


FILLET ENCROACHMENT CHART



$k-t_f$ (in)	encr. (in)
5/16	1/8
3/8	3/16
7/16	3/16
1/2	3/16
9/16	1/4
5/8	1/4
11/16	1/4
3/4	1/4
13/16	1/4
7/8	5/16
1	5/16

G CJP GROOVE-WELDED DETAIL



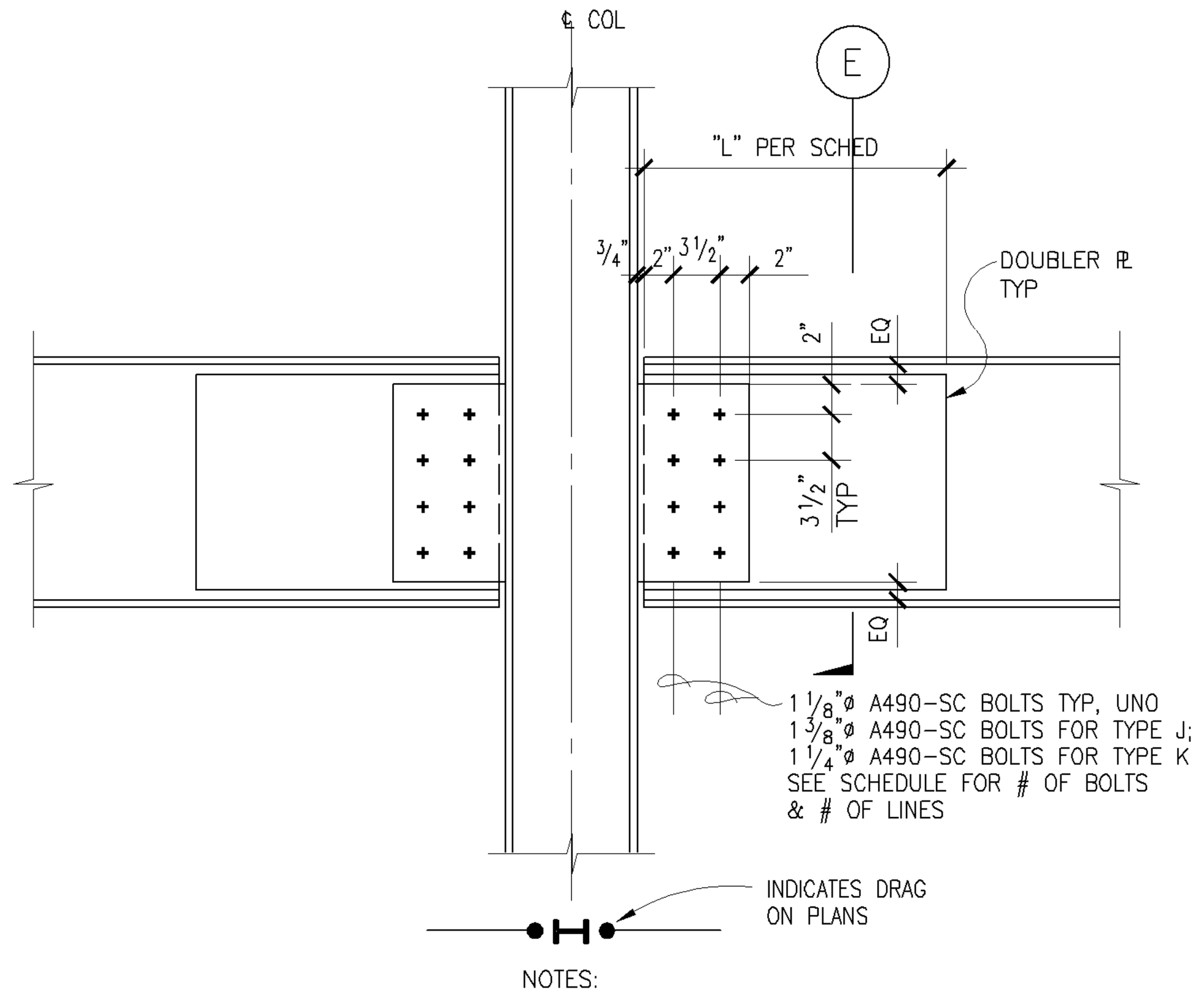
NOTE: BEAM "K" & "T" VALUES ARE FROM AISC "K" & "T" VALUES FOR REQUIRED DRAG BEAM, TYPICAL.

H DOUBLER PLATE ASSEMBLY DETAIL

DRAG CONNECTION SCHEDULE										
CONNECTION TYPE	Wx SIZE	NO. OF LINES	BOLTS/ LINE	PLATE A (IN)	PLATE B (IN)	DBL PLATE THICKNESS (IN)	L (IN)	WELD a (IN)	WELD b (IN)	WELD C (IN)
A	W33x	2	9	1/2	1/2	1/4	12	3/8	3/8	1/4
B	W24x	2	6	1/2	1/2	1/2	12	3/8	3/8	1/4
C	W33x	2	9	1/2	–	–	–	3/8	–	–
D	W33x	2	9	1/2	1/2	–	–	3/8	3/8	1/4
E	W27x	2	7	1/2	1/2	–	–	3/8	3/8	1/4
F	W18x	2	4	1/2	1/2	1/4	12	1/2	1/2	1/4
G	W24x	2	6	1/2	–	–	–	3/8	–	–
H	W30x	2	8	1/2	–	–	–	3/8	–	–
I	W18x	2	4	1/2	–	–	–	3/8	–	–
J	W18x	2	4	5/8	5/8	3/8	12	1/2	1/2	1/4
K	W27x	2	7	1/2	1/2	–	–	1/2	1/2	1/4
L	W36x	2	9	1/2	1/2	–	–	1/2	1/2	1/4
M	W30x	2	8	1/2	1/2	–	–	3/8	3/8	1/4
N	W27x	2	7	1/2	–	–	–	3/8	–	–
O	W36x	2	9	1/2	1/2	–	–	3/8	3/8	1/4
P	W16x	2	4	1/2	1/2	3/8	12	1/2	1/2	1/4
Q	W21x	2	5	1/2	1/2	1/4	12	1/2	1/2	1/4
R	W18x	2	4	1/2	1/2	–	–	3/8	3/8	1/4

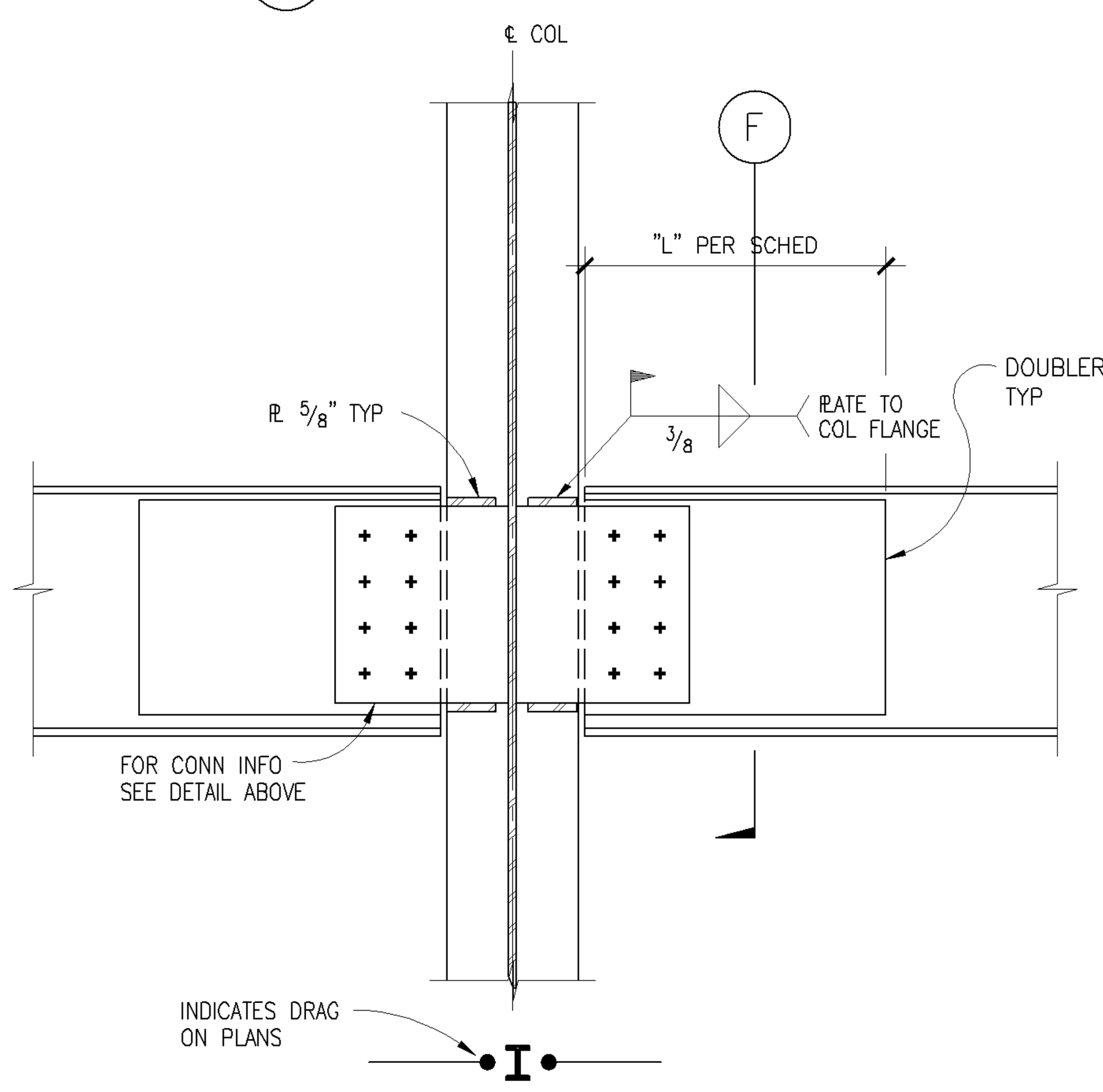
NOTES:

1. ALL PLATE MATERIAL SHALL BE GRADE 50, A572. FOR CONNECTION
2. FOR CONNECTION TYPE J, USE 1 3/8"ø BOLTS
   FOR 1 3/8"ø BOLTS,
   VERT EDGE DISTANCE =1 3/4"
   VERT BOLT SPACING =3 3/8"
   HORIZONTAL EDGE DISTANCE =2 1/8"
   HORIZONTAL BOLT SPACING =4 1/8"
3. FOR CONNECTION TYPE K, USE 1 1/4"ø BOLTS.
   FOR 1 1/4"ø BOLTS,
   VERT EDGE DISTANCE =1 3/4"
   VERT BOLT SPACING =3 1/2"
   HORIZONTAL EDGE DISTANCE =2"
   HORIZONTAL BOLT SPACING =3 3/4"



NOTES:
1. PROVIDE 1 1/2" VERT EDGE DISTANCE FOR W16 BEAMS.
2. FOR W24 BEAMS VERT EDGE DIST= 1 3/4".
3. FOR W27 BEAMS
   VERT EDGE DIST= 1 3/4"
   VERT BOLT SPACING= 3 3/8"
4. FOR W33 BEAMS VERT BOLT SPACING= 3".

C DRAG CONNECTION AT COL FLANGE



NOTE:
1. FOR W33 BEAMS VERT BOLT SPACING= 3".

D DRAG CONNECTION TO COL WEB

DRAG CONNECTION DETAILS 1"=1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

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3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

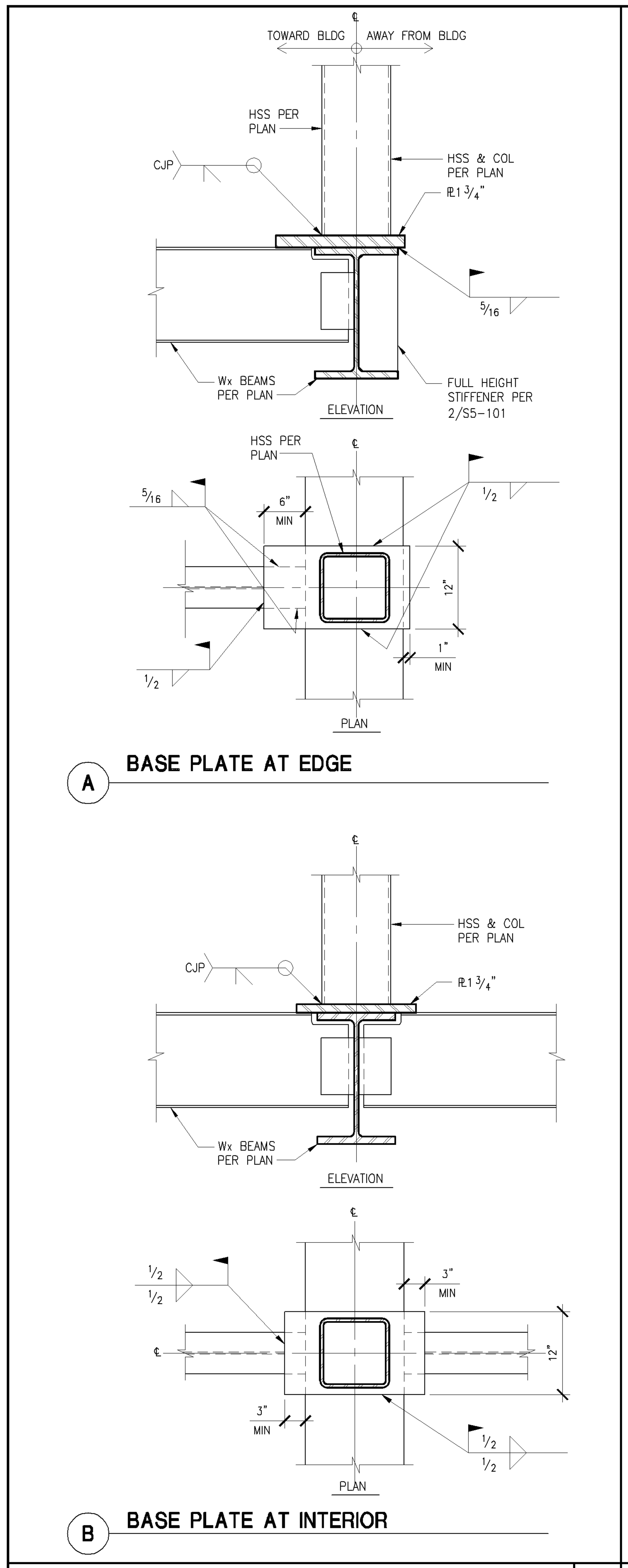
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

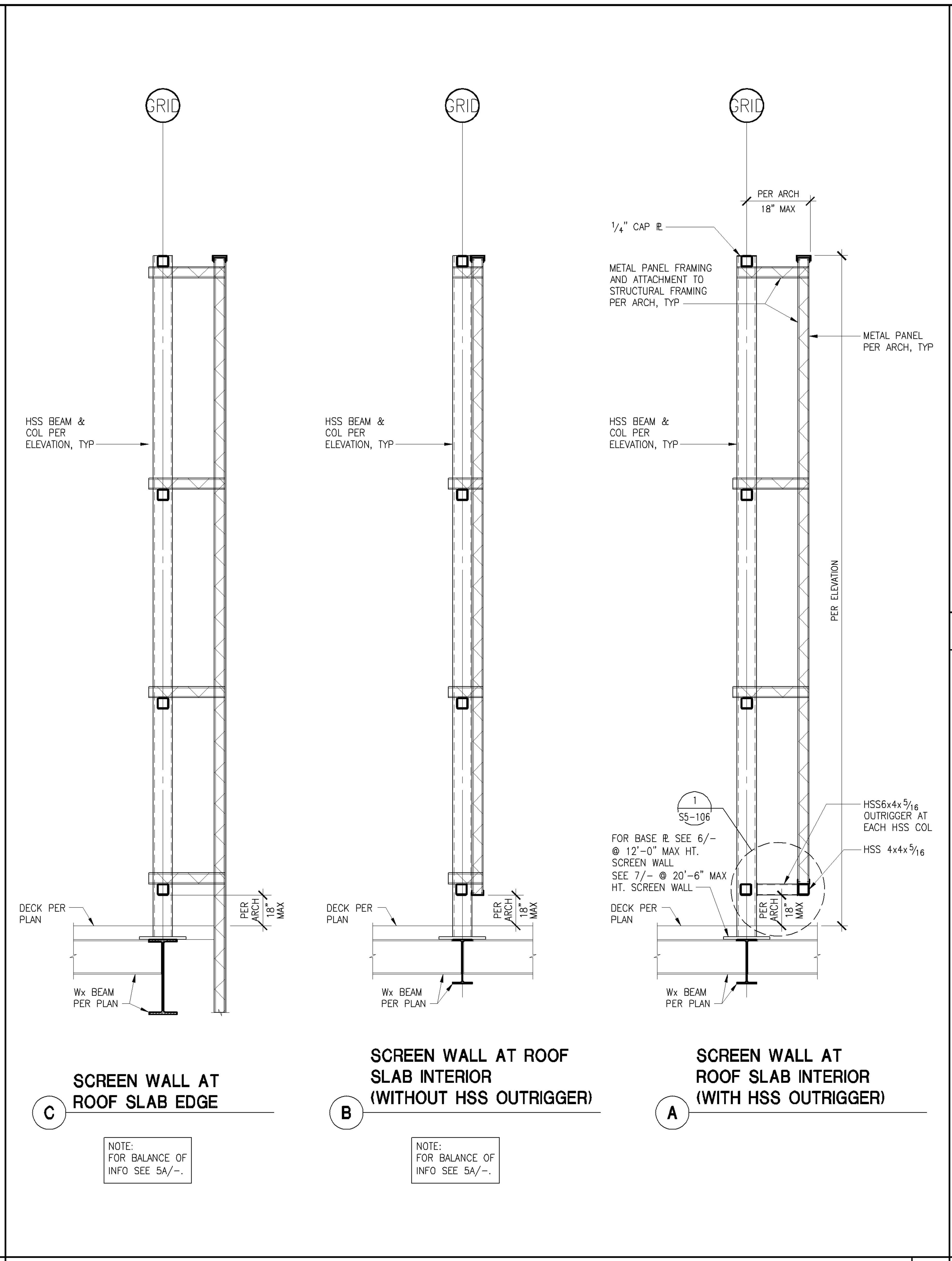
SHEET TITLE

TYPICAL STEEL DETAILS

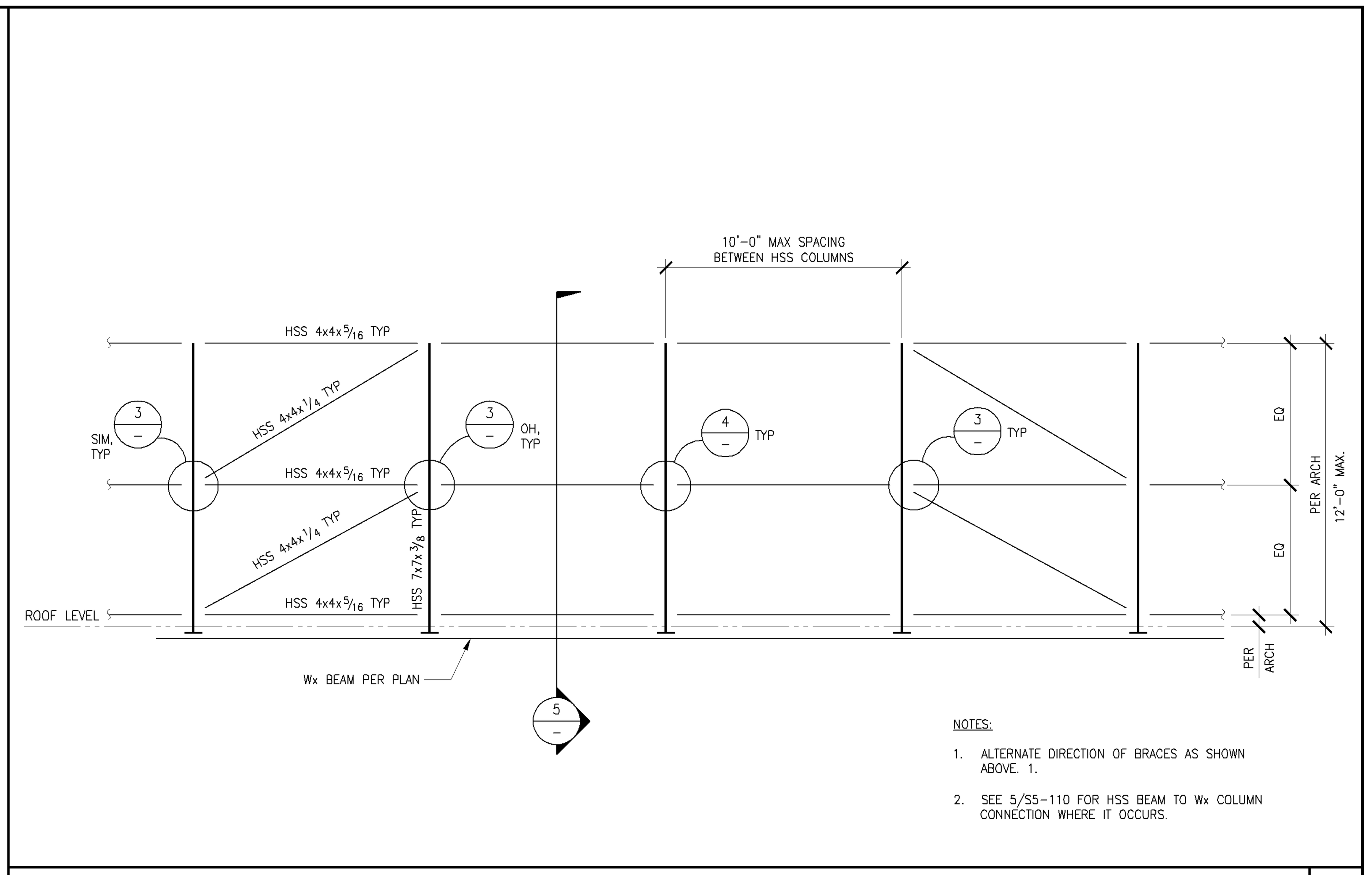
S5-104



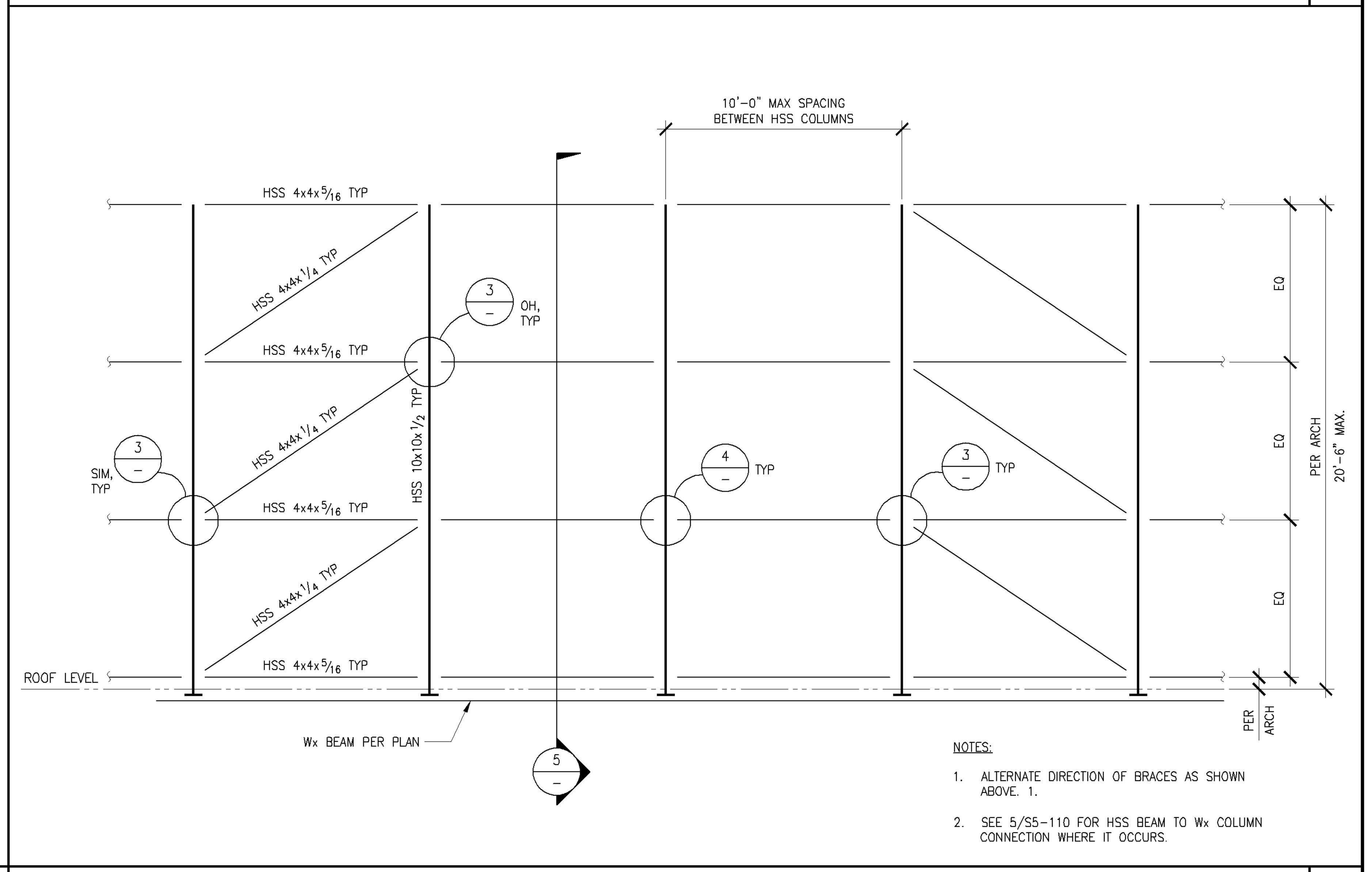
SCREEN BASE PLATE: 20'-6" MAX HT WALL 7



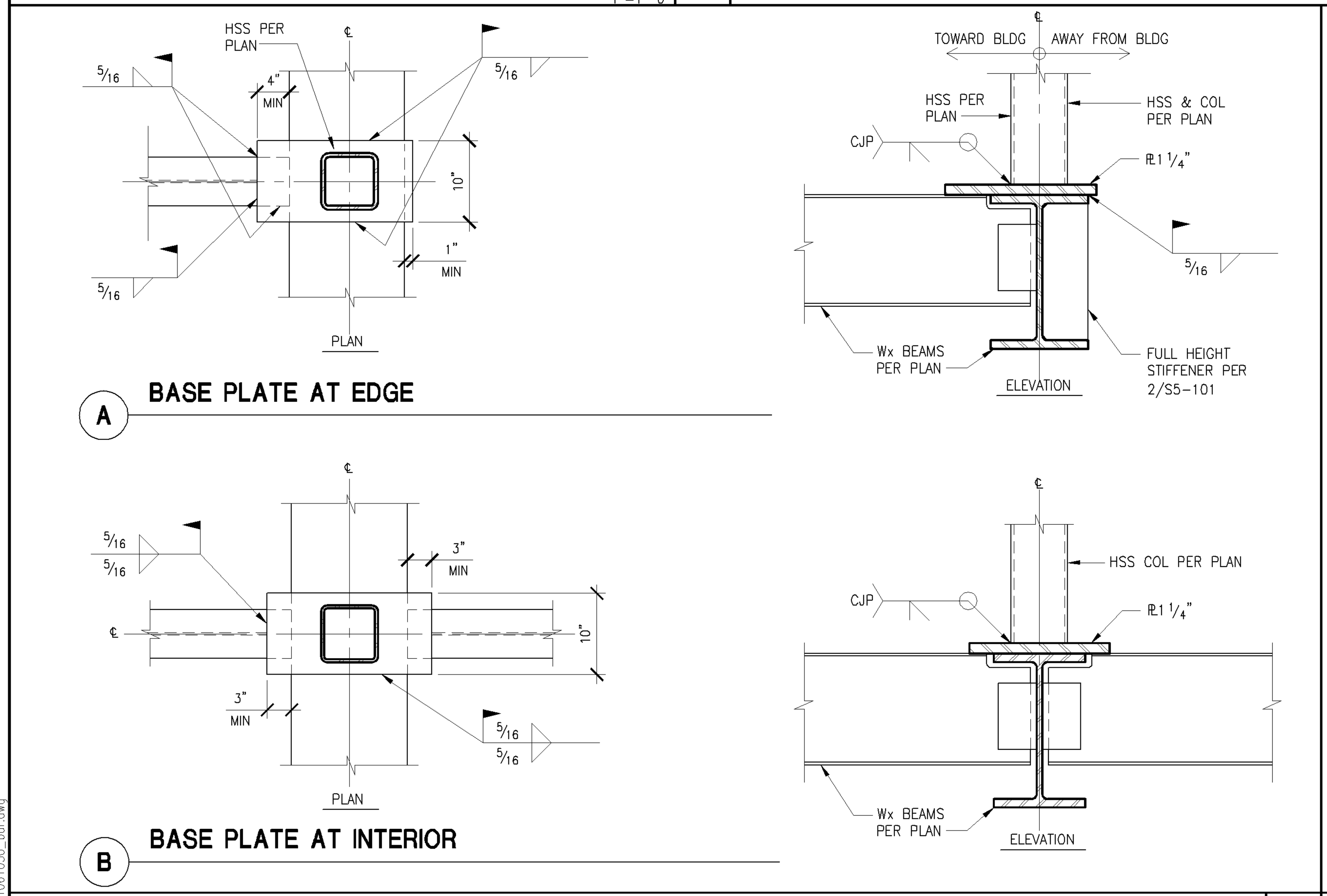
SCREEN WALL SECTIONS 1/2"=1'-0" 5



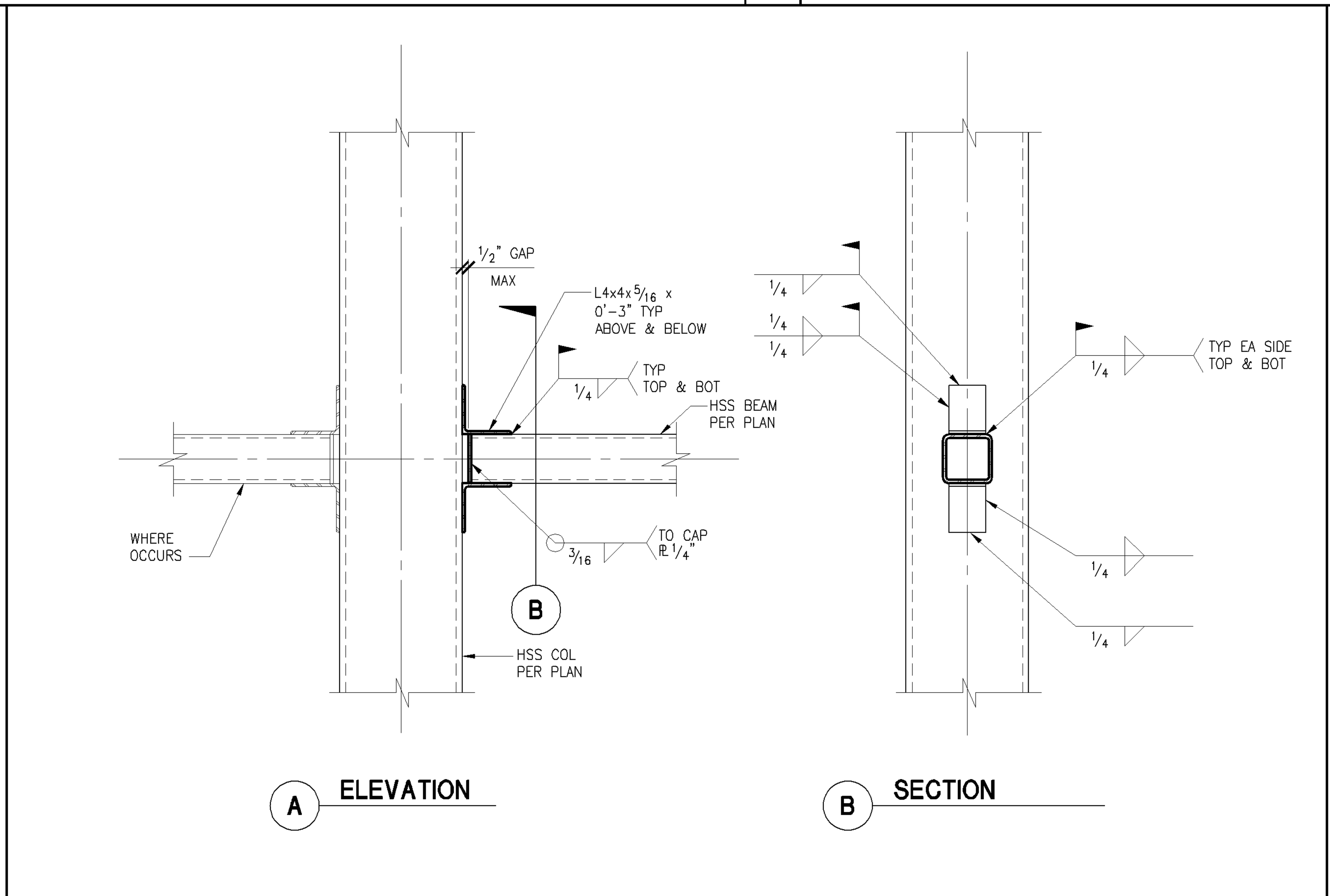
SCREEN WALL FRAMING ELEVATION - 12'-0" MAX HEIGHT 1/4"=1'-0" 1



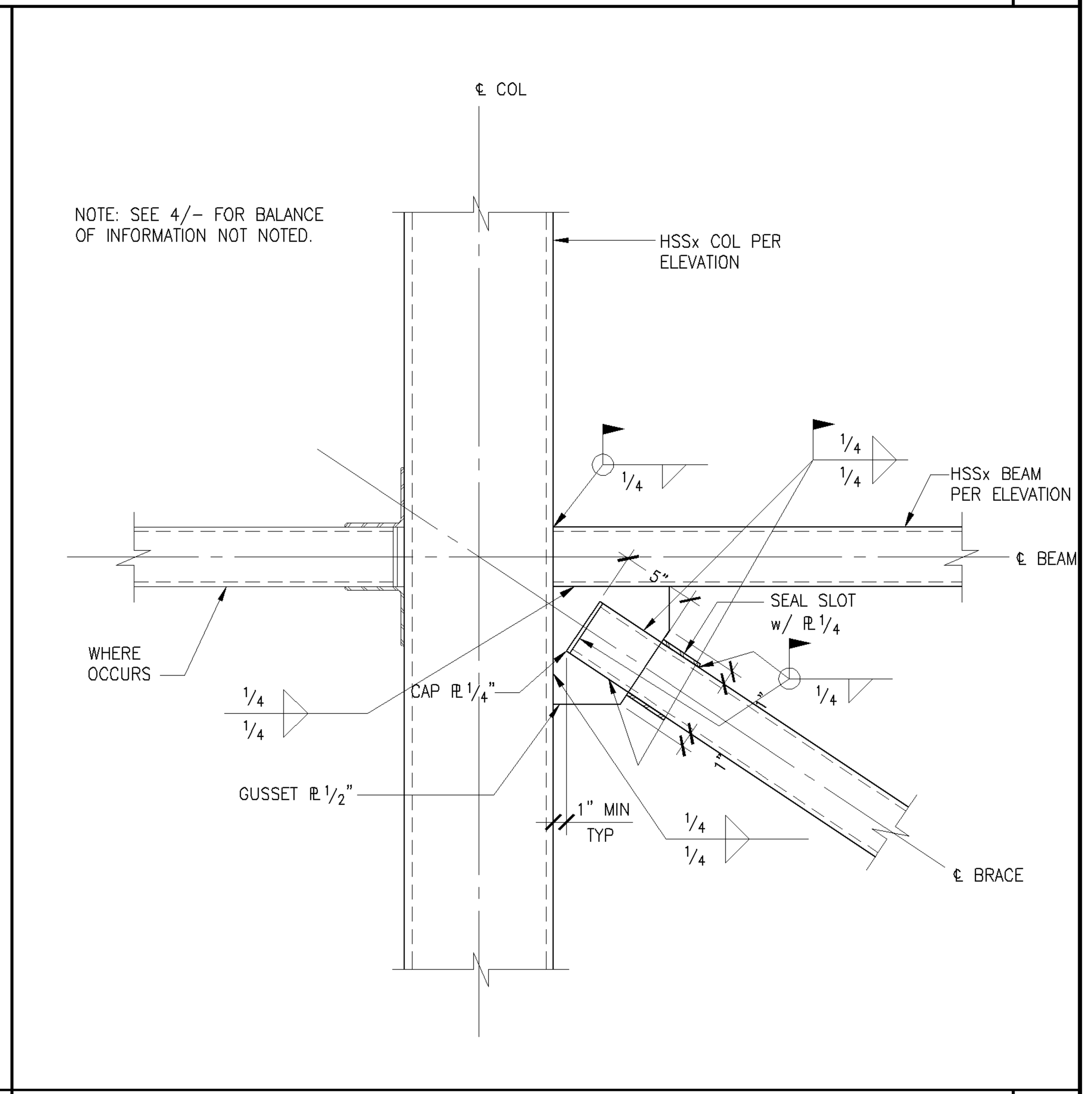
SCREEN WALL FRAMING ELEVATION - 20'-6" MAX HEIGHT 1/4"=1'-0" 2



SCREEN BASE PLATE: 12'-0" MAX HT. WALL 1"=1'-0" 6



NON-FRAME HSSx BM TO COL CONNECTION 1 1/2"=1'-0" 4



FRAME CONNECTION DETAIL 1 1/2"=1'-0" 3

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
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BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

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PROJECT NO: 60004775
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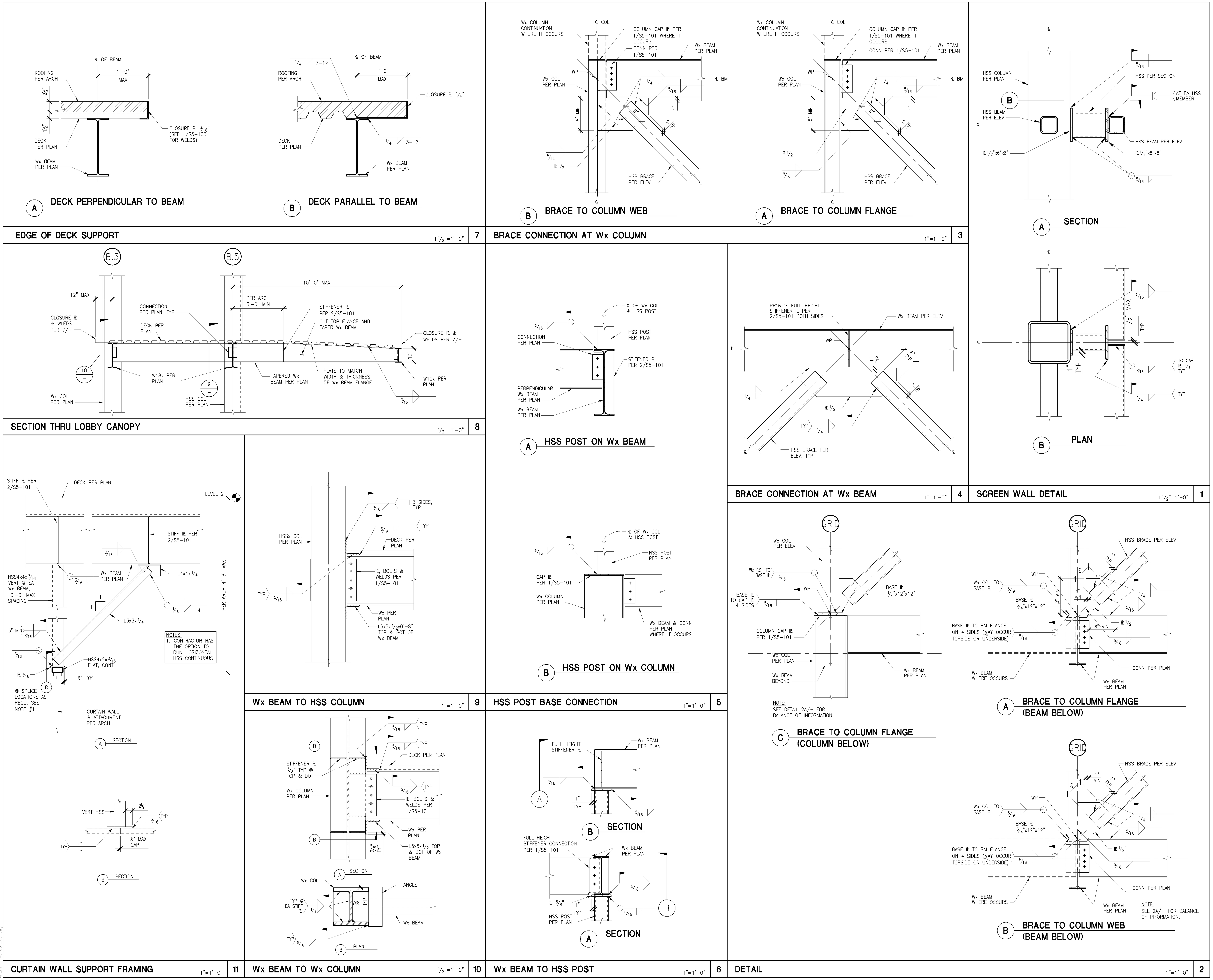
KEY PLAN

SHEET TITLE
STEEL DETAILS

S5-105


A DECK PERPENDICULAR TO BEAM
B DECK PARALLEL TO BEAM
EDGE OF DECK SUPPORT 1 1/2"=1'-0" 7
B BRACE TO COLUMN WEB
A BRACE TO COLUMN FLANGE
BRACE CONNECTION AT Wx COLUMN 1"=1'-0" 3
A SECTION
B PLAN
SCREEN WALL DETAIL 1 1/2"=1'-0" 1
SECTION THRU LOBBY CANOPY 1/2"=1'-0" 8
A HSS POST ON Wx BEAM
B HSS POST ON Wx COLUMN
HSS POST BASE CONNECTION 1"=1'-0" 5
BRACE CONNECTION AT Wx BEAM 1"=1'-0" 4
NOTES:
1. CONTRACTOR HAS THE OPTION TO RUN HORIZONTAL HSS CONTINUOUS
A SECTION
B SECTION
CURTAIN WALL SUPPORT FRAMING 1"=1'-0" 11
Wx BEAM TO HSS COLUMN 1"=1'-0" 9
A SECTION
B PLAN
Wx BEAM TO Wx COLUMN 1/2"=1'-0" 10
B SECTION
A SECTION
Wx BEAM TO HSS POST 1"=1'-0" 6
NOTE:
SEE DETAIL 2A/– FOR BALANCE OF INFORMATION.
C BRACE TO COLUMN FLANGE (COLUMN BELOW)
A BRACE TO COLUMN FLANGE (BEAM BELOW)
NOTE:
SEE 2A/– FOR BALANCE OF INFORMATION.
B BRACE TO COLUMN WEB (BEAM BELOW)
DETAIL 1"=1'-0" 2

OWNER/CLIENT

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TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

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LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

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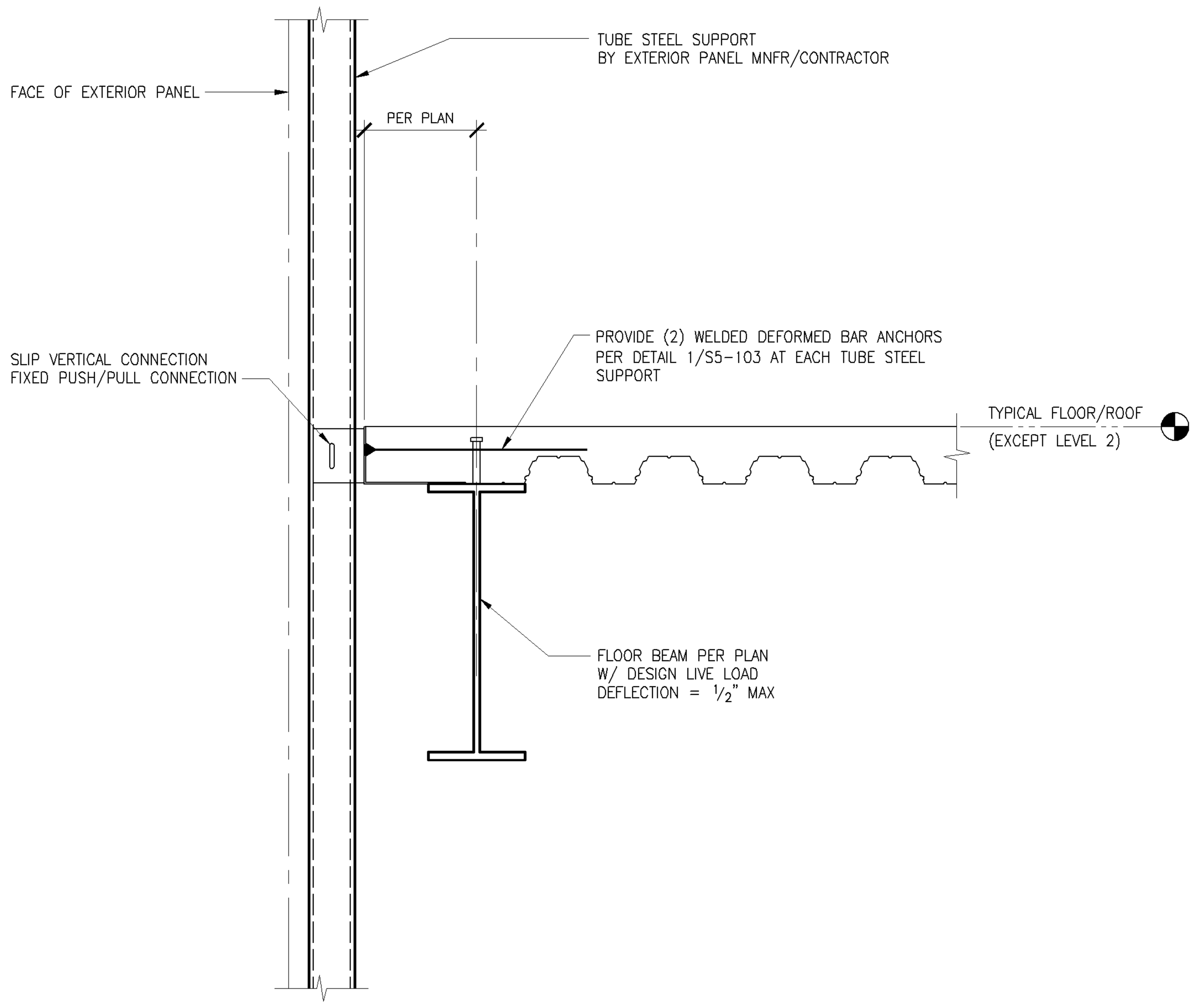
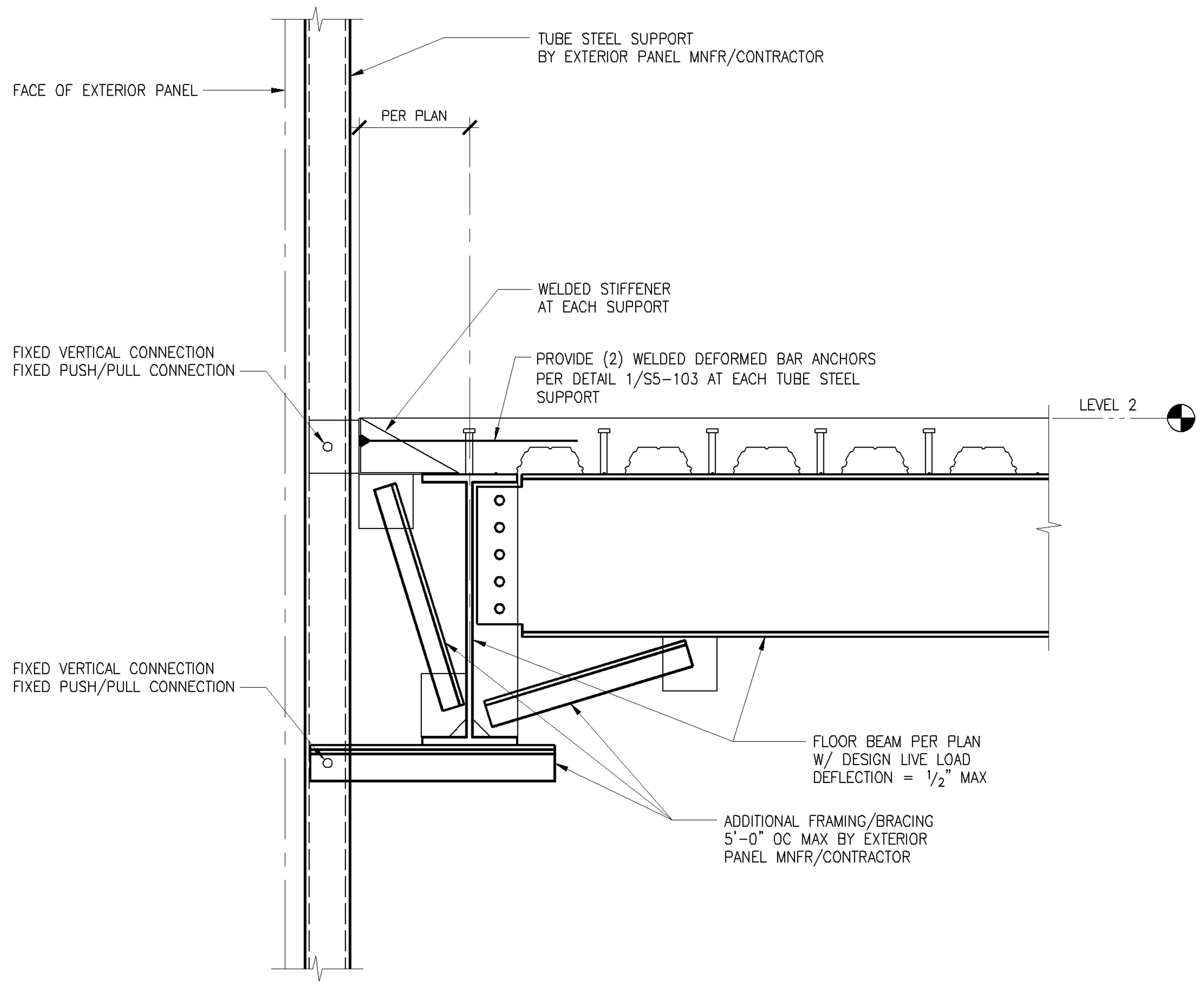
KEY PLAN

SHEET TITLE
STEEL DETAILS

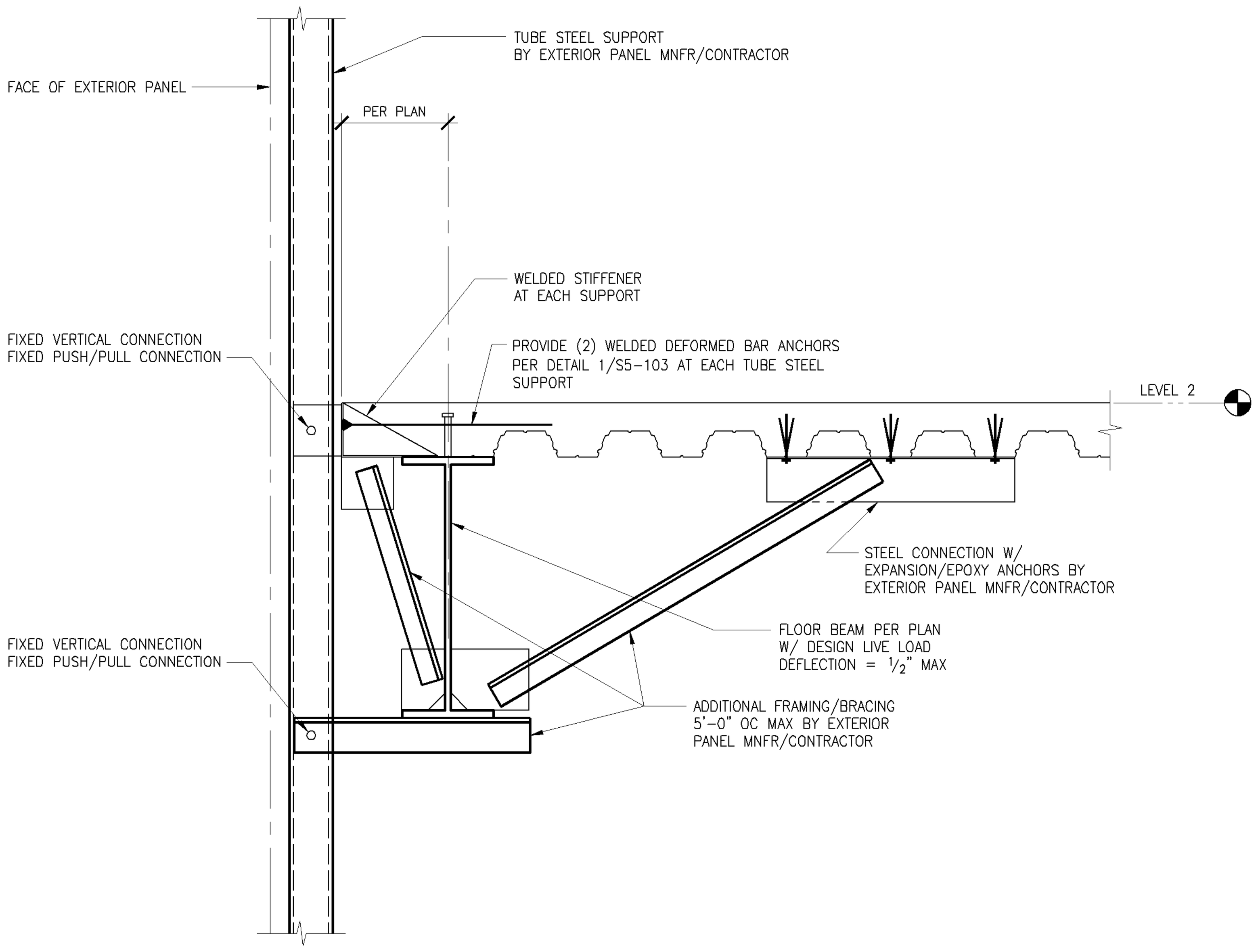
S5-106



EXTERIOR METAL PANEL WALL AT TYPICAL FLOOR/ROOF



EXTERIOR METAL PANEL WALL AT LEVEL 2



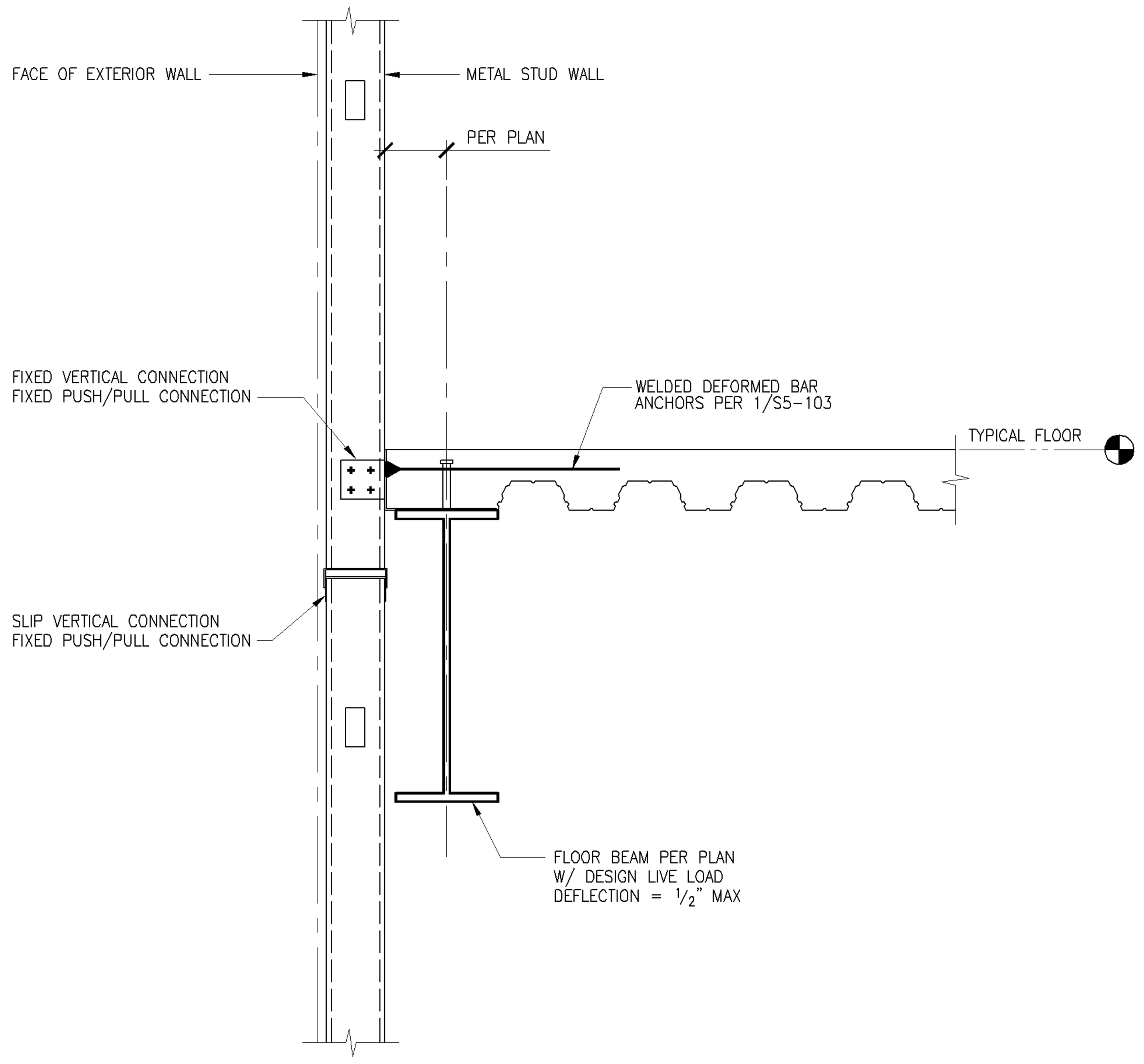
EXTERIOR METAL PANEL WALL AT LEVEL 2

NOTES:

1. SEE GENERAL NOTES ON SHEET S1-101 FOR NOTES REGARDING DEFERRED SUBMITTALS/DESIGN-BUILD ITEMS.
2. GRAVITY SUPPORT OF PANEL FRAMING TO OCCUR AT LEVEL 2 ONLY. ALL OTHER LEVELS SHALL HAVE VERTICAL SLIP CONNECTIONS. MAXIMUM WEIGHT OF EXTERIOR WALL PANELS IS 10 PSF.

TYPICAL EXTERIOR METAL PANEL AT SLAB EDGE

1"=1'-0" 1



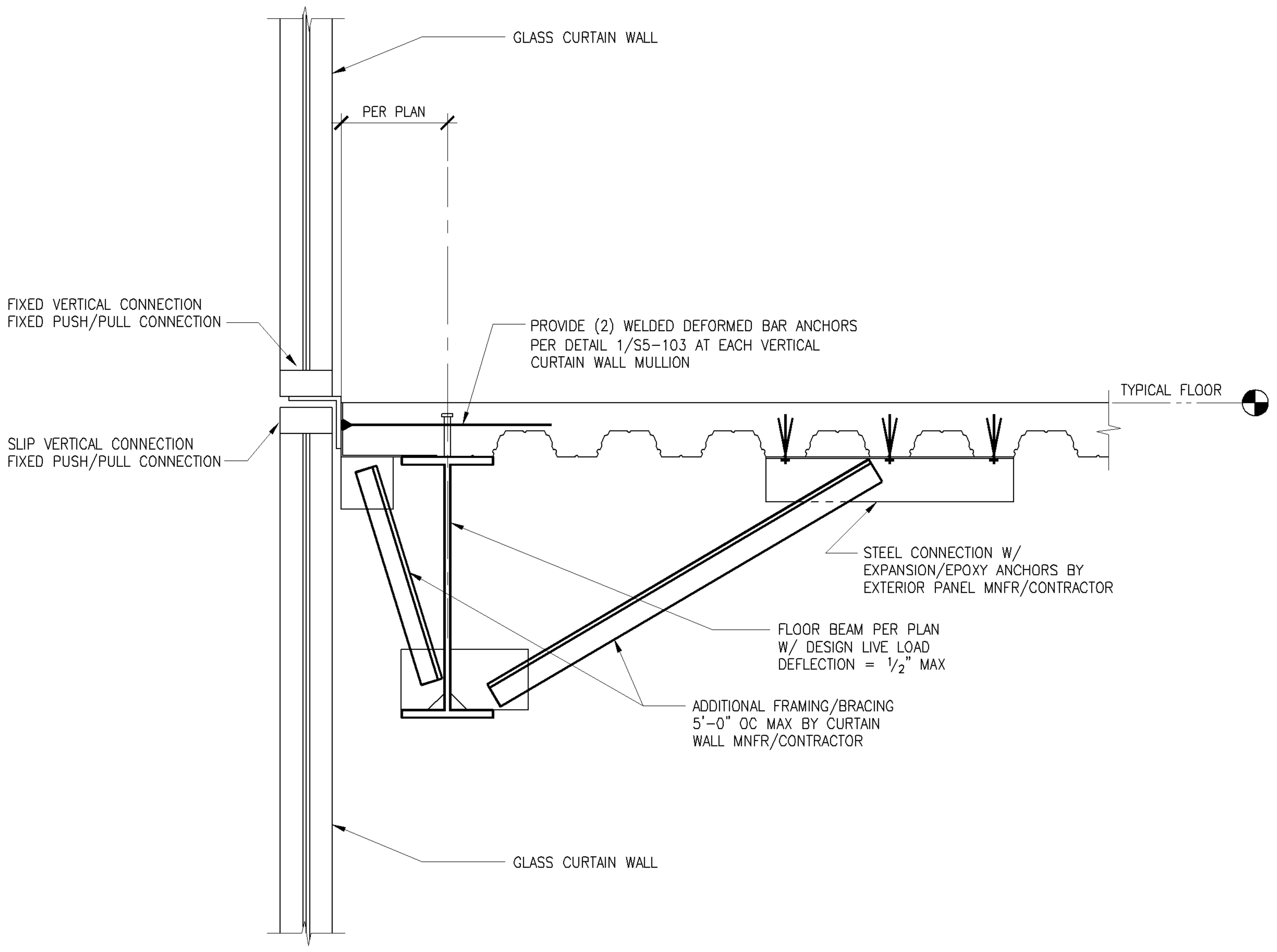
EXTERIOR METAL STUD WALL AT TYPICAL FLOOR

NOTES:

1. SEE GENERAL NOTES ON SHEET S1-101 FOR ITEMS REGARDING DEFERRED SUBMITTALS/DESIGN-BUILD ITEMS.

TYPICAL EXTERIOR METAL STUD AT SLAB EDGE

1"=1'-0" 3



TYPICAL GLASS CURTAIN WALL AT TYPICAL FLOOR

NOTES:

1. SEE GENERAL NOTES ON SHEET S1-101 FOR ITEMS REGARDING DEFERRED SUBMITTALS/DESIGN-BUILD ITEMS.

TYPICAL GLASS CURTAIN WALL AT SLAB EDGE

1"=1'-0" 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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PASADENA, CA 91101

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CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

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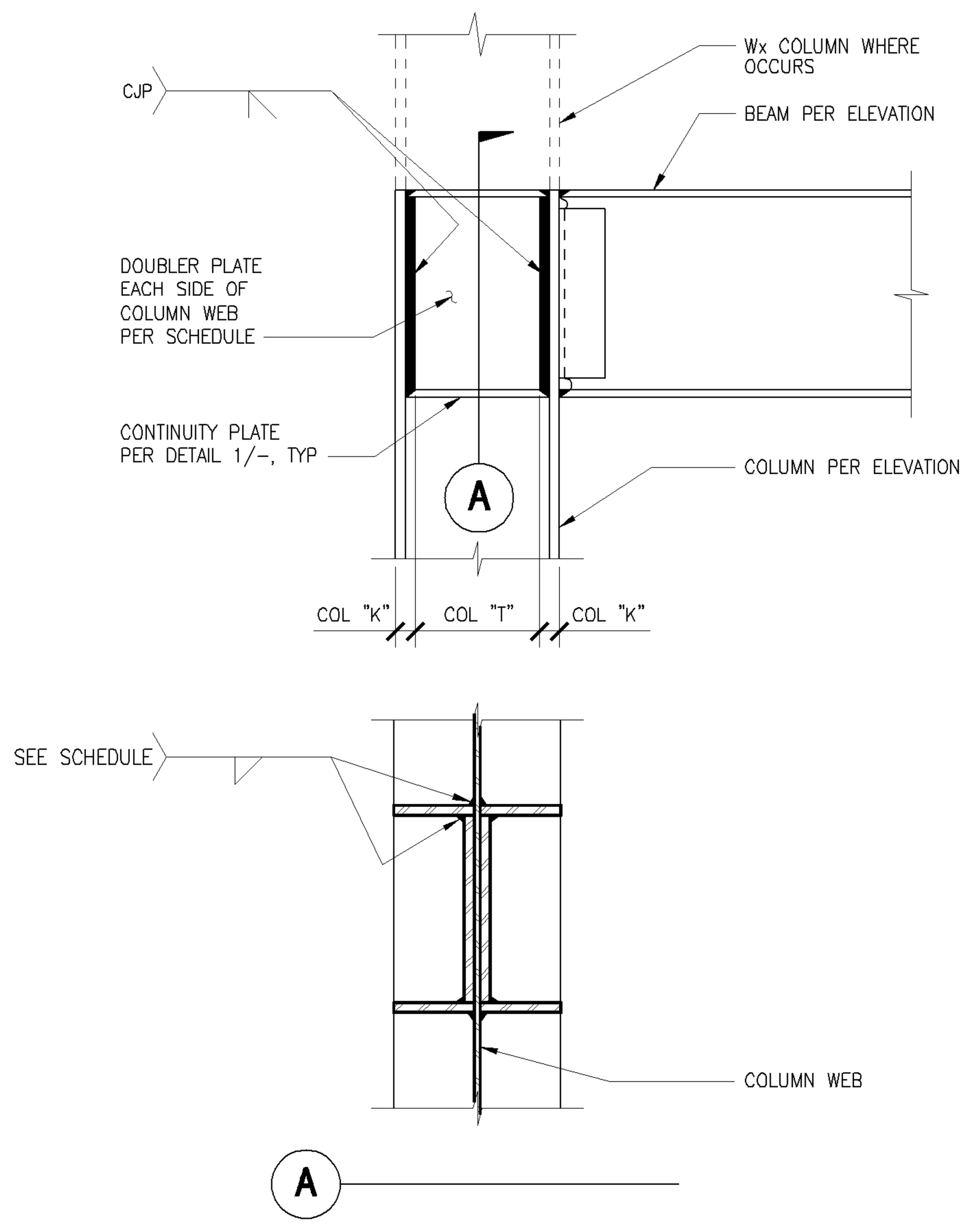
KEY PLAN

SHEET TITLE

EXTERIOR SLAB EDGE SECTIONS

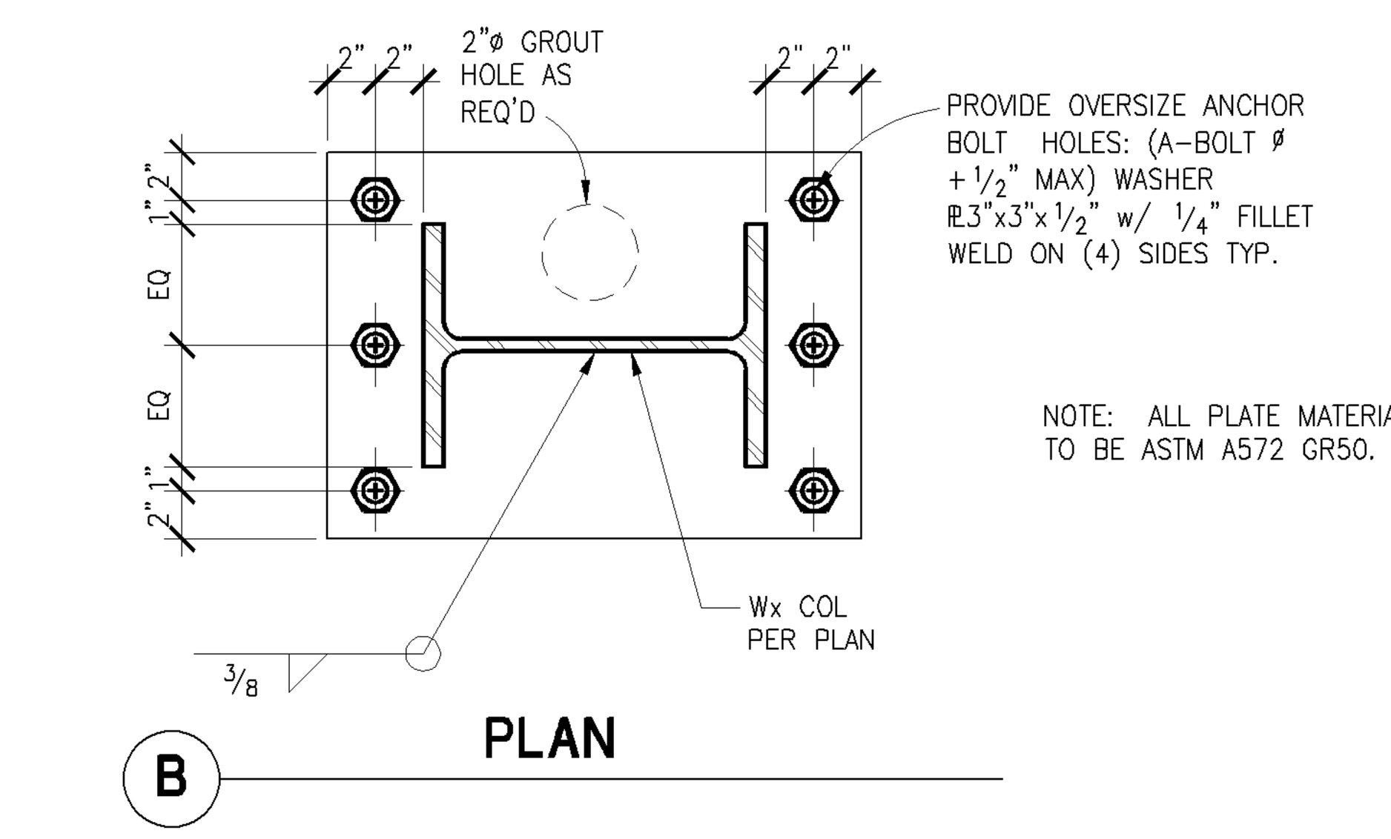
S5-107



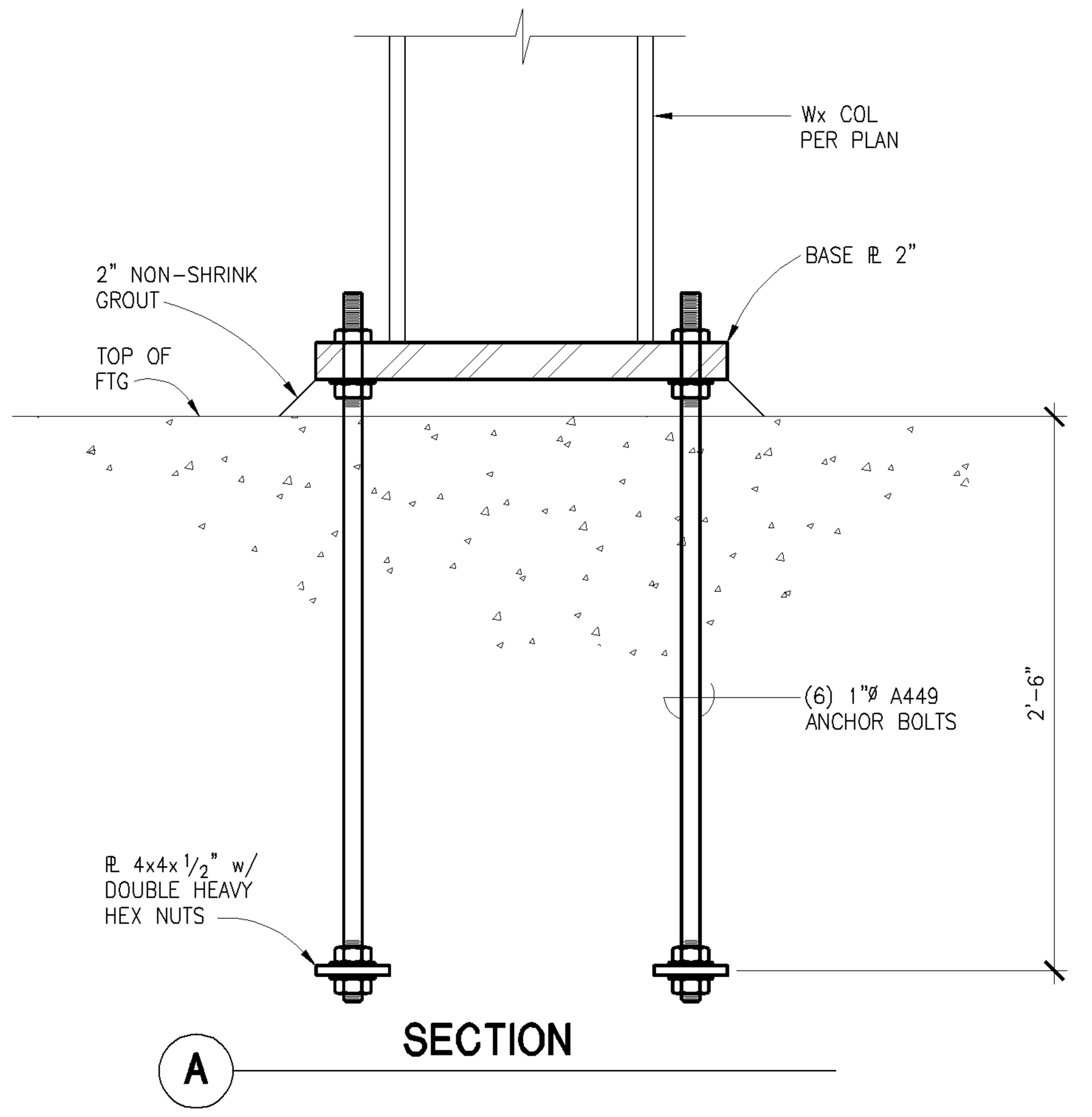
NOTE:
COL "K" AND COL "T" ARE FROM AISC "K" AND "T" VALUES FOR COLUMN.

DOUBLER PLATE SCHEDULE		
BEAM SIZE	DOUBLER PLATE	FILLET WELD
W18x86	3/8" EACH SIDE	5/16"
W18x65	3/8" EACH SIDE	5/16"
W18x50	3/8" EACH SIDE	5/16"

DOUBLER PLATE SCHEDULE AND DETAIL
NTS | 4



NOTE: ALL PLATE MATERIAL TO BE ASTM A572 GR50.



BASE PLATE DETAIL
SCALE: 1 1/2"=1'-0" | 5

STRUCTURAL STEEL WELDING (CONT)

18. REQUIREMENTS FOR WELDING THE BOTTOM FLANGE SHALL BE AS FOLLOWS:

A. THE ROOT PASS SHALL BEGIN IN THE CENTER OF THE JOINT, IN THE AREA OF THE WELD ACCESS HOLE, REACHING PAST THE BEAM WEB THROUGH THE COPE HOLE WHEN NEAR END OF WELDING. AFTER THE ARC IS INITIATED, TRAVEL SHALL PROGRESS TOWARD BOTH EDGES OF THE BEAM FLANGE, AND THE WELD SHALL BE TERMINATED ON THE WELD RUN OFF TAB.

B. THE HALF LENGTH ROOT PASS SHALL BE THOROUGHLY CLEANED.

C. THE START OF THE WELD IN THE WELD ACCESS HOLE AREA SHALL BE VISUALLY INSPECTED TO ENSURE FUSION, SOUNDNESS, FREE FROM SLAG INCLUSIONS AND EXCESSIVE POROSITY. THE RESULTING BEAD PROFILE SHALL BE SUITABLE FOR OBTAINING GOOD FUSION BY THE SUBSEQUENT PASS TO BE INITIATED ON THE OPPOSITE SIDE OF THE BEAM WEB. IF THE PROFILE IS NOT CONDUCIVE TO GOOD FUSION, THE START OF THE FIRST ROOT PASS SHALL BE GROUND, GOUGED CHIPPED, OR OTHERWISE PREPARED TO ENSURE ADEQUATE FUSION.

D. THE SECOND HALF OF THE WELD JOINT SHALL HAVE THE ROOT PASS APPLIED BEFORE ANY OTHER WELD PASSES ARE PERFORMED. THE ARC SHALL BE INITIATED IN THE AREA OF THE START OF THE FIRST ROOT PASS, AND TRAVEL SHALL PROGRESS TO THE END OF THE JOINT, TERMINATING ON THE WELD TAB.

E. EACH WELD LAYER SHALL BE COMPLETED ON BOTH SIDES OF THE JOINT BEFORE A NEW LAYER IS DEPOSITED.

19. ADHERE TO SECTION 5.3.2.1 OF AWS D1.1 FOR STORAGE OF ELECTRODES.

20. BACKING BAR SHALL BE REMOVED UPON COMPLETION OF CONNECTION.

21. BACKING BARS SHALL ALSO BE REMOVED AT ALL MOMENT FRAME COLUMN SPLICE LOCATIONS.

WELD TESTING & INSPECTION

1. TESTING SHALL BE IN ACCORDANCE WITH CBC SECTION 1703.

2. APART FROM VISUAL INSPECTION AND REVIEW OF FABRICATION AND ERECTION REPORTS OF THE FABRICATOR/ERECTOR'S OWN QUALITY CONTROL TESTING AND INSPECTION, THE OWNER'S STEEL TESTING AGENCY SHALL PERFORM THE INDICATED SHOP AND FIELD INSPECTION AND TESTING. THE STEEL TESTING AGENCY SHALL BE AWS Q.C.-1 CERTIFIED BY THE LOCAL BUILDING AUTHORITY AND SHALL PROVIDE REGISTERED INSPECTORS FOR CONTINUOUS INSPECTION OF ALL STEEL FABRICATION AND ERECTION OF THE MOMENT FRAMES.

A. ULTRASONIC TESTING IS REQUIRED FOR ALL (100%) PARTIAL AND COMPLETE PENETRATION WELDS. TEST GROOVE WELDING ON CONTINUITY PLATES BY ULTRASONIC TESTING AFTER BEAM FLANGE WELD CONNECTION. TESTING SHALL BE PERFORMED 24 HOURS OR MORE AFTER COMPLETION OF WELDING. WELD BACKING REMOVAL AREAS AND FILLET WELDS SHALL BE SUBJECTED TO MAGNETIC PARTICLE EXAMINATION.

B. CHECK A MINIMUM OF 10% OF FILLET WELDS BY MAGNETIC PARTICLE (ASTM 109 METHOD). CHECK A MINIMUM OF 25% OF CONTINUITY PLATE FILLET WELDS AND BEAM FILLET WELDS (100% MOMENT IN ZONES) BY MAGNETIC PARTICLE.

C. BASE METAL THICKER THAN 1-1/2, SUBJECTED TO THROUGH THICKNESS WELD SHRINKAGE SHALL BE ULTRASONICALLY TESTED DIRECTLY BEHIND SUCH WELDS 48 HOURS OR MORE AFTER COMPLETION OF WELDING.

D. ULTRASONICALLY TEST FOR LAMINATIONS IN COLUMN FLANGE PLATES AT ALL MOMENT CONNECTIONS AND BASE PLATES WHERE COLUMN FLANGE OR BASE PLATE THICKNESS EXCEEDS 1 ½" MAXIMUM. EVALUATE BY TESTING A MINIMUM ZONE EXTENDING 6' ABOVE AND BELOW THE CENTER LINE OF EACH BEAM FLANGE. ALL TESTING SHALL BE PERFORMED ON THE MATERIAL PRIOR TO FABRICATION, AFTER FABRICATION, AND AFTER FINAL WELDING OF THE BEAM.

STRUCTURAL STEEL WELDING

1. ALL WELDING SHALL BE IN STRICT CONFORMANCE WITH THE 2001 CALIFORNIA BUILDING CODE AND AWS D1.1 LATEST REVISION INCLUDING:

SECTION 3.5 FOR SECTIONS FOR PREHEAT AND INTERPASS TEMPERATURE REQUIREMENTS.
SECTION 4 FOR QUALIFICATION.
SECTION 5 FOR FABRICATION.
SECTION 6 FOR INSPECTION.

2. A PRE-CONSTRUCTION MEETING BETWEEN THE ENGINEER OF RECORD, THE FABRICATOR, THE ERECTOR, THE CONTRACTOR AND THE INSPECTORS SHALL TAKE PLACE TO DISCUSS THE WELDING PROCEDURE SPECIFICATIONS (WPS) PRIOR TO ANY WORK BEING PERFORMED. ALL WELDERS AND INSPECTORS SHALL BE INFORMED OF AND MUST ADHERE TO THE WPS.

3. THE CONTRACTOR SHALL PROVIDE THE WELDING PROCEDURE SPECIFICATIONS (WPS) AND DETAILED SEQUENCE OF WELDING SKETCH FOR REVIEW AT THE TIME OF SHOP DRAWING SUBMITTAL. THE SEQUENCE OF WELDING SHALL BE PLANNED TO MINIMIZE LOCKED IN STRESSES AND DISTORTION.

4. ALL WELDING ELECTRODES (FILLER METAL) SHALL BE E7XXX (70 KSI), U.N.O. EXCEPT E8XXX (80 KSI), SHALL BE USED AT COLUMN SPLICES BETWEEN TWO HIGH STRENGTH (65 KSI) COLUMNS.

5. COMPLETE PENETRATION GROOVE WELD SHALL HAVE A FILLER METAL WITH CHARPY V-NOTCH TOUGHNESS OF 20 FT/LBS AVERAGE AT - 20 DEGREES FAHRENHEIT.

6. CERTIFY CONFORMANCE TO CHARPY V-NOTCH TOUGHNESS REQUIREMENTS WITH TESTS BY AN INDEPENDENT TESTING LABORATORY FOR EACH AWS CLASSIFICATION, MANUFACTURER AND TRADE NAME. THE SIZES SPECIFIED BY AWS SHALL BE TESTED.

7. FIELD WELDING OF FULL PENETRATION WELDS AND PARTIAL PENETRATION WELDS AT THE STEEL MOMENT FRAME CONNECTIONS SHALL BE PERFORMED USING LOW HYDROGEN ELECTRODES IN CONFORMANCE WITH THE AWS.

8. LENGTHS OF WELDS ARE EFFECTIVE LENGTHS AS SPECIFIED IN THE UNIFORM BUILDING CODE WHERE LENGTH OF WELD IS NOT SHOWN IT SHALL BE FULL LENGTH OF JOINT. ALL BUTT/GROOVE WELDS SHALL BE COMPLETE PENETRATION, UNLESS NOTED OTHERWISE.

9. ALL SHOP WELDS SHALL BE PERFORMED BY A FABRICATOR LICENSED BY THE BUILDING DEPARTMENT.

10. WELDERS SHALL BE QUALIFIED FOR THE WORK THEY WILL BE DOING AND SHALL HAVE CURRENT CERTIFICATIONS CERTIFIED BY THE BUILDING DEPARTMENT.

11. FACES OF FILLET WELDS EXPOSED TO VIEW SHALL HAVE AS-WELDED SURFACES THAT ARE REASONABLY SMOOTH AND UNIFORM. NO FINISHING OR GRINDING SHALL BE REQUIRED, EXCEPT WHERE CLEARANCES OR FIT OF OTHER ITEMS MAY SO NECESSITATE.

12. ALL PARTIAL AND COMPLETE PENETRATION WELDS WHICH ARE EXPOSED TO VIEW SHALL BE GROUND SMOOTH AND FLUSH WITH FINISH SURFACE OF STEEL. HOLES SHALL BE FILLED WITH WELD METAL OR BODY SOLDER AND SMOOTHED BY GRINDING OR FILING.

13. CLEAN GROOVE PREPARATION THERMAL CUTS BY GRINDING.

14. WELDS SHALL BE TERMINATED AT THE END OF A JOINT IN A MANNER THAT WILL ENSURE SOUND WELDS. WHENEVER NECESSARY THIS SHALL BE DONE BY USE OF EXTENSION BARS AND RUN OFF TABS. RUN OFF TABS USED AT BEAM FLANGE CONNECTIONS SHALL BE REMOVED AND THE ENDS OF THE WELDS SHALL BE MADE SMOOTH AND FLUSH WITH THE EDGES OF ABUTTING PARTS PER 5.31 OF AWS D1.1. NO WELD DAMS ARE ALLOWED.

15. AFTER FULL PENETRATION WELDING, THE BACKING BAR IS TO BE REMOVED, THE WELD ROOT INSPECTED AND TESTED FOR IMPERFECTIONS, WHICH IF FOUND, ARE TO BE REMOVED BY BACKGOUGING TO SOUND MATERIAL & CLEANED BY GRINDING IF BACKGOUGED BY AIR ARC. THE BACKGOUGED AREA IS TO BE WELDED AND A FILLET WELD SHALL BE APPLIED TO REINFORCE THE JOINT. THE SIZE OF THE REINFORCING FILLET WELD SHALL BE EQUAL TO ¼ THE PLATE THICKNESS, BUT NOT LESS THAN ¼ NOR MORE THAN 3/8" PER NOTE J OF FIGURE 3.4 OF AWS D1.1.

16. TO ASSURE THE PROPER AMPERAGE AND VOLTAGE OF THE WELDING PROCESS, THE USE OF A HAND HELD CALIBRATED AMP AND VOLT METER SHALL BE USED. THIS EQUIPMENT SHALL BE USED BY THE FABRICATOR, ERECTOR AND THE INSPECTORS. AMPERAGE AND VOLTAGE SHALL BE MEASURED AT THE ARC WITH THIS EQUIPMENT. TRAVEL SPEED AND ELECTRODE STICK OUT SHALL BE VERIFIED TO BE IN COMPLIANCE WITH THE ELECTRODE MANUFACTURER'S RECOMMENDATIONS AND WITH THE APPROVED WPS.

17. THE FOLLOWING PROVISIONS APPLY TO ALL WELDING AT BEAM-COLUMN MOMENT CONNECTIONS:

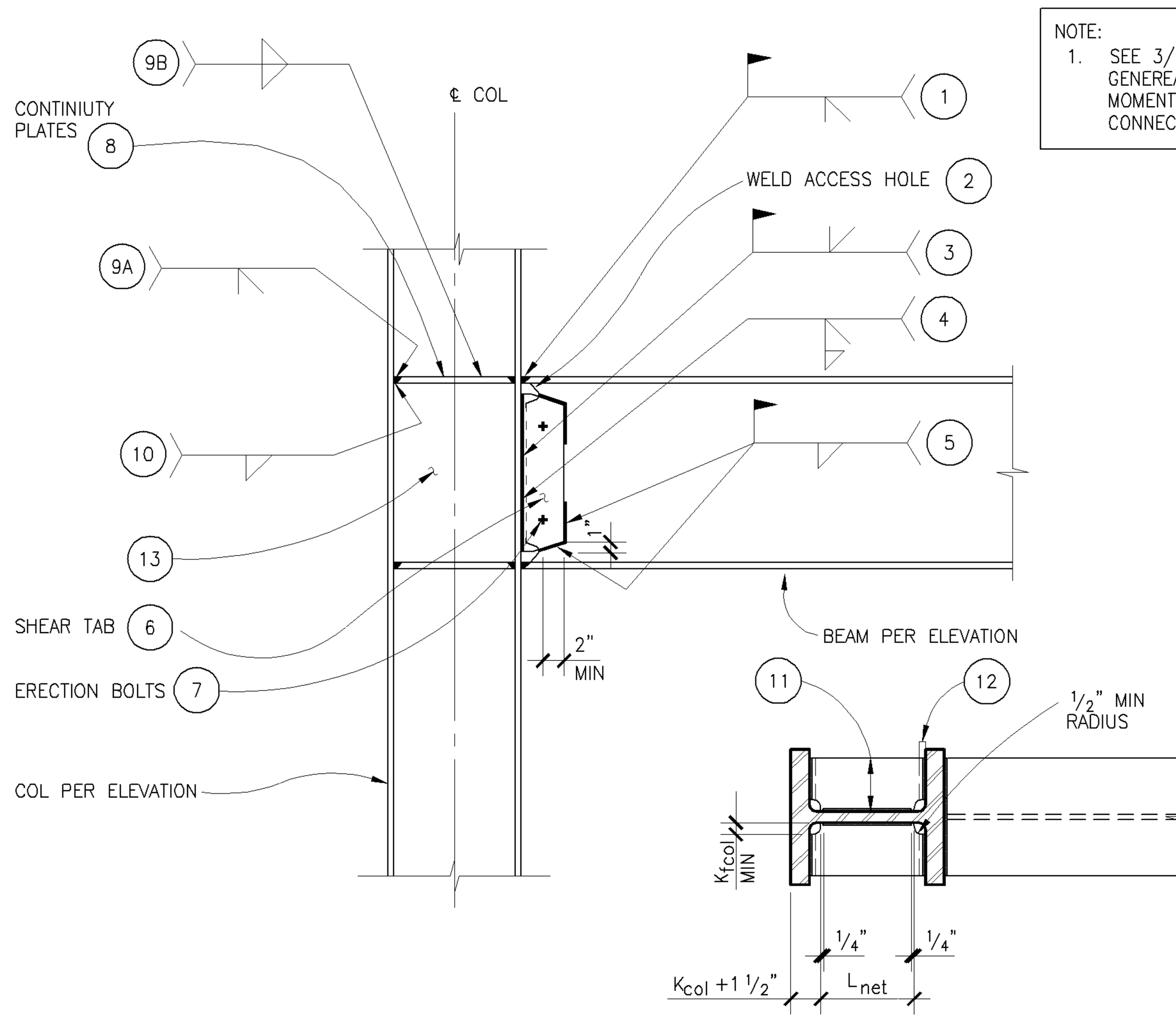
A. MINIMUM INITIAL PREHEAT TO BE 225 DEGREES F MEASURED + - ONE FOOT FROM THE WELD JOINT. FOR JUMBO SECTIONS MINIMUM PREHEAT TO BE 350 DEGRESS F. MAXIMUM INTERPASS TEMPERATURE 600 DEGRESS F. TO BE MONITORED ON COLUMN FLANGE. MAINTAIN PREHEAT TEMPERATURE WHEN WELDING IS INTERRUPTED.

B. USE STRINGER PASSES ONLY, NO WEAVING. LAY PASSES IN HORIZONTAL LAYERS. EACH PASS SHALL BE THOROUGHLY DESLAGGED AND CLEAN BY WIRE BRUSHING.

C. BOTH BEAM FLANGES SHOULD BE WELDED PRIOR TO ANY SUPPLEMENTAL WELDING TO THE SHEAR TAB. WELD RUN OF TABS SHALL BE REMOVED AND GROUND FLUSH TO THE BEAM FLANGE WITH MINIMAL DISTURBANCE.

D. PEEN EACH PASS, EXCEPT FIRST AND LAST, IMMEDIATELY AFTER DESLAGGING AND CLEAN USING A POWER SLAGGING GUN WITH A BLUNT TOOL. KEEP GUN AT RIGHT ANGLES TO WELD AND MAKE 4 - 5 PASSES THE LENGTH OF THE WELD WITH NO NICKS, CUTS OR DEEP INDENTATIONS BEING EVIDENT.

WELDING GENERAL NOTES FOR MOMENT FRAME CONNECTIONS
NTS | 3



NOTE:
1. SEE 3/- FOR WELDING GENERAL NOTES FOR MOMENT FRAME CONNECTIONS.

(1) CJP GROOVE WELD AT TOP AND BOTTOM FLANGES. AT TOP AND BOTTOM FLANGES, REMOVE WELD BACKING, BACKGOUGE, AND ADD 5/16" MINIMUM FILLET WELD.

(2) SEE DETAIL 2/- FOR DIMENSION OF WELD ACCESS HOLE.

(3) CJP GOOVE WELD FULL LENGTH OF WEB BETWEEN WELD ACCESS HOLES. PROVIDE NON-FUSIBLE WELD TABS. REMOVE WELD TABS AFTER WELDING AND GRIND END OF WELD SMOOTH AT WELD ACCESS HOLE.

(4) FULL-DEPTH PARTIAL PENETRATION FROM FAR SIDE.

(5) FILLET WELD SHEAR TAB TO BEAM WEB. WELD SIZE SHALL BE EQUAL TO THE THICKNESS OF THE SHEAR TAB MINUS 1/16". WELD SHALL EXTEND OVER THE TOP AND BOTTOM ONE-THIRD OF THE SHEAR TAB HEIGHT AND ACROSS THE TOP AND BOTTOM.

(6) SHEAR TAB OF THICKNESS EQUAL TO THAT OF BEAM WEB. SHEAR TAB LENGTH SHALL BE SO AS TO ALLOW 1/8" OVERLAP WITH WELD ACCESS HOLE AT TOP AND BOTTOM, AND THE WIDTH SHALL EXTEND 2" MINIMUM BACK ALONG THE BEAM, BEYOND THE END OF THE WELD ACCESS HOLE.

(7) ERECTION BOLTS: NUMBER, TYPE, AND SIZE SELECTED FOR ERECTION LOADS.

(8) FOR ONE-SIDED CONNECTIONS, CONTINUITY PLATE THICKNESS SHOULD BE EQUAL TO THE THICKNESS OF THE TWO BEAM FLANGES. FOR TWO SIDED CONNECTIONS, THE CONTINUITY PLATES SHOULD BE EQUAL IN THICKNESS TO THE THICKER OF THE TWO BEAM FLANGES ON EITHER SIDE OF COLUMN (IN ALL CASES F_y OF CONTINUITY PLATES SHALL MATCH F_y OF BEAMS AND COLUMNS).

(9A) CJP GROOVE WELD TO COLUMN FLANGES TYPICAL.

(9B) FILLET WELD TO COLUMN WEB PER DETAIL 4/- TYPICAL.

(10) AISC MINIMUM CONTINUOUS FILLET WELD UNDER BACKING TYPICAL.

(11) MINIMUM WIDTH TO MATCH BEAM FLANGE

(12) REMOVE WELD TABS TO 1/4" MAXIMUM FROM EDGE OF CONTINUITY PLATE, GRIND END OF WELD SMOOTH (500 μ-in), NOT FLUSH. DO NOT GOUGE COLUMN FLANGE.

(13) DOUBLER PLATE PER DETAIL 4/-.

WELDED UNREINFORCED FLANGE - WELDED WEB CONNECTION
NTS | 1

(1) BEVEL AS REQUIRED BY AWS D1.1. FOR SELECTED GROOVE WELD PROCEDURE.

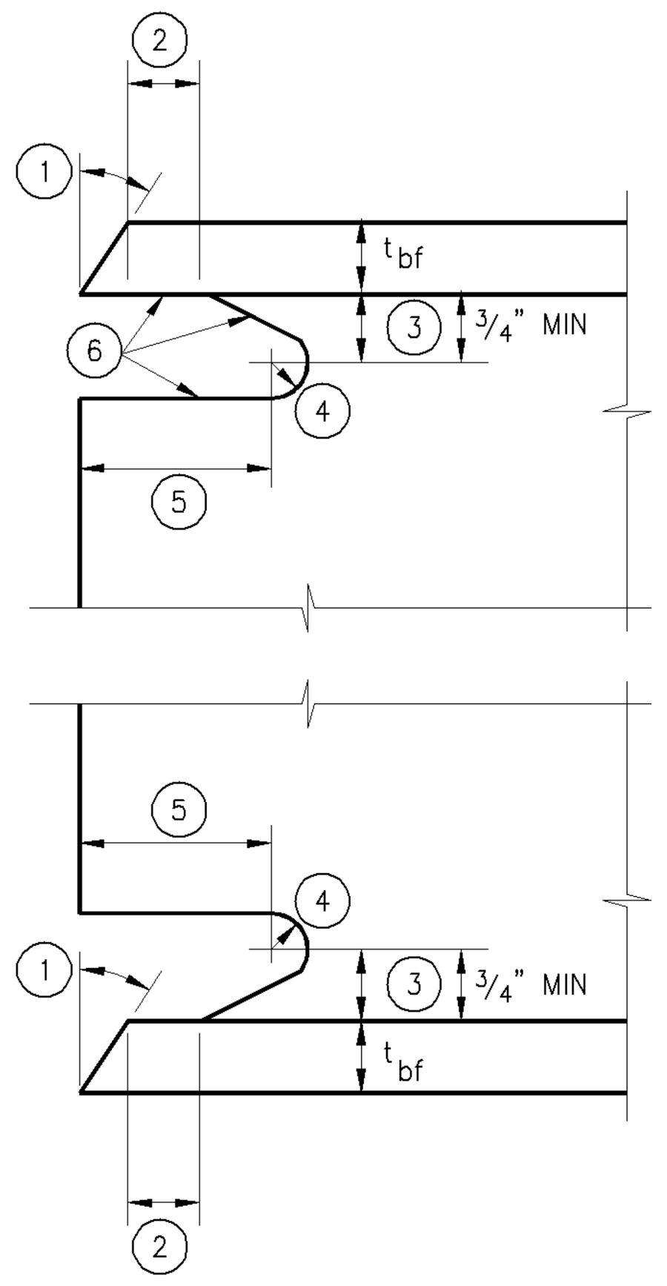
(2) LARGER OF t_{bf} OR 1/2 INCH. (PLUS 1/2 t_{bf}, OR MINUS 1/4 t_{bf})

(3) 3/4 t_{bf} TO t_{bf}, 3/4" MINIMUM (± 1/4")

(4) 3/8" MINIMUM RADIUS (PLUS NOT LIMITED, OR MINUS 0)

(5) $3t_{bf}$. (± 1/2 INCH)

(6) SEE FEMA-353, RECOMMENDED SPECIFICATIONS AND QUALITY ASSURANCE GUIDELINES FOR STEEL MOMENT-FRAME CONSTRUCTION FOR SEISMIC APPLICATIONS, FOR FABRICATION DETAILS INCLUDING CUTTING METHODS AND SMOOTHNESS REQUIREMENTS.



NOTE:
1. TOLERANCES SHALL NOT ACCUMULATE TO THE EXTENT THAT THE ANGLE OF THE ACCESS HOLE CUT TO THE FLANGE SURFACE EXCEEDS 25°

WELDED ACCESS HOLE DETAIL
NTS | 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

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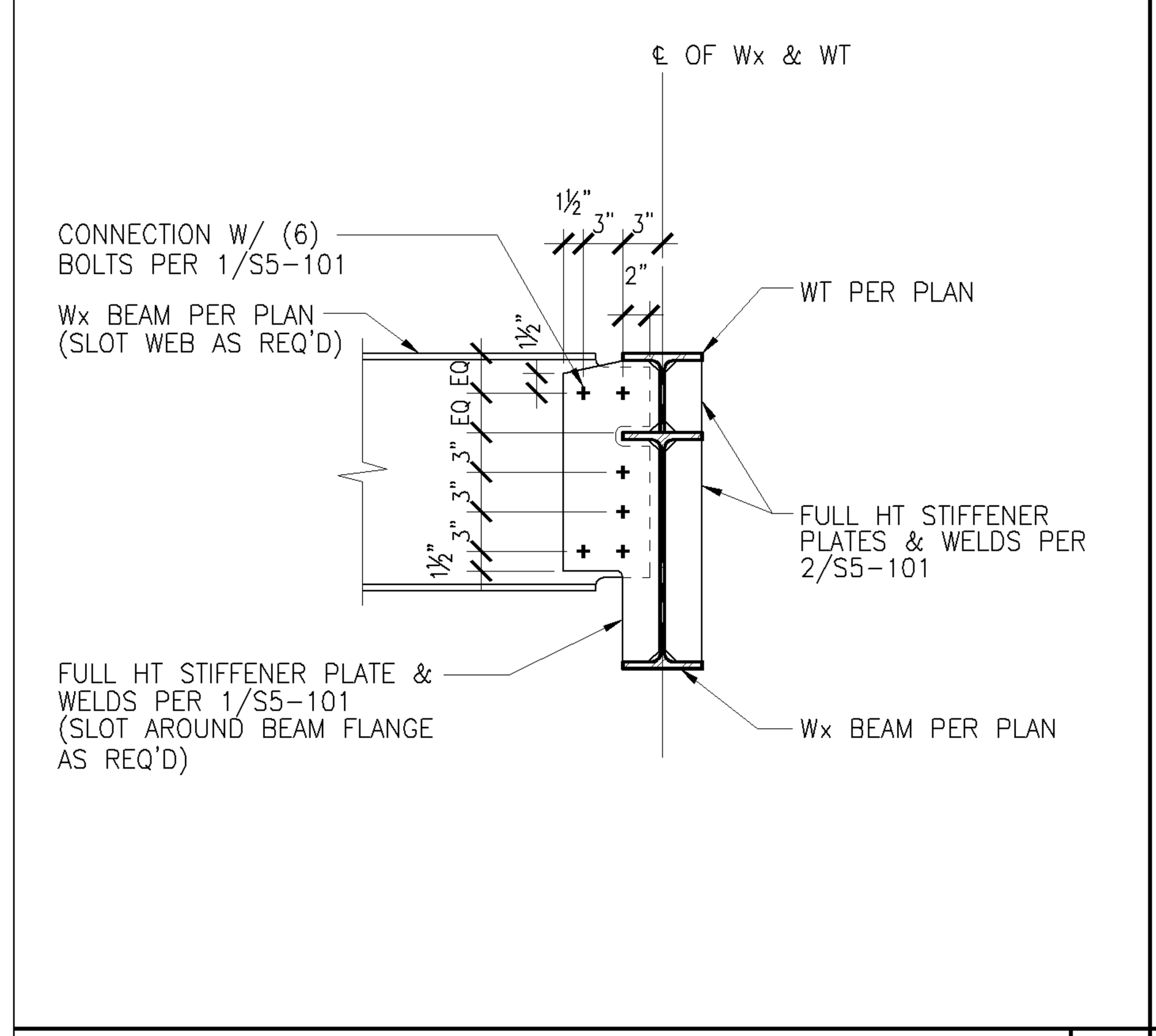
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

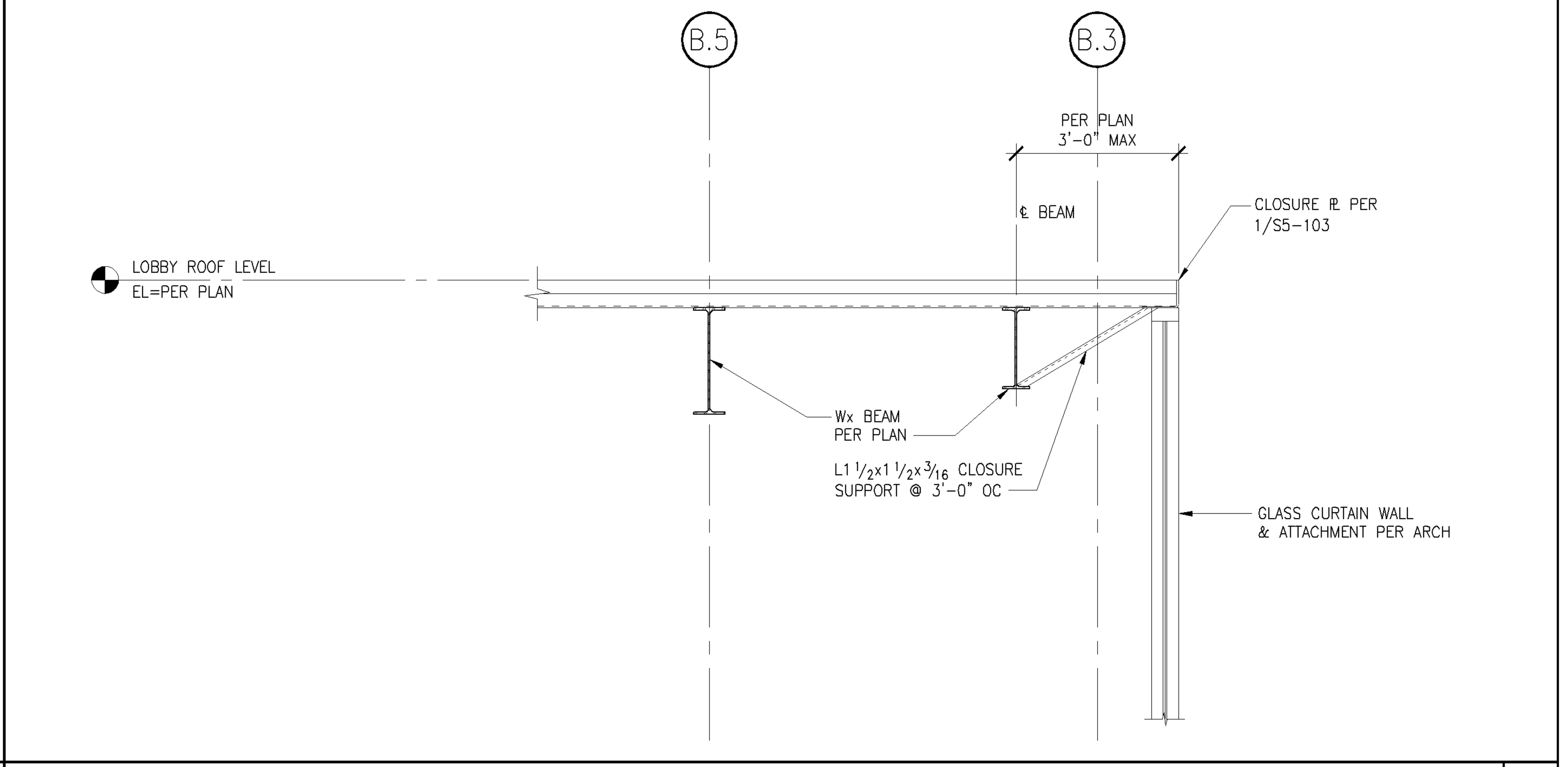
KEY PLAN

SHEET TITLE
STEEL MOMENT FRAME DETAILS AND WELDING GENERAL NOTES

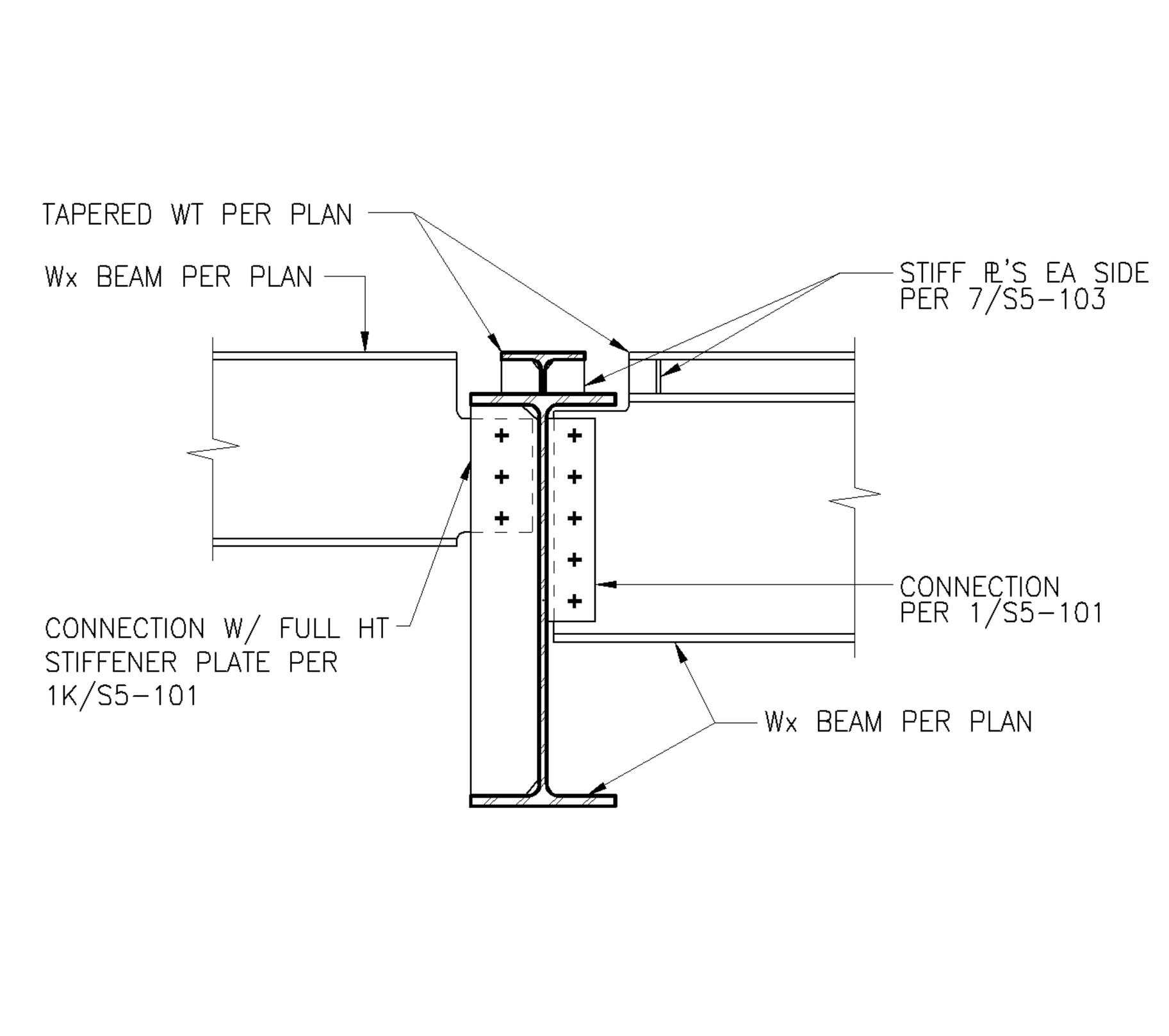
S5-108



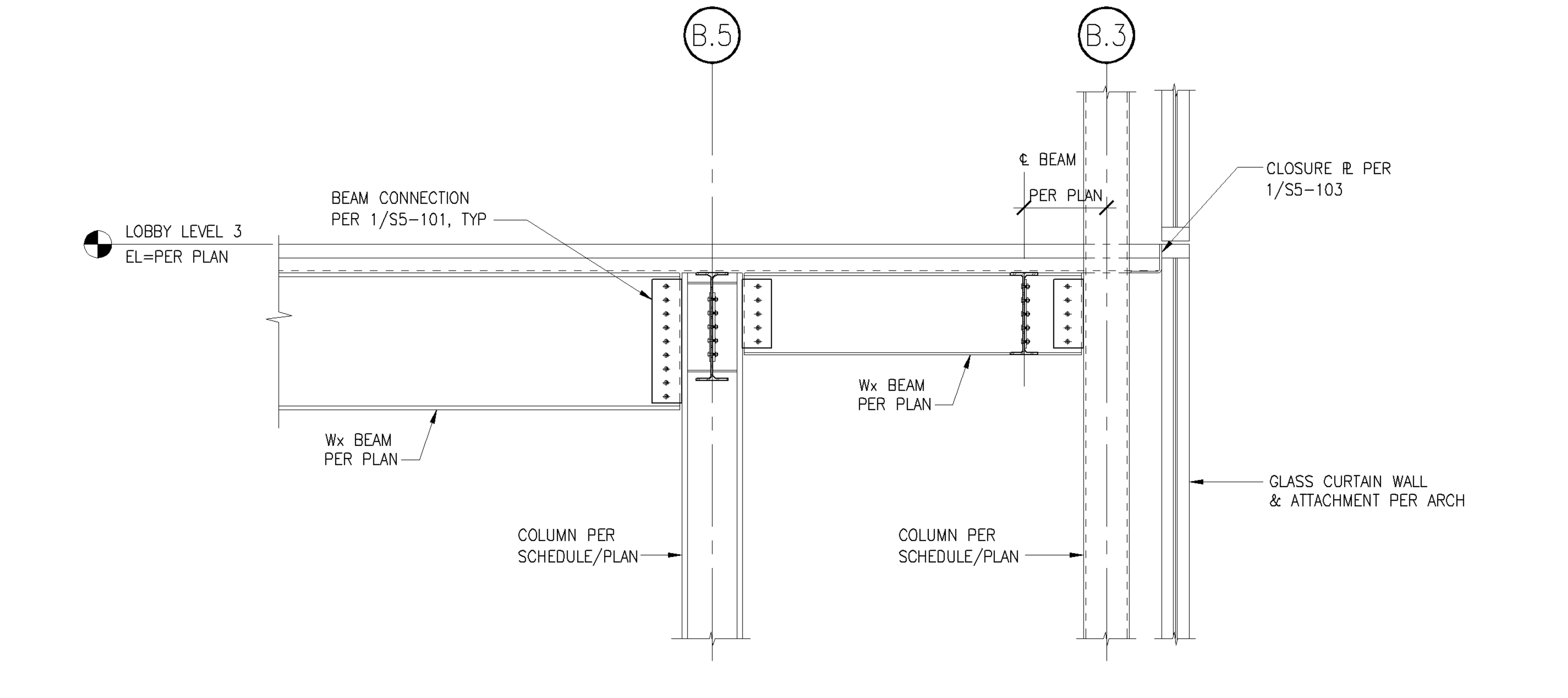
BEAM TO BEAM W/ WT CONNECTION — SCALE: 1"=1'-0" — 8



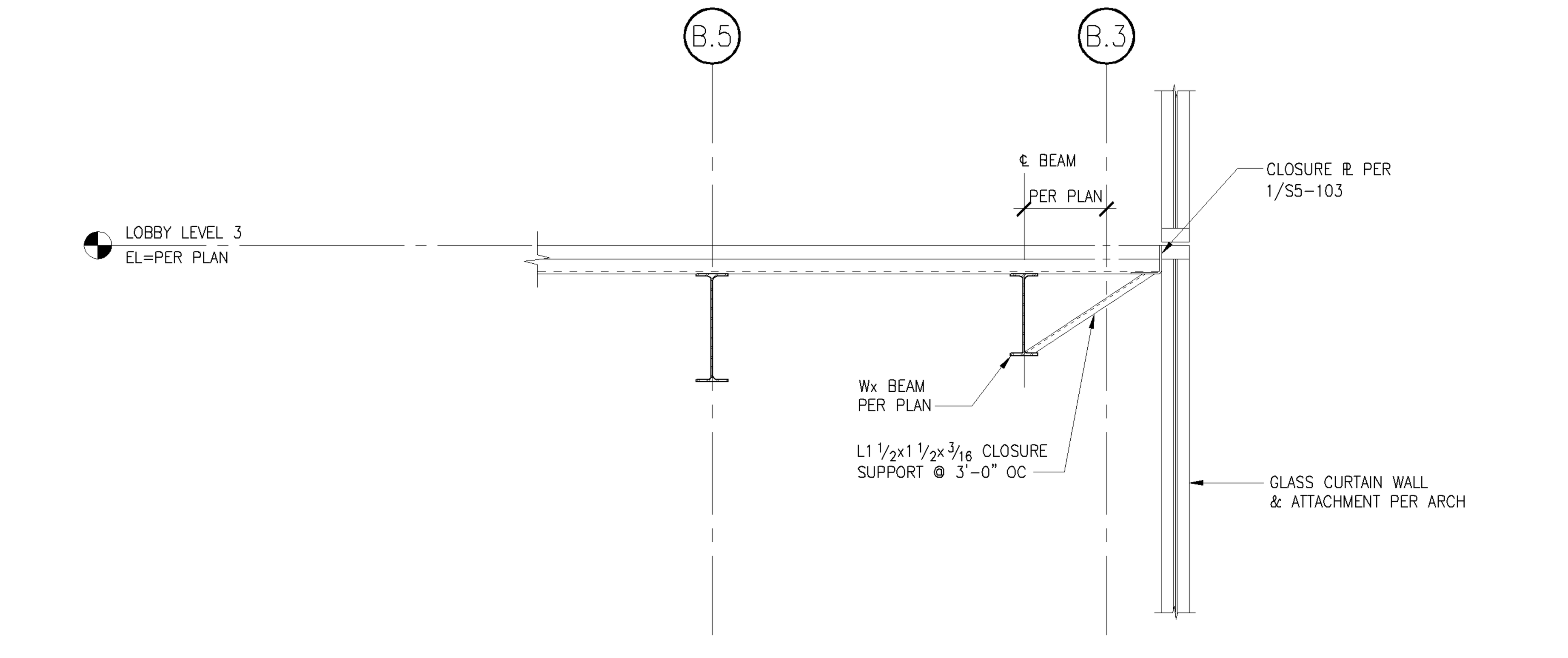
SECTION AT LOBBY ROOF BETWEEN COLUMNS — SCALE: 1/2"=1'-0" — 3



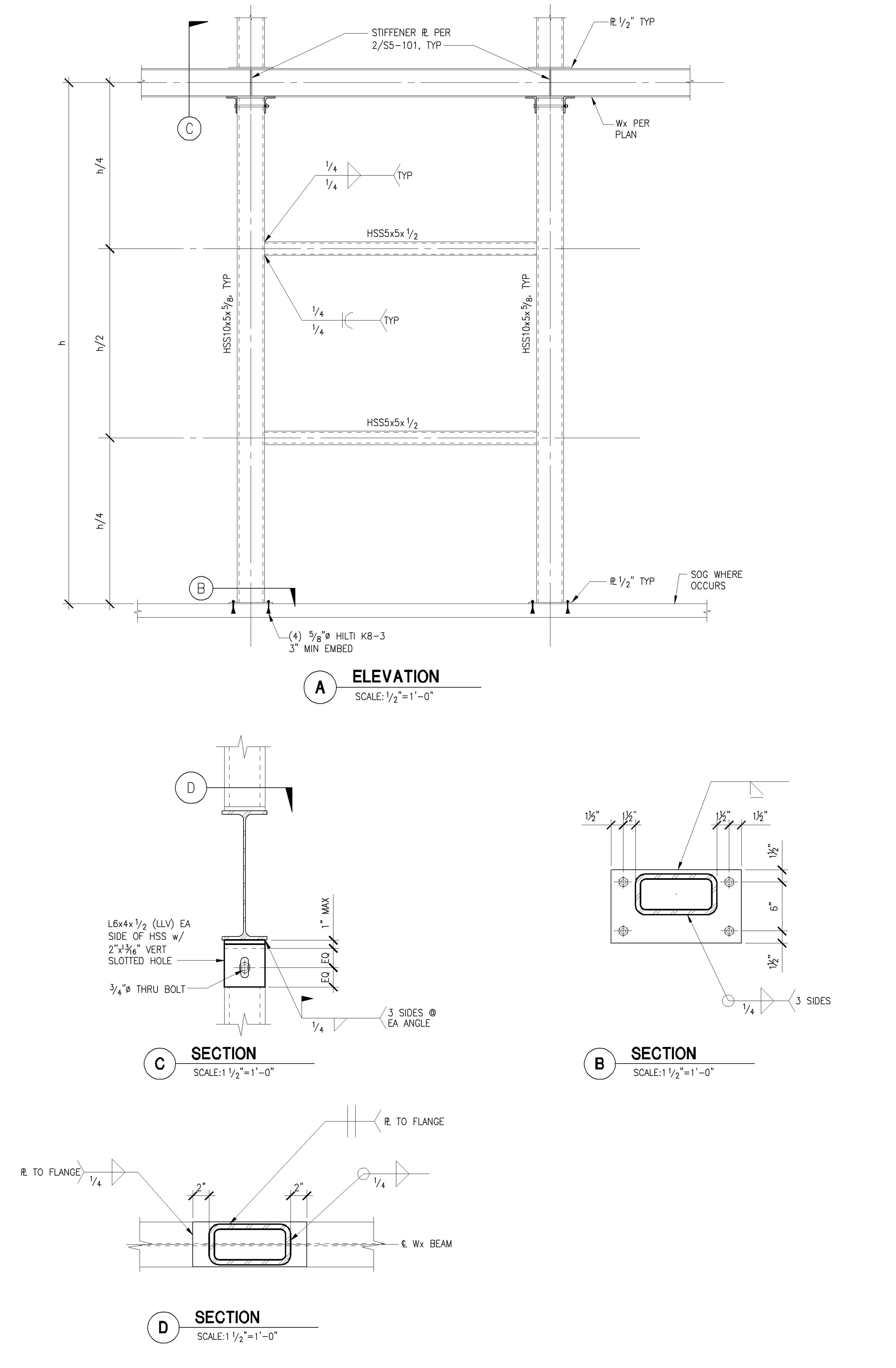
BEAM TO BEAM W/ WT CONNECTION — SCALE: 1"=1'-0" — 9



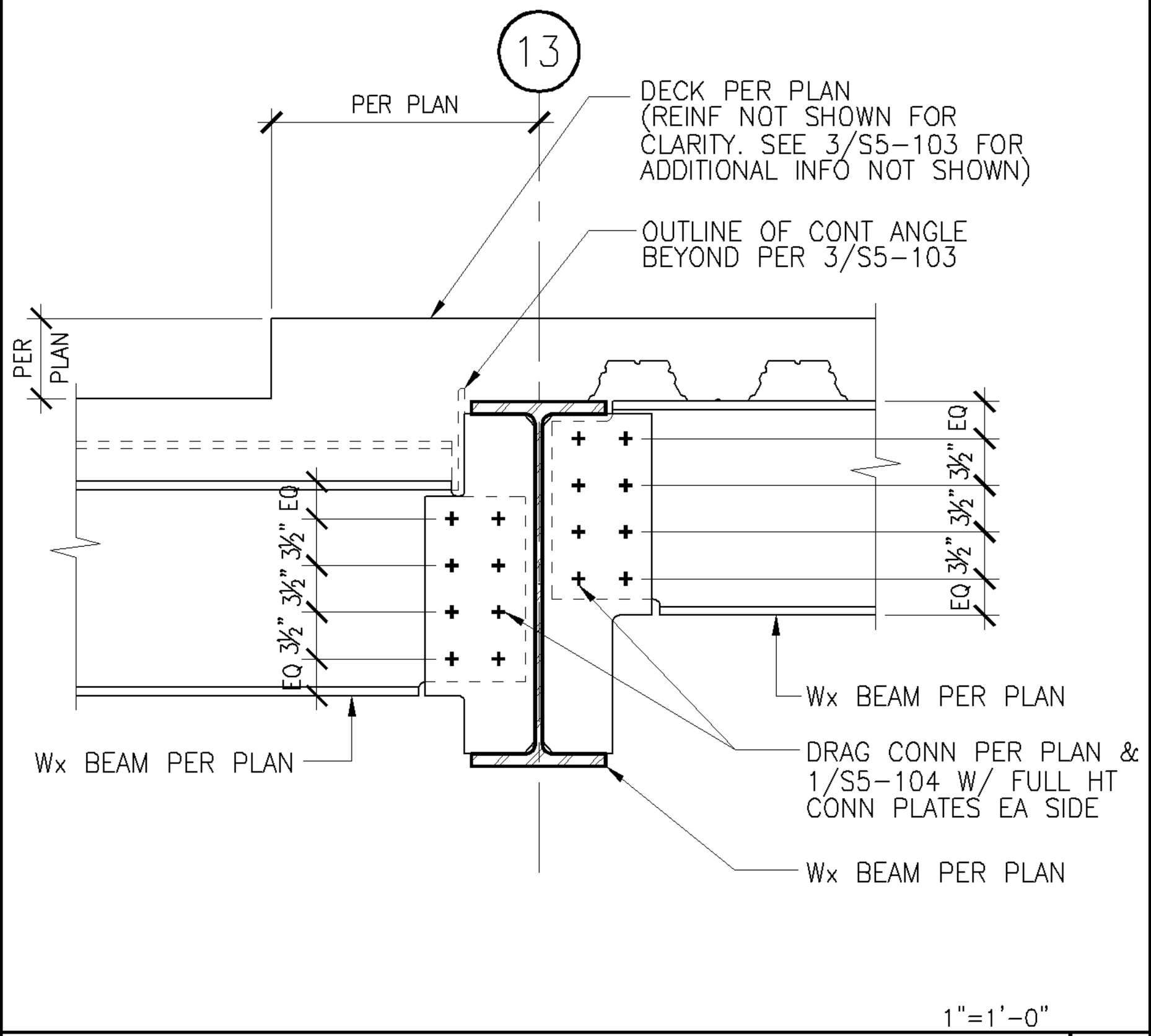
SECTION AT LOBBY LEVEL 3 AT COLUMNS — SCALE: 1/2"=1'-0" — 4



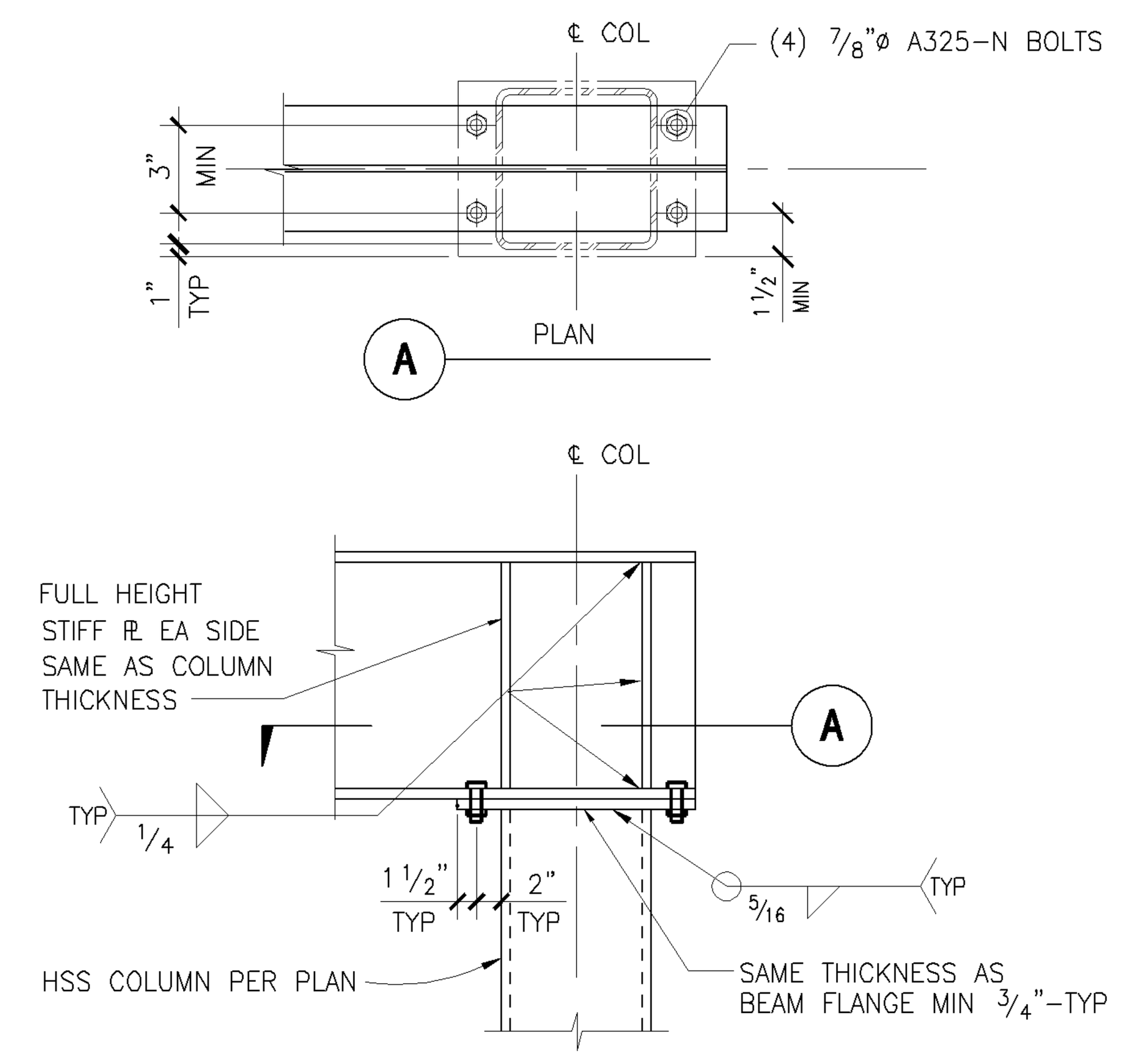
SECTION AT LOBBY LEVEL 3 BETWEEN COLUMNS — SCALE: 1/2"=1'-0" — 5



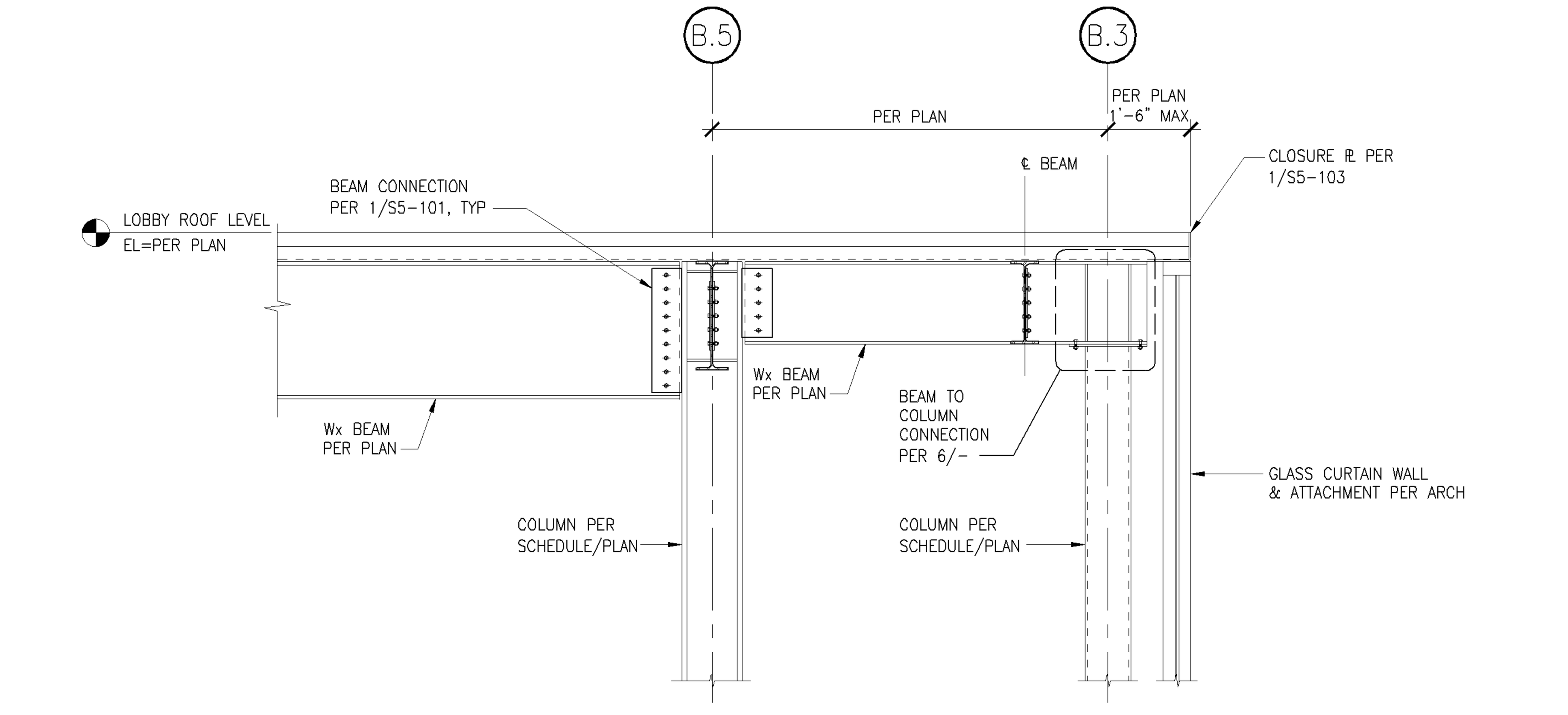
ELEVATOR GUIDE RAIL SUPPORT — SCALE: AS NOTED — 1



DRAG BEAM CONN BTWN LOBBY ROOF & BLDG G LEVEL 5 — 7



DETAIL — SCALE: 1"=1'-0" — 6



SECTION AT LOBBY ROOF AT COLUMNS — SCALE: 1/2"=1'-0" — 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

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CHECKED BY:

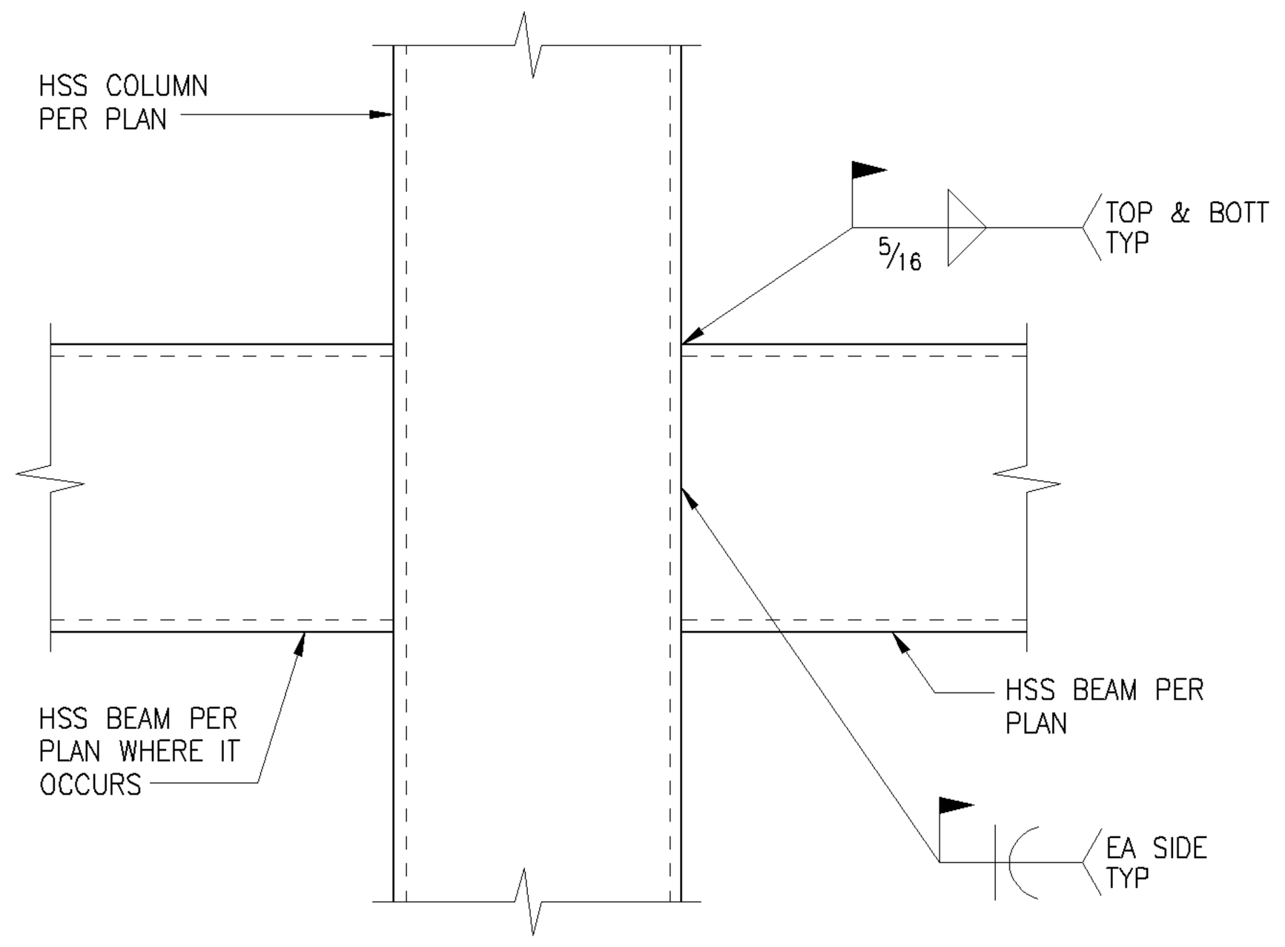
KEY PLAN

SHEET TITLE
STEEL DETAILS

S5-109

HSS BM TO HSS COL DETAIL

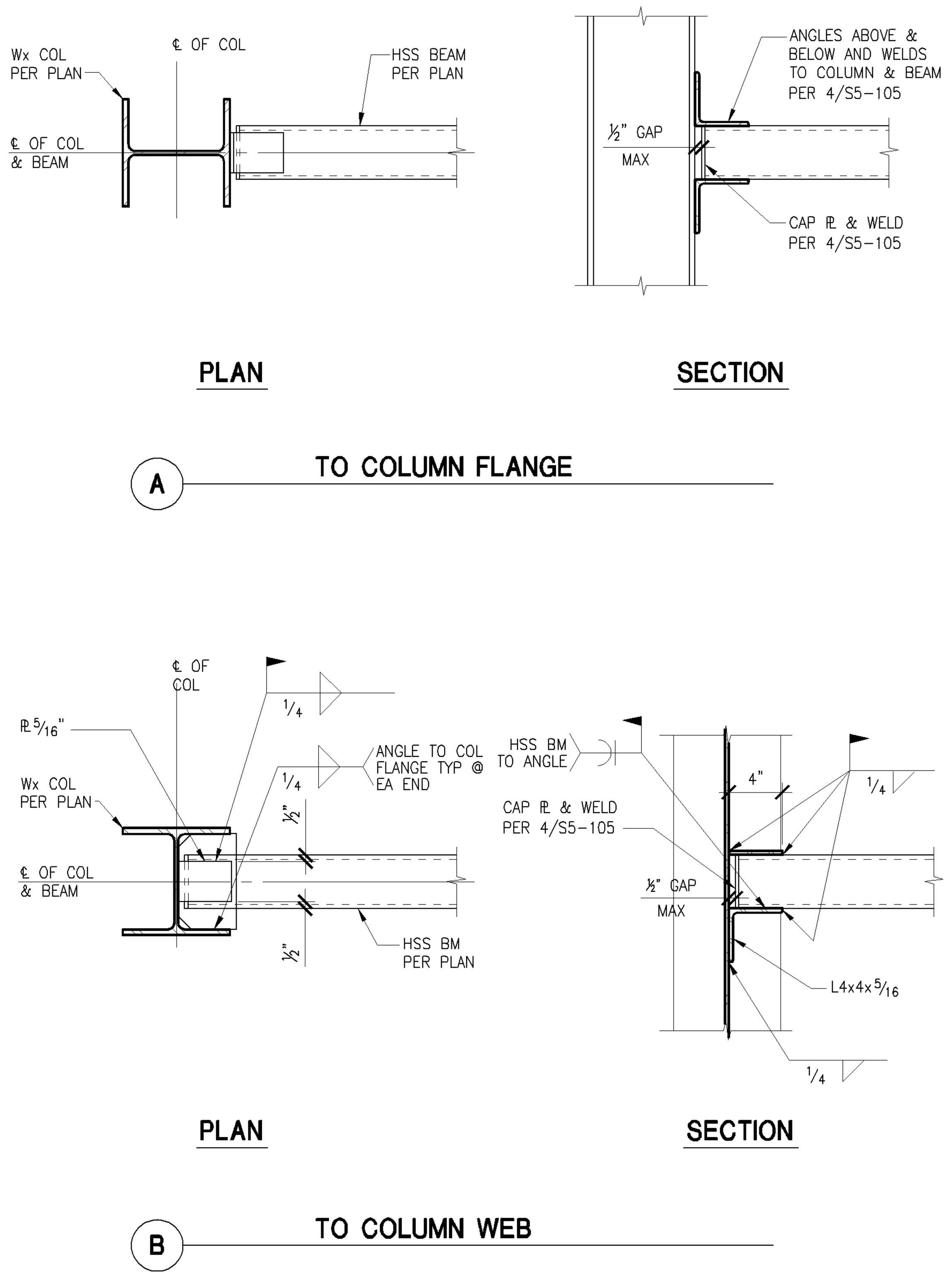
1 1/2"=1'-0" 4



NOTES:
STEEL MEMBERS AND WELDING SHALL BE DONE PER REQUIREMENTS FOR ARCHITECTURAL EXPOSED STRUCTURAL STEEL (AESS).

HSS BEAM TO Wx COLUMN CONNECTION

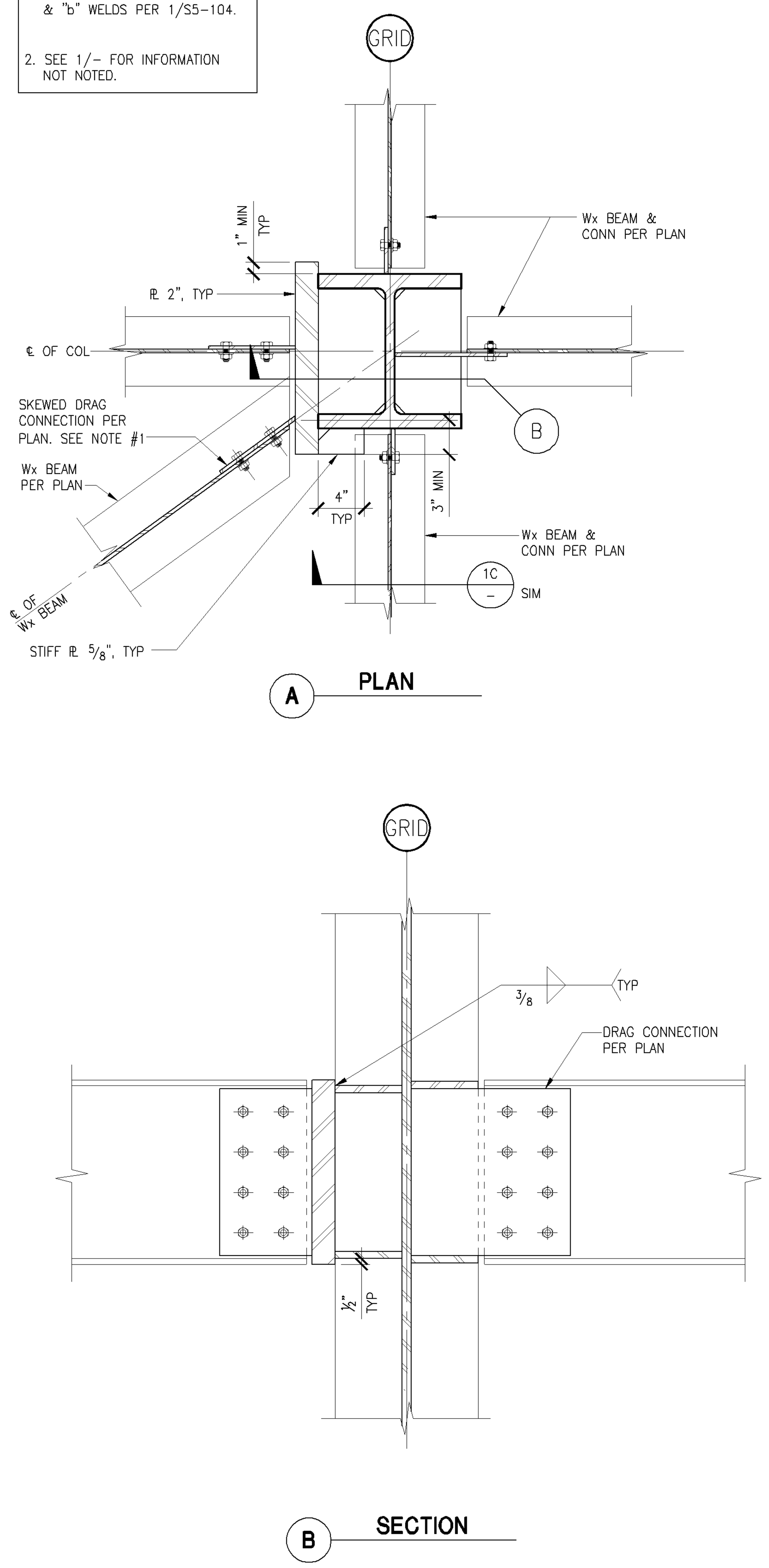
1 1/2"=1'-0" 5



Wx COLUMN TO Wx BEAM

1 1/2"=1'-0" 2

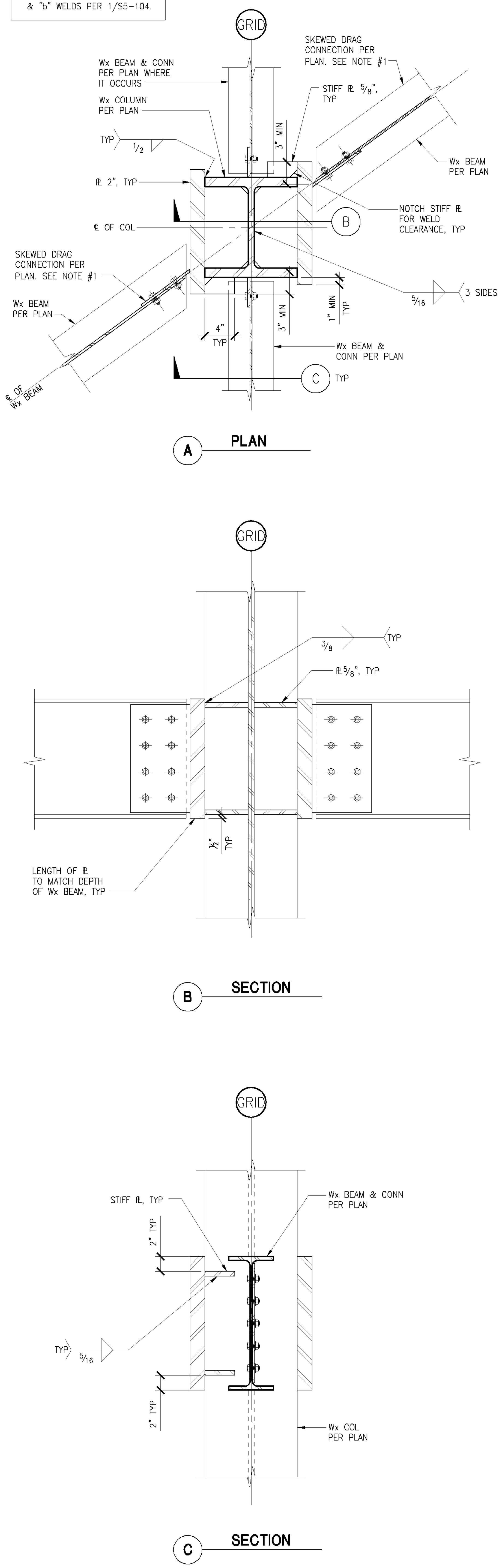
NOTES:
1. AT SKEWED DRAG CONNECTIONS PROVIDE CJP WELDS FOR "a" & "b" WELDS PER 1/S5-104.
2. SEE 1/– FOR INFORMATION NOT NOTED.



Wx COLUMN TO Wx BEAM

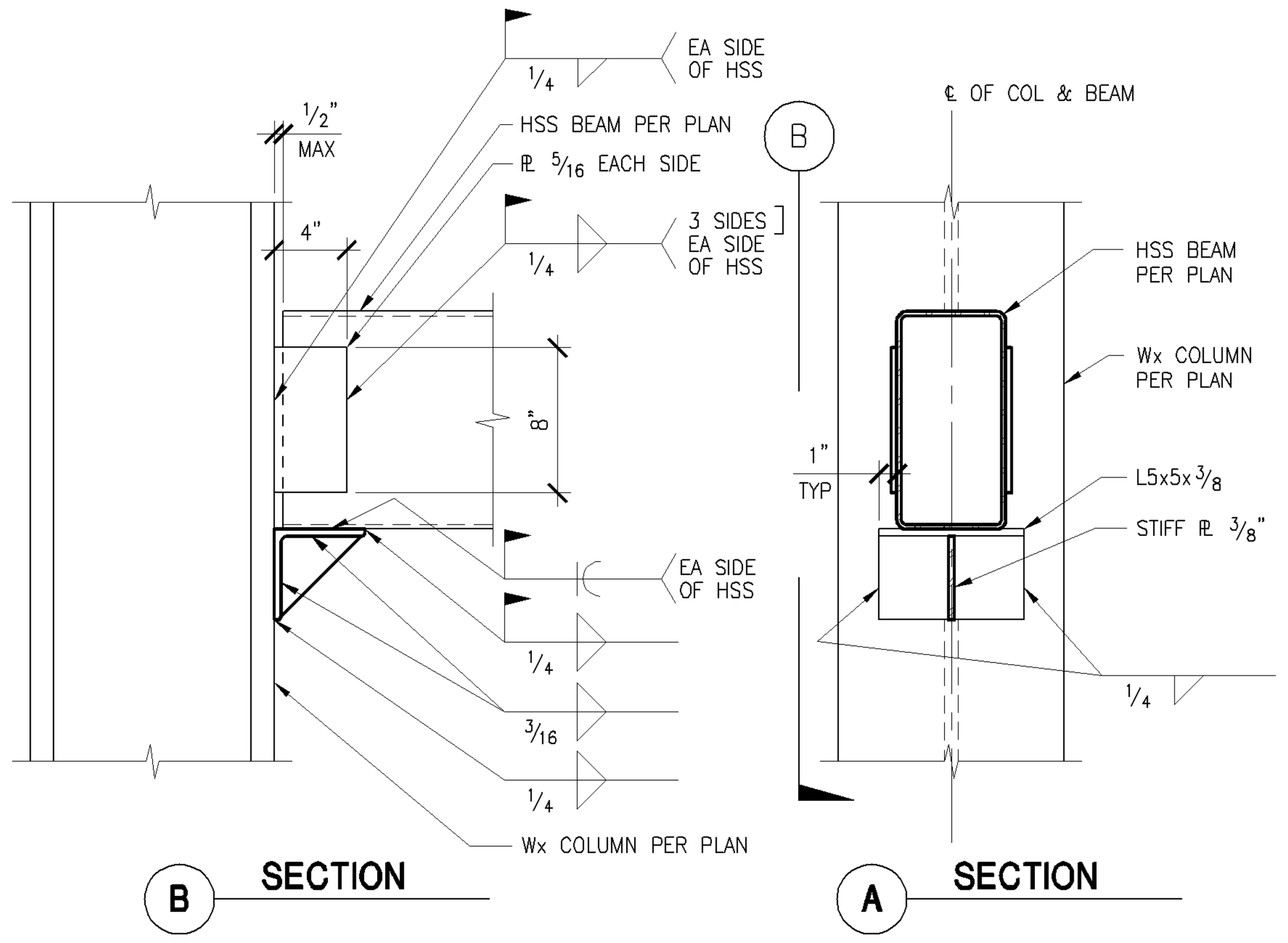
1 1/2"=1'-0" 1

NOTE:
1. AT SKEWED DRAG CONNECTIONS PROVIDE CJP WELDS FOR "a" & "b" WELDS PER 1/S5-104.



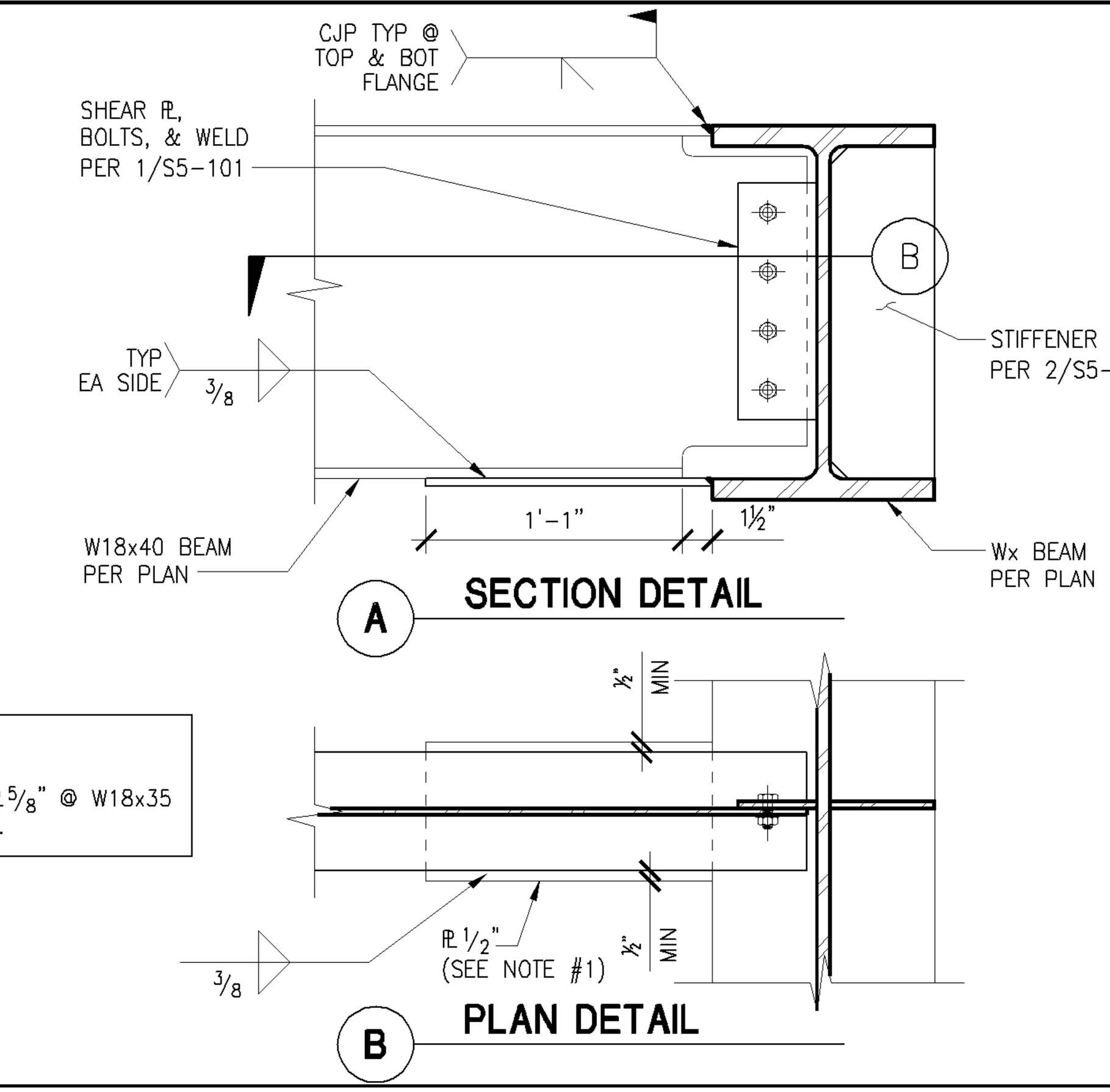
HSS BEAM TO Wx COLUMN CONNECTION

1 1/2"=1'-0" 6



BEAM TO BEAM MOMENT CONNECTION

1 1/2"=1'-0" 3



NOTE:
1. USE ℙ 5/8" @ W18x35 BEAMS.

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CONSULTANTS

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CIVIL ENGINEER
440 STATE PLACE
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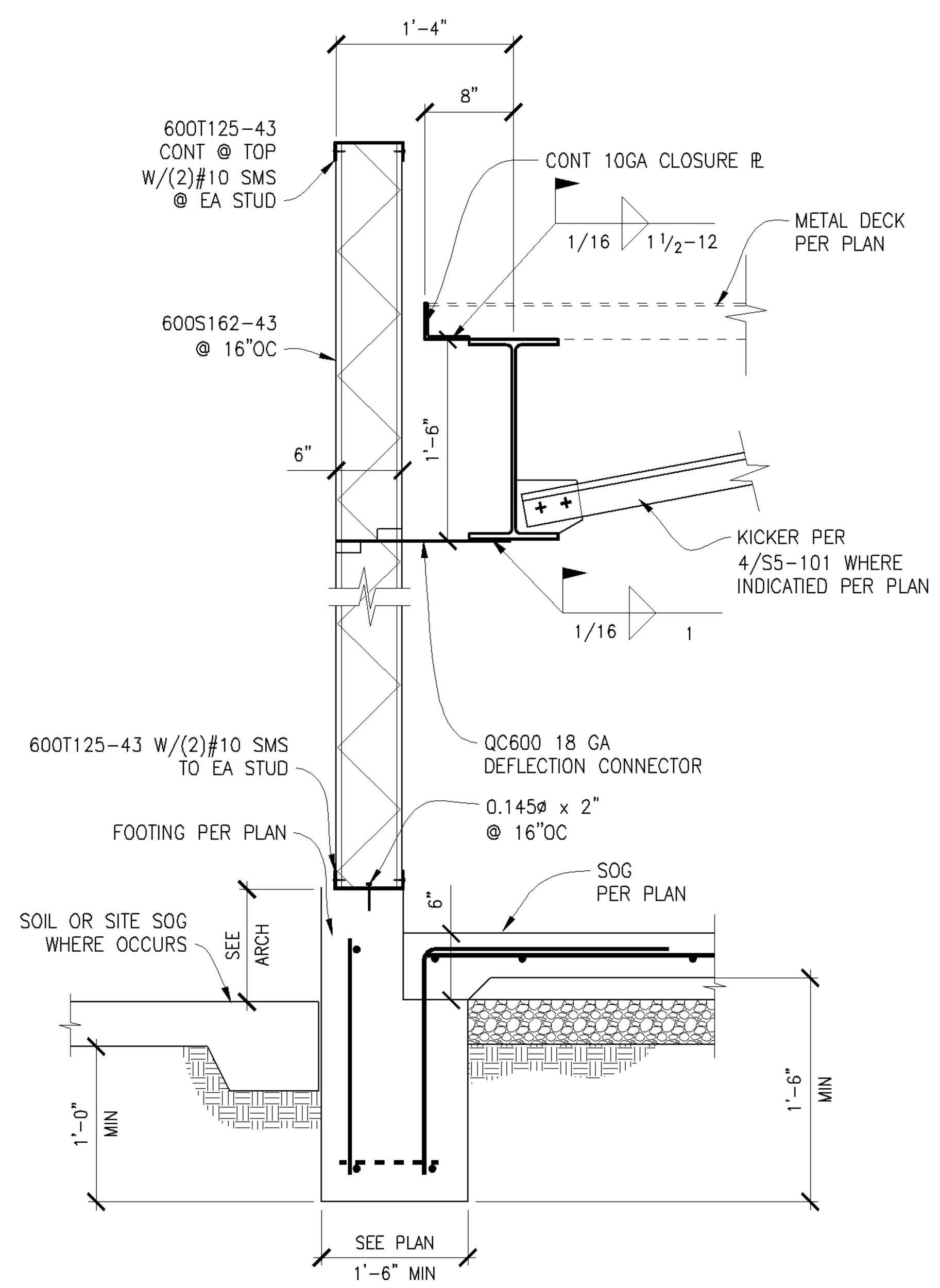
KEY PLAN

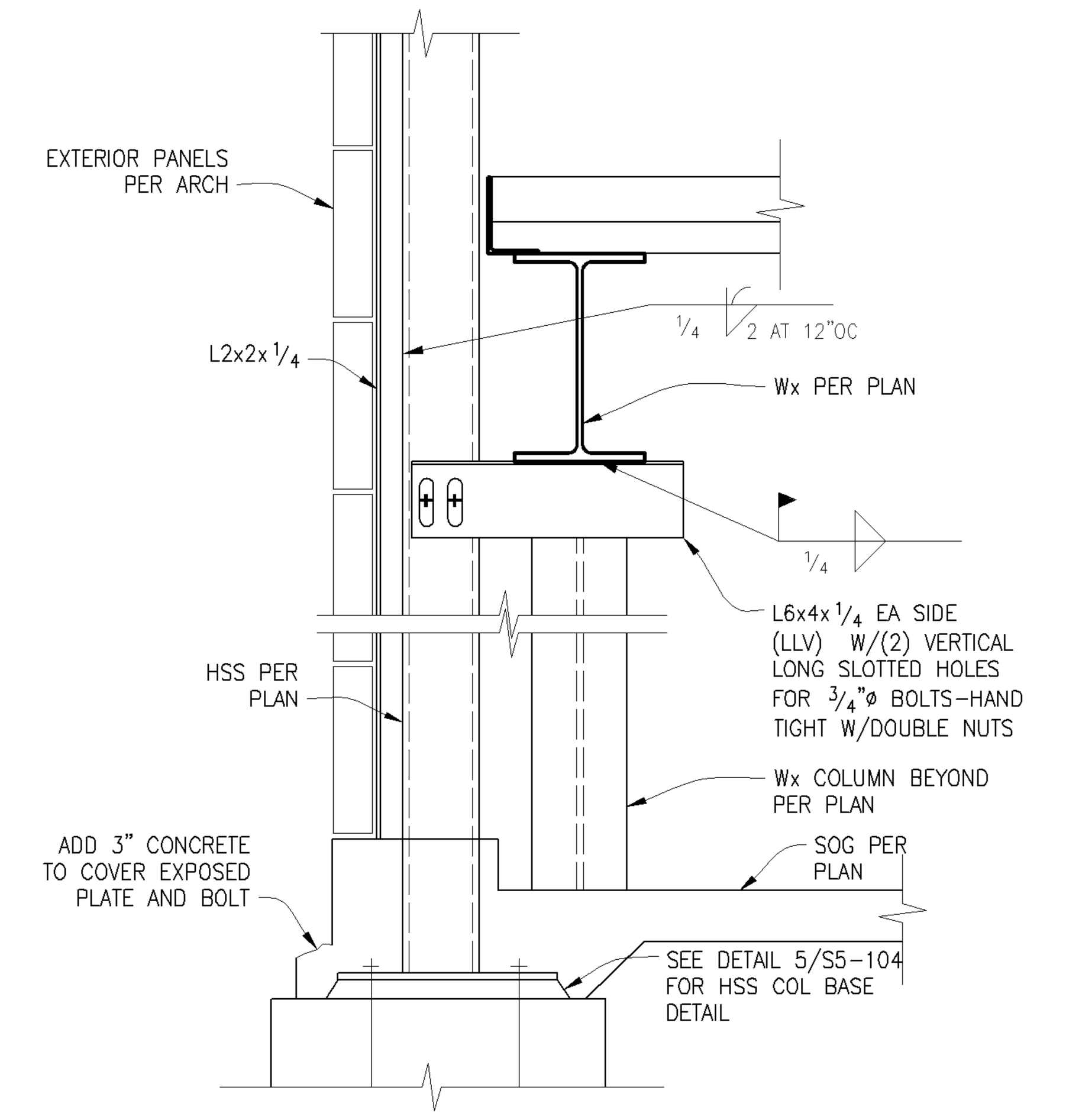
SHEET TITLE

STEEL DETAILS

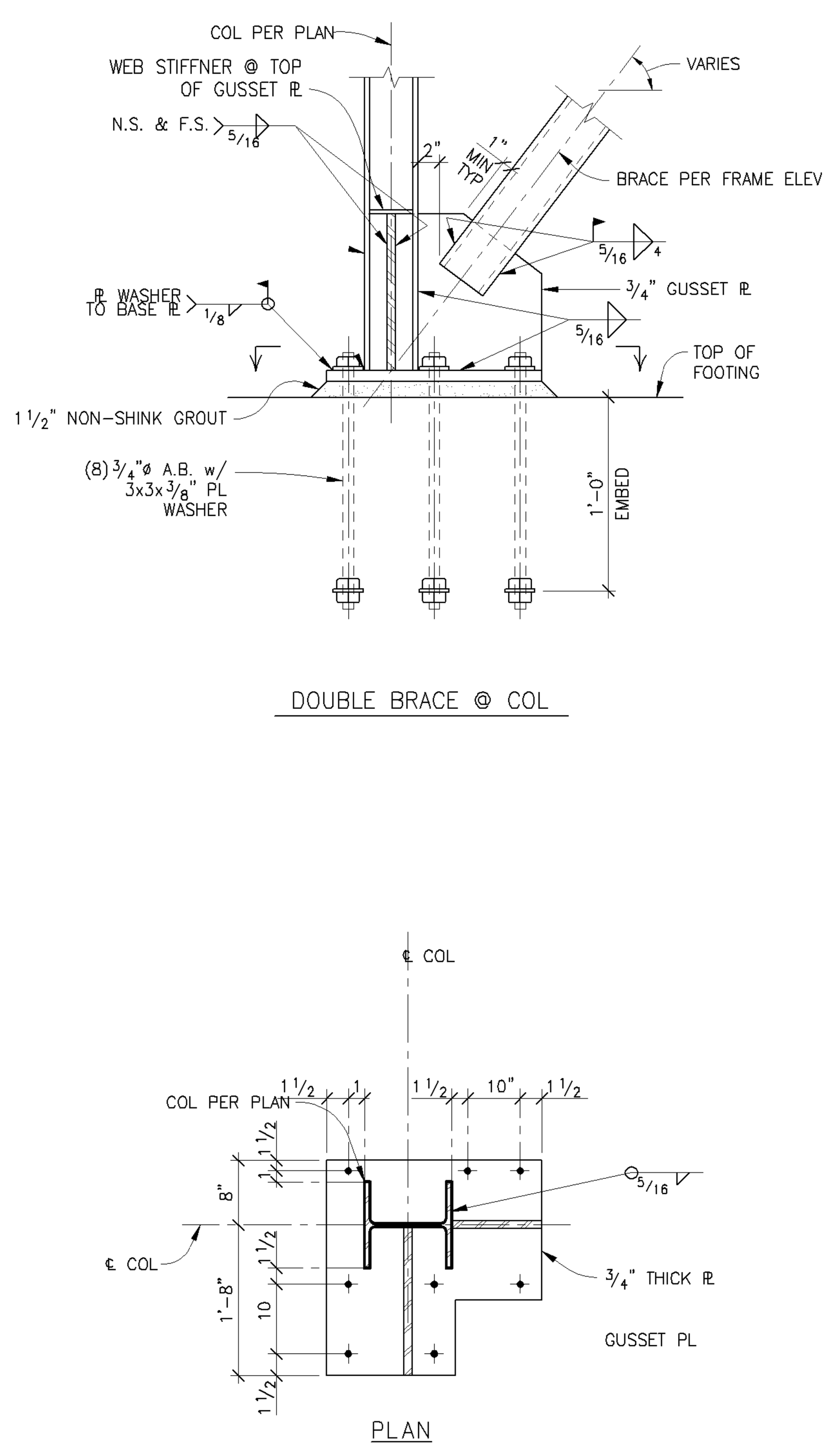
S5-110



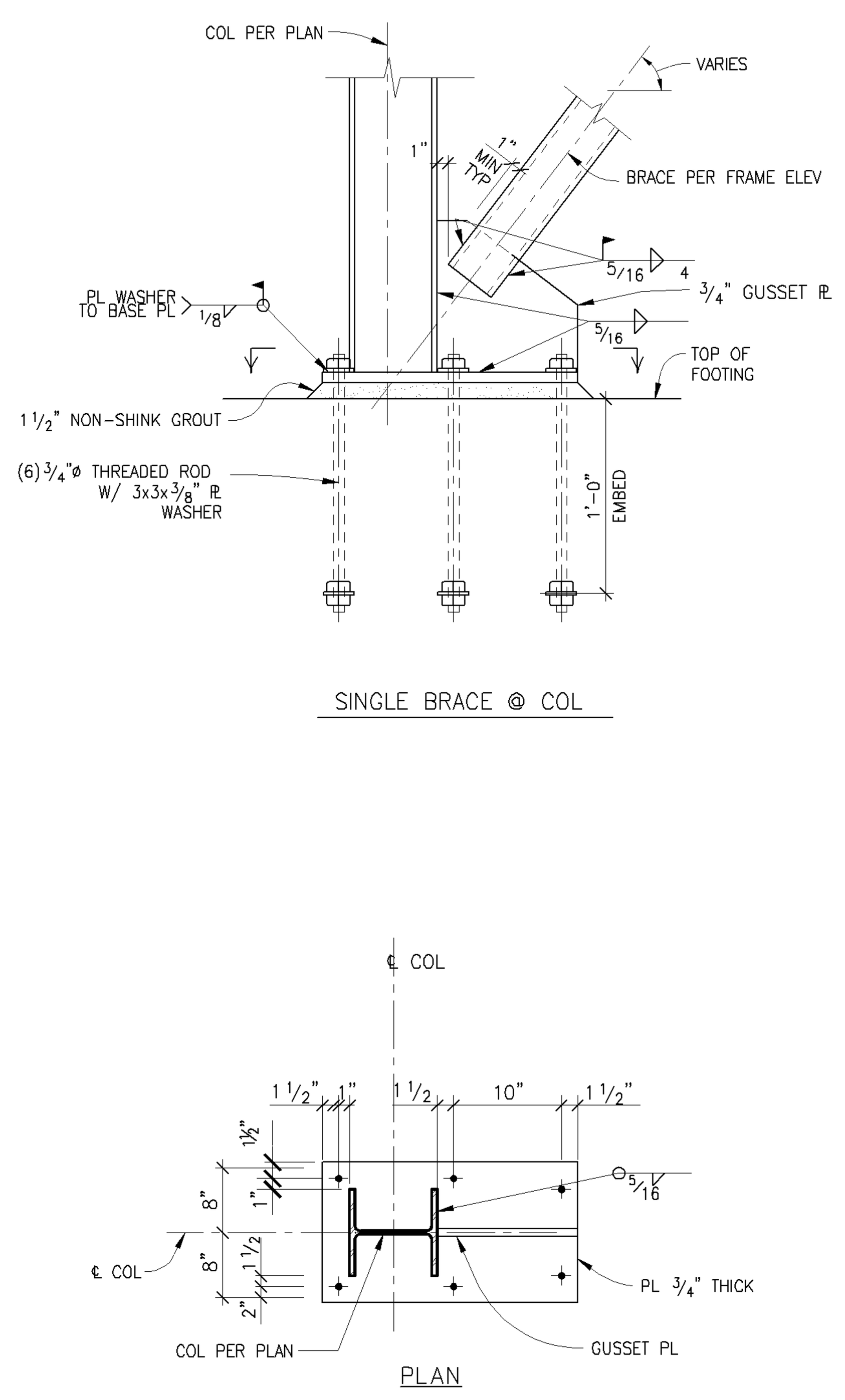
EXTERIOR WALL SECTION
1"=1'-0" | 10



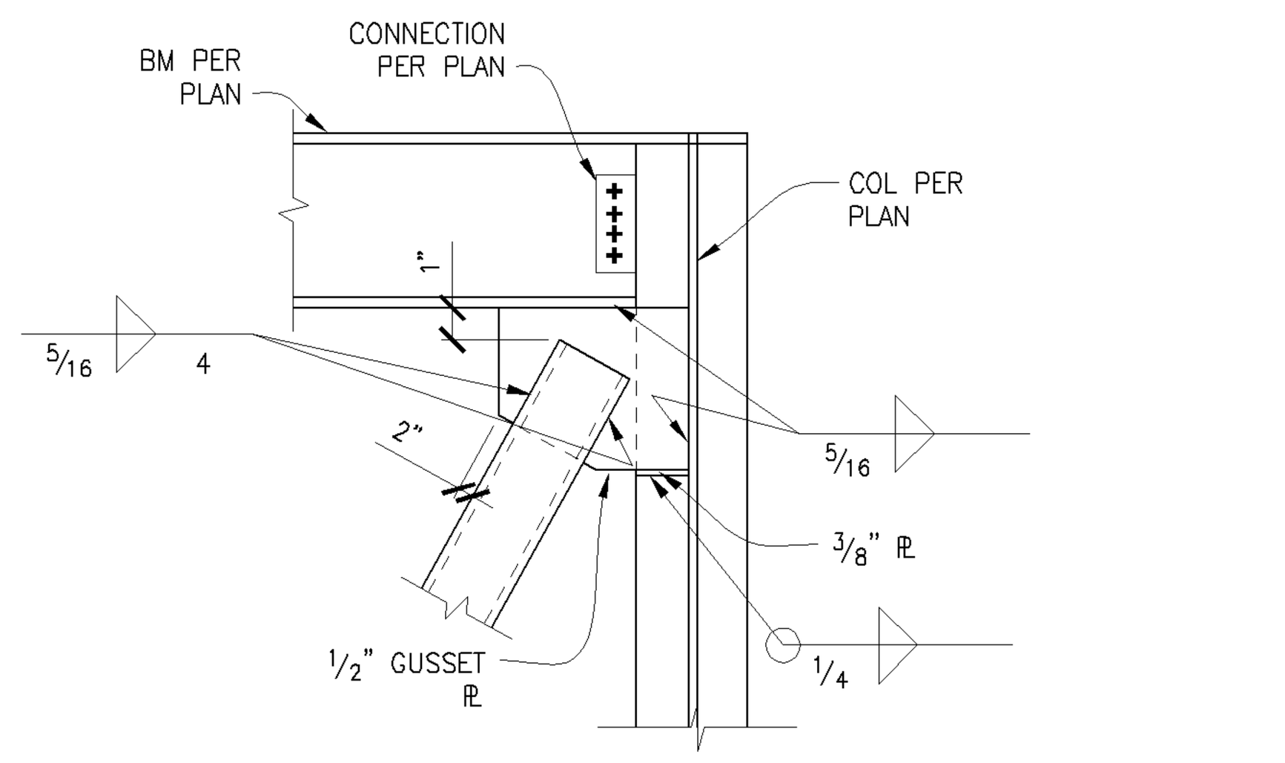
EXTERIOR PANEL SECTION
1"=1'-0" | 9



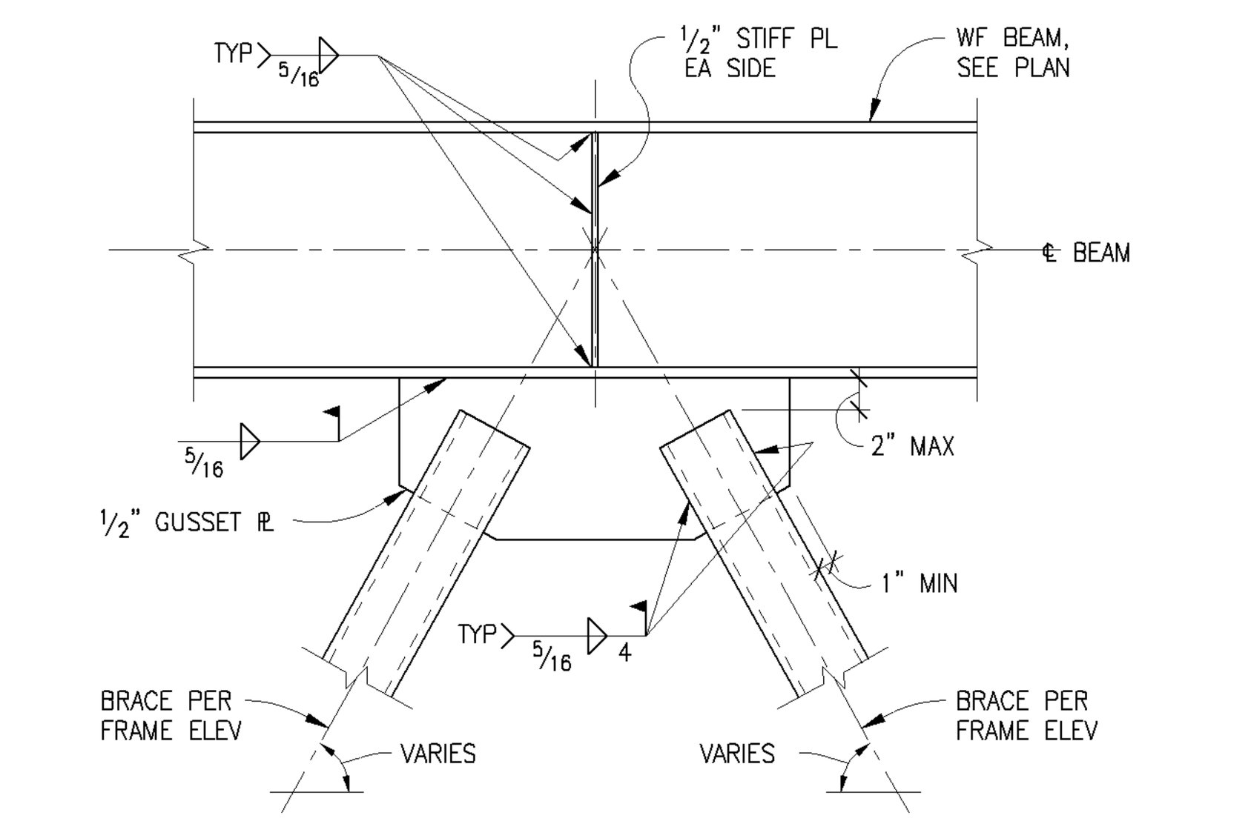
BRACED FRAME DETAIL
1"=1'-0" | 7



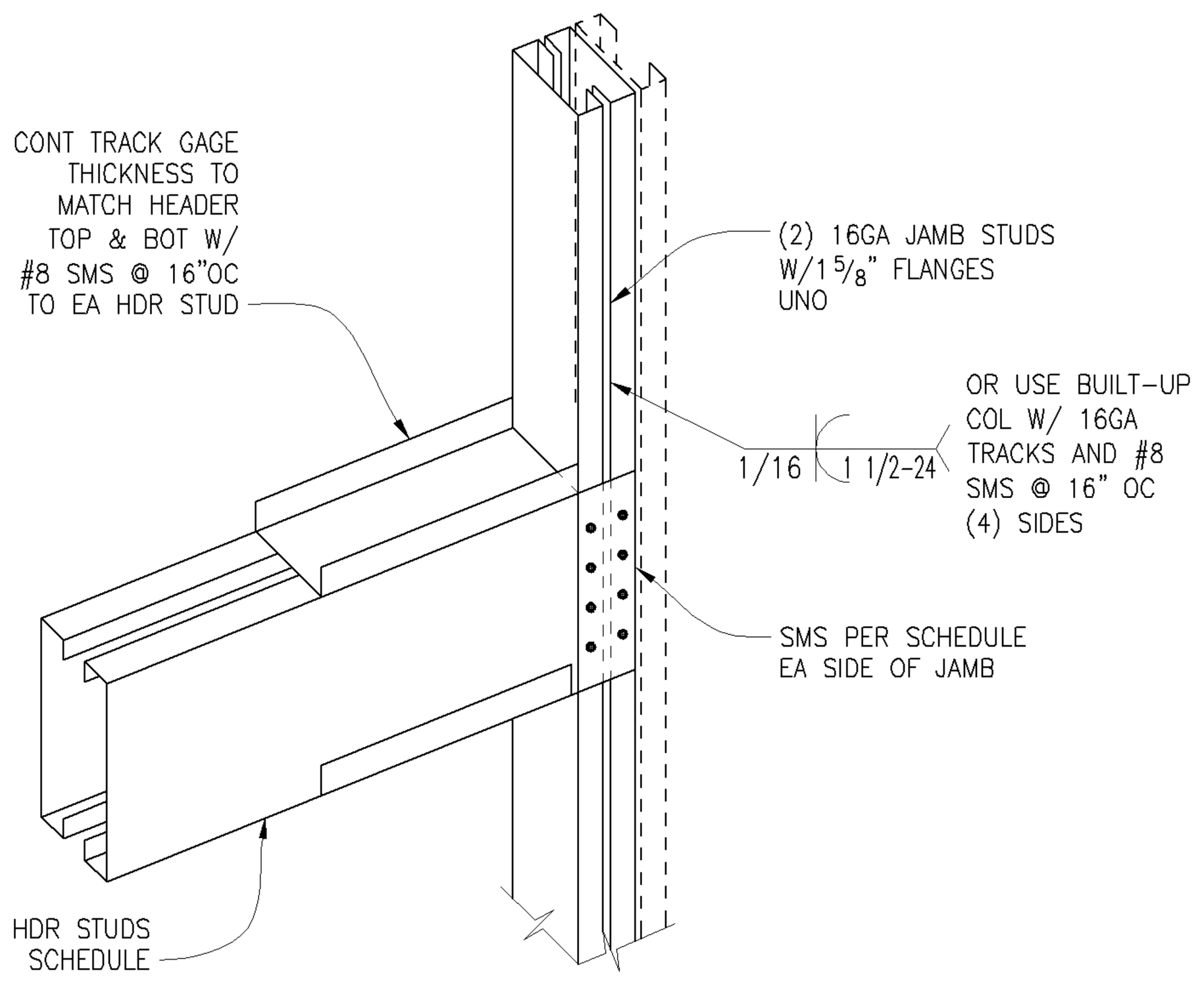
BRACED FRAME DETAIL
1"=1'-0" | 5



BRACED FRAME DETAIL
1"=1'-0" | 1

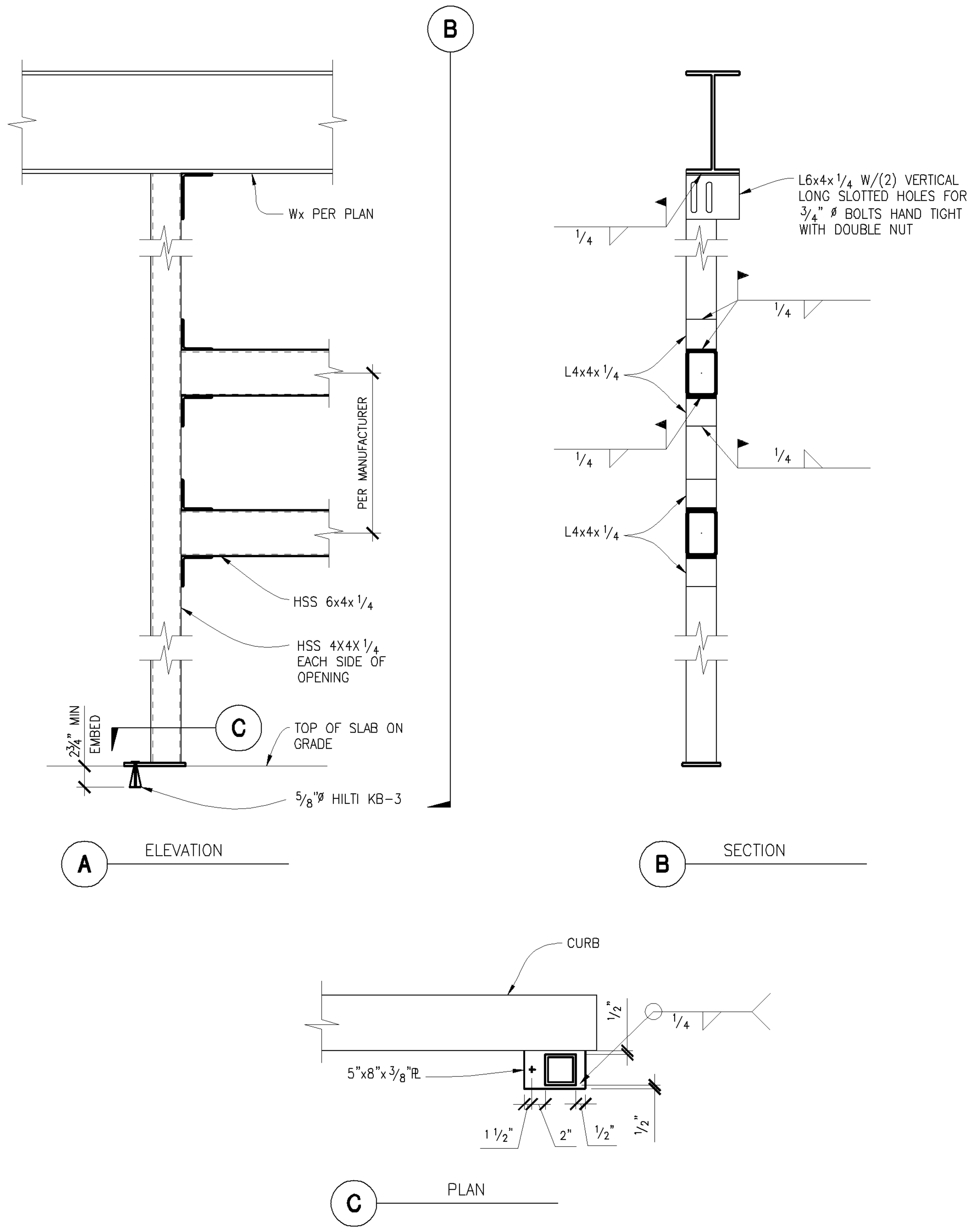


BRACED FRAME DETAIL
1"=1'-0" | 2

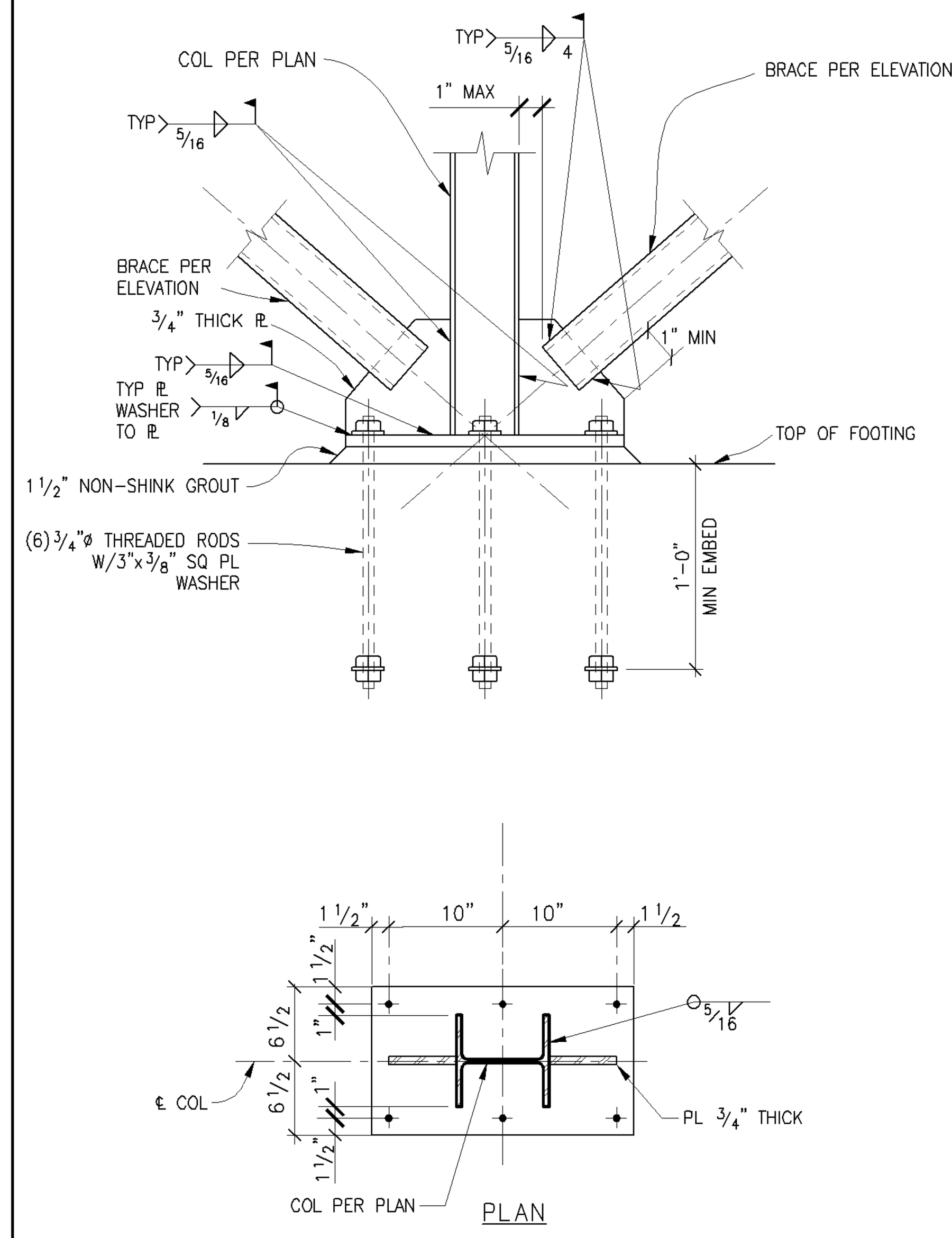


HEADER LENGTH	HORIZ STUD SIZE	HDR STUD FLANGE WIDTH	ATTACHMENT EA SIDE OF JAMB
20'	10" x 16GA	1 5/8"	(6) #10 SMS
≤12'	6" x 16GA	1 5/8"	(4) #10 SMS

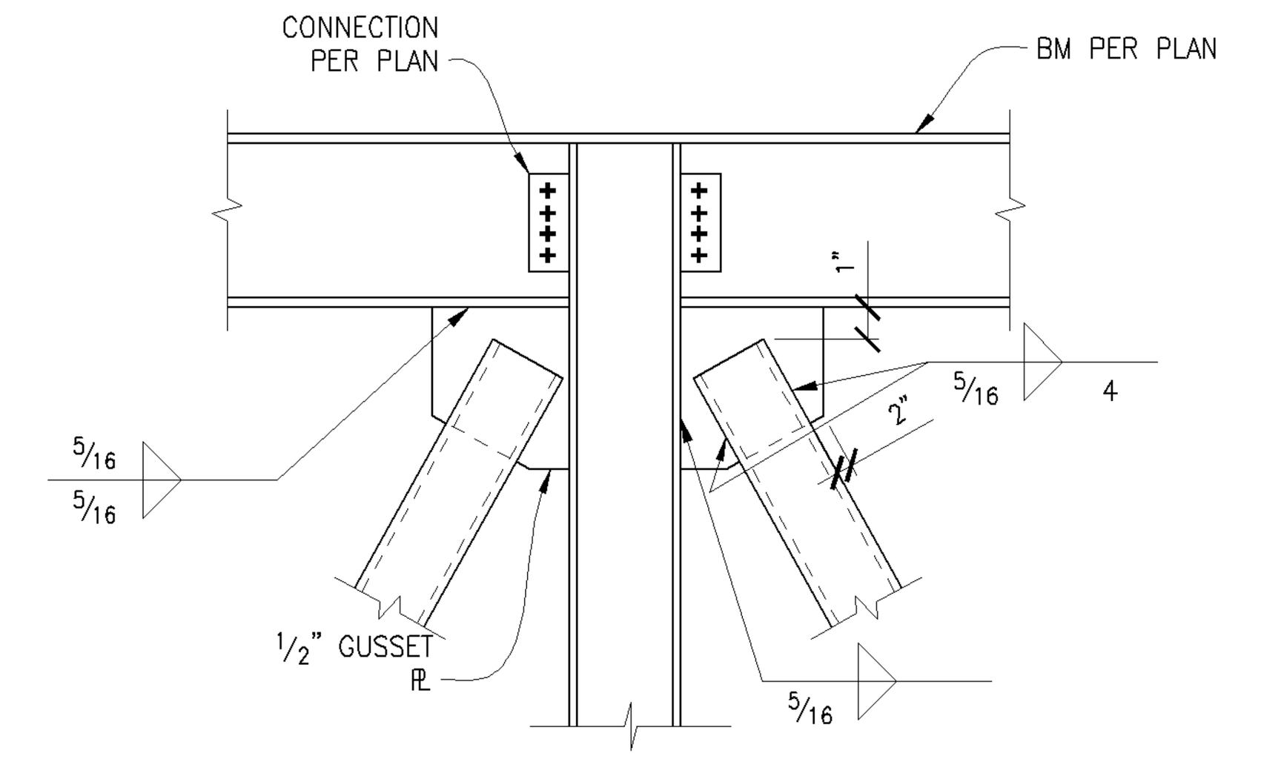
MTL STUD HDR SCHEDULE
1"=1'-0" | 11



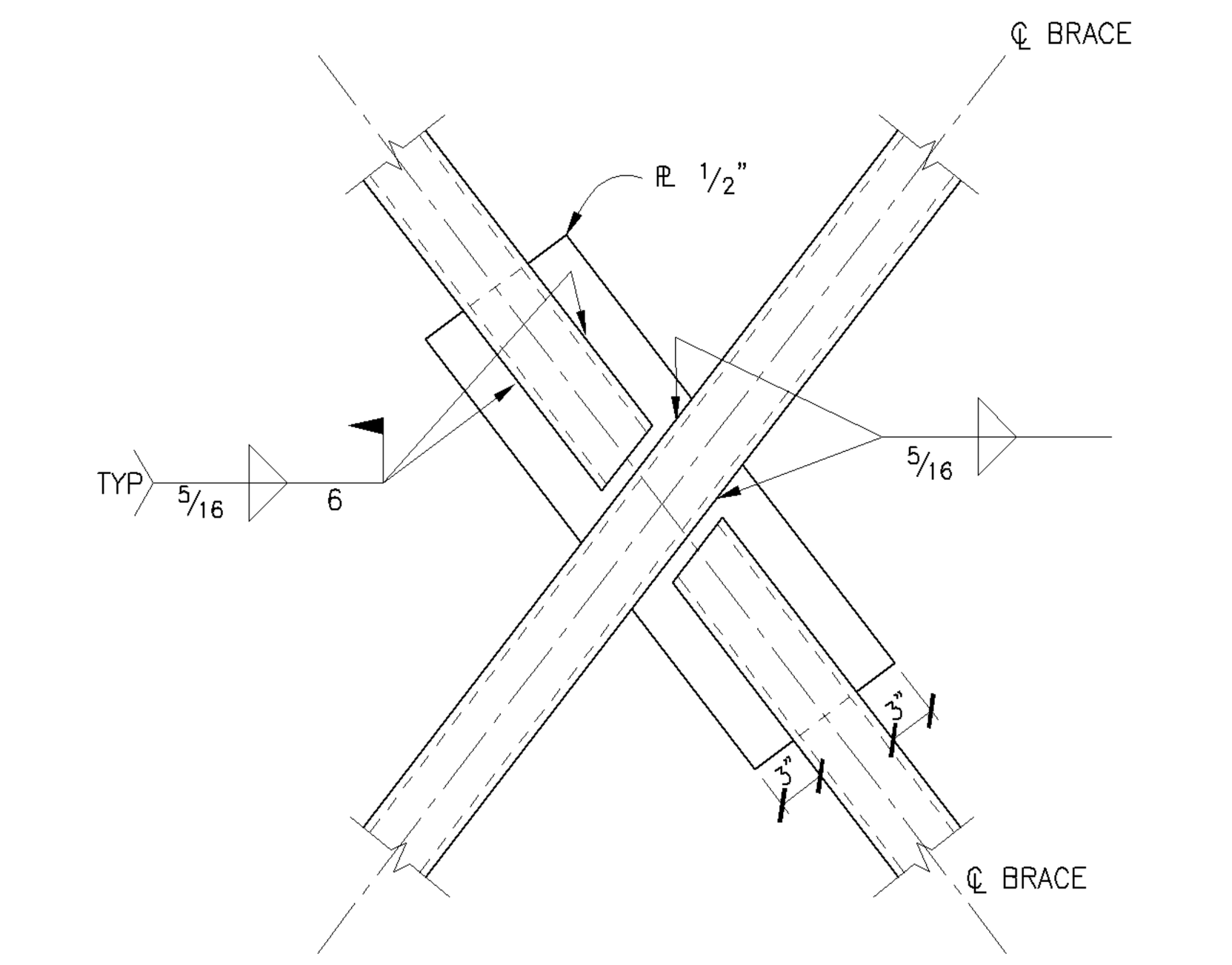
ROLL-UP DOOR SUPPORT FRAME
1"=1'-0" | 8



BRACED FRAME DETAIL
1"=1'-0" | 6



BRACED FRAME DETAIL
1"=1'-0" | 3



BRACED FRAME DETAIL
1"=1'-0" | 4

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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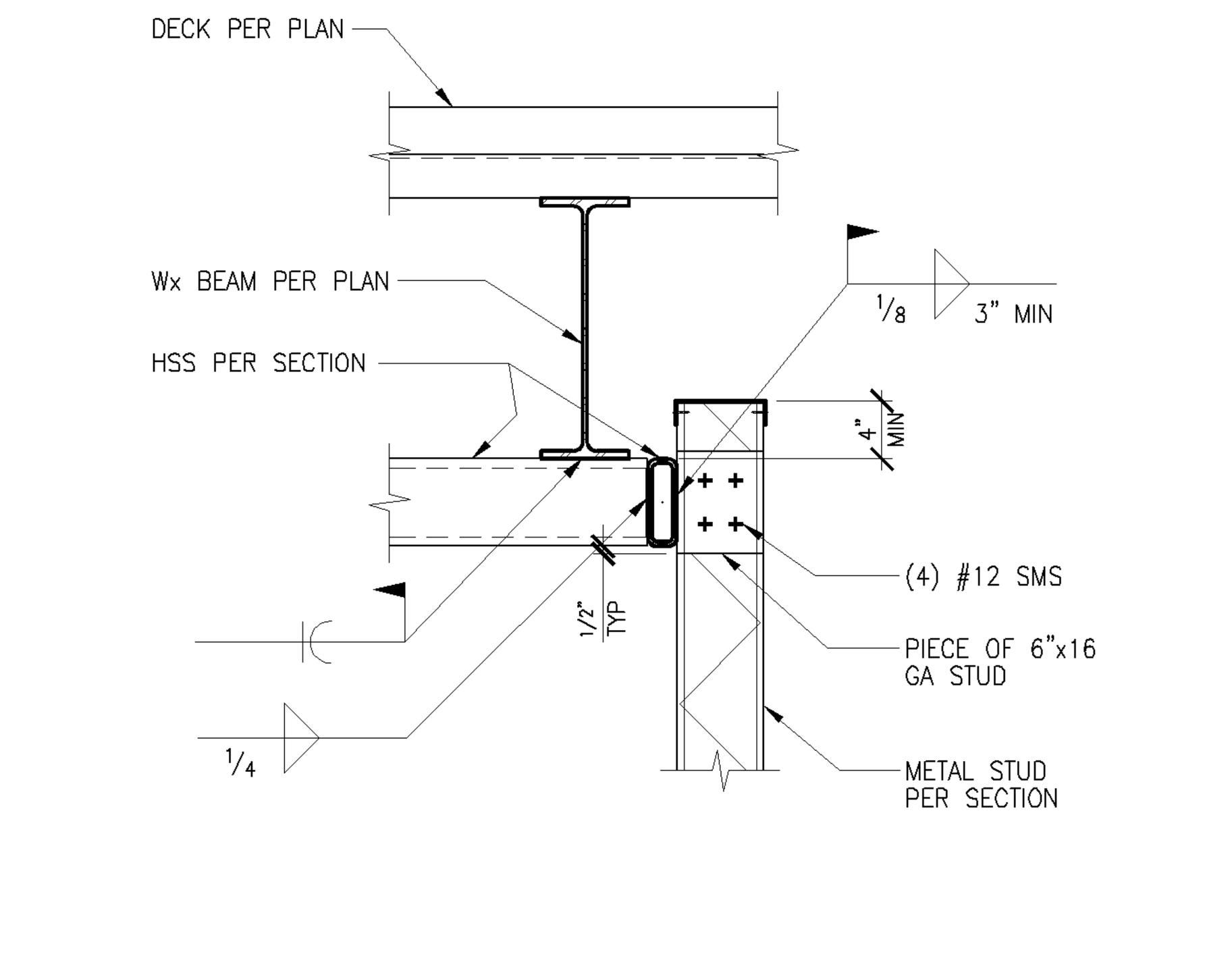
KEY PLAN

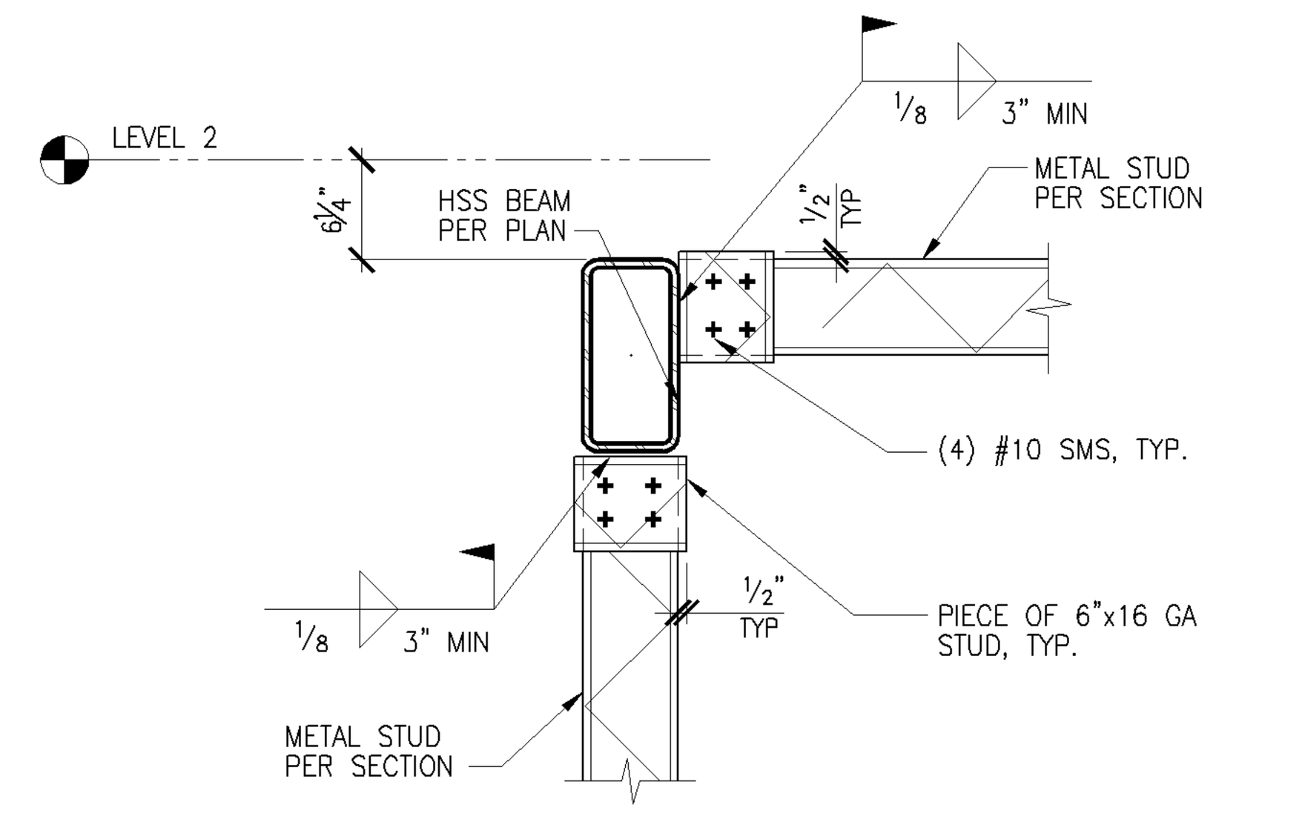
SHEET TITLE

STEEL DETAILS

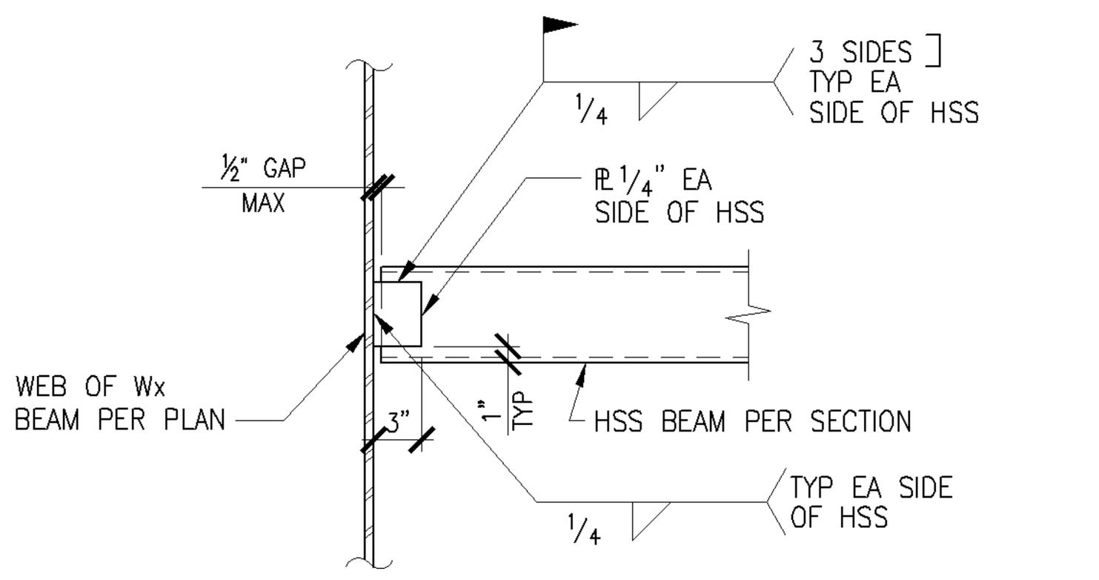
S5-111



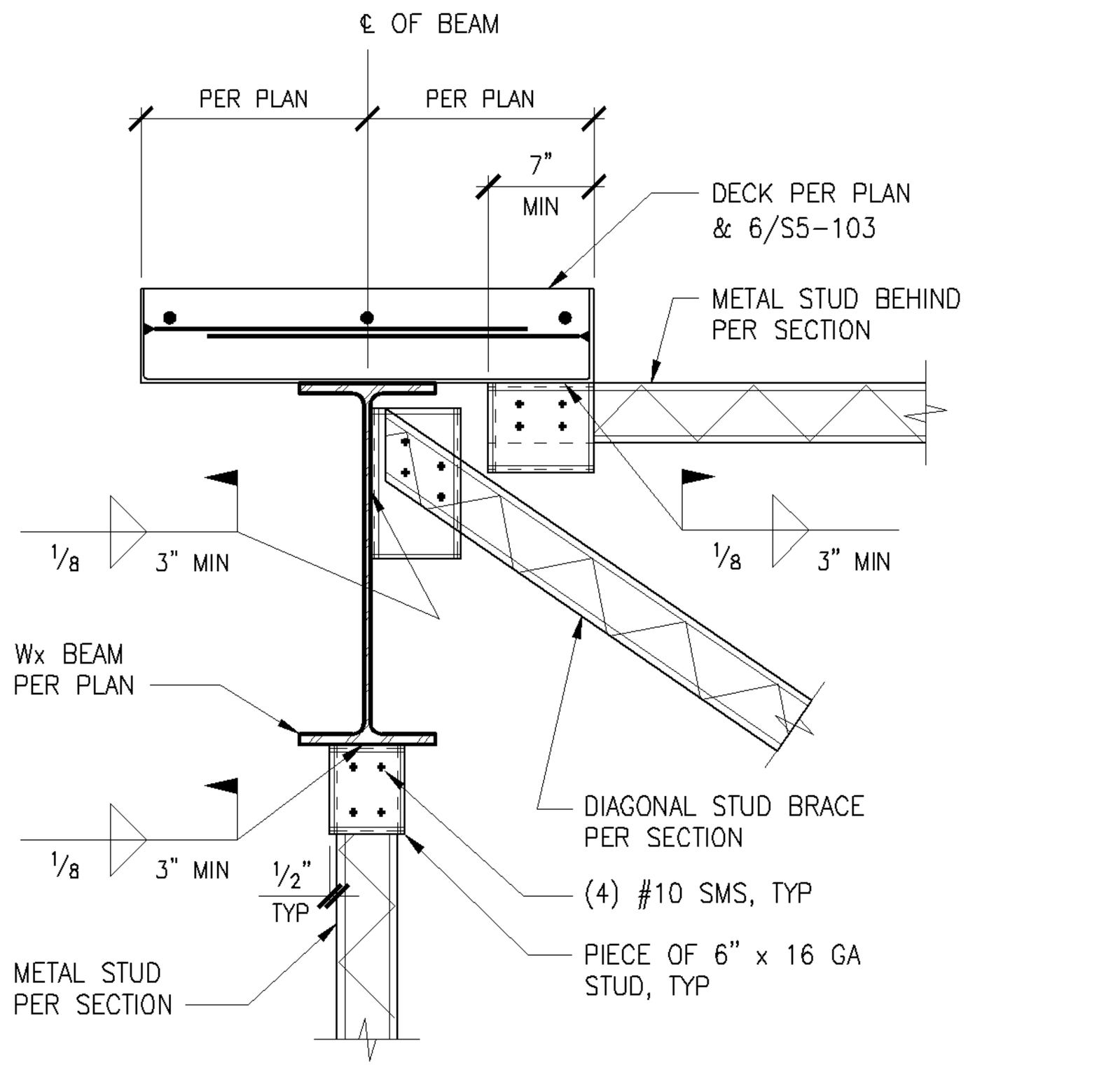
DETAIL 1"=1'-0" 3



DETAIL 1"=1'-0" 4

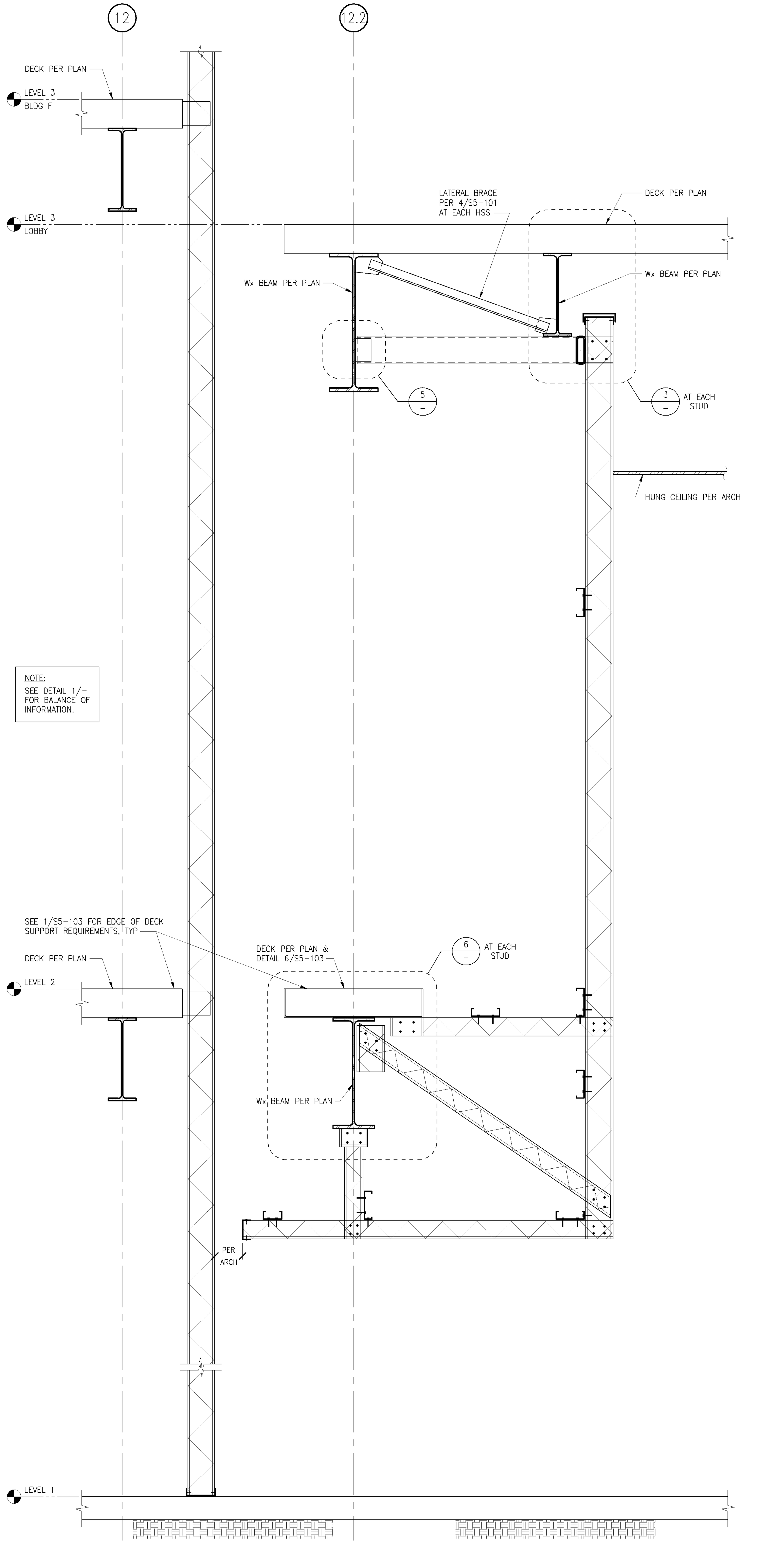


DETAIL 1"=1'-0" 5

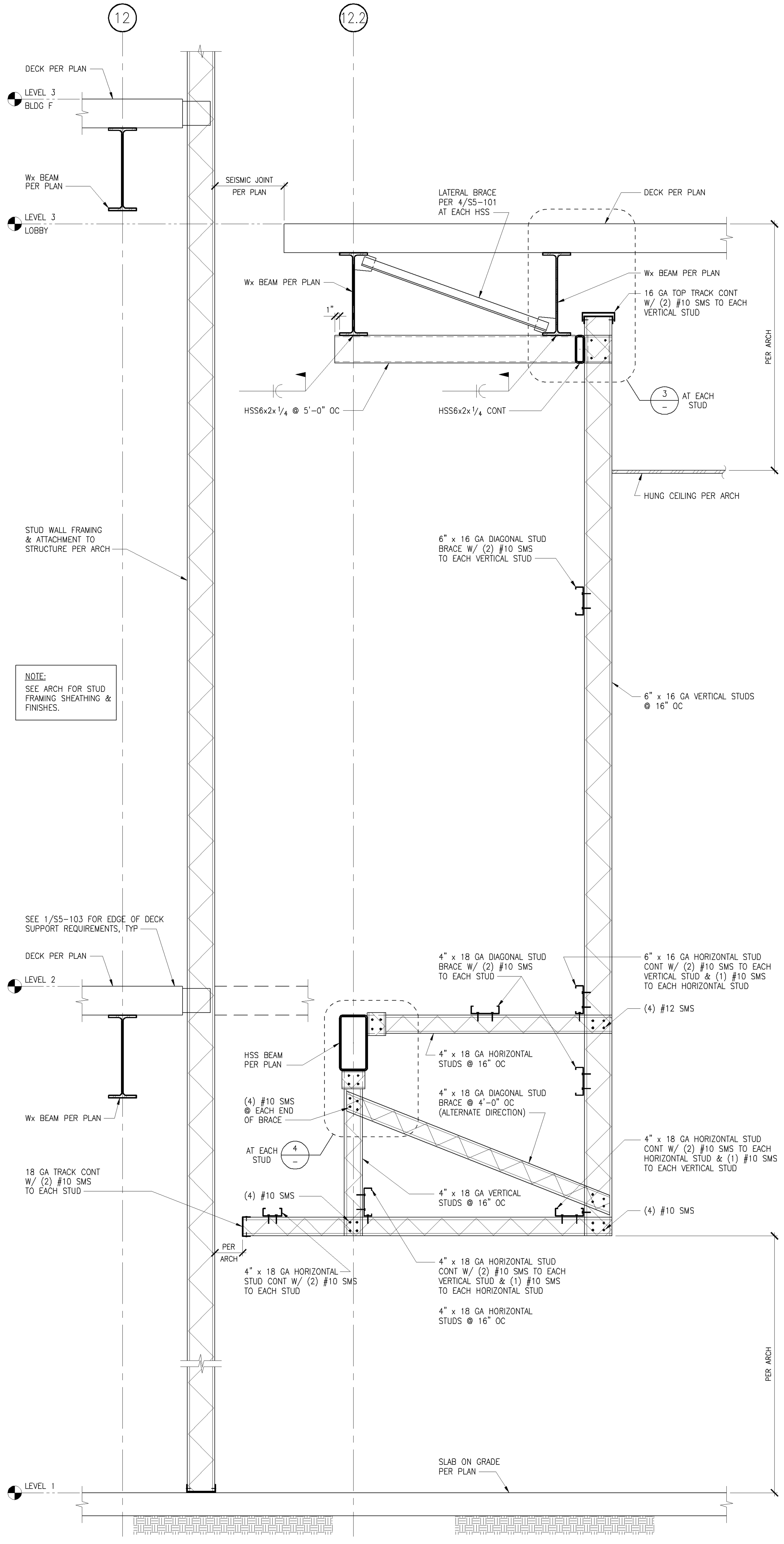


NOTE:
SEE 1/S5-103 FOR EDGE OF DECK SUPPORT REQUIREMENTS.

DETAIL 1"=1'-0" 6



SECTION THRU LOBBY SOFFIT ALONG LINE 12.2 (LINES C.3 TO C.6) 1"=1'-0" 2



SECTION THRU LOBBY SOFFIT ALONG LINE 12.2 (LINES B.5 TO C.3) 1"=1'-0" 1

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Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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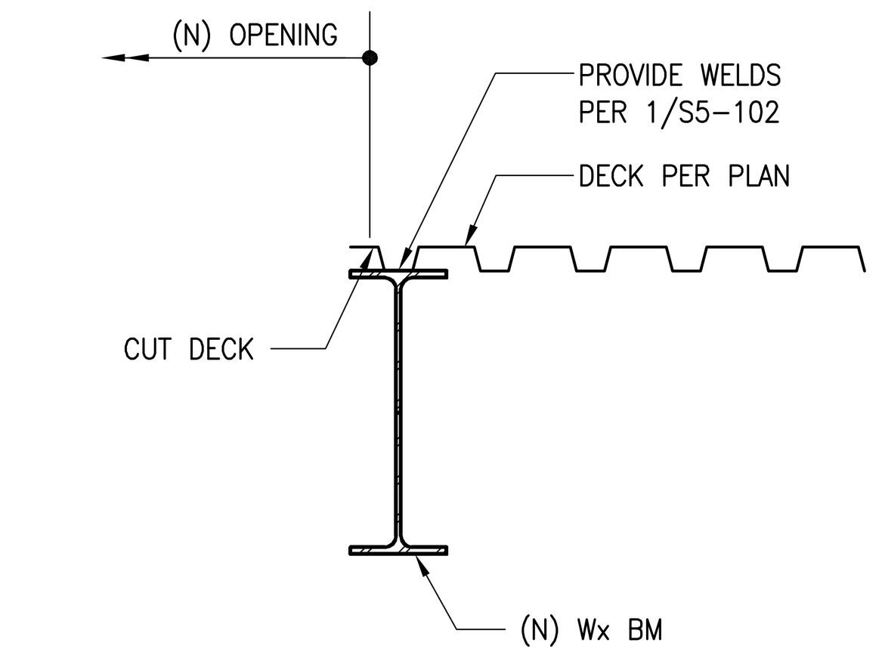
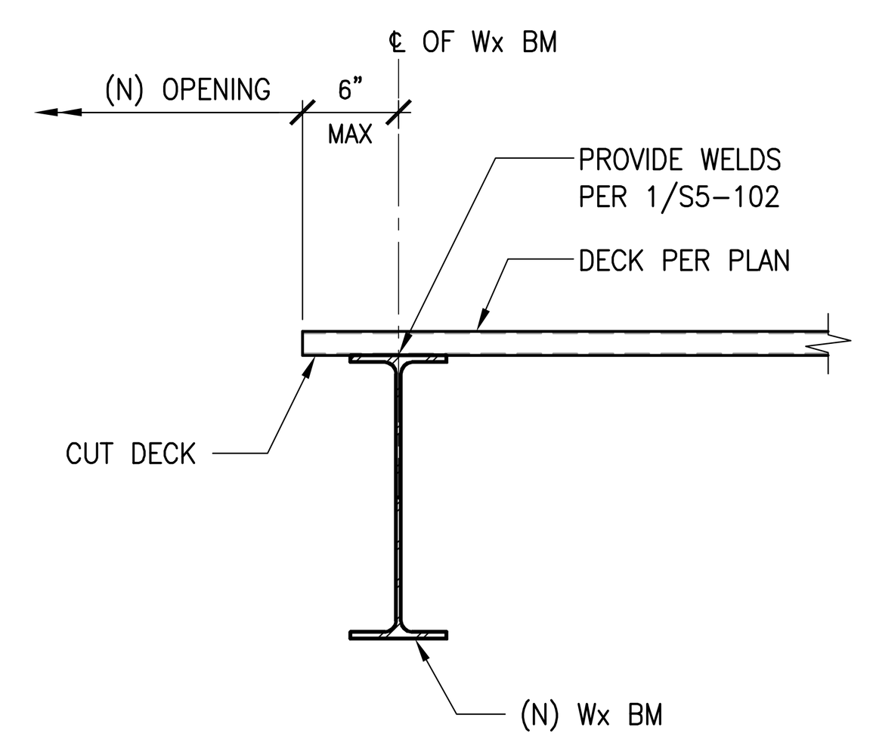
STEEL DETAILS

S5-112



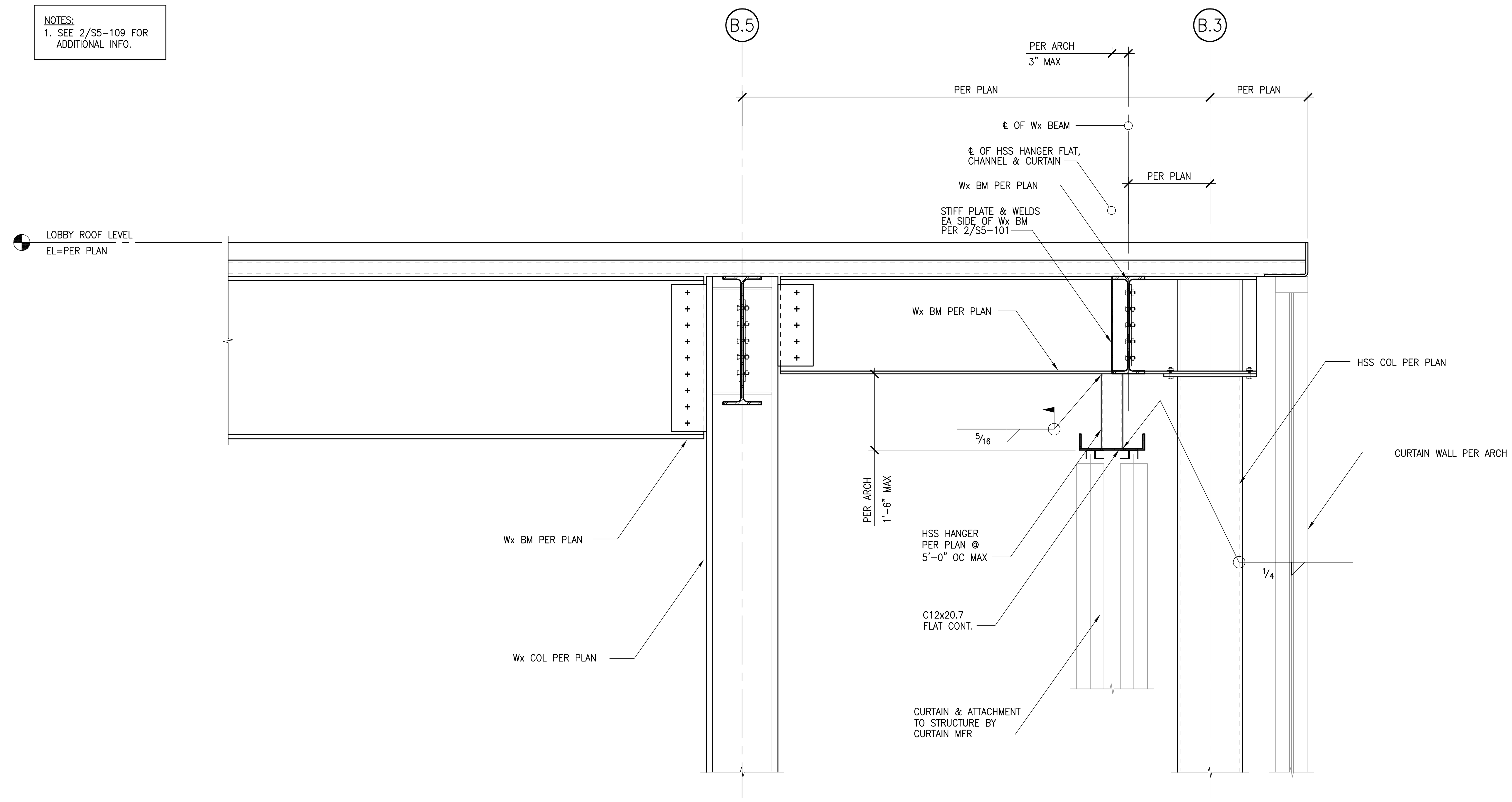
A DECK PARALLEL TO BEAM



B DECK PERPENDICULAR TO BEAM

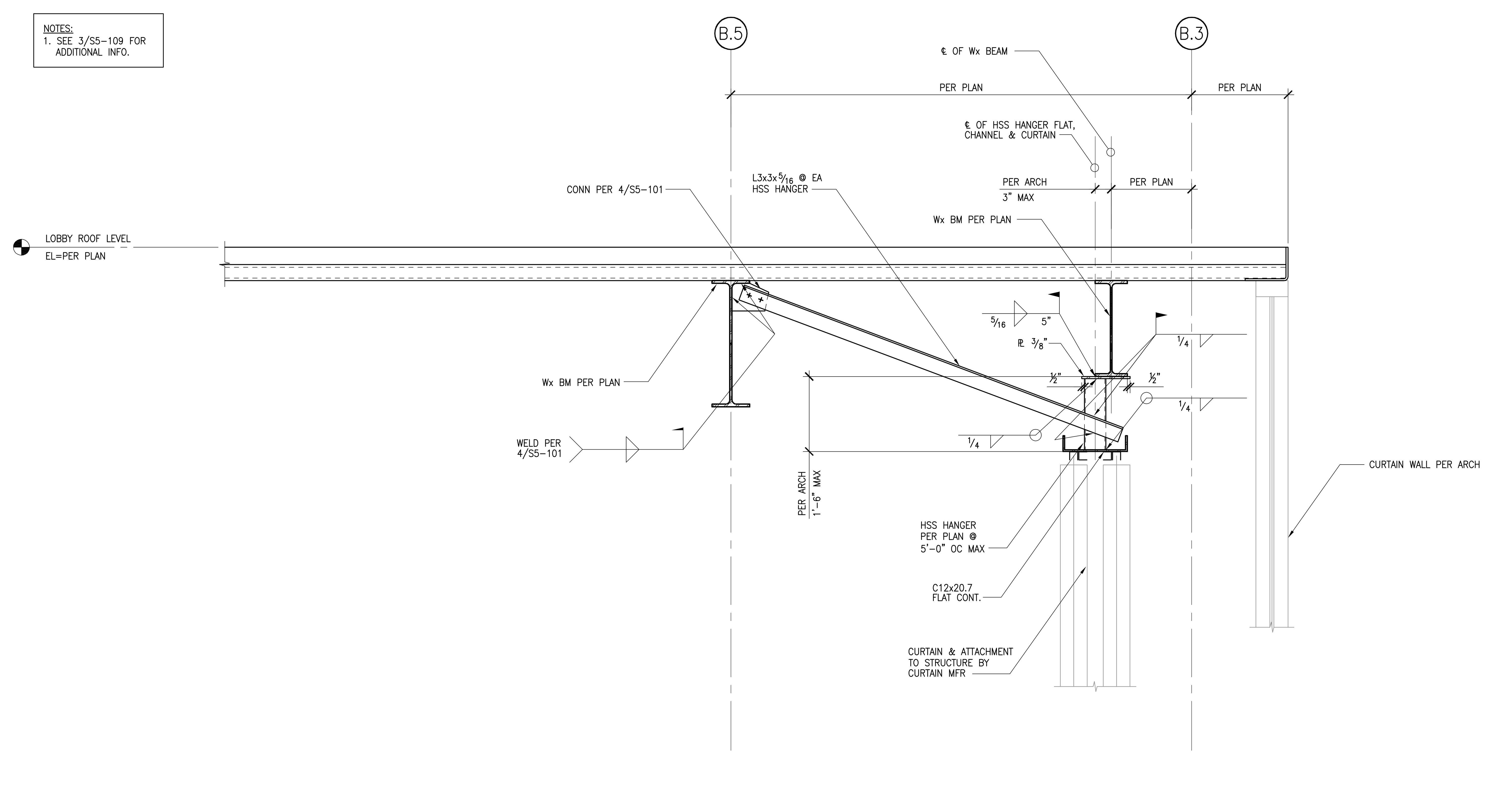
1"=1'-0"

EDGE OF DECK SUPPORT AT (N) ROOF OPENINGS 3



SECTION THRU LOBBY ROOF AT COLUMNS

1/2"=1'-0" 1



SECTION THRU LOBBY ROOF BETWEEN COLUMNS

1/2"=1'-0" 2

OWNER/CLIENT

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TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

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515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
2	12-21-07	BULLETIN 015 – HAZMAT
1	01-16-08	BULLETIN 013 SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

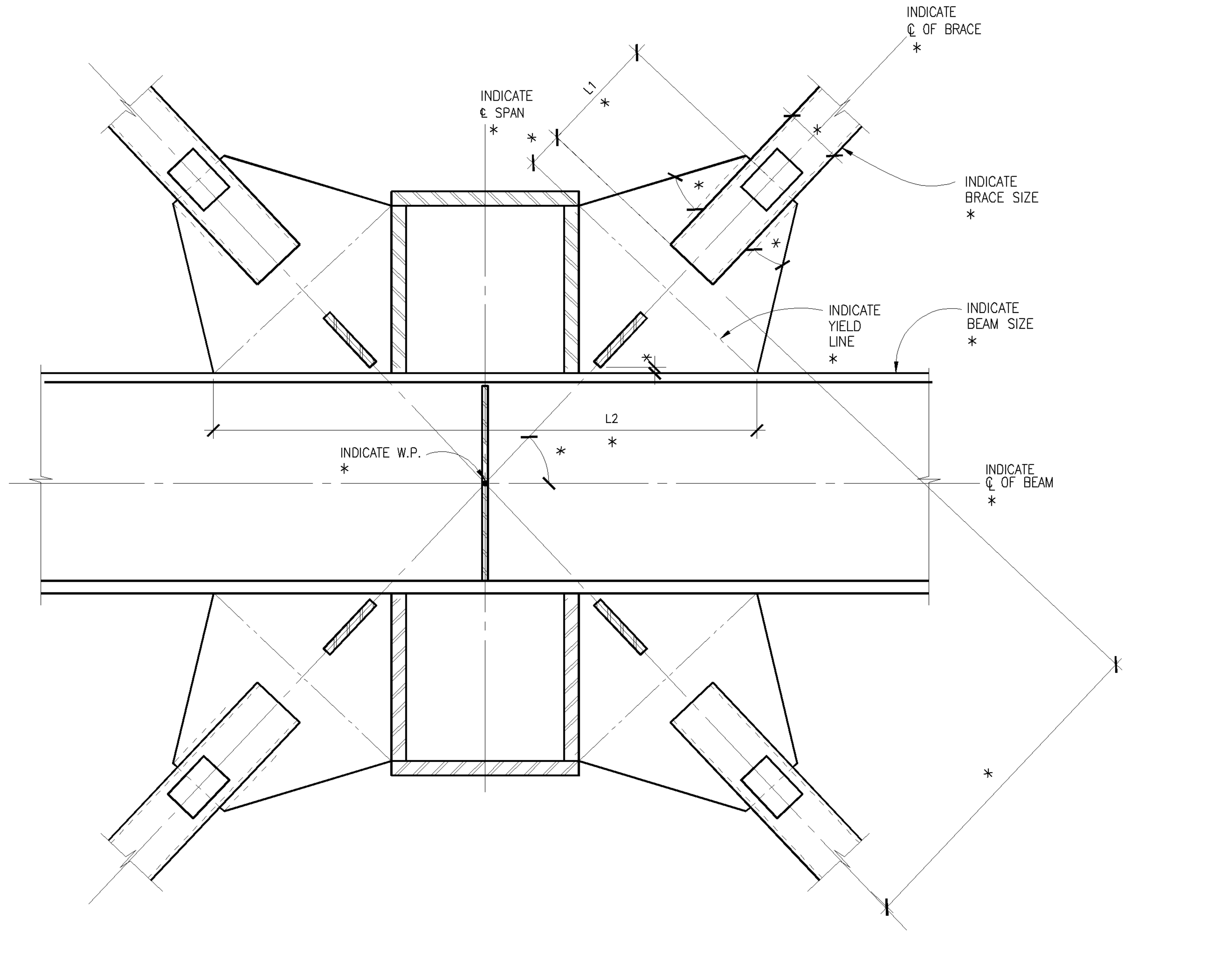
SHEET TITLE

STEEL DETAILS

S5-113

NOTE:

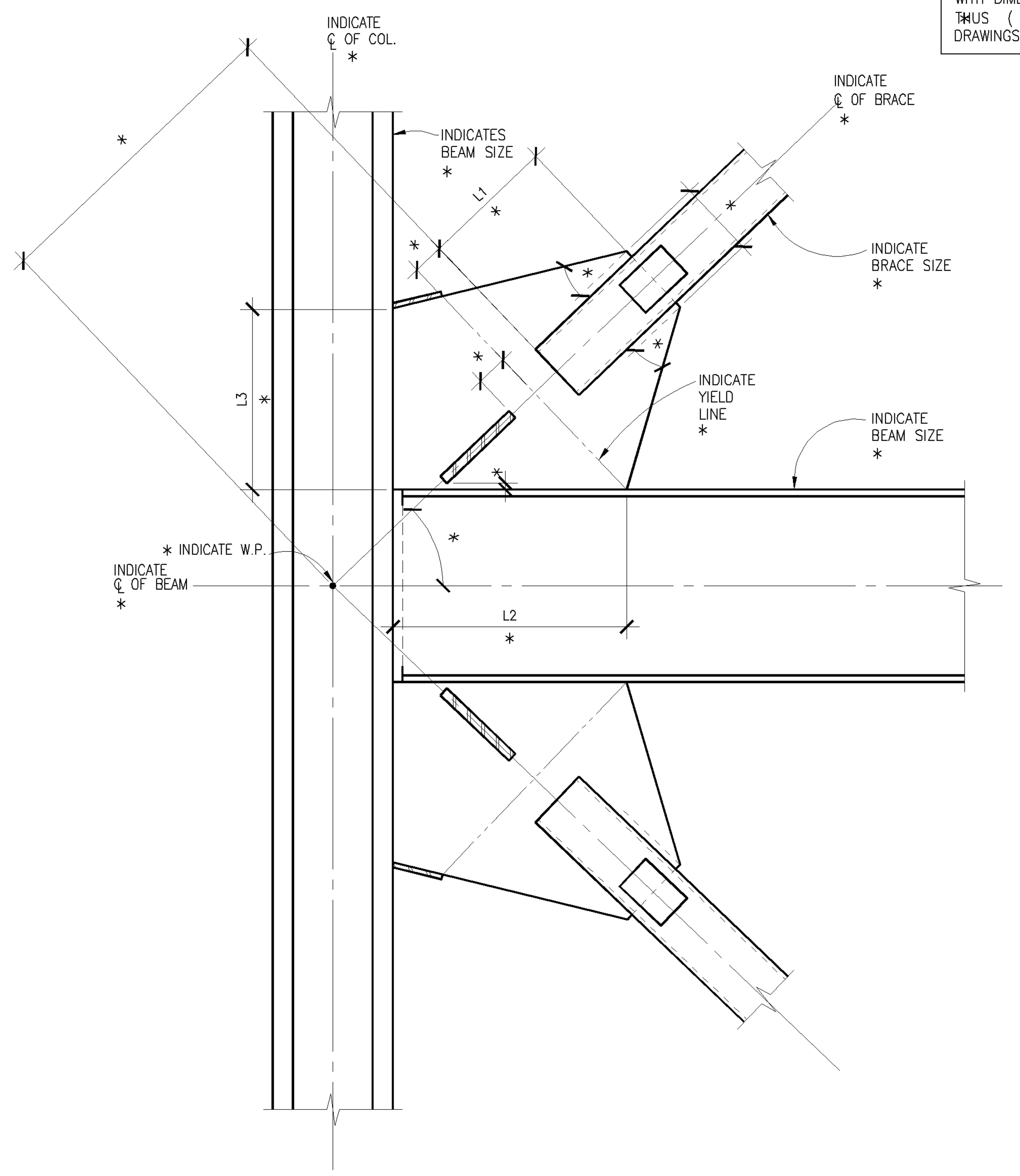
SHOP DRAWINGS SHALL DEPICT CONNECTION DRAWN TO SCALE WITH DIMENSIONS, ANGLES AND OTHER INFORMATION INDICATED THUS ( ) FOR EACH CONNECTION INCLUDED IN THE SHOP DRAWINGS.



BRACE FRAME CONNECTION DETAIL 3/4"=1'-0" **3**

NOTE:

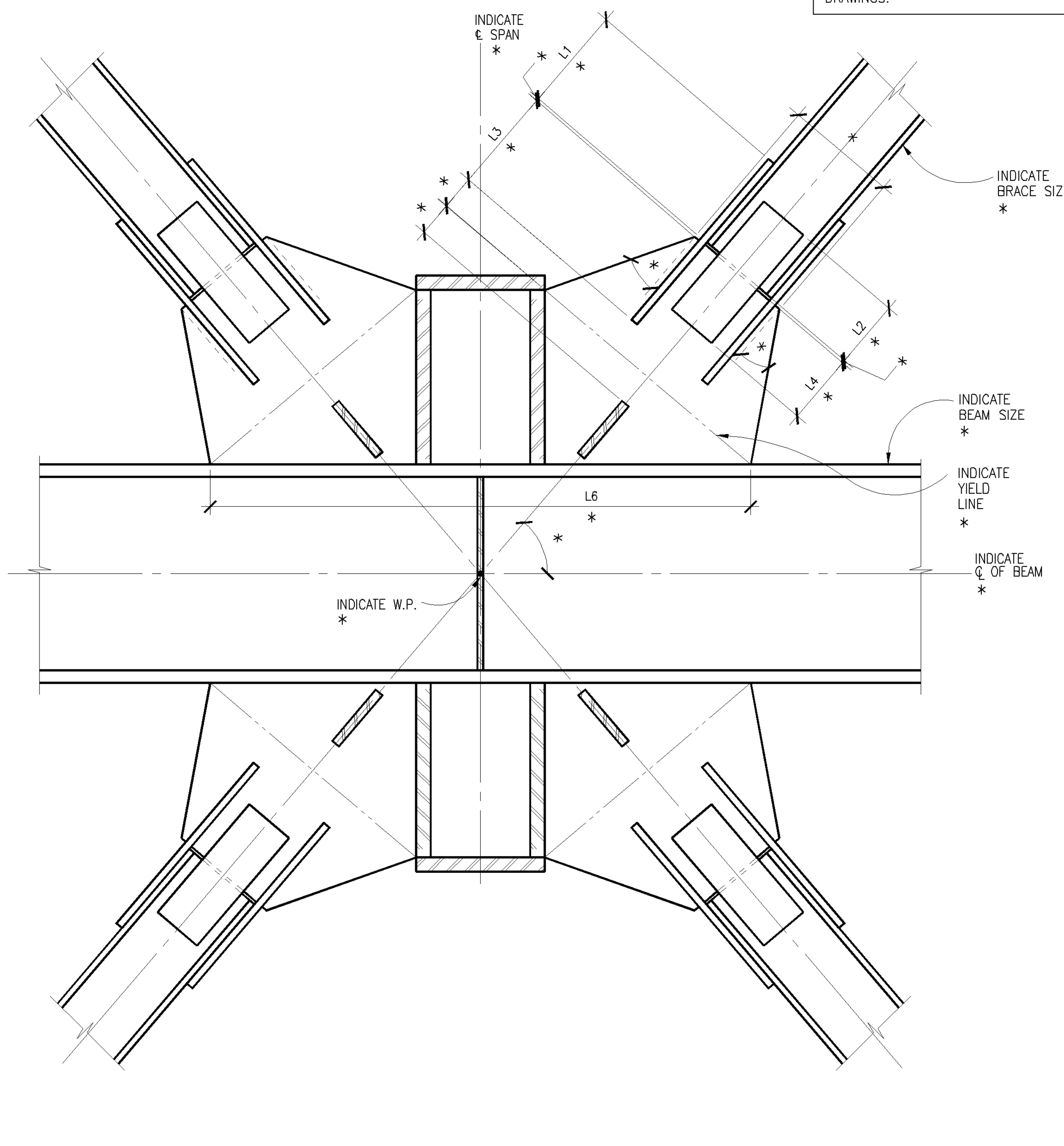
SHOP DRAWINGS SHALL DEPICT CONNECTION DRAWN TO SCALE WITH DIMENSIONS, ANGLES AND OTHER INFORMATION INDICATED THUS ( ) FOR EACH CONNECTION INCLUDED IN THE SHOP DRAWINGS.



BRACE FRAME CONNECTION DETAIL 3/4"=1'-0" **1**

NOTE:

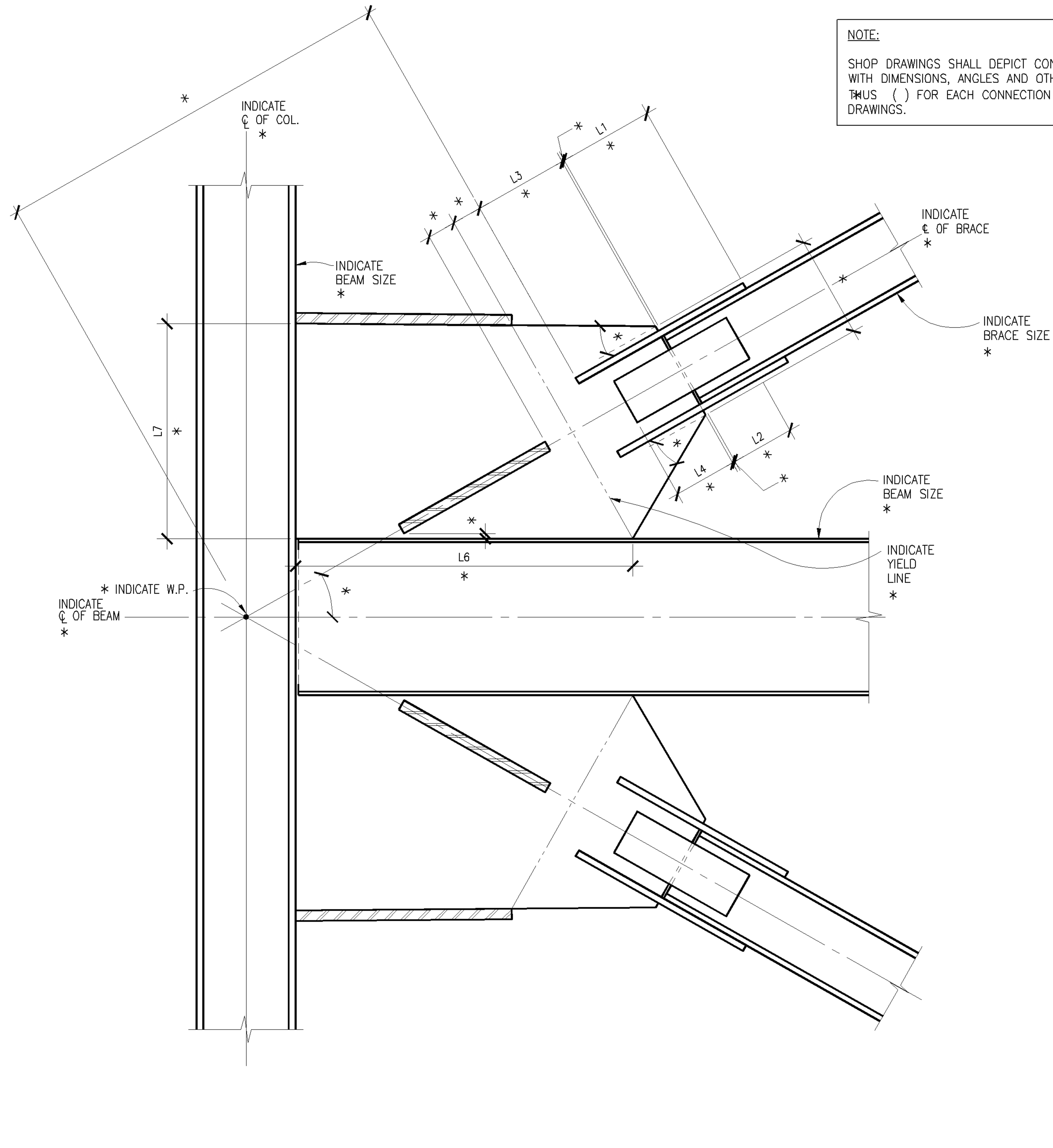
SHOP DRAWINGS SHALL DEPICT CONNECTION DRAWN TO SCALE WITH DIMENSIONS, ANGLES AND OTHER INFORMATION INDICATED THUS ( ) FOR EACH CONNECTION INCLUDED IN THE SHOP DRAWINGS.



BRACE FRAME CONNECTION DETAIL 3/4"=1'-0" **4**

NOTE:

SHOP DRAWINGS SHALL DEPICT CONNECTION DRAWN TO SCALE WITH DIMENSIONS, ANGLES AND OTHER INFORMATION INDICATED THUS ( ) FOR EACH CONNECTION INCLUDED IN THE SHOP DRAWINGS.



BRACE FRAME CONNECTION DETAIL 3/4"=1'-0" **2**

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
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CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

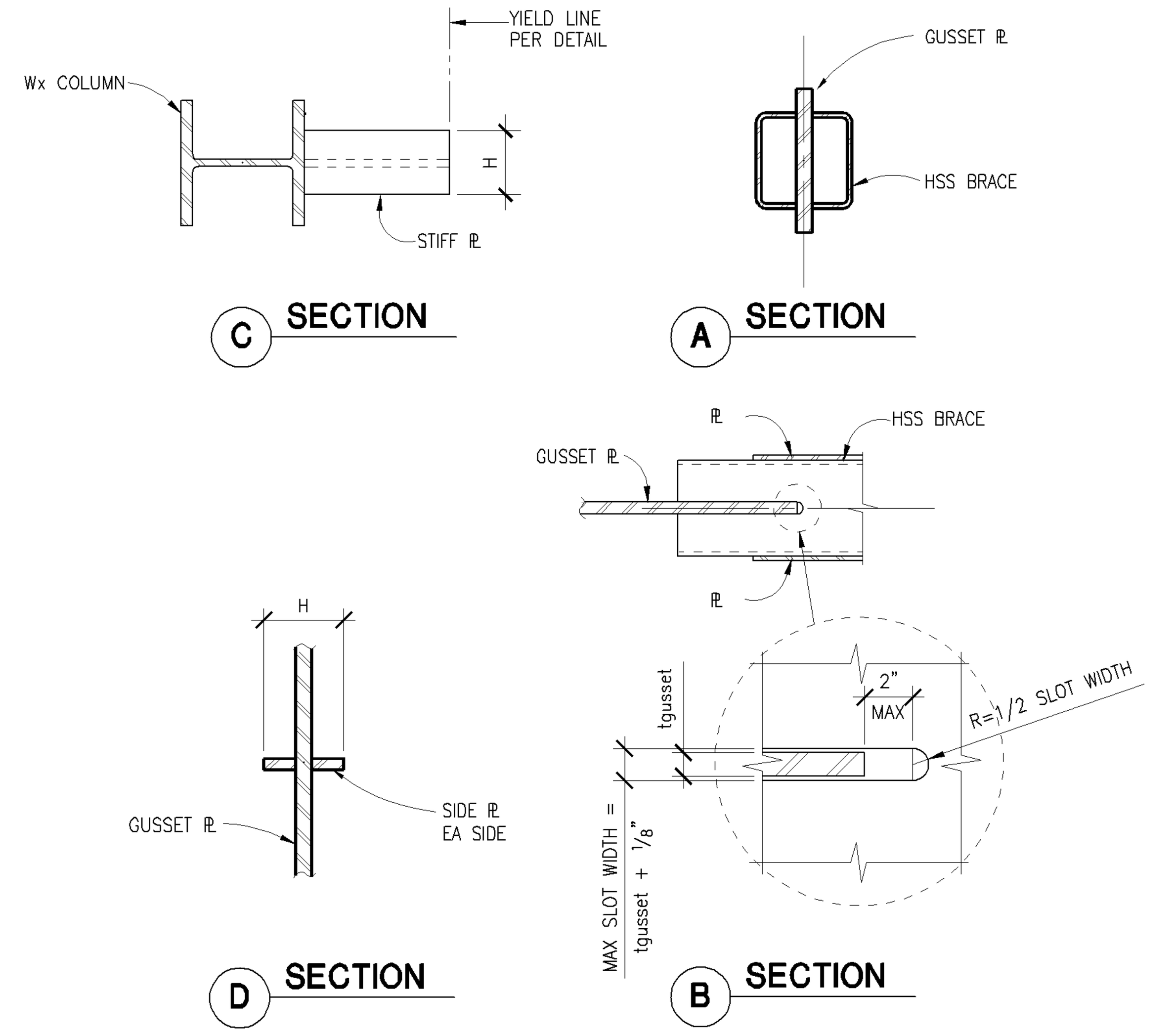
SHEET TITLE

BRACED FRAME DETAILS—SHOP DRAWING REFERENCE INFORMATION

S6-100

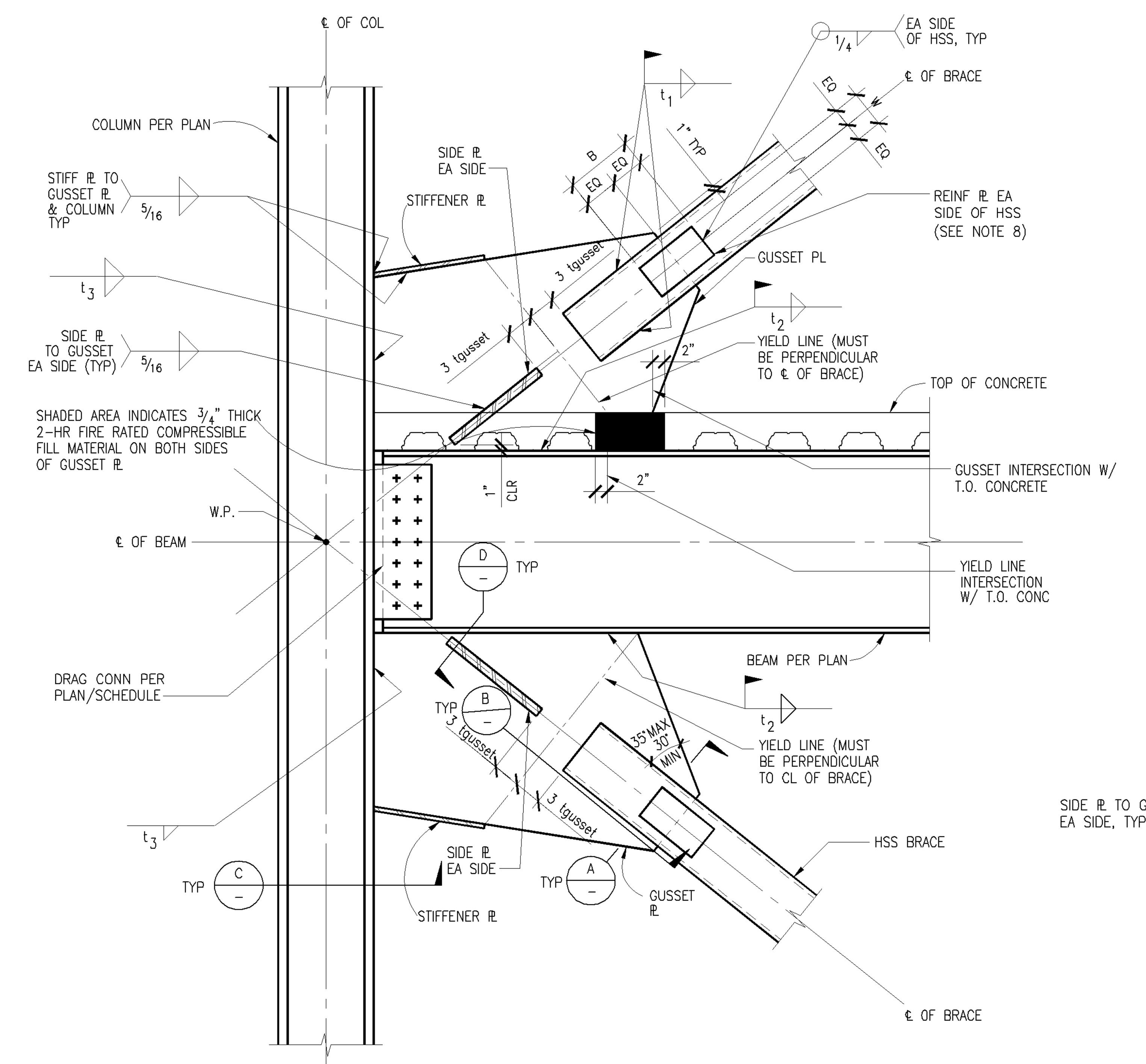
File: W:\2006\1061030 - Guidant Corporate Campus, Temecula, CA\Sdraft\1061030_S6.100.dwg
Plotted: 3/21/07 at 1:14pm By: SGutierrez
XREFs: 1061030_bdr.dwg

CONNECTION SCHEDULE A											
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)			WELD THICKNESS (IN)			PLATE INTERFACE LENGTH (IN)		
			GUSSET	STIFF	SIDE	t1	t2	t3	L1	L2	L3
A1	HSS10x10x5/8	–	1 1/2	–	–	3/4	5/8	5/8	21	45	30
A2	HSS10x10x5/8	–	1 1/2	–	–	5/8	1/2	1/2	21	45	30
A3	HSS10x10x5/8	–	1 1/4	–	–	5/8	5/8	5/8	22	37	28
A4	HSS10x10x5/8	–	1 1/4	–	–	5/8	1/2	1/2	19	32	32
A5	HSS8x8x1/2	–	1 1/4	–	–	1/2	3/8	3/8	18	30	36
A6	HSS10x10x5/8	–	1 1/2	–	–	5/8	3/8	1/2	21	35	42
A7	HSS10x10x5/8	–	1 1/2	–	–	3/4	1/2	5/8	24	35	41
A8	HSS8x8x1/2	–	1	–	–	1/2	5/8	5/8	22	32	31
A9	HSS10x10x5/8	–	1 1/4	–	–	3/4	3/4	3/4	25	37	36
A10	HSS10x10x5/8	–	1 1/2	–	–	3/4	7/8	7/8	26	38	30
A11	HSS8x8x1/2	–	1 1/2	–	–	5/8	3/8	3/8	18	32	41
A12	HSS10x10x5/8	–	1 3/4	–	–	3/4	3/8	1/2	23	40	50
A13	HSS10x10x5/8	–	1 1/2	–	–	3/4	1/2	5/8	25	38	46
A14	HSS10x10x5/8	–	1 1/2	–	–	3/4	3/4	5/8	24	41	31
A15	HSS10x10x5/8	–	1	–	–	5/8	5/8	5/8	20	31	26
A16	HSS10x10x5/8	–	1 1/2	–	–	3/4	7/8	3/4	24	36	30

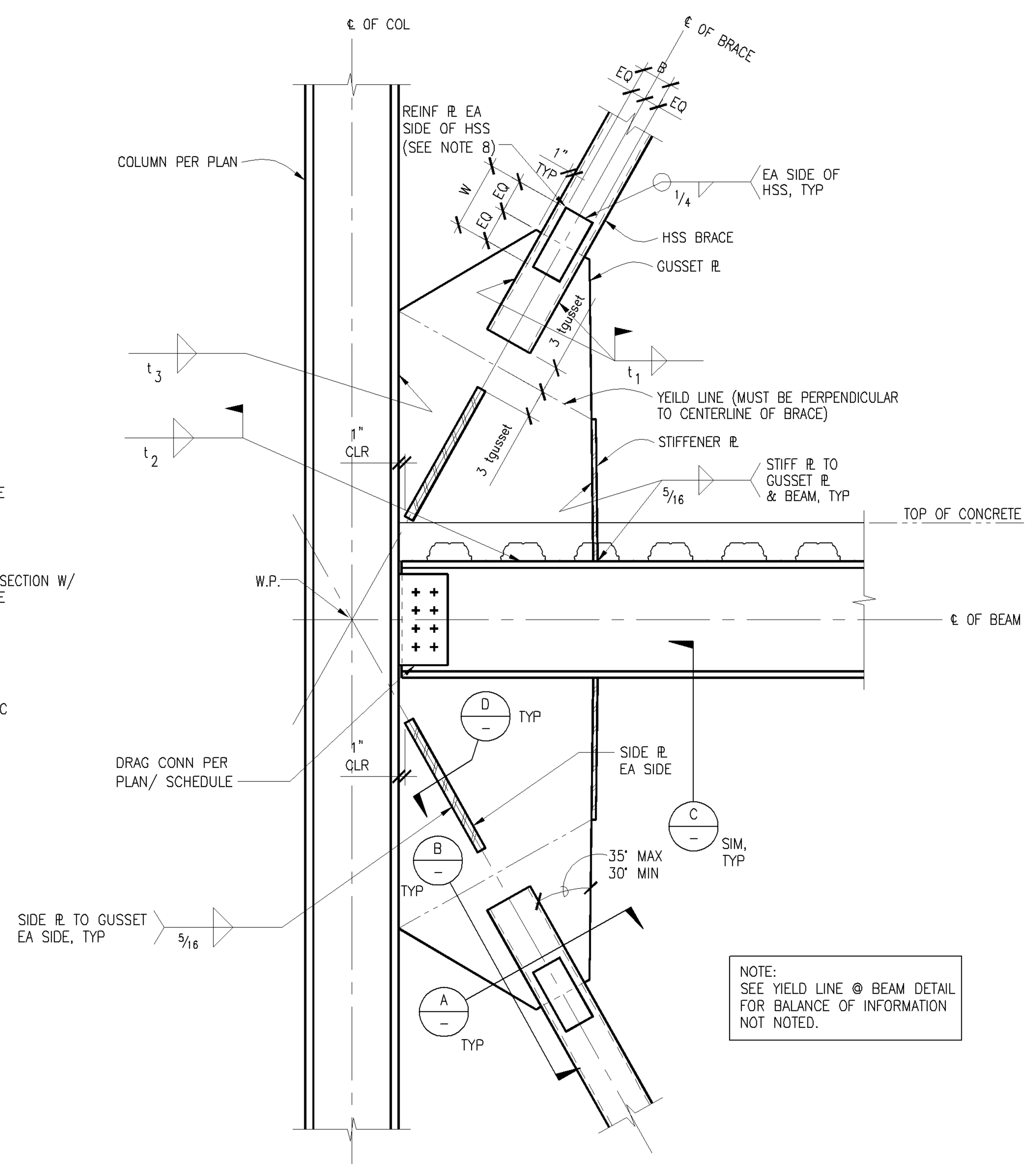


NOTES:

1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. BRACES MAY OCCUR ABOVE AND BELOW, ONLY ABOVE, OR ONLY BELOW. SEE BRACED FRAME ELEVATION FOR BRACE CONFIGURATION.
4. SEE S6–100 FOR GRAPHICAL DEPICTION OF DIMENSIONS L1 THRU L3
5. SEE S6–100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
6. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
7. SEE 2/S6–103 FOR GUSSET PLATES OCCURING AT EDGE OF SLAB.
8. REINF ℙ DIMENSIONS:
   AT HSS10x10: B = 7" W = 16" T = 3/8"
   AT HSS8x8: B = 5" W = 12" T = 3/8"



YIELD LINE @ BEAM



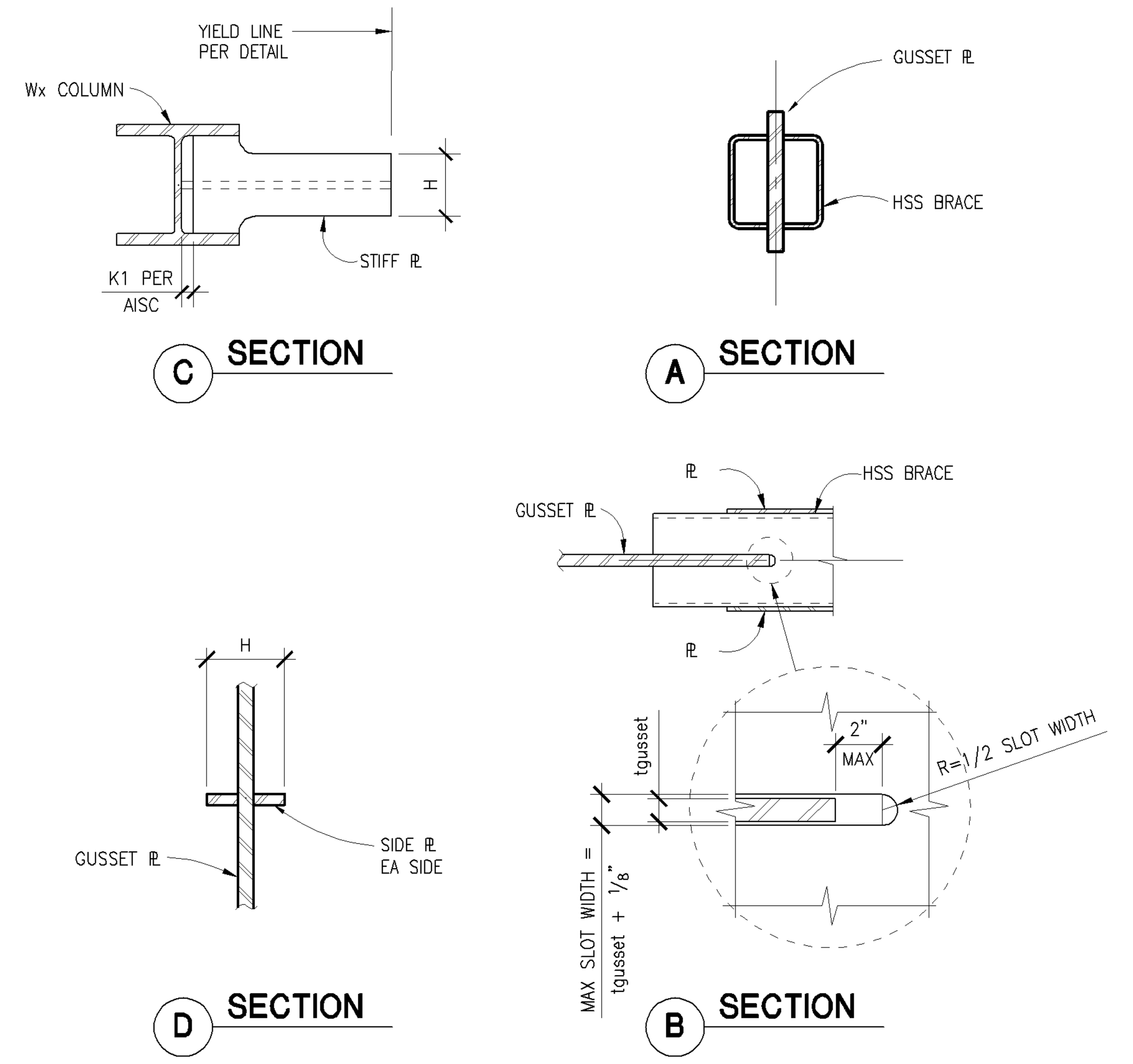
YIELD LINE @ COLUMN

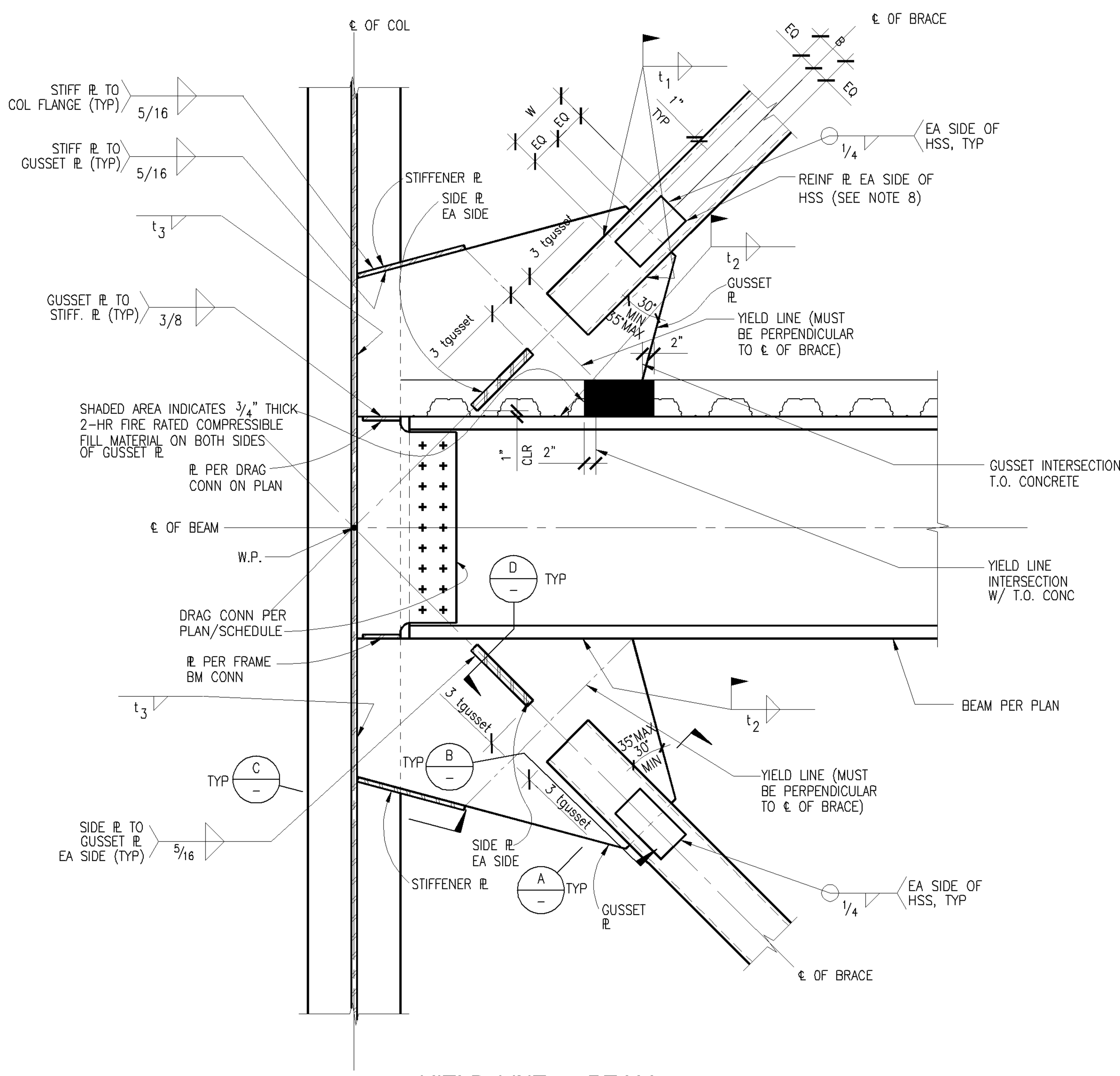
NOTE:
SEE YIELD LINE @ BEAM DETAIL FOR BALANCE OF INFORMATION NOT NOTED.

BRACE FRAME CONNECTION SCHEDULE AND DETAIL (TYPE A)

3/4"=1'-0" 1

CONNECTION SCHEDULE B											
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)			WELD THICKNESS (IN)			PLATE INTERFACE LENGTH (IN)		
			GUSSET	STIFF	SIDE	t1	t2	t3	L1	L2	L3
B1	HSS10x10x5/8	–	1 1/4	–	–	5/8	1/2	5/8	19	46	23
B2	HSS10x10x5/8	–	1 1/2	–	–	5/8	5/8	5/8	23	50	26
B3	HSS10x10x5/8	–	1 1/4	–	–	1/2	1/2	1/2	24	44	30
B4	HSS10x10x5/8	–	1 1/4	–	–	3/4	5/8	5/8	24	44	30
B5	HSS10x10x5/8	–	1 1/2	–	–	3/4	5/8	1/2	24	49	33
B6	HSS10x10x5/8	–	1 1/2	–	–	3/4	5/8	5/8	24	48	30





YIELD LINE @ BEAM

NOTES:

1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. BRACES MAY OCCUR ABOVE AND BELOW, ONLY ABOVE, OR ONLY BELOW. SEE BRACED FRAME ELEVATION FOR BRACE CONFIGURATION.
4. SEE S6–100 FOR GRAPHICAL DEPICTION OF DIMENSIONS L1 THRU L3
5. SEE S6–100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
6. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
7. SEE 2/S6–103 FOR GUSSET PLATES OCCURING AT EDGE OF SLAB.
8. REINF ℙ DIMENSIONS:
   AT HSS10x10: B = 7" W = 16" T = 3/8"
   AT HSS8x8: B = 5" W = 12" T = 3/8"

BRACE FRAME CONNECTION SCHEDULE AND DETAIL (TYPE B)

3/4"=1'-0" 2

OWNER/CLIENT
Abbott Vascular
TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591–4628

ARCHITECT
DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
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CARLSBAD, CA 92010
Tel: 760.496.3714
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REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03–22–07	FOR CONSTRUCTION
2	01–31–07	B & S RESUBMITTAL
1	10–23–06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

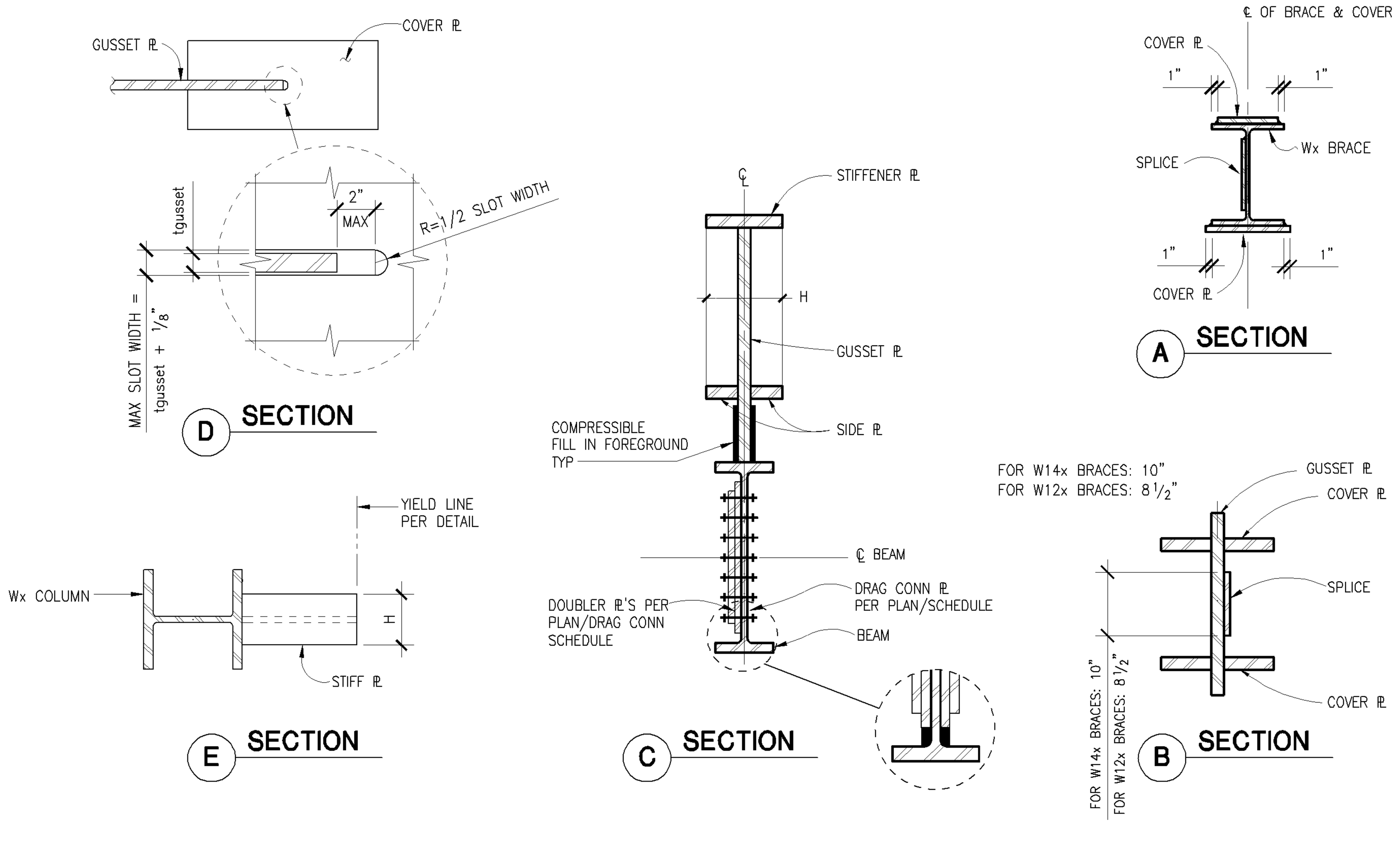
KEY PLAN

SHEET TITLE
BRACED FRAME DETAILS

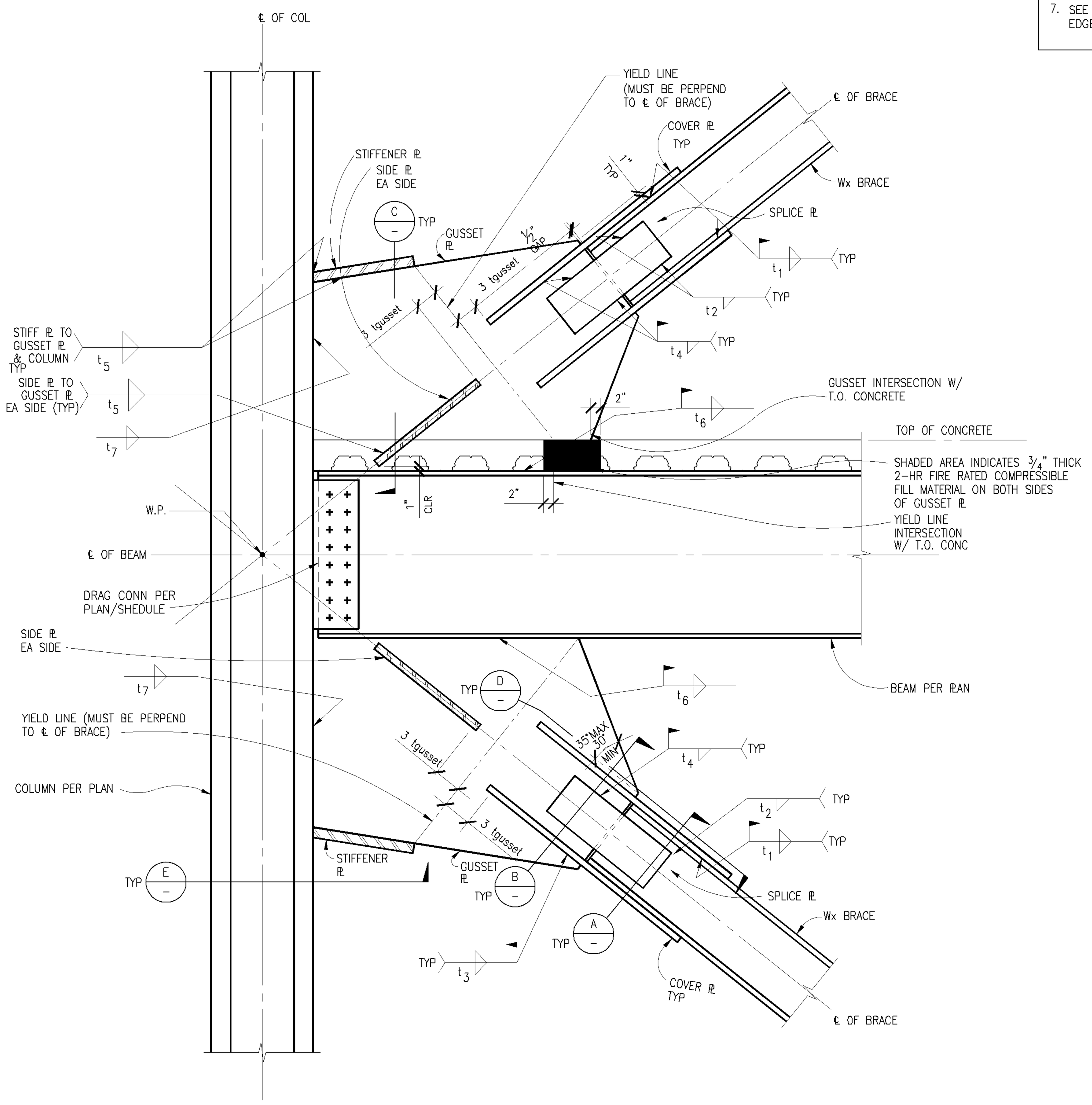
S6-101

CONNECTION SCHEDULE C

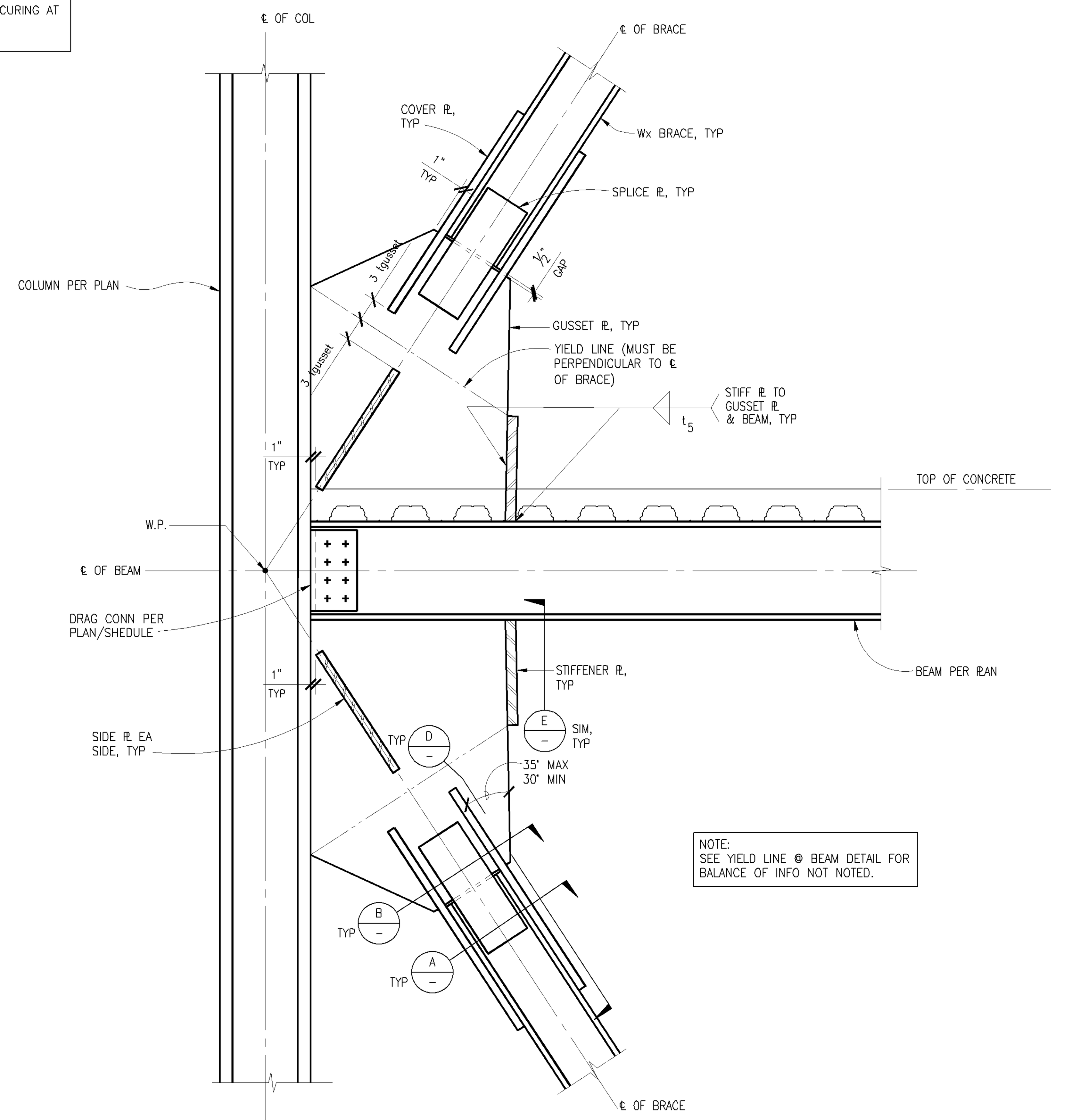
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)					
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
C1	W14x90	–	1 1/2	1	3/4	–	–	5/8	1/2	1/2	1/2	5/16	7/8	3/4	23	17	17	17	46	31
C2	W14x109	10	1 1/4	1 1/4	5/8	1/2	1"	3/4	3/8	5/8	3/8	5/16	3/4	5/8	25	17	17	17	59	38
C3	W14x145	–	2	1 3/4	3/4	–	–	3/4	1/2	1/2	1/2	5/16	7/8	7/8	33	17	28	17	57	44
C4	W14x90	–	1 3/4	1	1/2	–	–	5/8	3/8	1/2	3/8	5/16	3/4	3/4	22	17	17	17	47	31
C5	W14x109	10	1 1/4	1	5/8	1/2	1"	3/4	3/8	1/2	3/8	5/16	5/8	5/8	20	17	17	17	62	37
C6	W14x145	–	2	1 1/4	1	–	–	3/4	5/8	1/2	5/8	5/16	7/8	3/4	25	17	24	17	54	40
C7	W14x90	–	1 3/4	1	3/4	–	–	5/8	1/2	3/8	5/8	5/16	7/8	7/8	23	17	20	17	41	32
C8	W14x109	–	2	1 1/4	1/2	–	–	3/4	3/8	5/8	3/8	5/16	3/4	3/4	23	17	17	17	50	35
C9	W14x145	–	1 3/4	1 1/2	1	–	–	3/4	5/8	1/2	5/8	5/16	1	1	30	17	30	17	48	42
C10	W14x90	–	1 1/2	1	1/2	–	–	5/8	3/8	1/2	3/8	5/16	3/4	3/4	21	17	17	17	36	36
C11	W14x109	–	1 3/4	1 1/4	1/2	–	–	3/4	3/8	1/2	3/8	5/16	7/8	7/8	23	17	22	17	38	34
C12	W14x145	–	2	1 1/2	3/4	–	–	3/4	1/2	5/8	1/2	5/16	7/8	7/8	27	17	18	17	39	44
C13	W14x132	–	1 3/4	1 1/4	5/8	–	–	3/4	3/8	5/8	3/8	5/16	5/8	3/4	23	17	18	17	36	42
C14	W14x132	–	1 3/4	1 1/4	5/8	–	–	3/4	3/8	3/8	3/8	5/16	7/8	7/8	23	17	25	17	43	36
C15	W14x132	–	2	1 1/2	3/4	–	–	3/4	1/2	3/4	1/2	5/16	3/4	7/8	28	17	17	17	38	47
C16	W14x132	–	1 3/4	1 1/2	1	–	–	3/4	5/8	1/2	5/8	5/16	7/8	7/8	28	17	28	17	53	42
C17	W14x132	–	1 3/4	1 1/2	1	–	–	3/4	5/8	5/8	5/8	5/16	7/8	7/8	28	17	22	17	46	39
C18	W14x132	–	1 3/4	1 1/2	1	–	–	3/4	5/8	5/8	5/8	5/16	7/8	7/8	28	17	22	17	46	39
C19	W14x132	–	1 1/2	1 1/2	1	–	–	3/4	5/8	1/2	5/8	5/16	7/8	7/8	28	17	27	17	42	40
C20	W14x132	–	1 1/2	1 1/2	1	–	–	3/4	5/8	1/2	5/8	5/16	7/8	7/8	28	17	27	17	42	40



NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. SIDE PLATE AND STIFFENER PLATE THICKNESS PER SCHEDULE BELOW.
3. BRACES MAY OCCUR ABOVE AND BELOW, ONLY ABOVE, OR ONLY BELOW. SEE BRACED FRAME ELEVATION FOR BRACE CONFIGURATION.
4. SEE S6-100 FOR DIMENSIONS L1 THRU L7
5. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
6. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
7. SEE 2/S6-103 FOR GUSSET PLATES OCCURING AT EDGE OF SLAB.



YIELD LINE @ BEAM



YIELD LINE @ COLUMN

BRACE FRAME CONNECTION SCHEDULE AND DETAIL (TYPE C)

3/4"=1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

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REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

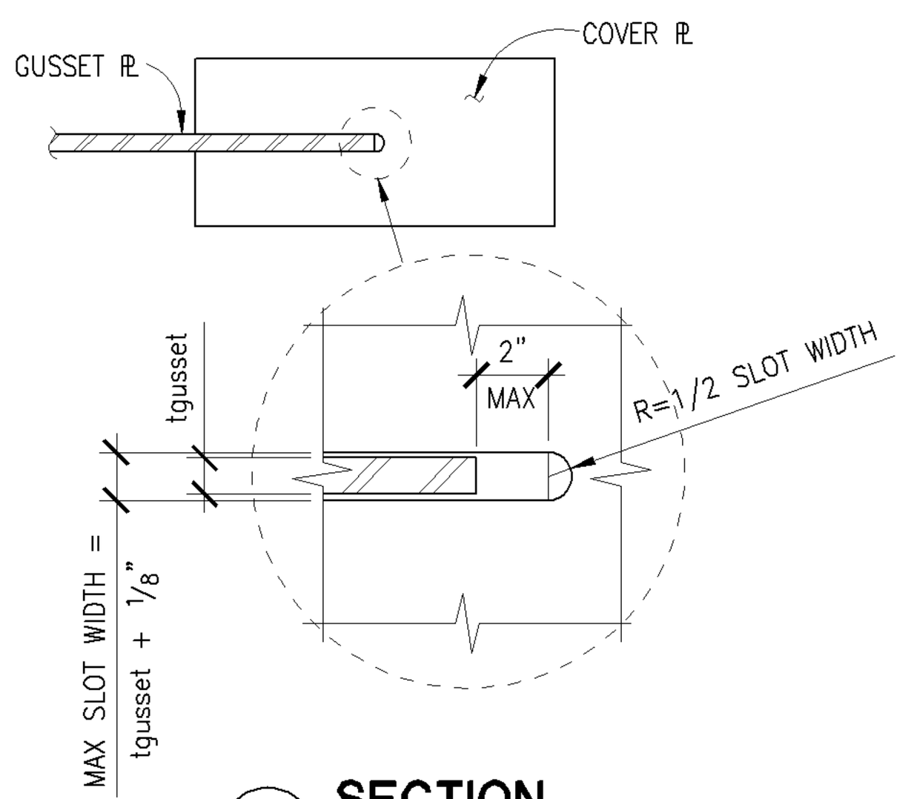
SHEET TITLE

BRACED FRAME DETAILS

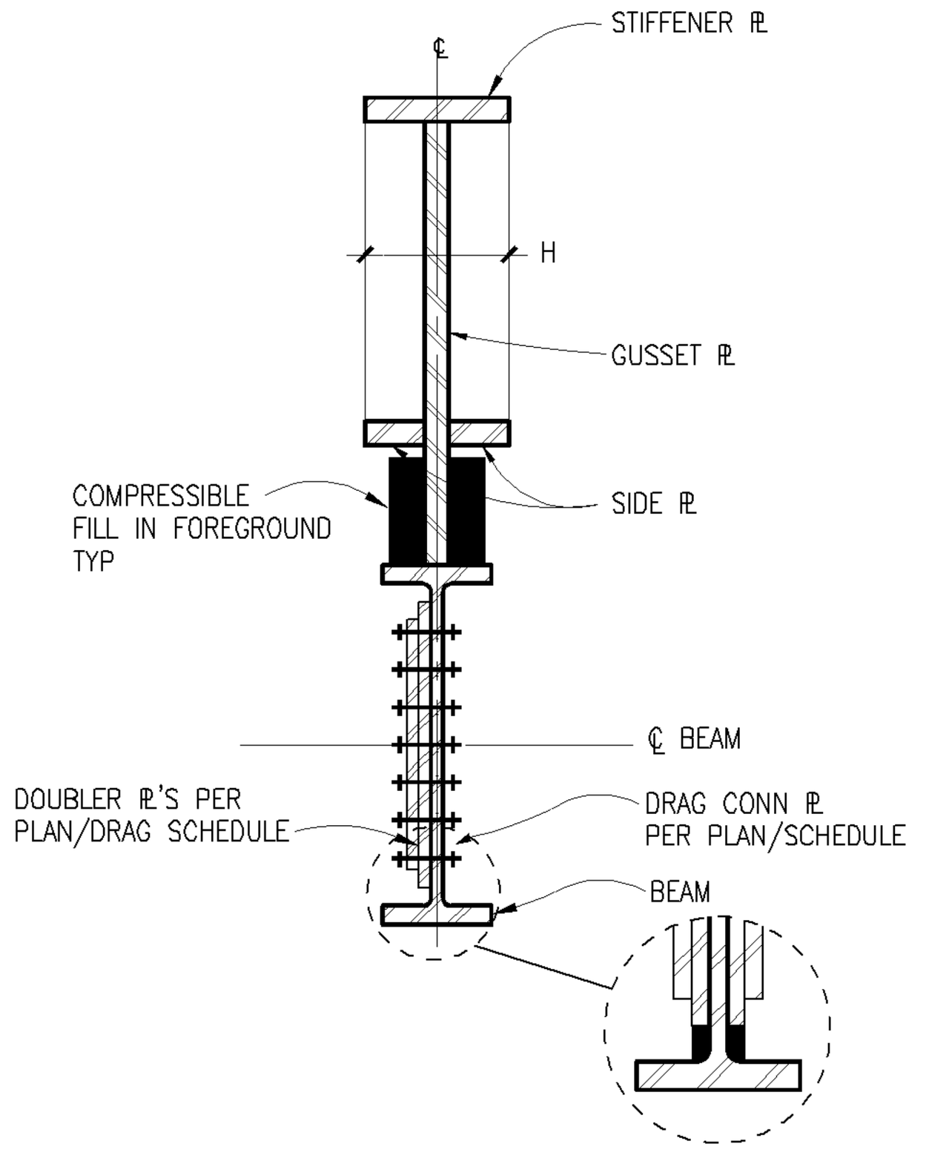
S6-102

CONNECTION SCHEDULE D

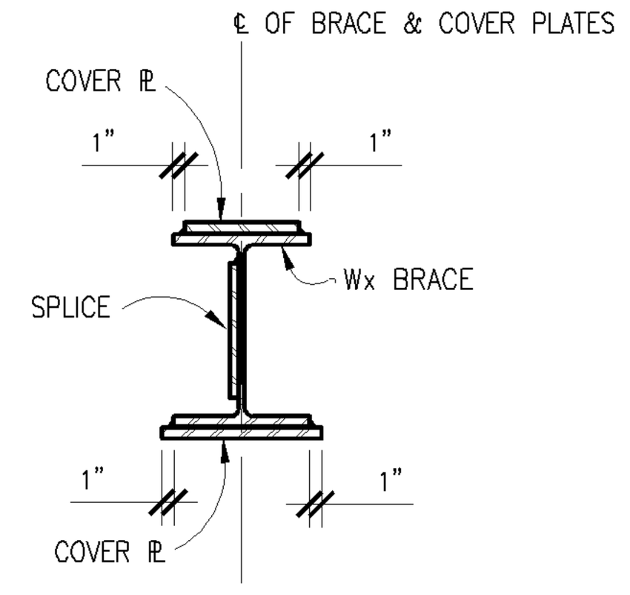
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)					
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
D1	W14x90	10	2	1	1/2	–	1"	5/8	1/4	3/8	1/4	5/16	1/2	1/2	18	17	17	17	63	33
D2	W14x109	–	1 3/4	1 1/2	3/4	–	–	3/4	3/8	3/8	3/8	5/16	3/4	3/4	20	17	23	17	55	32
D3	W14x132	–	2	1 1/2	1	–	–	3/4	5/8	1/2	5/8	5/16	7/8	3/4	28	17	26	17	59	42
D4	W14x132	–	1 3/4	1 3/4	3/4	–	–	3/4	1/2	1/2	1/2	5/16	7/8	3/4	30	17	28	17	56	41
D5	W14x132	–	2	1 1/2	1	–	–	3/4	5/8	1/2	5/8	5/16	7/8	3/4	28	17	24	17	57	41
D6	W14x132	–	2	1 1/2	1	–	–	3/4	5/8	1/2	5/8	5/16	7/8	3/4	28	17	24	17	57	41



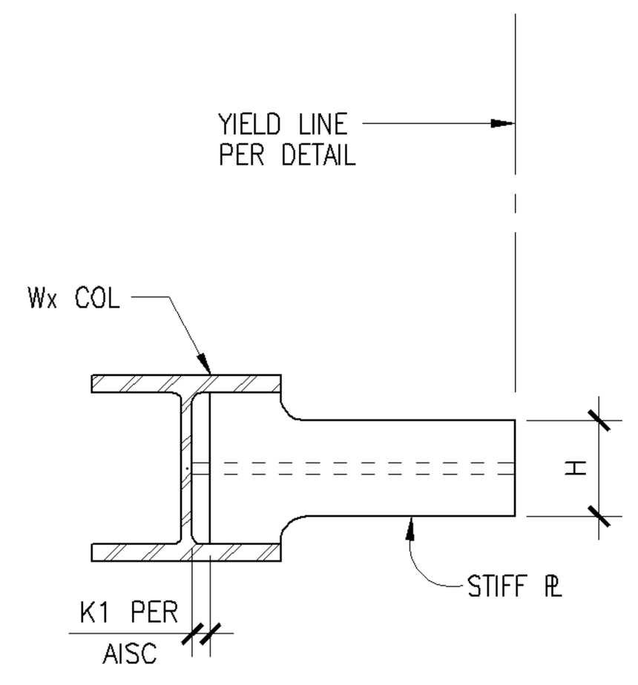
D SECTION



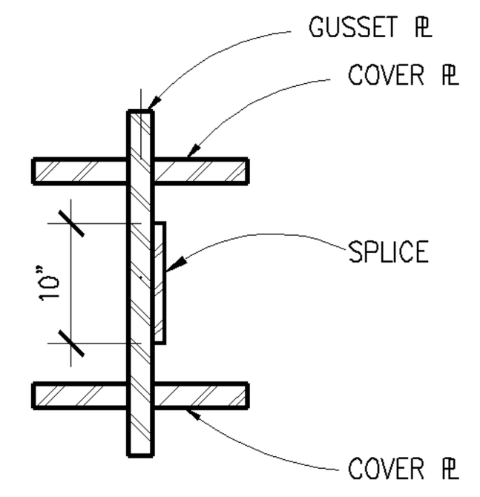
C SECTION



A SECTION

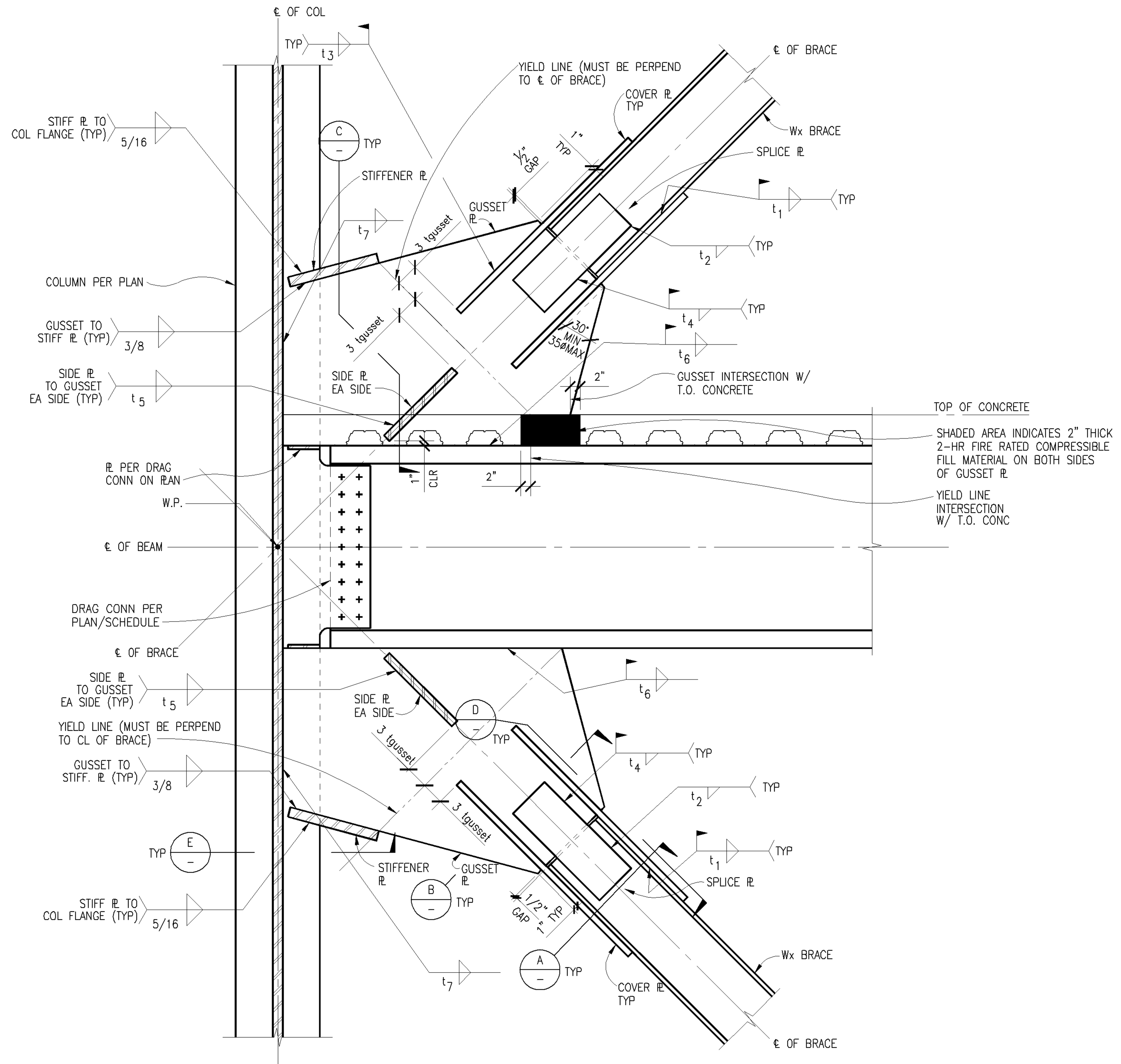


E SECTION

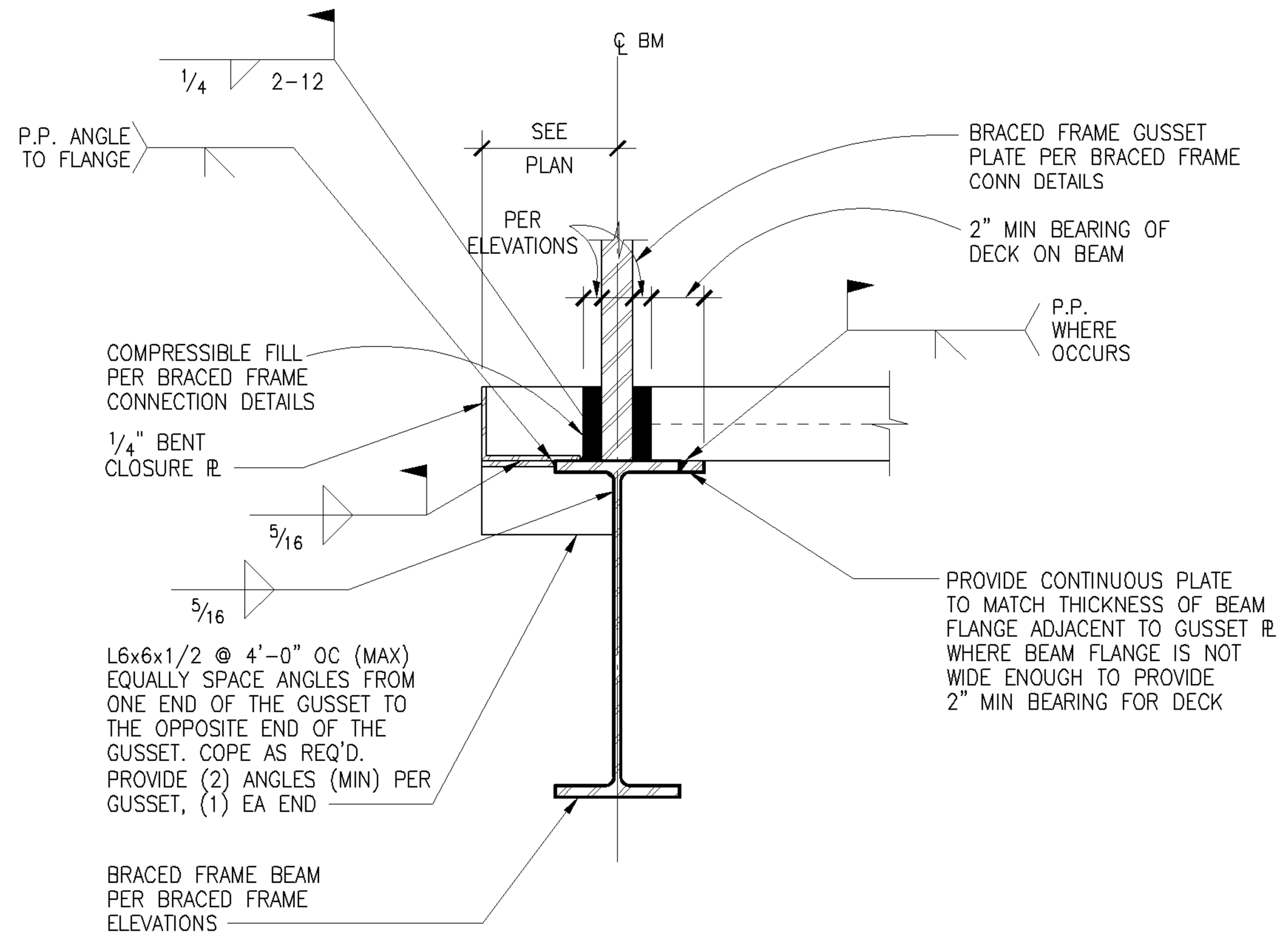


B SECTION

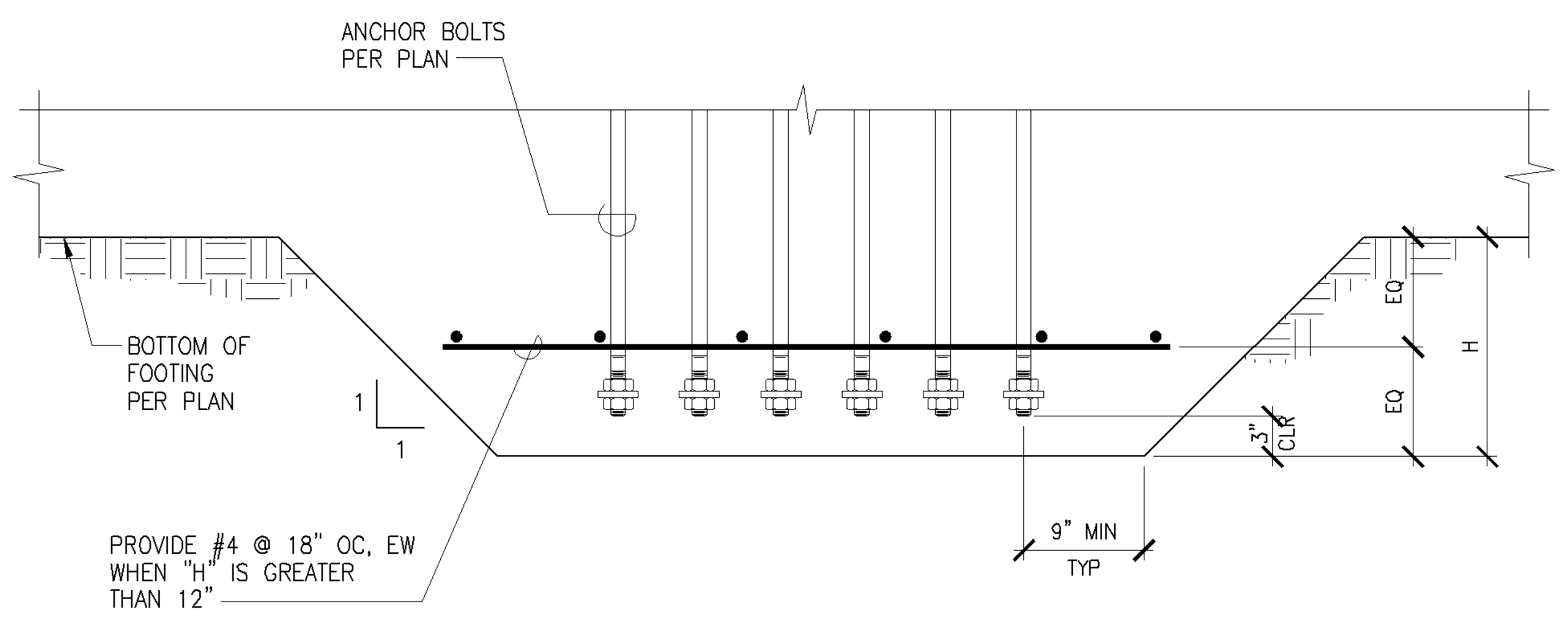
NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. SIDE PLATE AND STIFFENER PLATE THICKNESS PER SCHEDULE BELOW.
3. BRACES MAY OCCUR ABOVE AND BELOW, ONLY ABOVE, OR ONLY BELOW. SEE BRACED FRAME ELEVATION FOR BRACE CONFIGURATION.
4. SEE S6-100 FOR DIMENSIONS L1 THRU L7
5. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
6. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
7. SEE 2/– FOR GUSSET PLATES OCCURING AT EDGE OF SLAB.



YIELD LINE @ BEAM



EDGE OF SLAB DETAIL AT GUSSET PLATE | 1"=1'-0" | 2



NOTES:
WHEN ANCHOR BOLT EMBEDMENTS EXCEED THE FOOTING DEPTH, DEEPEN FOOTING AROUND THE ANCHOR BOLTS AS SHOWN.

DEEPENED FOOTING AT ANCHOR BOLTS | NTS | 3

BRACE FRAME CONNECTION SCHEDULE AND DETAIL (TYPE D) | 3/4"=1'-0" | 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
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2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
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CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

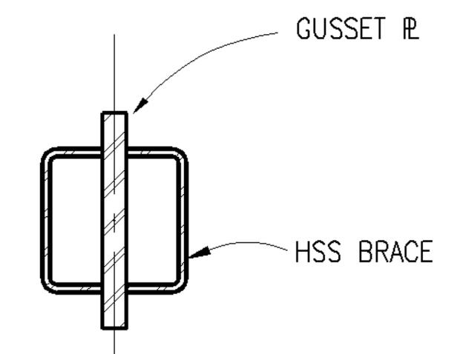
SHEET TITLE
BRACED FRAME DETAILS

S6-103

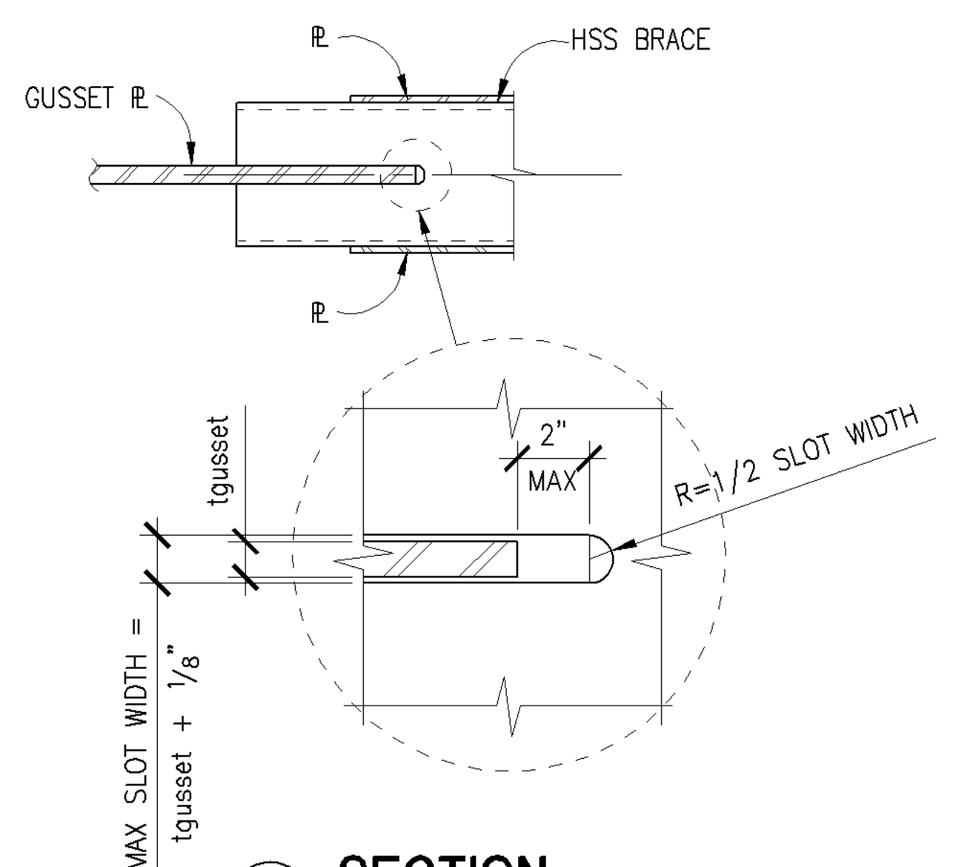
CONNECTION SCHEDULE E								
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)		WELD THICKNESS (IN)		PLATE INTERFACE LENGTH (IN)	
			GUSSET	SIDE	t1	t2	L1	L2
E1	HSS10x10x5/8	–	1 1/4	–	7/8	1/2	21	102
E2	HSS10x10x5/8	–	1 1/4	–	5/8	1/2	21	102
E3	HSS10x10x5/8	–	1	–	5/8	5/8	19	88
E4	HSS10x10x5/8	–	1	–	5/8	5/8	23	97
E5	HSS10x10x5/8	–	1	–	5/8	5/8	22	88
E6	HSS10x10x5/8	–	1 1/4	–	5/8	7/8	19	65
E7	HSS8x8x1/2	–	1	–	5/8	1/2	18	59
E8	HSS10x10x5/8	–	1 1/4	–	5/8	5/8	21	69
E9	HSS10x10x5/8	–	1 1/4	–	3/4	5/8	24	72
E10	HSS8x8x1/2	–	1	–	1/2	1/2	22	68
E11	HSS10x10x5/8	–	1	–	3/4	3/4	25	75
E12	HSS10x10x5/8	–	1 1/4	–	3/4	5/8	26	82
E13	HSS8x8x1/2	–	1	–	5/8	1/2	18	61
E14	HSS10x10x5/8	–	1 1/2	–	3/4	1/2	23	78
E15	HSS10x10x5/8	–	1 1/4	–	3/4	5/8	25	76
E16	HSS10x10x5/8	–	1 1/4	–	1/2	1/2	23	86
E17	HSS10x10x5/8	–	1 1/2	–	5/8	1/2	27	94
E18	HSS10x10x5/8	–	1 1/2	–	5/8	1/2	26	103
E19	HSS10x10x5/8	–	1 1/4	–	3/4	1/2	24	86
E20	HSS10x10x5/8	–	1	–	5/8	3/4	20	68
E21	HSS10x10x5/8	–	1 1/4	–	3/4	5/8	24	78

NOTES:

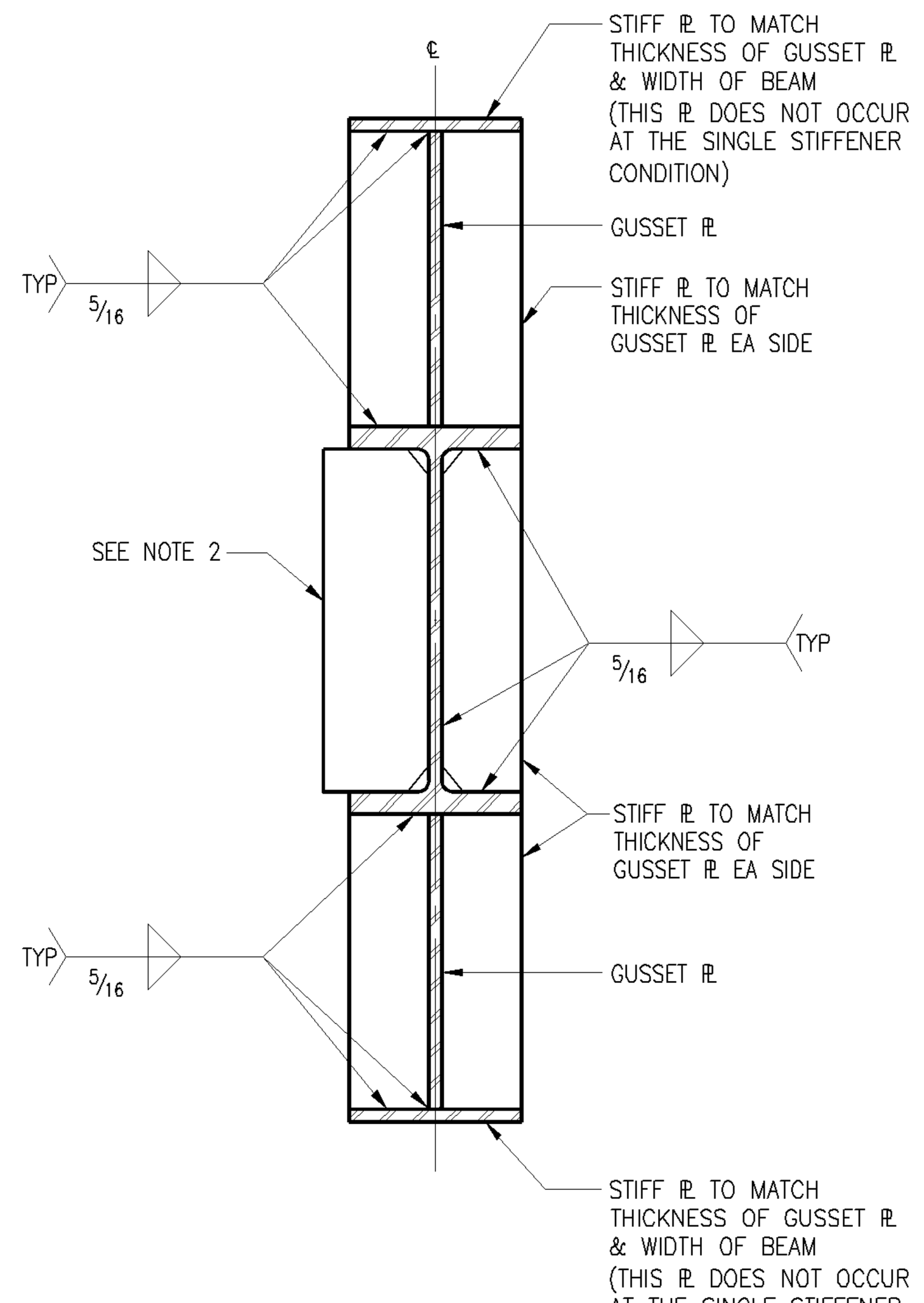
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50
2. SEE DETAIL 8/S5-101 FOR LATERAL BRACE AT CHEVRON BRACE INTERSECTION.
3. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
4. BRACES MAY OCCUR ABOVE AND BELOW, ONLY ABOVE, OR ONLY BELOW. SEE BRACED FRAME ELEVATION FOR BRACE CONFIGURATION.
5. SEE S6-100 FOR GRAPHICAL DEPICTION OF DIMENSIONS L1 THRU L2.
6. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
7. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
8. SEE 2/S6-103 FOR GUSSET PLATES OCCURING AT EDGE OF SLAB.
9. REINF ℙ DIMENSIONS:
   AT HSS10x10: B = 7", W = 16", T = 3/8"
   AT HSS8x8: B = 5", W = 12", T = 3/8"



A SECTION



C SECTION



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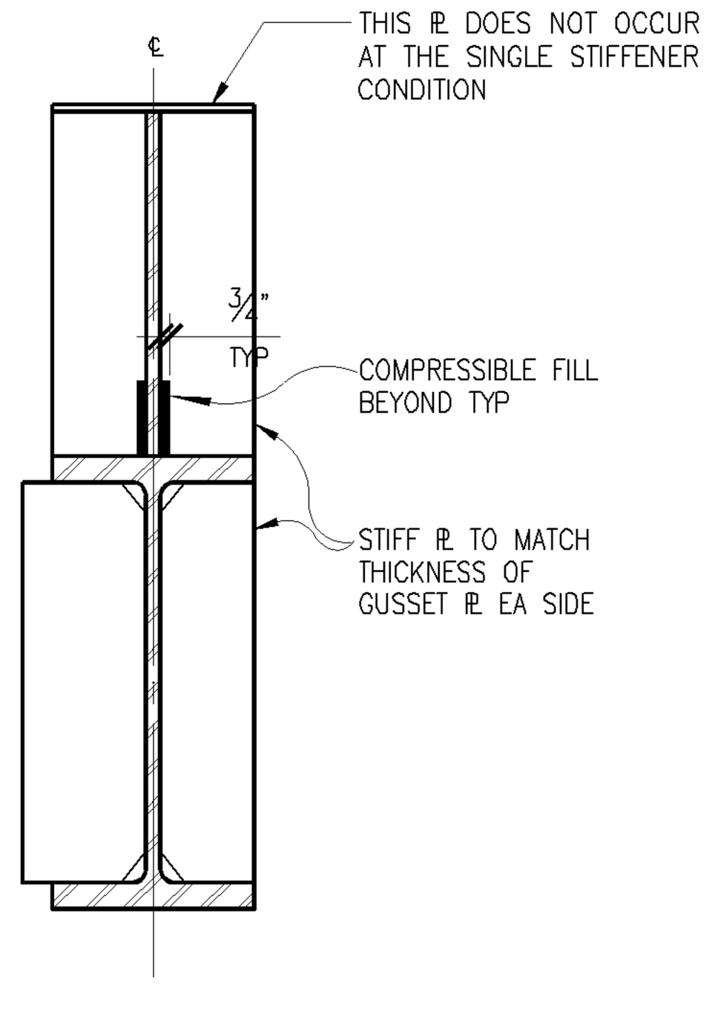
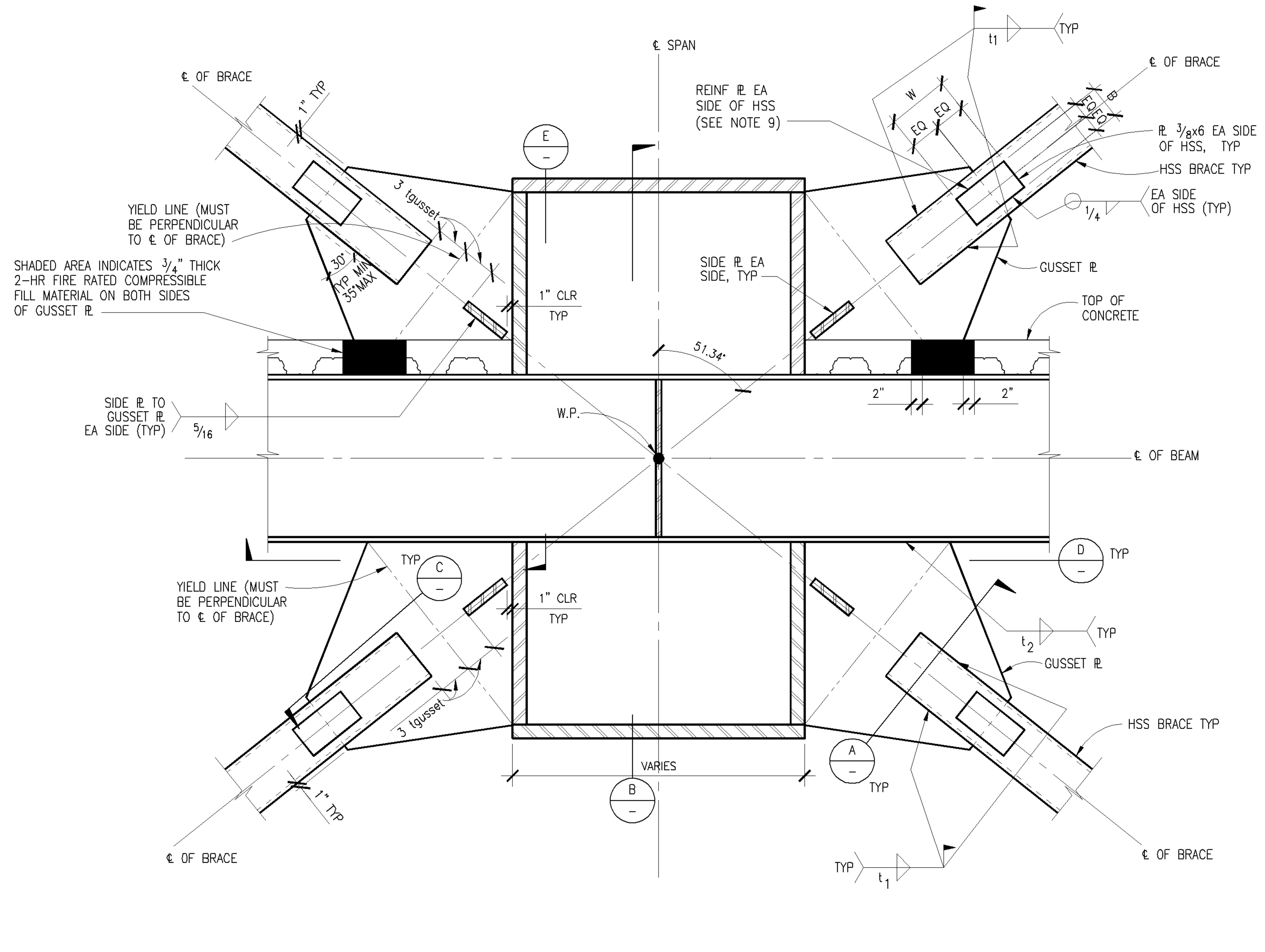
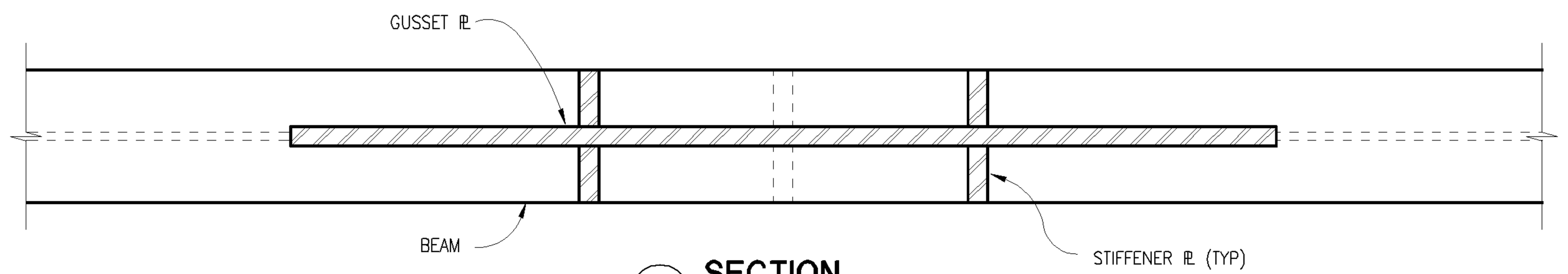


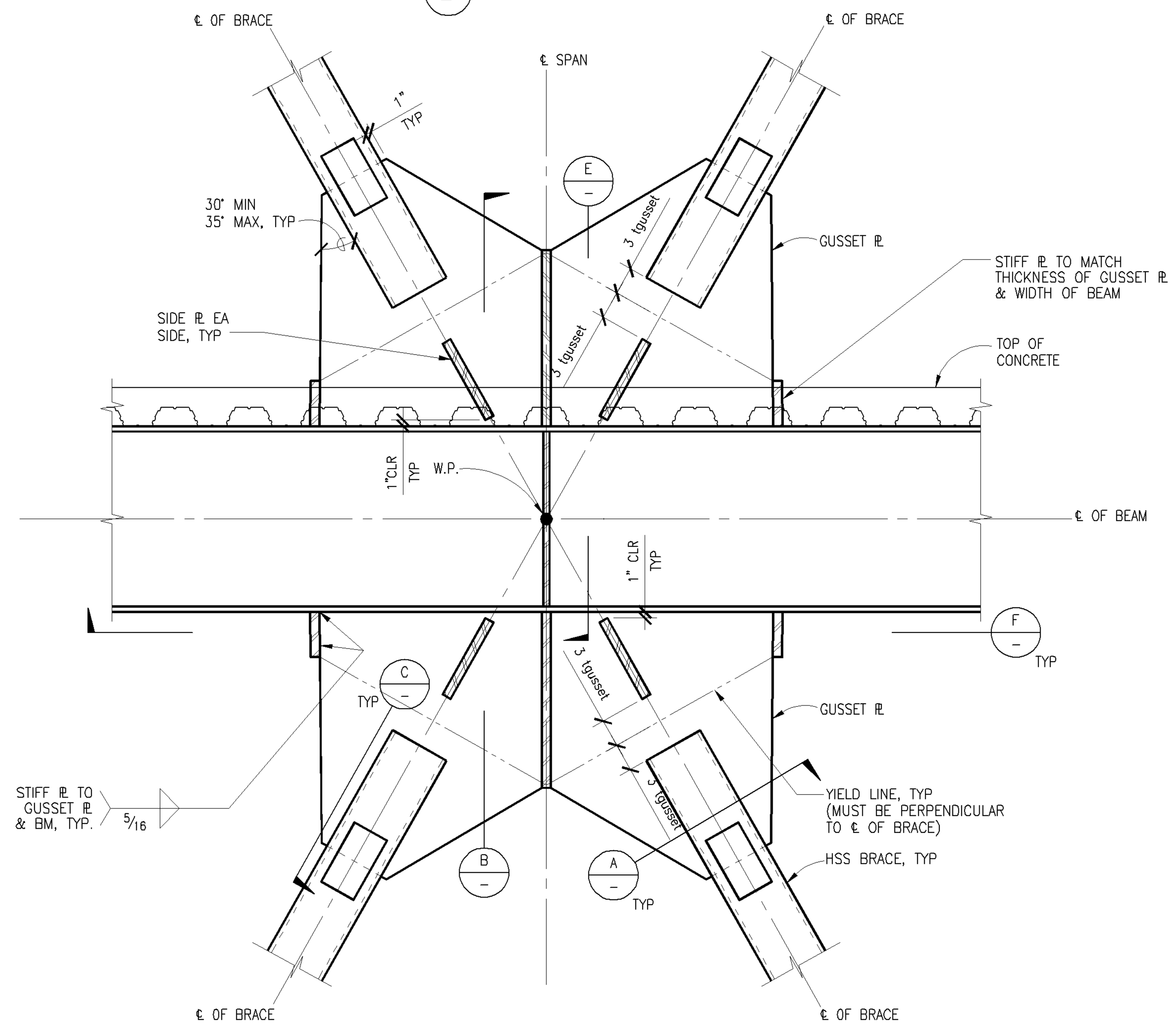
E SECTION



DOUBLE STIFFENER CONDITION

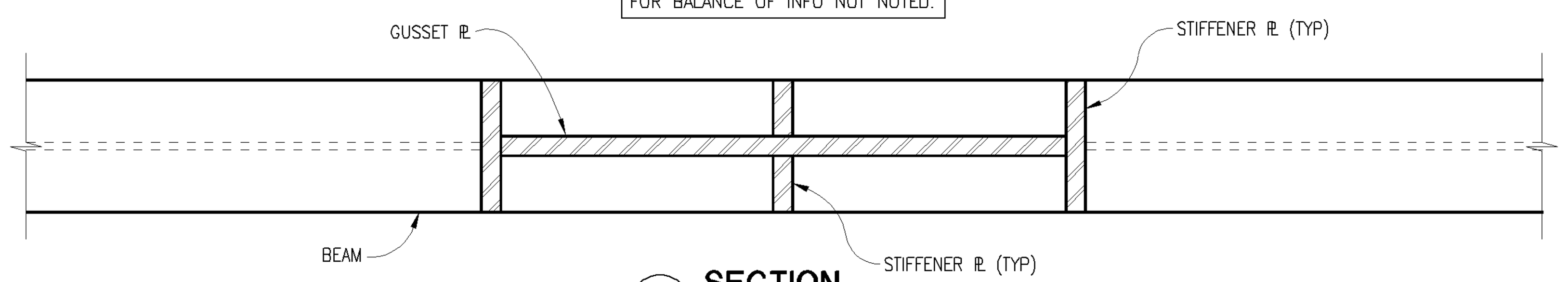


D SECTION



SINGLE STIFFENER CONDITION

NOTE:
SEE DOUBLE STIFFENER CONDITION FOR BALANCE OF INFO NOT NOTED.



F SECTION

BRACED FRAME CONNECTION SCHEDULE AND DETAIL (TYPE E)

3/4" = 1'-0" 1

OWNER/CLIENT

TEMECULA EAST CAMPUS
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Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
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REGISTRATION

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PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE
BRACED FRAME DETAILS

S6-104

CONNECTION SCHEDULE F

CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)				WELD THICKNESS (IN)					PLATE INTERFACE LENGTH (IN)				
			GUSSET	COVER	SPLICE	SIDE	t1	t2	t3	t4	t5	L1	L2	L3	L4	L5
F1	W14x90	–	1 1/2	1	3/4	–	5/8	1/2	1/2	1/2	1/2	23	17	17	17	107
F2	W14x109	10	1 1/4	1 1/4	5/8	1/2	3/4	3/8	5/8	3/8	3/8	25	17	17	17	138
F3	W14x145	–	1 3/4	1 3/4	3/4	–	3/4	1/2	3/4	1/2	5/8	33	17	18	17	113
F4	W14x176	–	2	2	3/4	–	3/4	1/2	3/4	1/2	3/4	39	17	22	17	116
F5	W14x90	–	1 1/2	1	1/2	–	5/8	3/8	1/2	3/8	1/2	22	17	17	17	110
F6	W14x109	10	1 1/4	1	5/8	1/2	3/4	3/8	1/2	3/8	3/8	20	17	17	17	143
F7	W14x145	–	1 3/4	1 1/4	1	–	3/4	5/8	5/8	5/8	5/8	25	17	17	17	114
F8	W14x176	–	2	1 1/2	1	–	3/4	5/8	3/4	5/8	3/4	31	17	17	17	115
F9	W14x90	–	1 1/2	1	1/2	–	5/8	1/4	3/8	1/4	3/8	18	17	17	17	126
F10	W14x109	–	1 1/2	1	1/2	–	3/4	3/8	1/2	3/8	5/8	20	17	17	17	100
F11	W14x120	–	1 1/2	1	1/2	–	3/4	3/8	1/2	3/8	5/8	21	17	17	17	102
F12	W14x90	–	1 1/4	1	3/4	–	5/8	1/2	1/2	1/2	1/2	23	17	17	17	90
F13	W14x109	–	1 3/4	1 1/4	1/2	–	3/4	3/8	5/8	3/8	1/2	23	17	17	17	118
F14	W14x145	–	1 3/4	1 1/2	1	–	3/4	5/8	3/4	5/8	7/8	30	17	17	17	95
F15	W14x176	–	1 3/4	1 3/4	7/8	–	3/4	5/8	3/4	5/8	7/8	35	17	22	17	97
F16	W14x90	–	1 1/2	1	1/2	–	5/8	3/8	1/2	3/8	5/8	21	17	17	17	73
F17	W14x109	–	1 3/4	1 1/4	1/2	–	3/4	3/8	5/8	3/8	3/4	23	17	17	17	88
F18	W14x145	–	1 3/4	1 1/2	3/4	–	3/4	1/2	5/8	1/2	7/8	27	17	20	17	84
F19	W14x176	–	2	1 1/2	1 1/4	–	3/4	3/4	5/8	3/4	7/8	31	17	22	17	93
F20	W14x132	–	1 1/2	1 1/4	5/8	–	3/4	3/8	1/2	3/8	3/4	23	17	18	17	75
F21	W14x132	–	2	1 1/2	3/4	–	3/4	1/2	3/4	1/2	5/8	28	17	17	17	84
F22	W14x132	–	1 1/2	1 1/2	5/8	–	3/4	3/8	3/8	3/8	3/4	23	17	25	17	101
F23	W14x132	–	1 1/2	1 1/2	1	–	3/4	5/8	1/2	5/8	7/8	28	17	22	17	88
F24	W14x132	–	1 3/4	1 1/2	3/4	–	3/4	1/2	3/4	1/2	3/4	28	17	17	17	77
F25	W14x132	–	2	1 1/2	3/4	–	3/4	1/2	3/4	1/2	5/8	28	17	17	17	88
F26	W14x132	–	2	1 1/2	1	–	3/4	5/8	3/4	5/8	5/8	28	17	17	17	102
F27	W14x132	–	1 3/4	1 1/2	1	–	3/4	5/8	3/4	5/8	5/8	28	17	17	17	84
F28	W14x132	–	1 3/4	1 1/2	1	–	3/4	5/8	3/4	5/8	5/8	28	17	17	17	91
F29	W14x132	–	2	1 1/2	1	–	3/4	5/8	3/4	5/8	5/8	28	17	17	17	102
F30	W14x132	–	2	1 1/2	1	–	3/4	5/8	3/4	5/8	5/8	28	17	17	17	102
F31	W14x132	–	1 3/4	1 1/2	1	–	3/4	5/8	3/4	5/8	5/8	28	17	17	17	87
F32	W14x132	–	1 3/4	1 1/2	1	–	3/4	5/8	3/4	5/8	3/4	28	17	17	17	86
F33	W14x132	–	1 3/4	1 1/2	1	–	3/4	5/8	3/4	5/8	3/4	28	17	17	17	86
F34	W14x132	–	1 3/4	1 1/2	1	–	3/4	5/8	3/4	5/8	3/4	28	17	17	17	77
F35	W14x132	–	1 1/2	1 1/2	1	–	3/4	5/8	1/2	5/8	7/8	28	17	22	17	87
F36	W14x132	–	1 3/4	1 1/2	1	–	3/4	5/8	3/4	5/8	5/8	28	17	17	17	84
F37	W14x132	–	1 3/4	1 1/2	1	–	3/4	5/8	3/4	5/8	3/4	28	17	17	17	74
F38	W14x176	–	1 3/4	1 3/4	7/8	–	3/4	5/8	3/4	5/8	7/8	35	17	20	17	97
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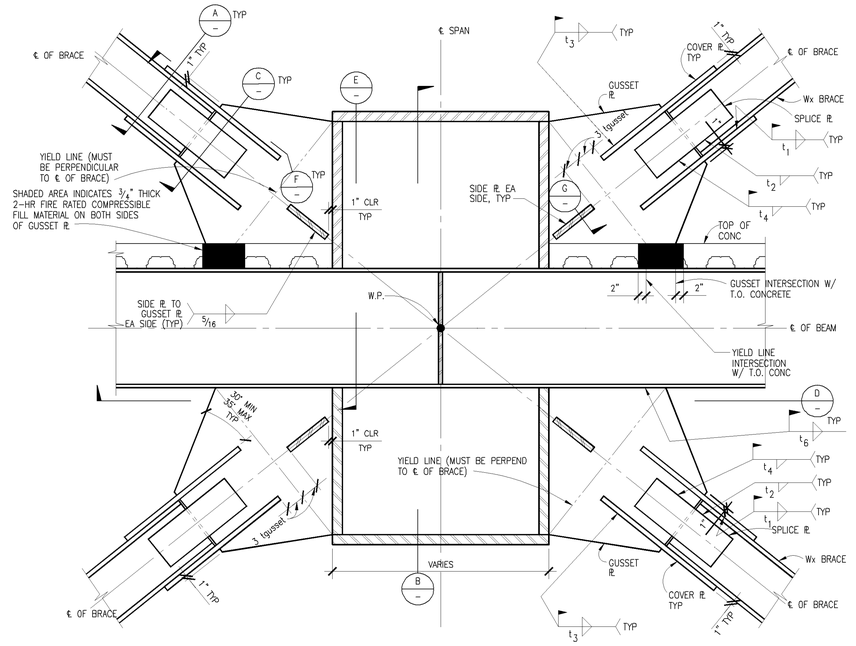
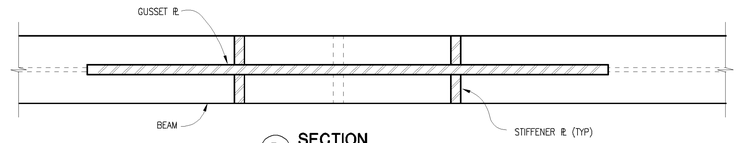
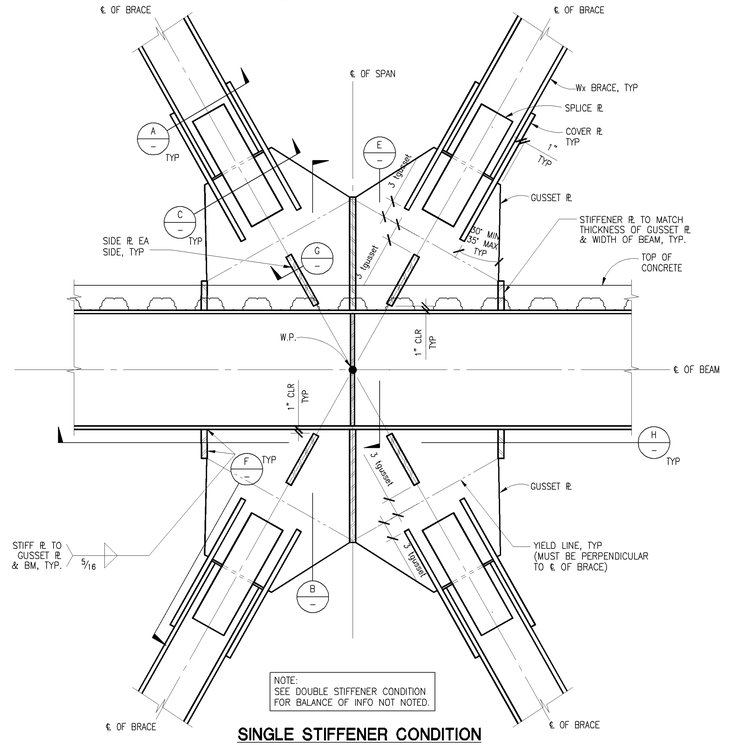


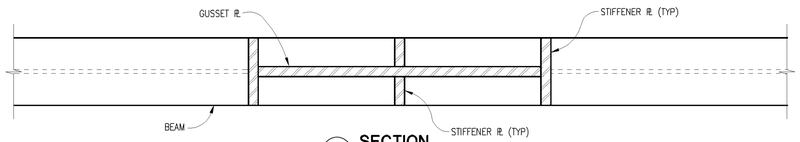
DOUBLE STIFFENER CONDITION



D SECTION



SINGLE STIFFENER CONDITION



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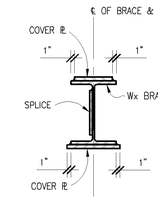
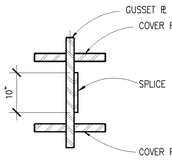
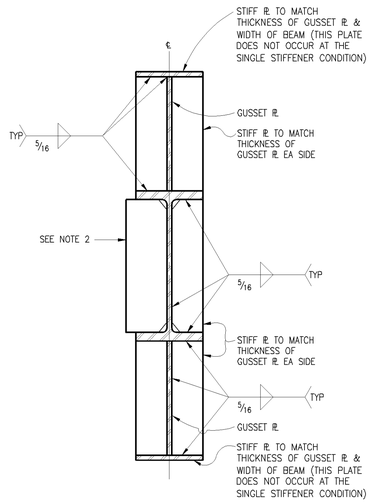


A SECTION

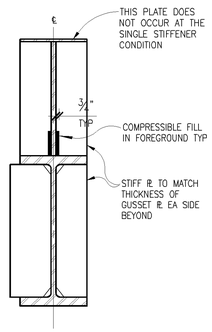


C SECTION

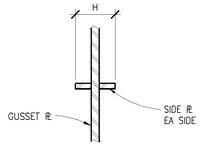
NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. SEE DETAIL 8/S5-101 FOR LATERAL BRACE AT CHEVRON BRACE INTERSECTION.
3. GUSSET PLATE AND SIDE PLATE THICKNESS PER SCHEDULE BELOW.
4. BRACES MAY OCCUR ABOVE AND BELOW, ONLY ABOVE, OR ONLY BELOW. SEE BRACED FRAME ELEVATION FOR BRACE CONFIGURATION.
5. SEE S6-100 FOR GRAPHICAL DEPICTION OF DIMENSIONS L1 THRU L6
6. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
7. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
8. SEE 2/S6-103 FOR GUSSET PLATES OCCURING AT EDGE OF SLAB.



B SECTION



E SECTION



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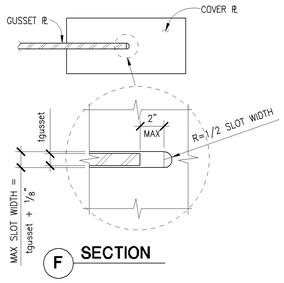

F SECTION

BRACED FRAME CONNECTION SCHEDULE AND DETAIL (TYPE F)

3/4" = 1'-0"

1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
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T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3774
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER ADRIN V. DIVE No. S-4399 Exp. 12/31/07 STRUCTURAL STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

BRACED FRAME DETAILS

S6-105

File: W:\2005\106103 - Guidant Corporate Campus, Temecula, CA\Sdraft\106103_S6_105.dwg
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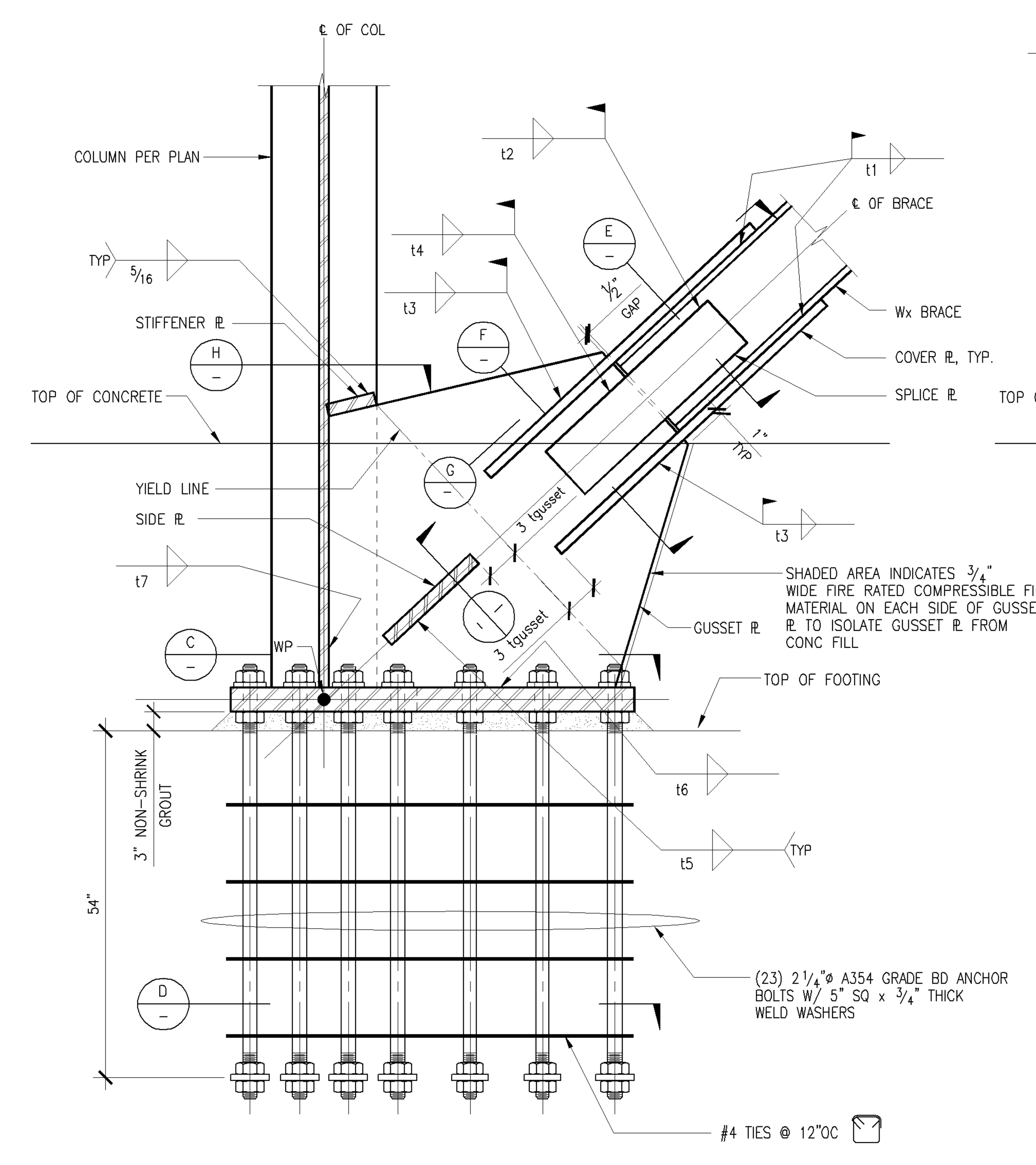
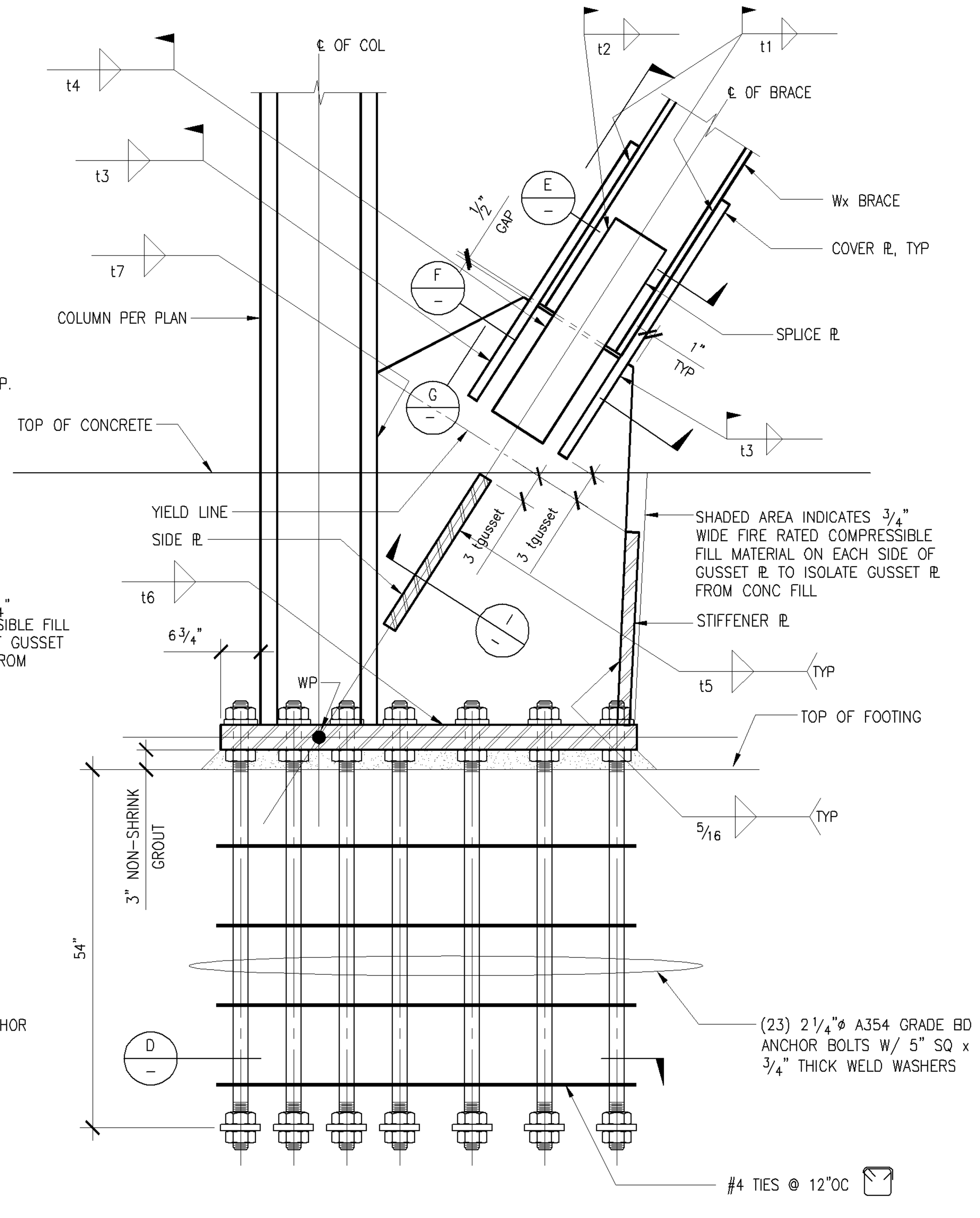
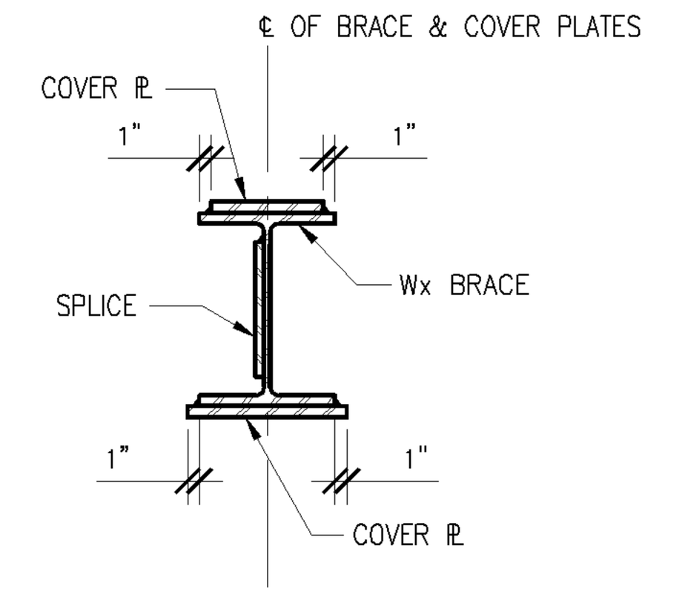


B ELEVATION



A ELEVATION



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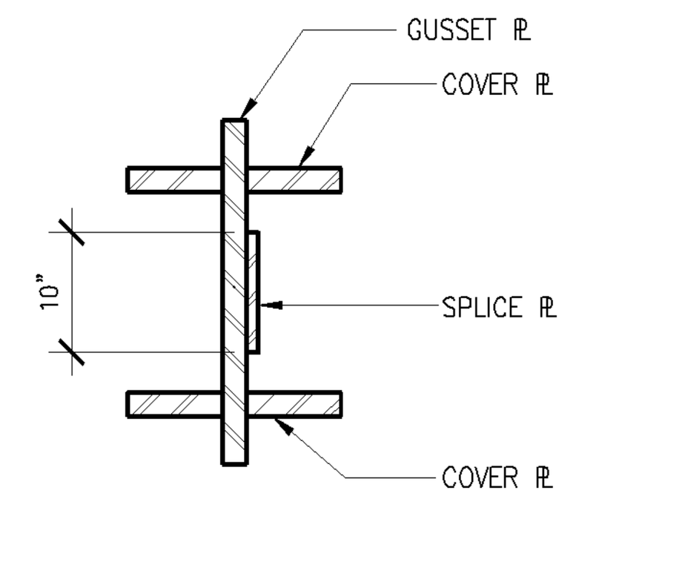
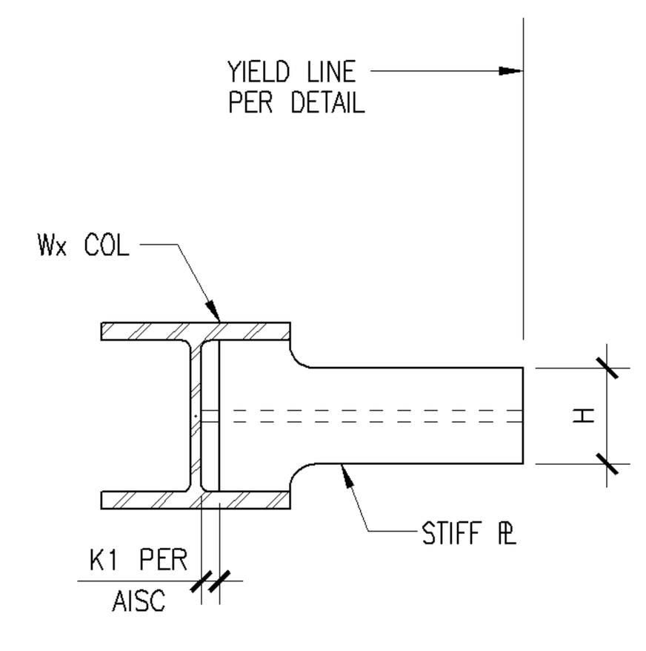
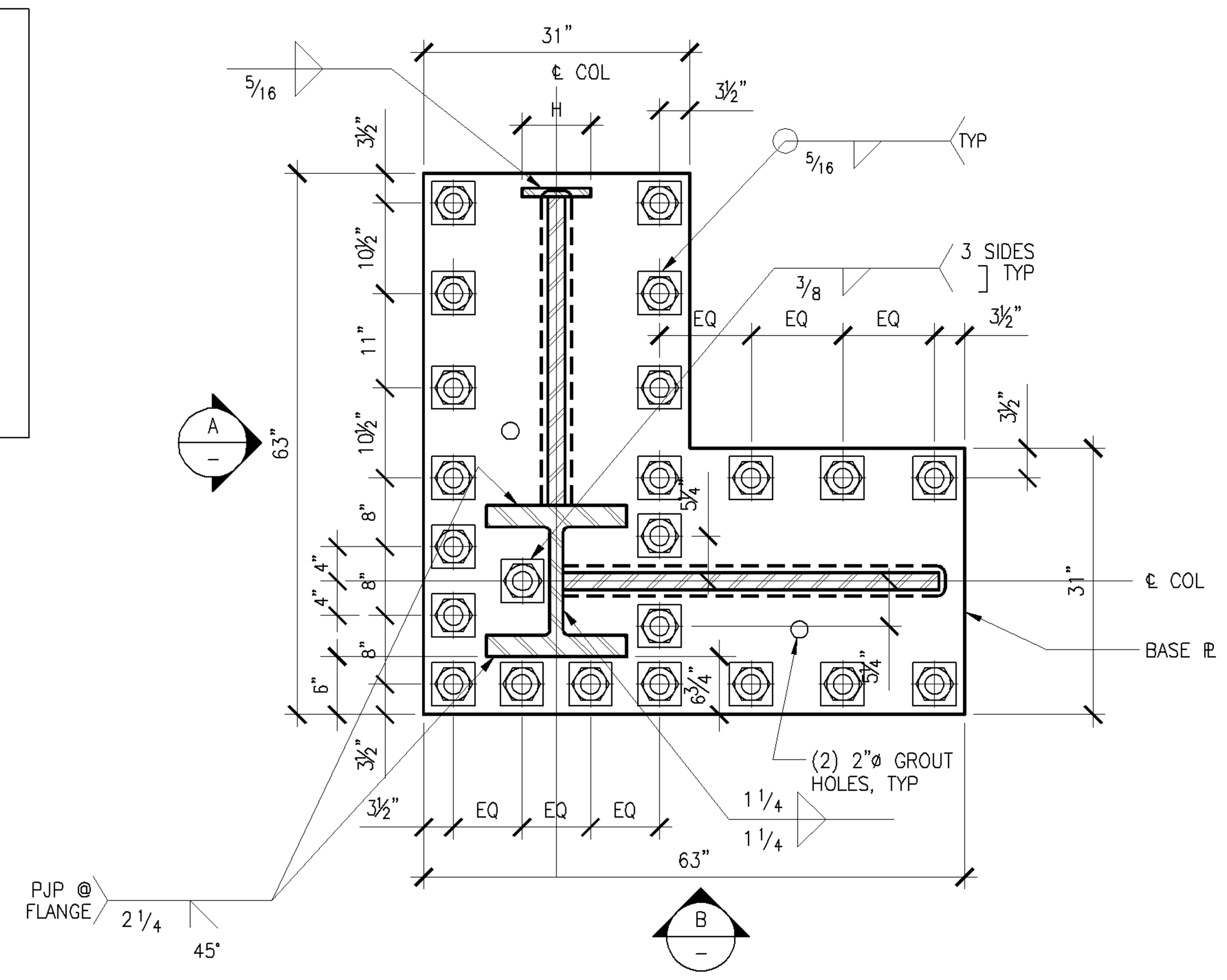
NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.



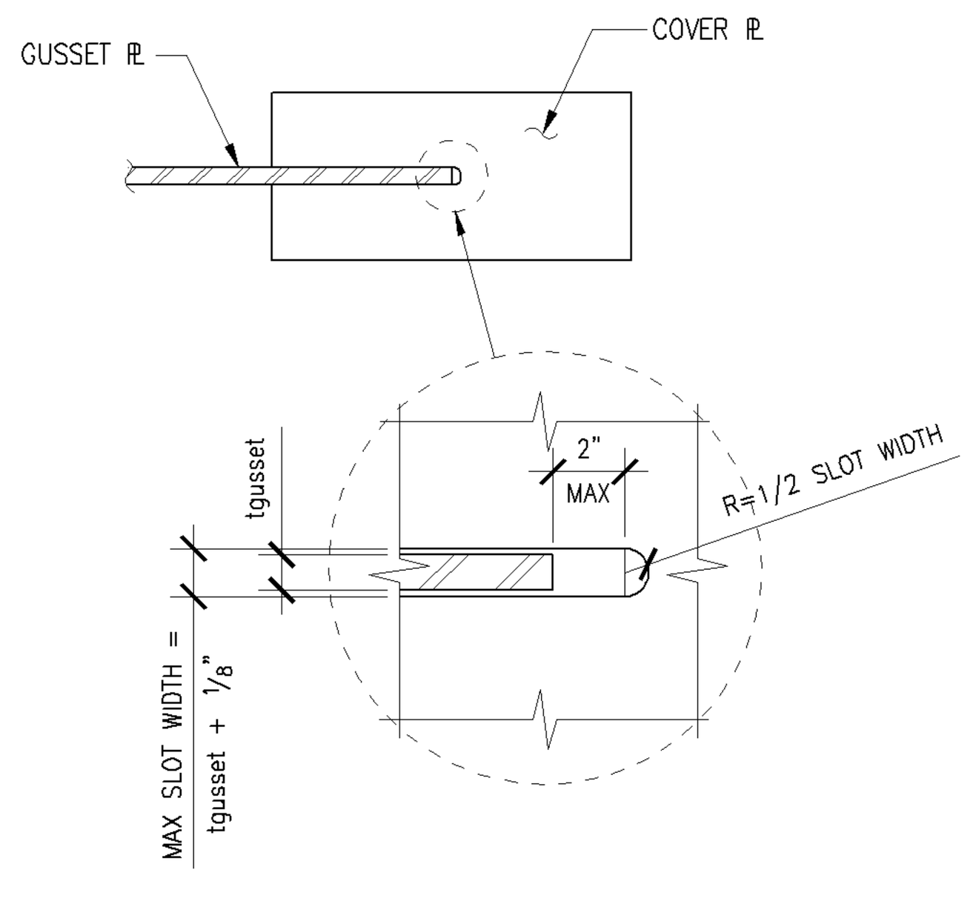
F SECTION



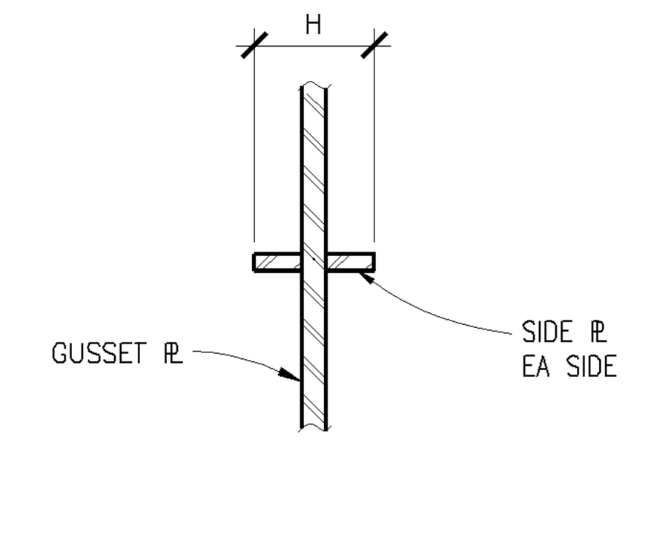
H SECTION



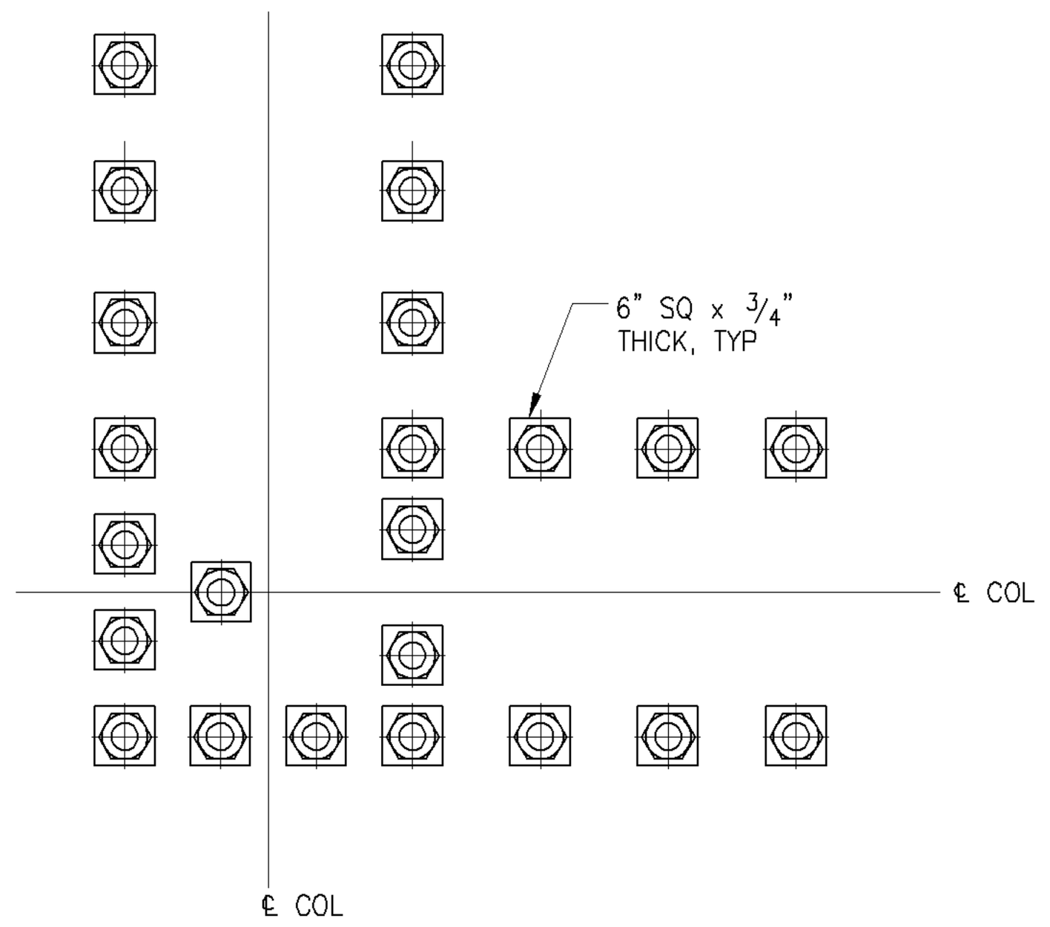
C PLAN - BASE PLATE



G SECTION



I SECTION

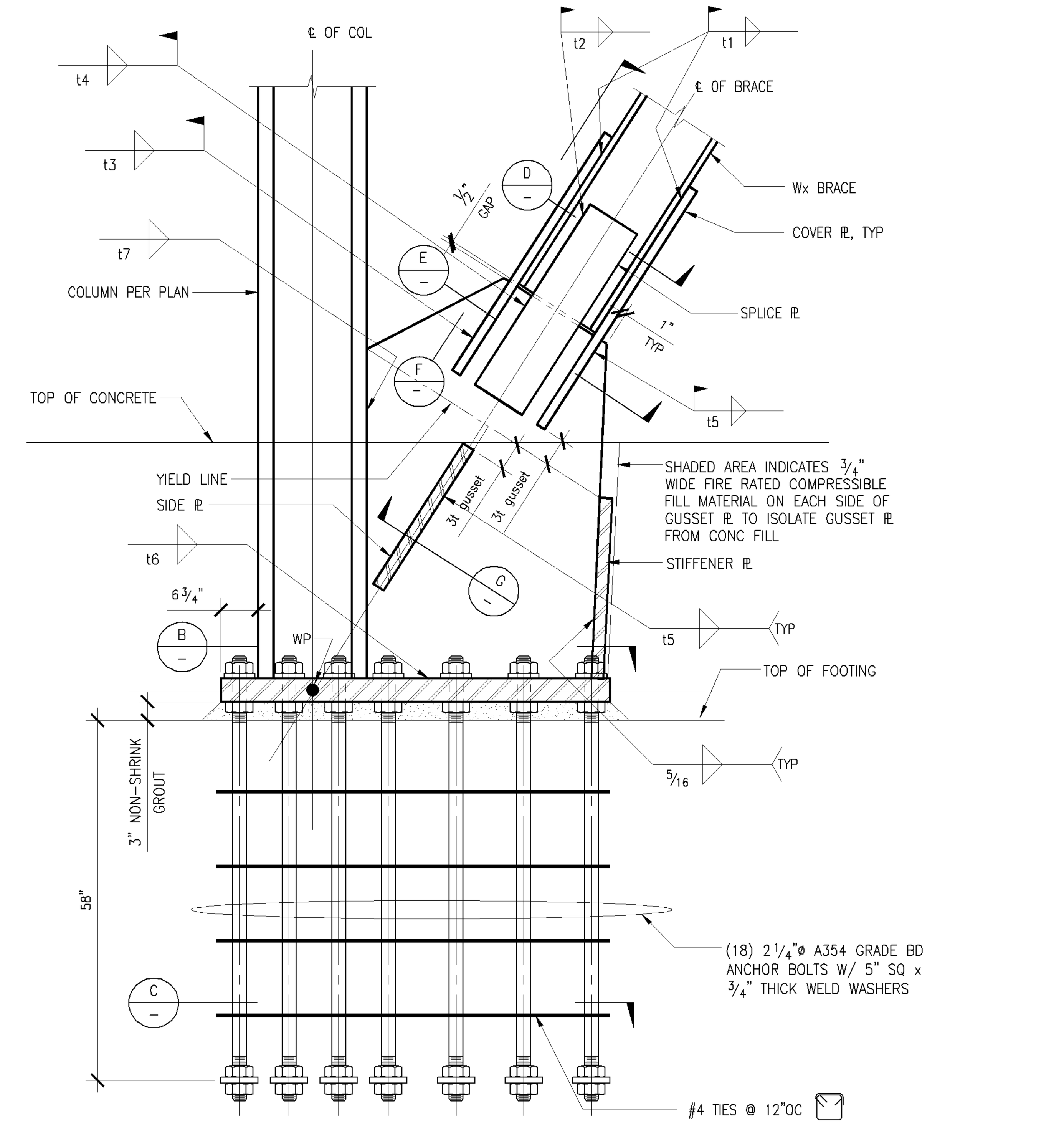


D PLAN - BEARING PLATES

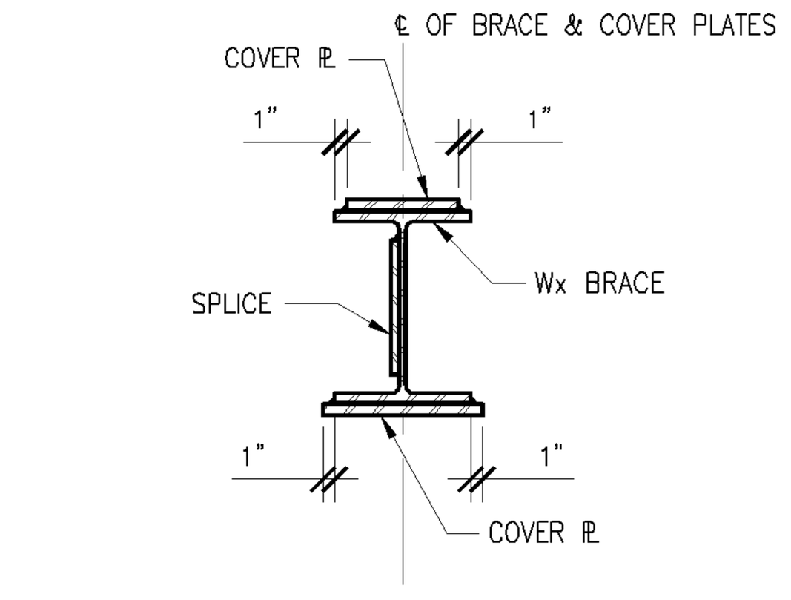
CONNECTION SCHEDULE H																					
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)					
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
H1(ELV A)	W14x132	–	1 1/2	1 1/2	1	1/2	–	3/4	5/8	3/4	5/8	5/16	1/2	7/8	28	17	17	17	35	50
H1(ELV B)	W14x132	–	1 3/4	1 1/2	3/4	–	–	3/4	1/2	1/2	1/2	5/16	7/8	7/8	30	17	26	17	45	42
H2(ELV A)	W14x132	10	1 1/2	1 1/2	1	1/2	1/2	3/4	5/8	3/4	5/8	5/16	1/2	7/8	28	17	17	17	36	53
H2(ELV B)	W14x132	–	1 3/4	1 1/2	3/4	–	–	3/4	1/2	1/2	1/2	5/16	3/4	3/4	30	17	26	17	45	44

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE H) 3/4" = 1'-0" 2

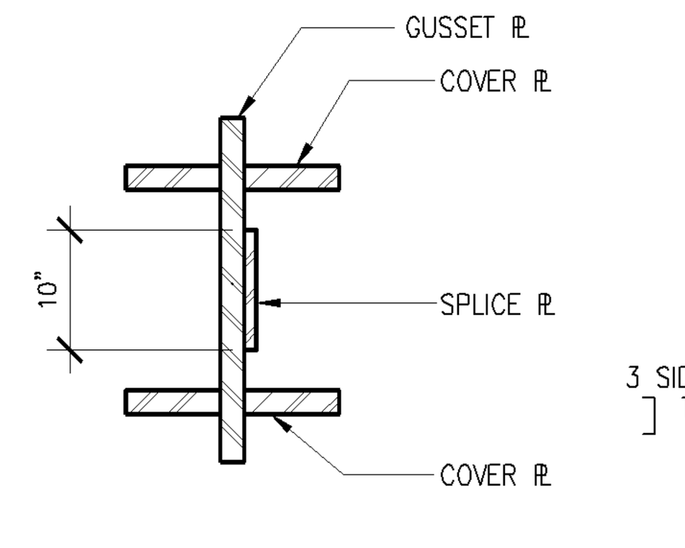
NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.



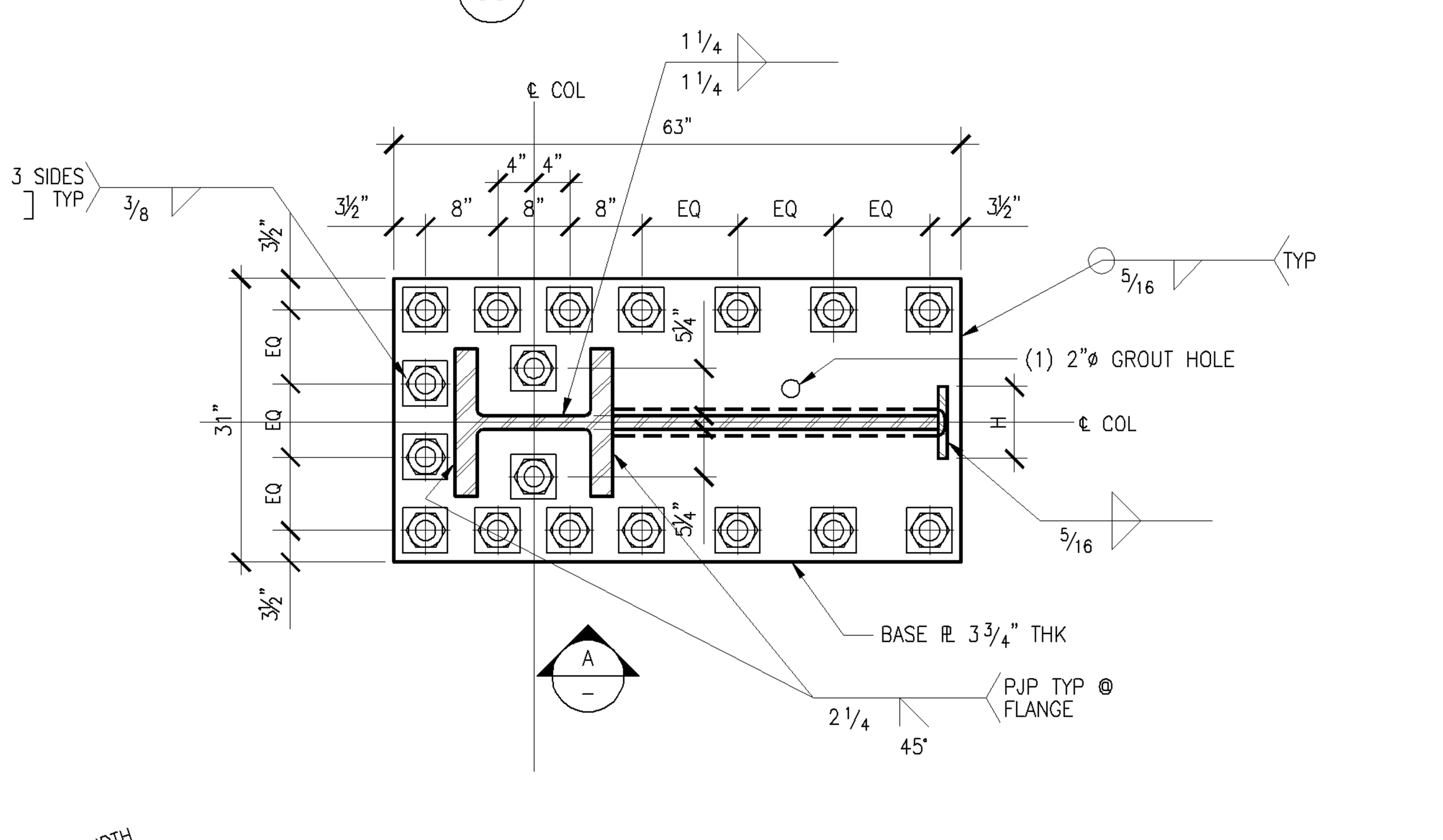
A ELEVATION



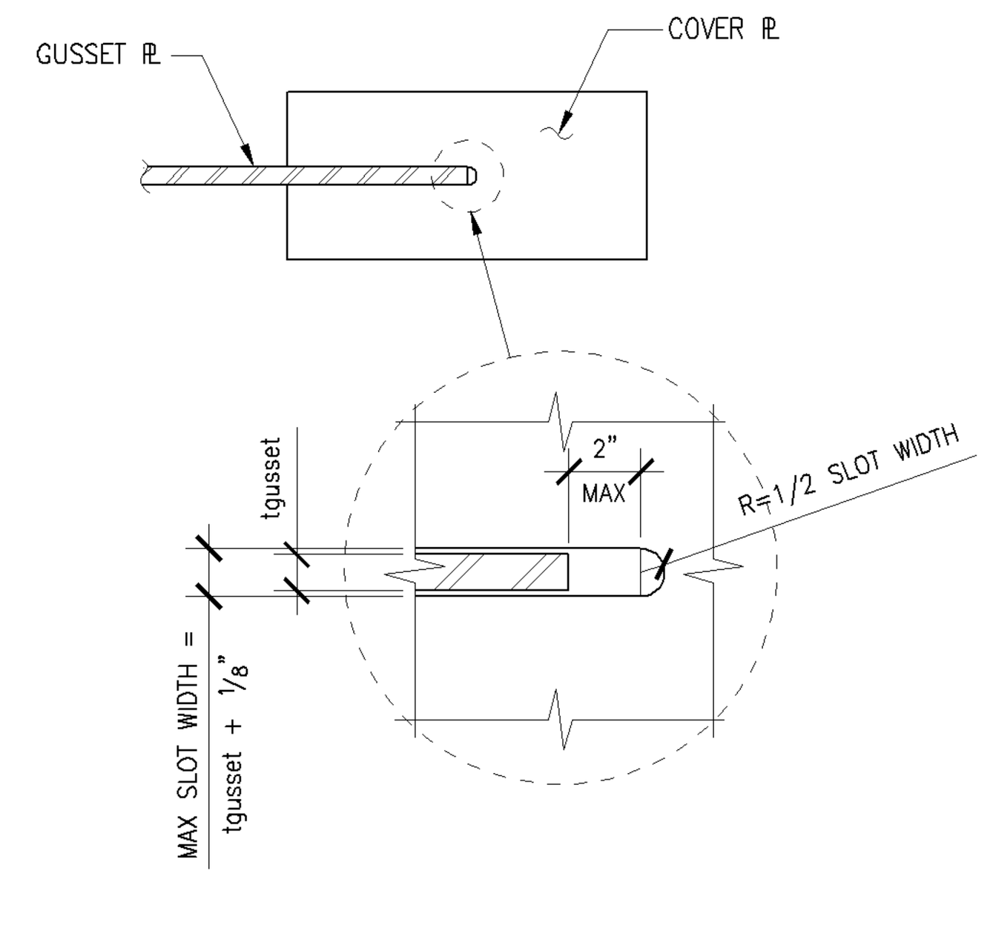
D SECTION



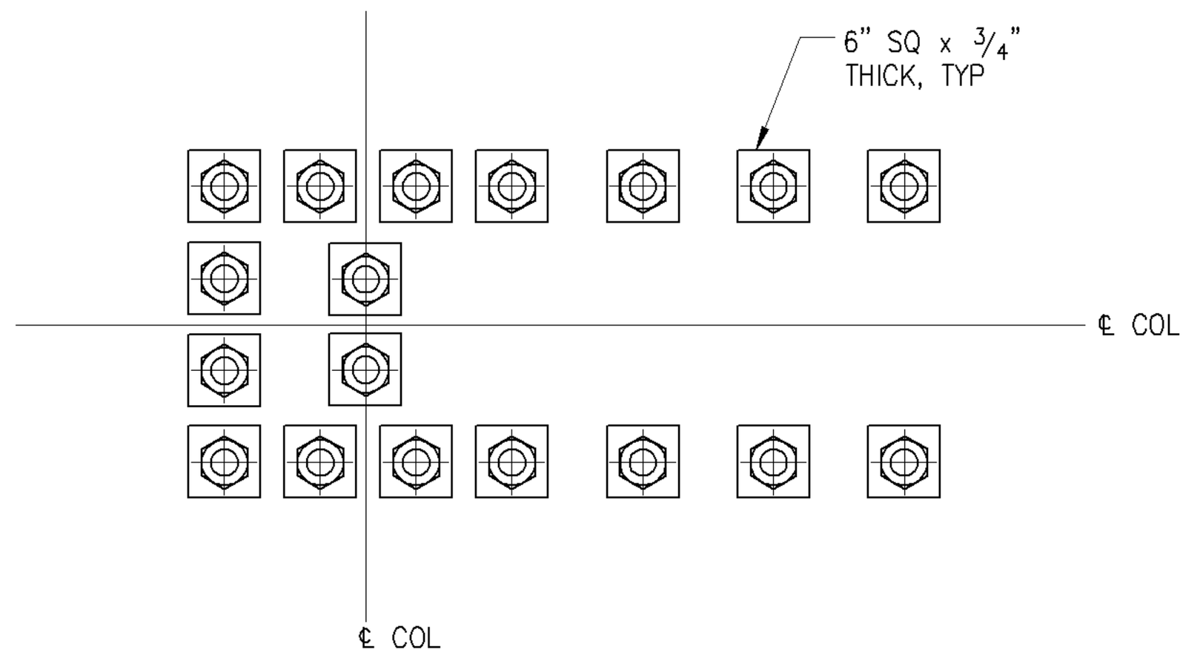
E SECTION



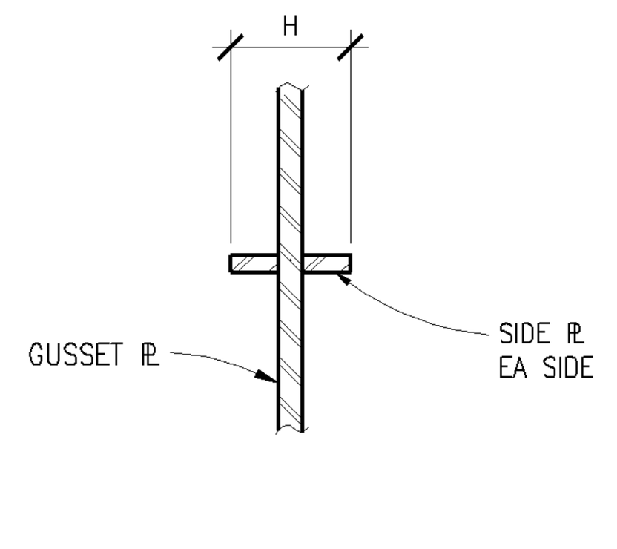
B PLAN - BASE PLATE



F SECTION



C PLAN - BEARING PLATES



G SECTION

CONNECTION SCHEDULE G																					
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)					
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
G1	W14x132	10	1 1/2	1 1/2	1	1/2	–	3/4	5/8	3/4	5/8	5/16	1/2	7/8	28	17	17	17	35	50
G2	W14x132	10	1 1/2	1 1/2	1	1/2	1/2	3/4	5/8	3/4	5/8	5/16	1/2	7/8	28	17	17	17	36	53

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE G) 3/4" = 1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

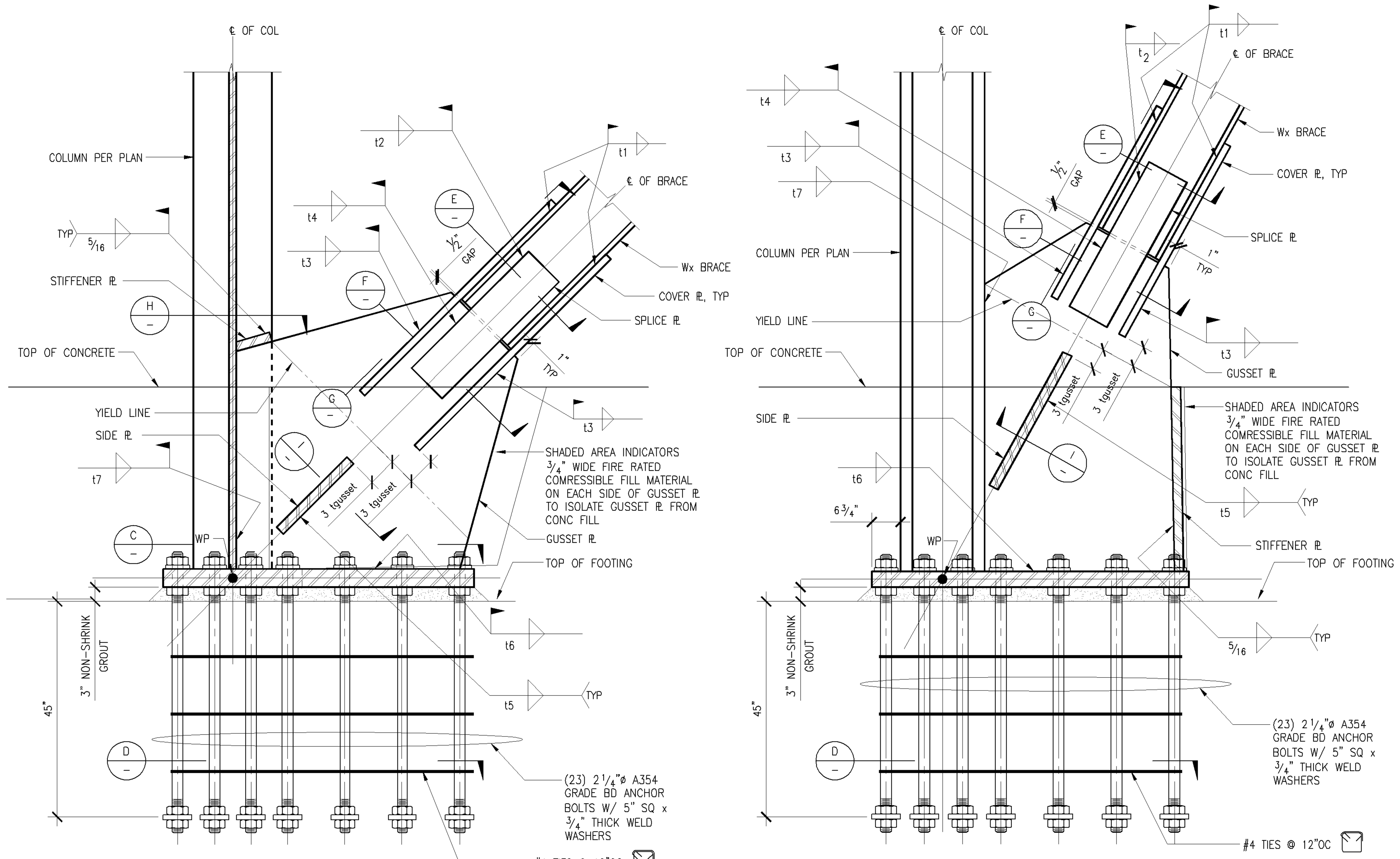
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

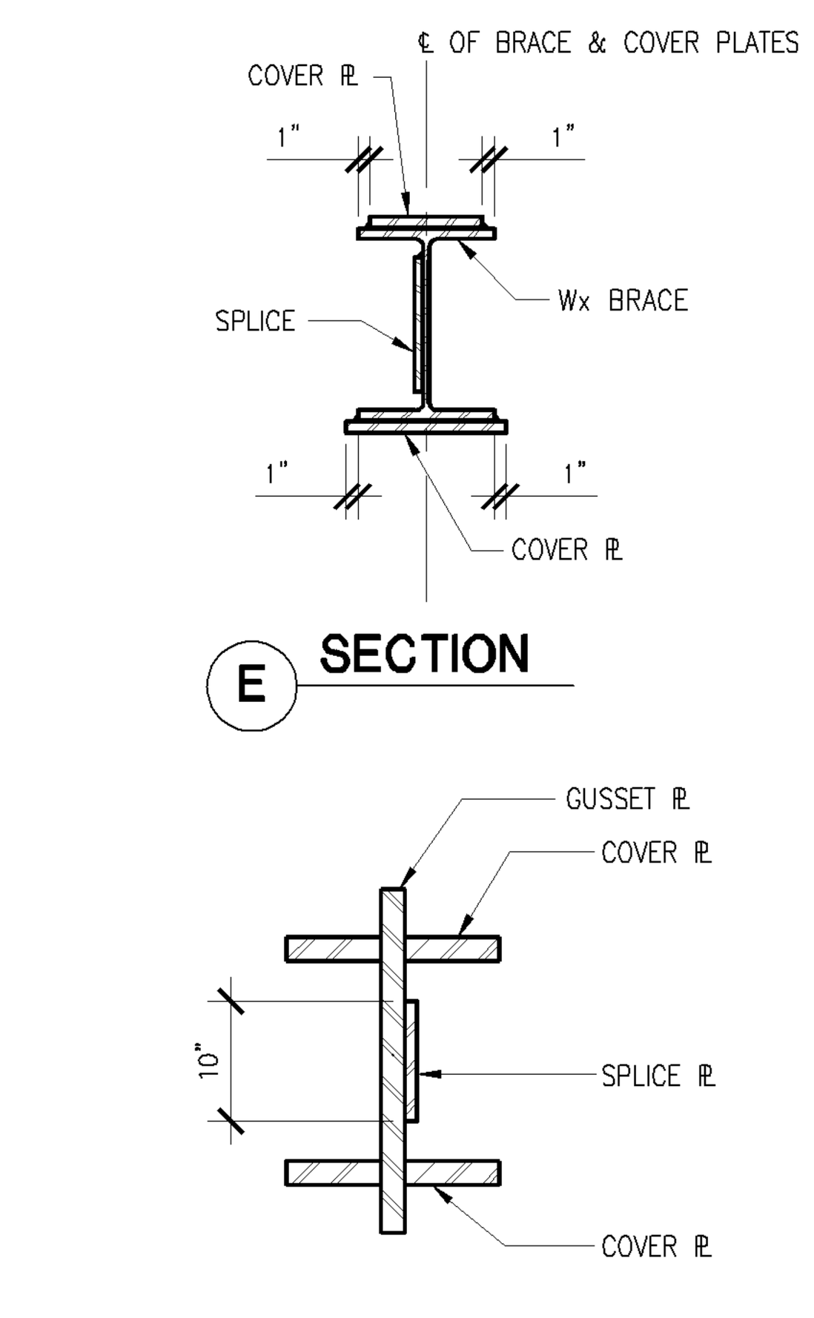
BRACED FRAME DETAILS

S6-106



B ELEVATION

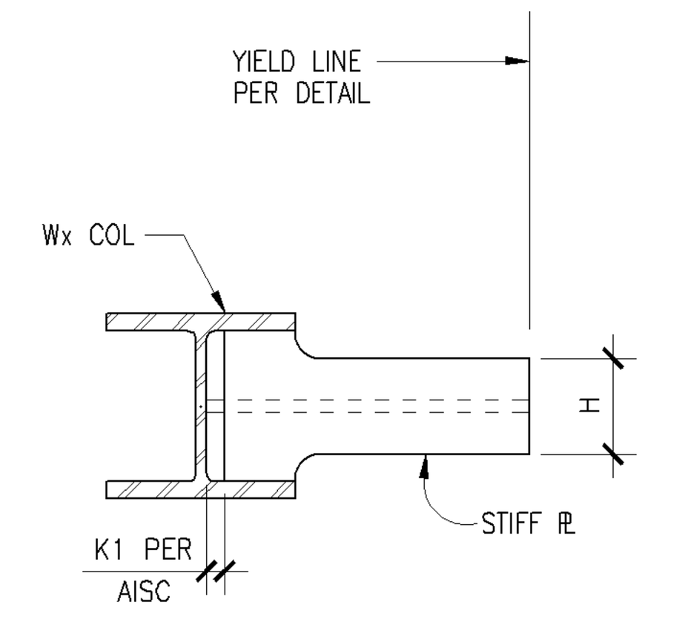
A ELEVATION



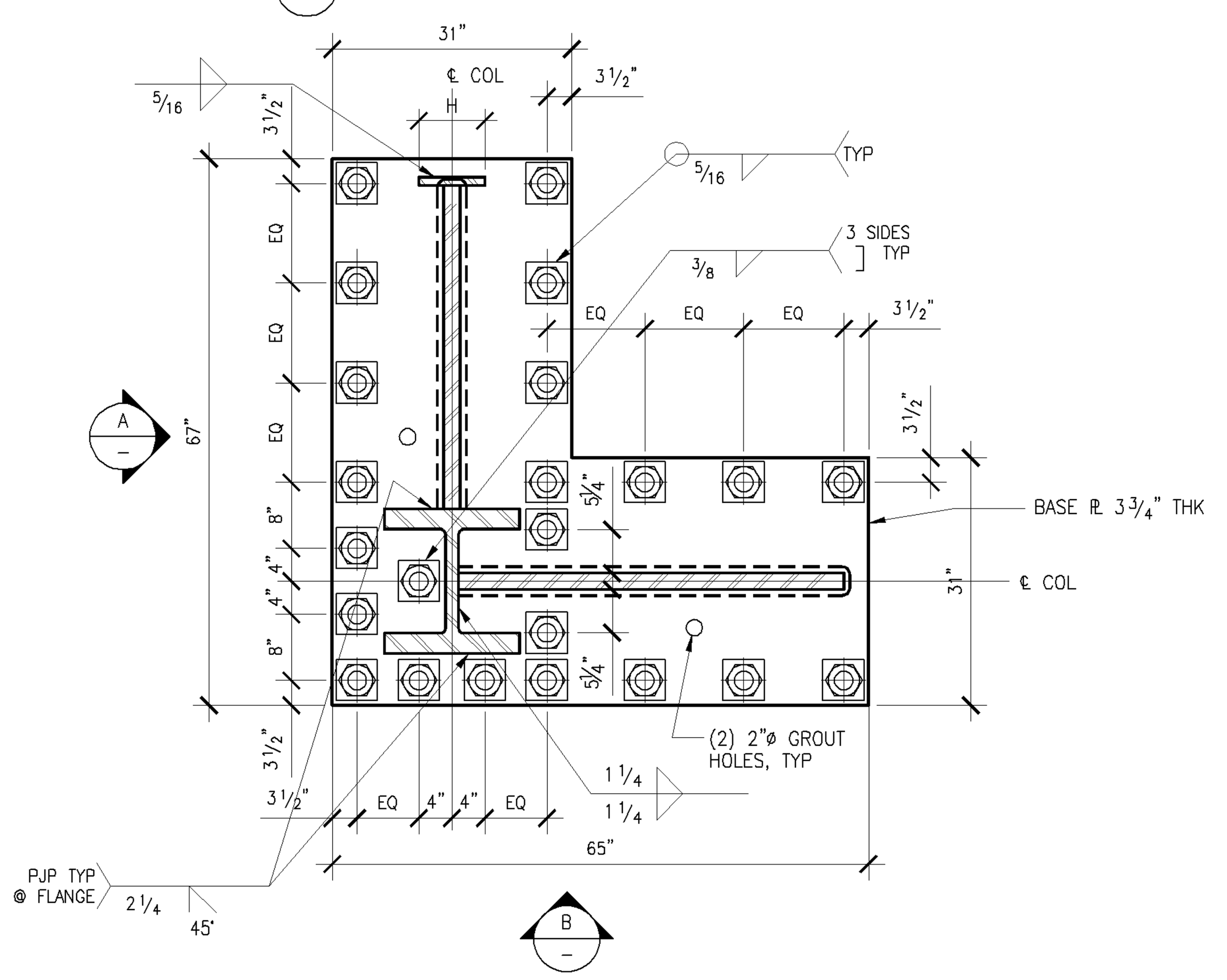
E SECTION

F SECTION

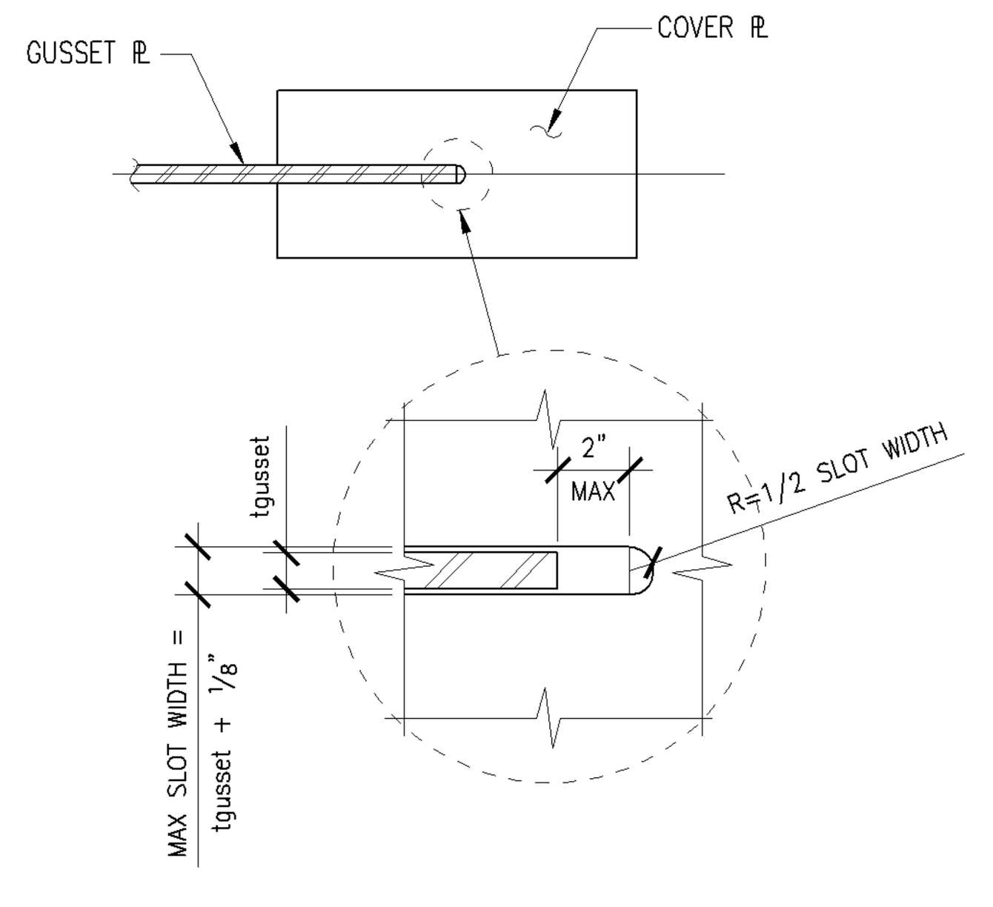
NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.



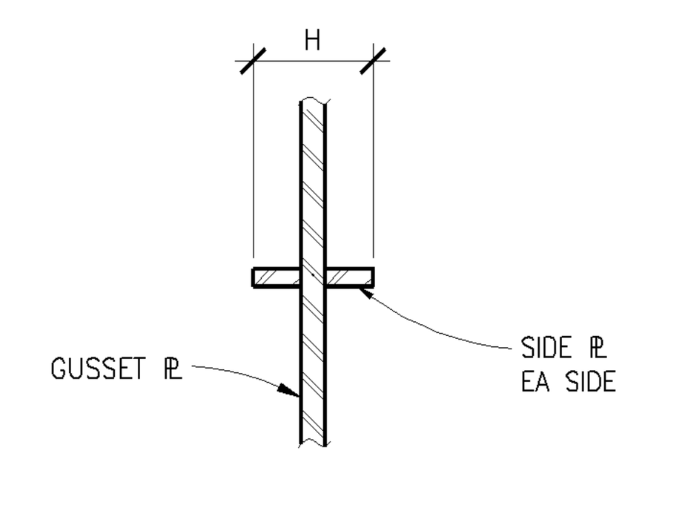
H SECTION



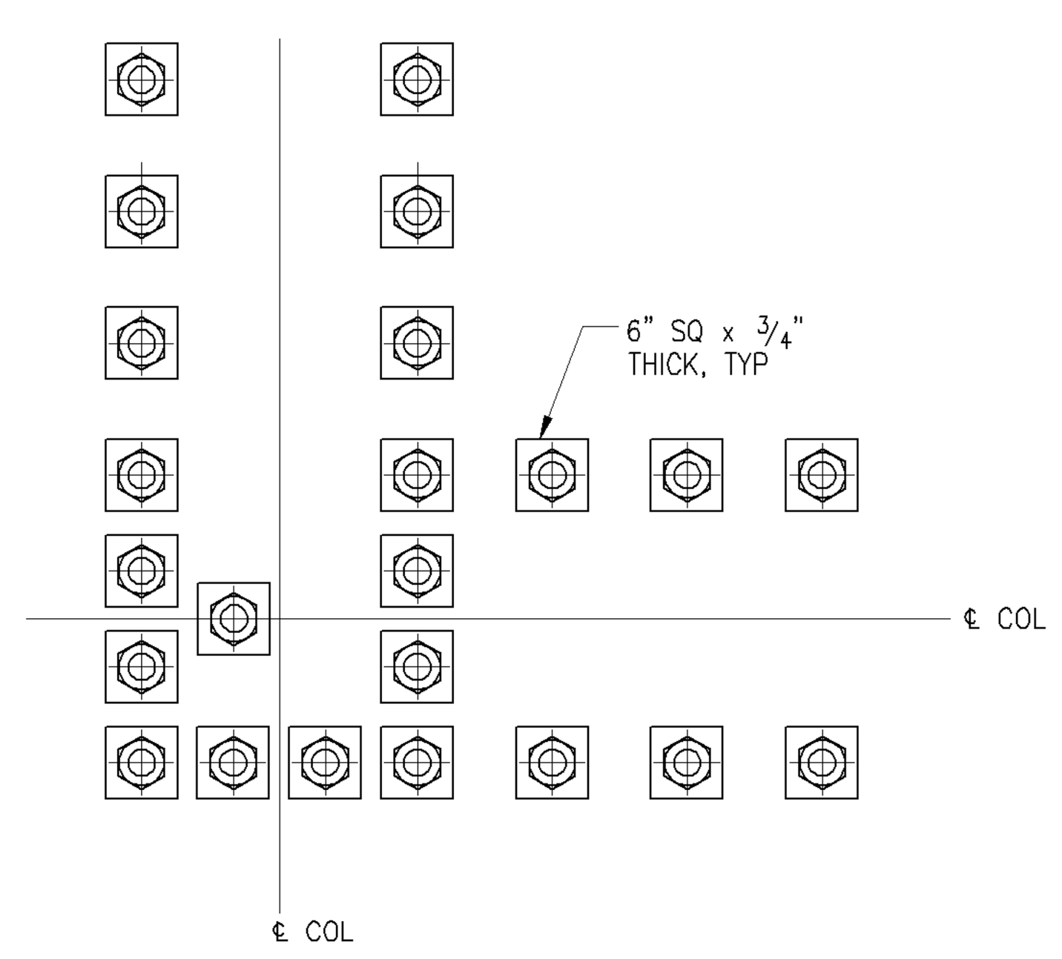
C PLAN - BASE PLATE



G SECTION



I SECTION

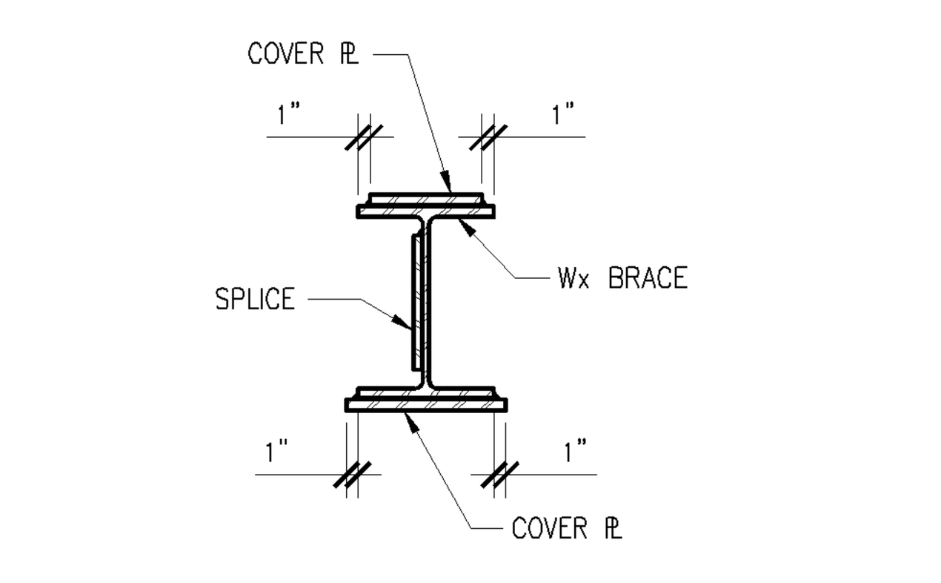


D PLAN - BEARING PLATES

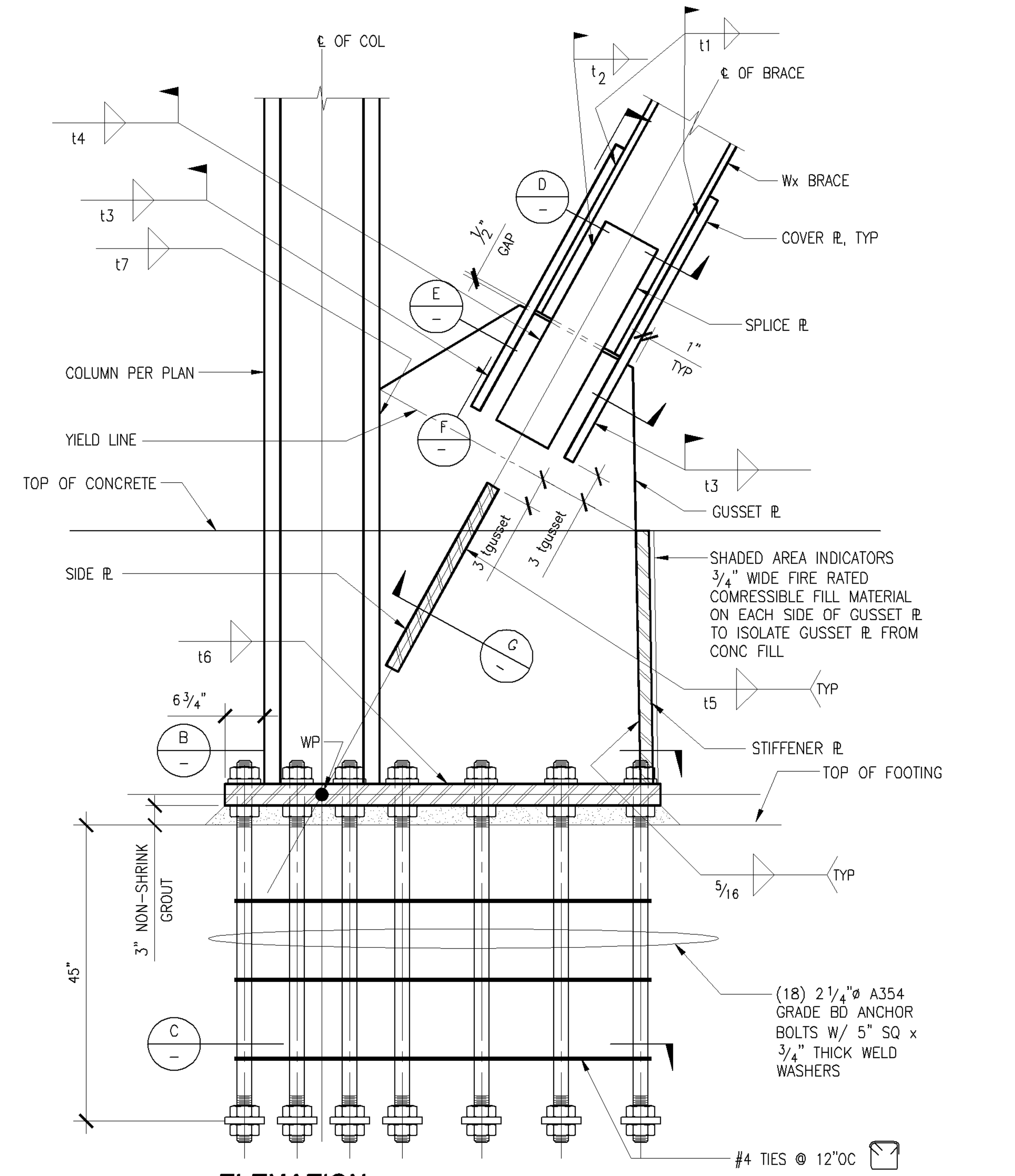
CONNECTION SCHEDULE J																					
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)						
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
J1 (ELV A)	W14x132	10	1 1/2	1 1/2	3/4	1/2	1/2	3/4	1/2	3/4	1/2	5/16	3/8	3/4	28	17	17	17	40	60
J1 (ELV B)	W14x132	10	1 3/4	1 1/2	1	–	–	3/4	5/8	3/8	5/8	5/16	3/4	3/4	28	17	28	17	47	46

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE J) 3/4" = 1'-0" 2

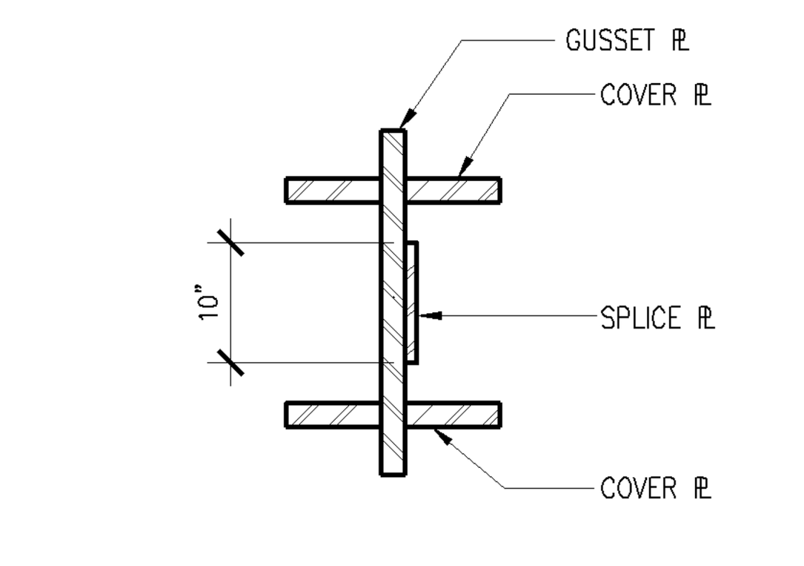
NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.



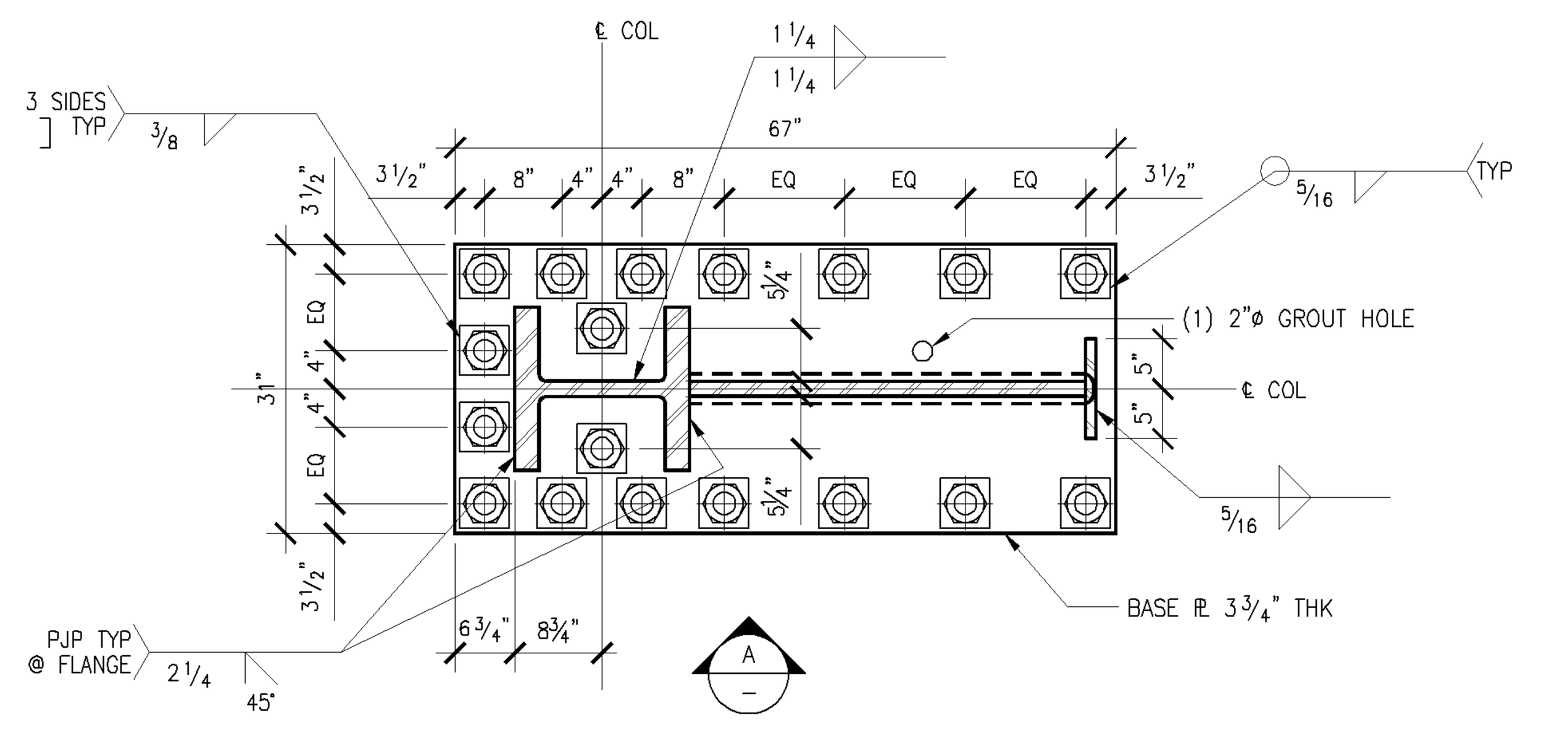
D SECTION



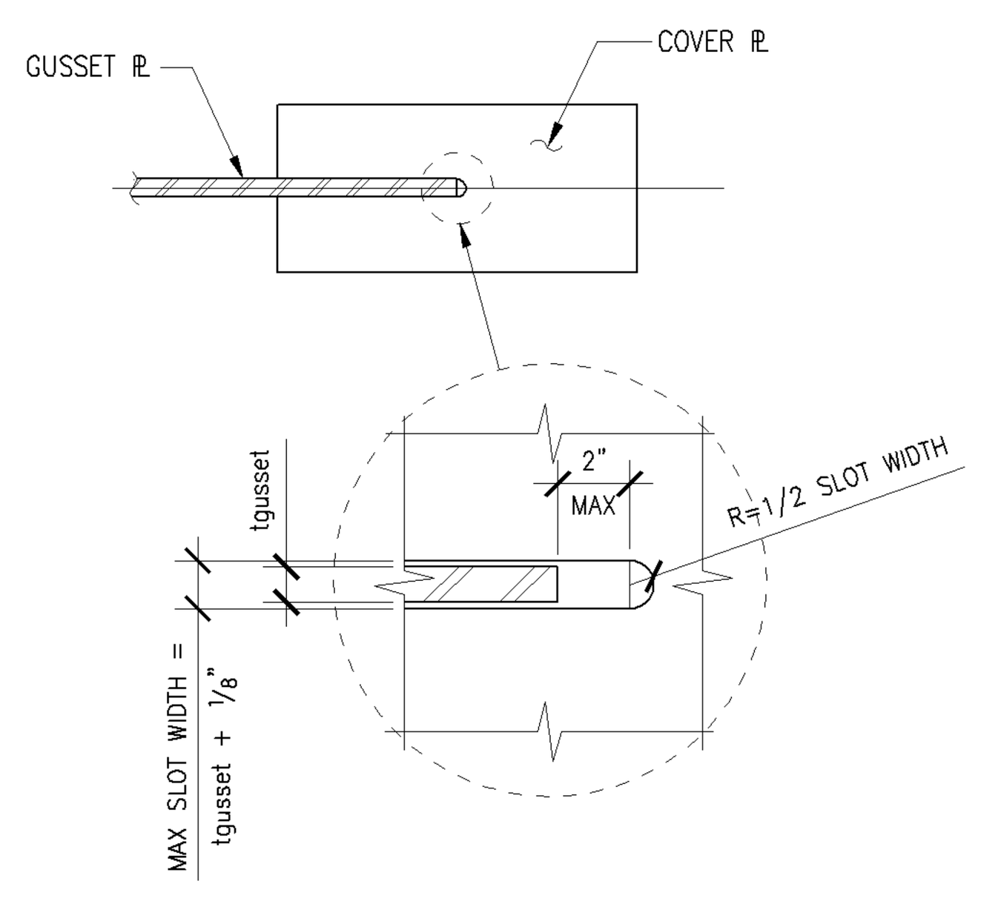
A ELEVATION



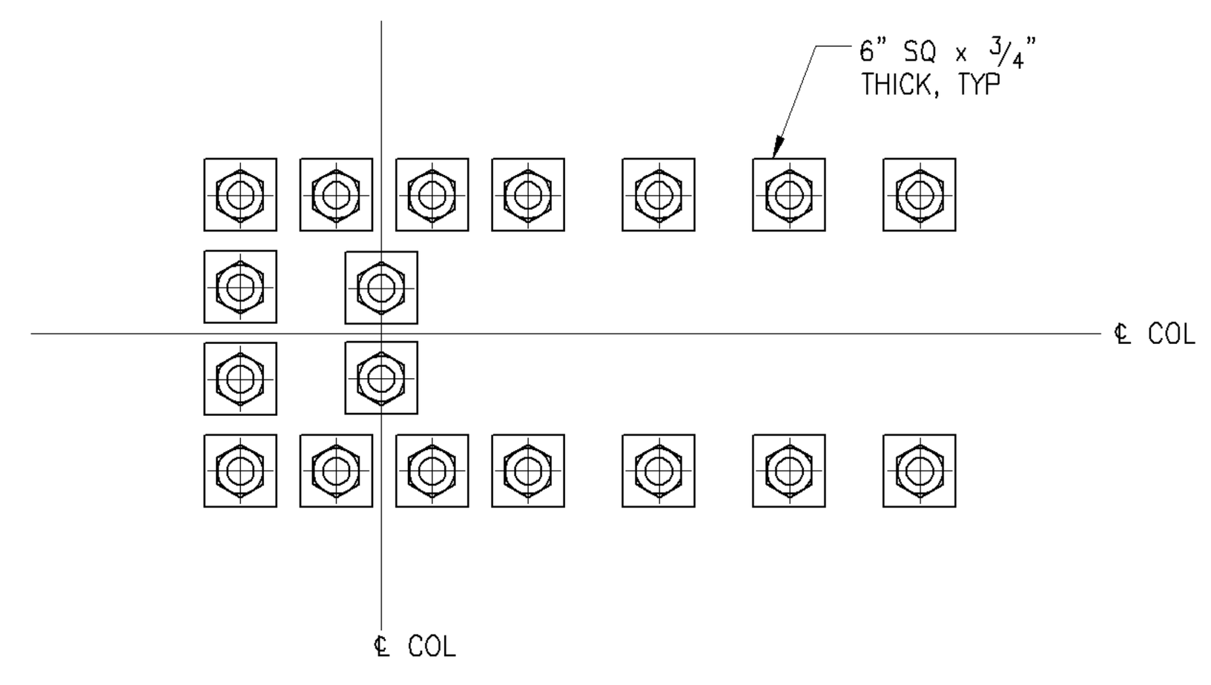
E SECTION



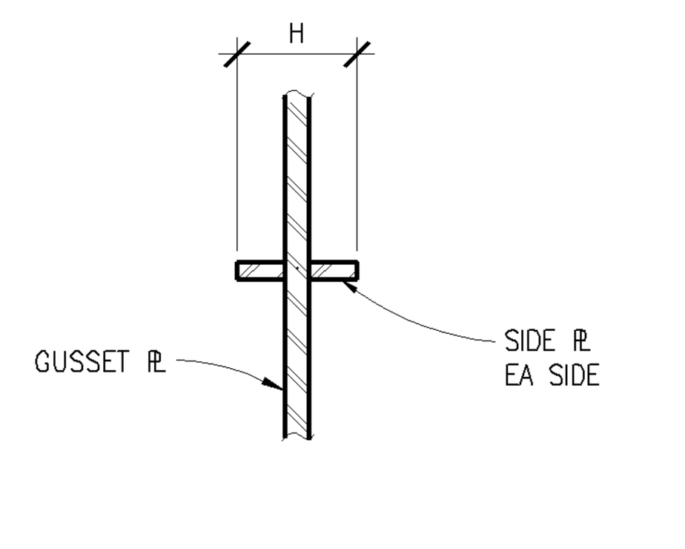
B PLAN - BASE PLATE



F SECTION



C PLAN - BEARING PLATES



G SECTION

CONNECTION SCHEDULE I																					
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)						
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
I1	W14x132	10	1 1/2	1 1/2	3/4	1/2	1/2	3/4	1/2	3/4	1/2	5/16	3/8	3/4	28	17	17	17	40	60

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE I) 3/4" = 1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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STRUCTURAL ENGINEER
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PASADENA, CA 91101

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CARLSBAD, CA 92010

BENNITT + MITCHELL
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2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

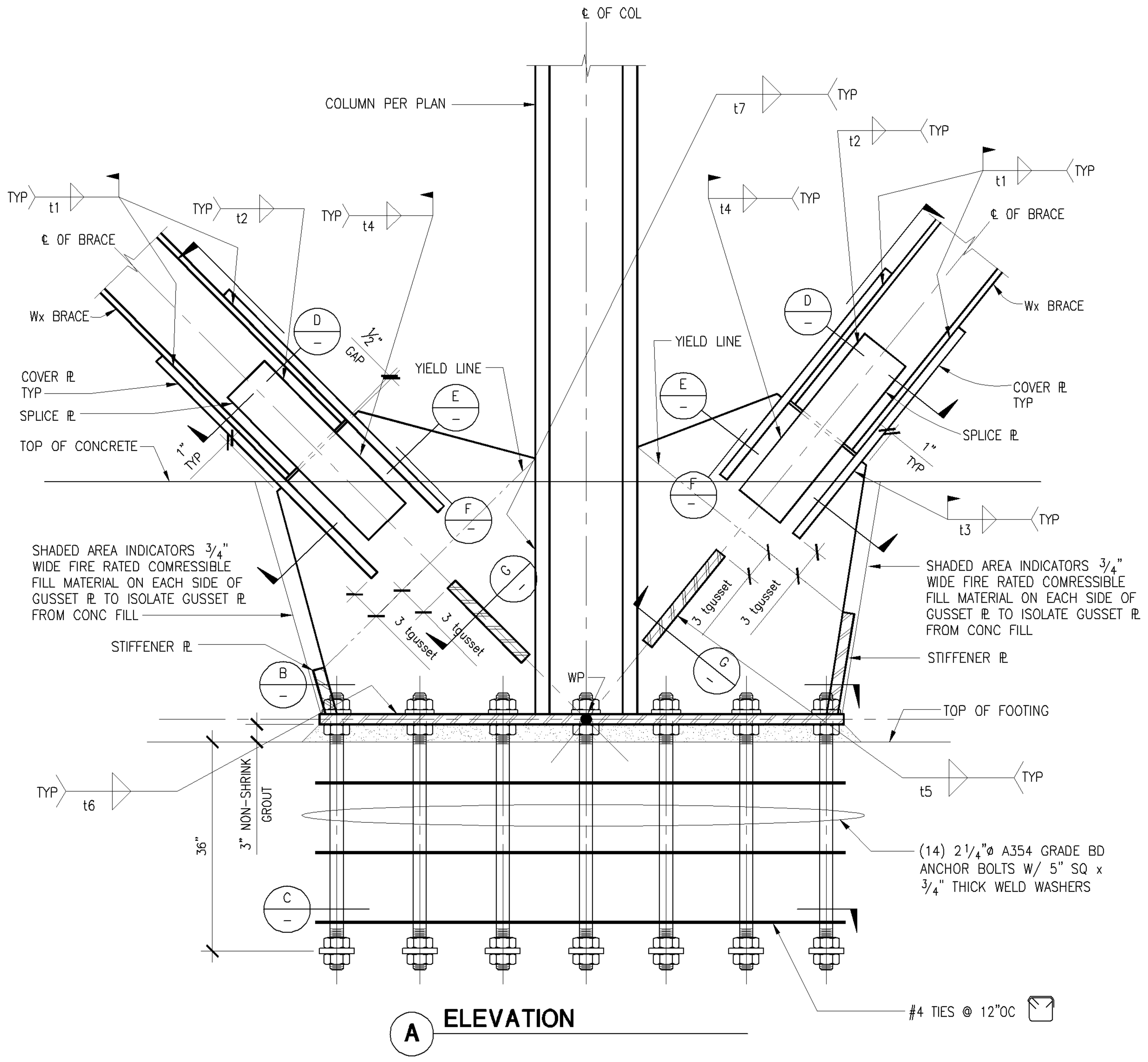
KEY PLAN

SHEET TITLE

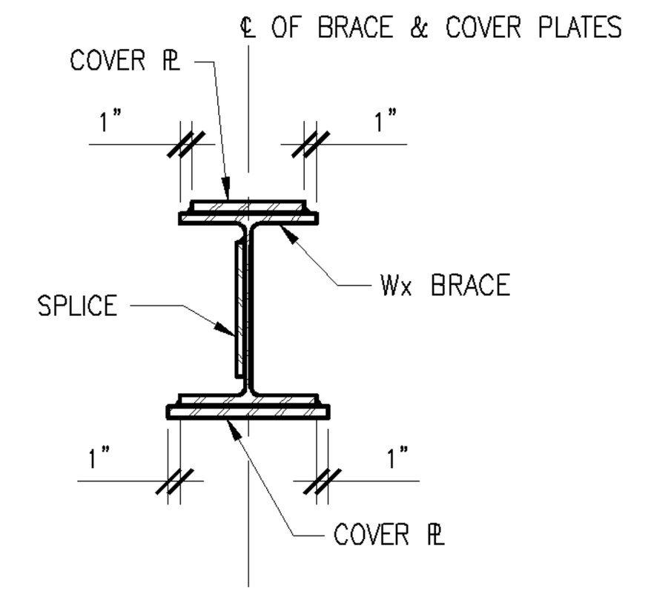
BRACED FRAME DETAILS

S6-107

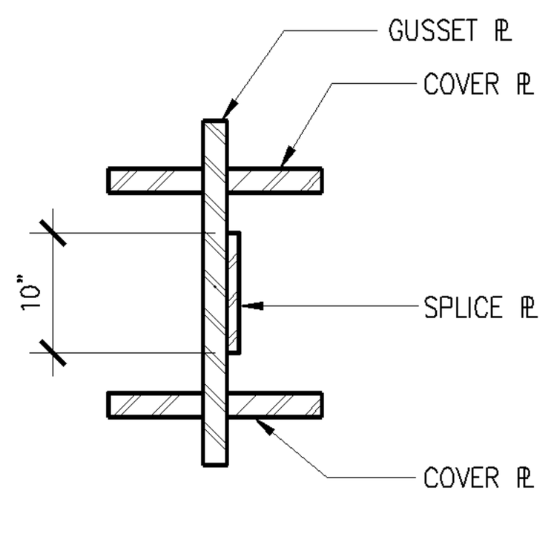
NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.



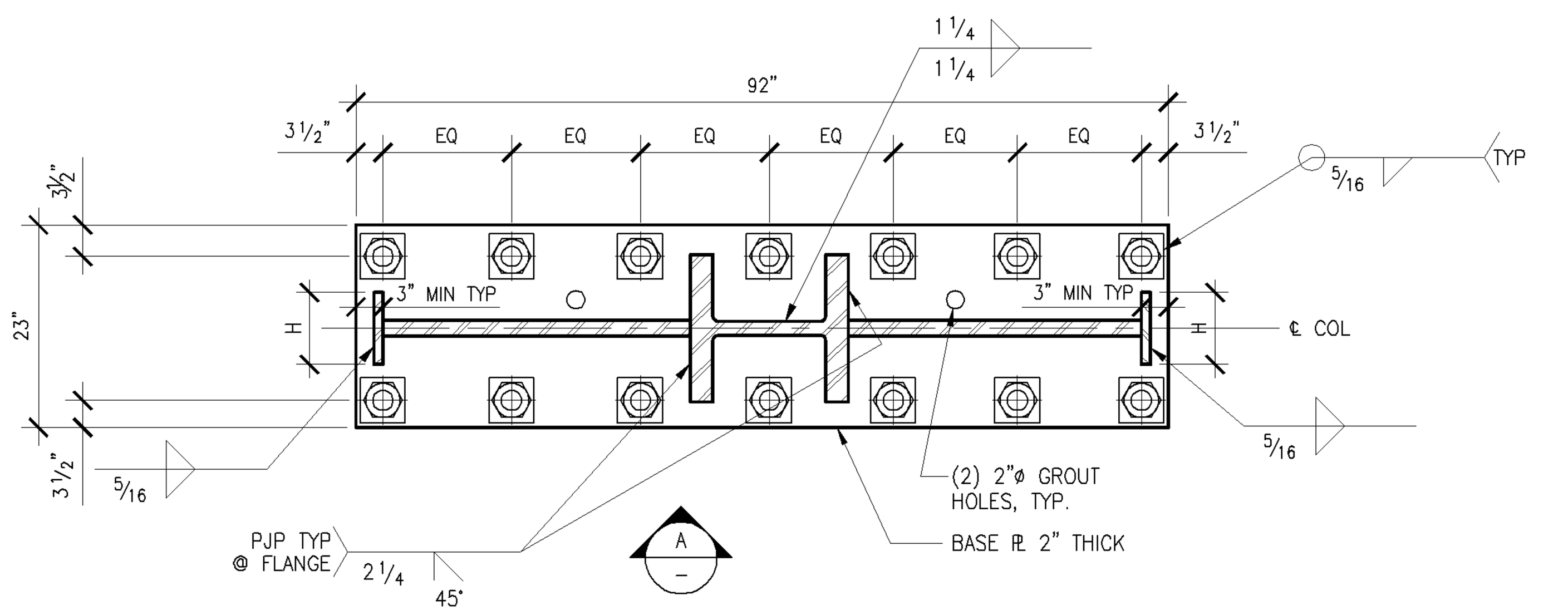
(A) ELEVATION



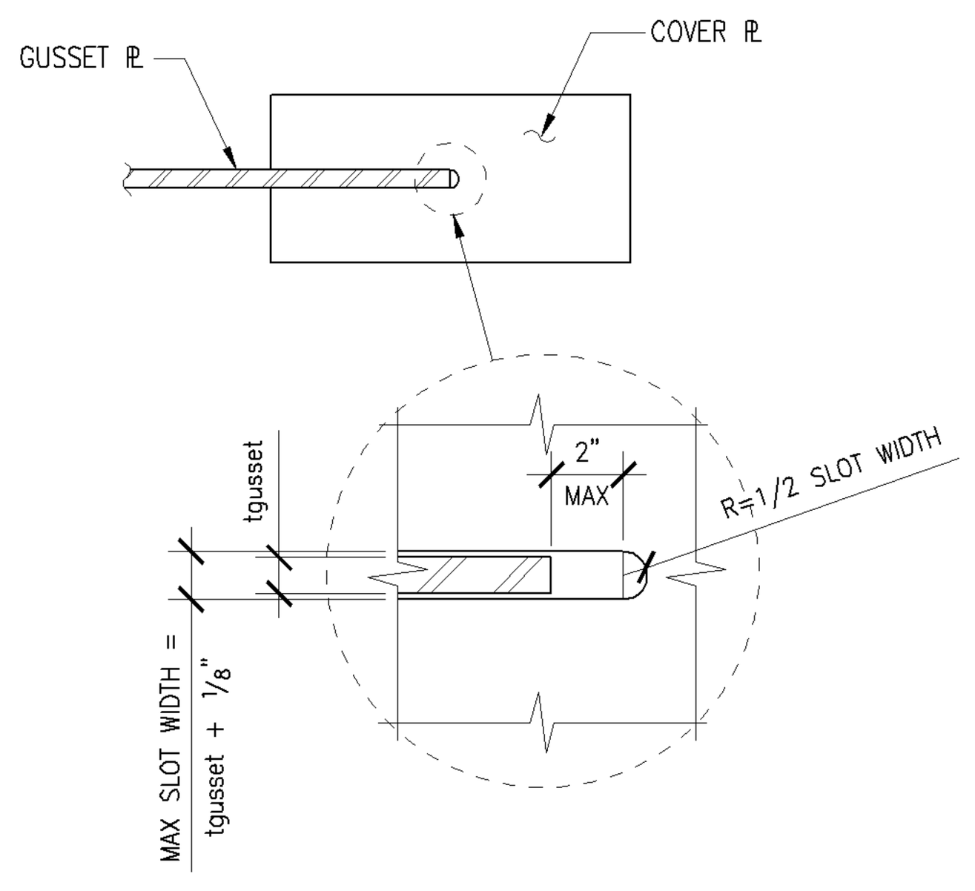
(D) SECTION



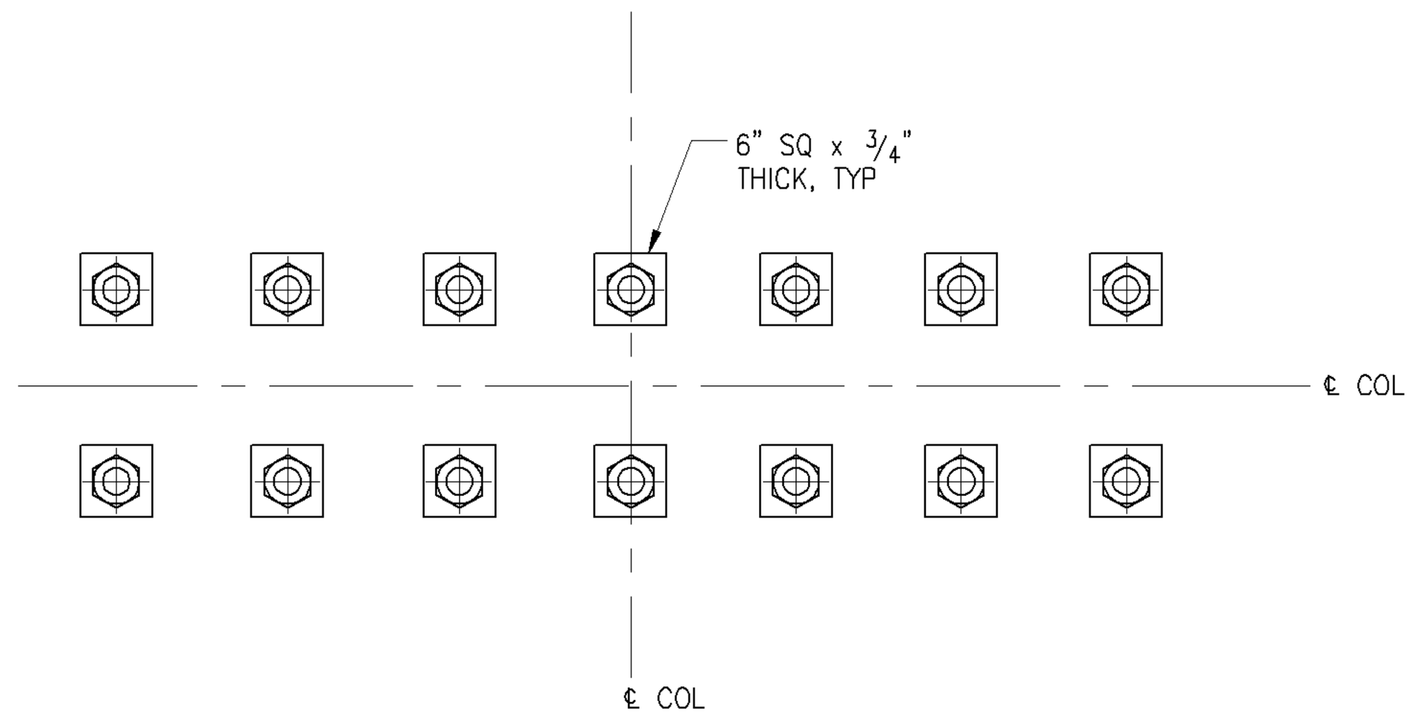
(E) SECTION



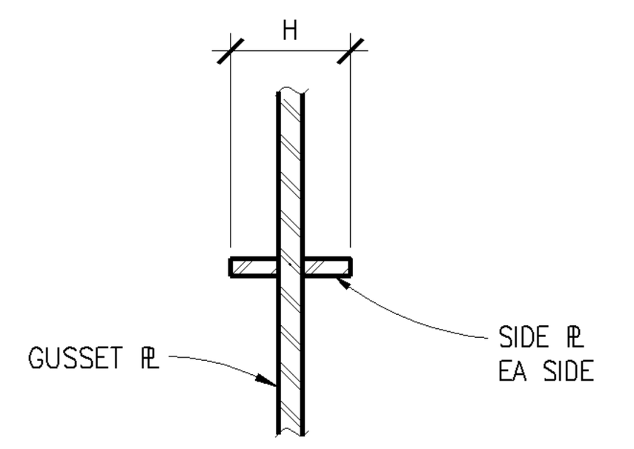
(B) PLAN - BASE PLATE



(F) SECTION



(C) PLAN - BEARING PLATES



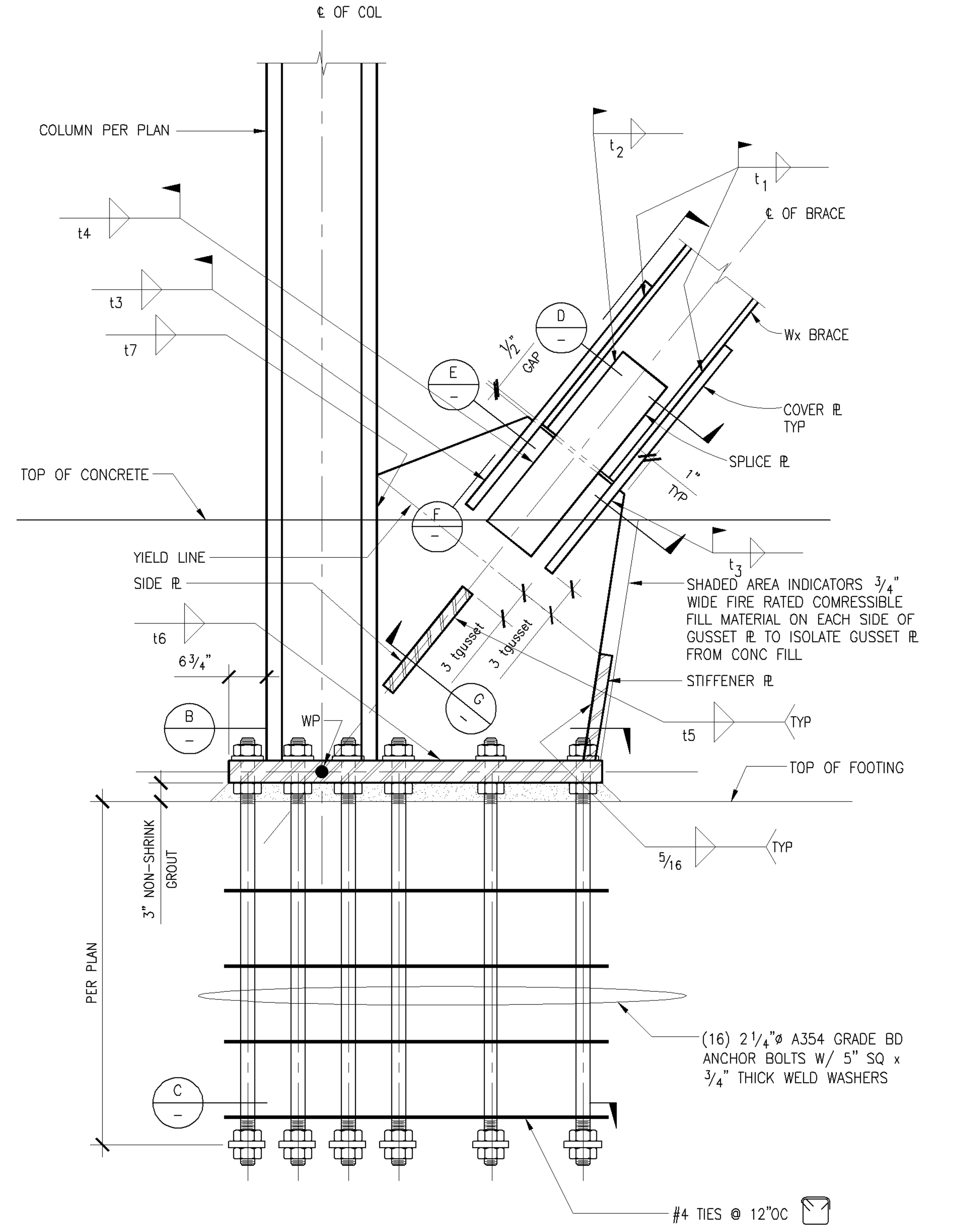
(G) SECTION

CONNECTION SCHEDULE L																				
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)					
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
L1	W14x132	–	1 3/4	1 1/2	1	–	–	3/4	5/8	5/8	5/8	5/16	3/4	7/8	28	17	18	17	33	44
L2	W14x132	–	1 3/4	1 1/2	3/4	–	–	3/4	1/2	5/8	1/2	5/16	3/4	7/8	30	17	22	17	34	42
L3	W14x132	–	1 3/4	1 1/2	1	–	–	3/4	5/8	3/4	5/8	5/16	5/8	7/8	28	17	17	17	33	45
L4	W14x132	–	1 3/4	1 1/2	3/4	–	–	3/4	1/2	5/8	1/2	5/16	3/4	7/8	30	17	22	17	34	43

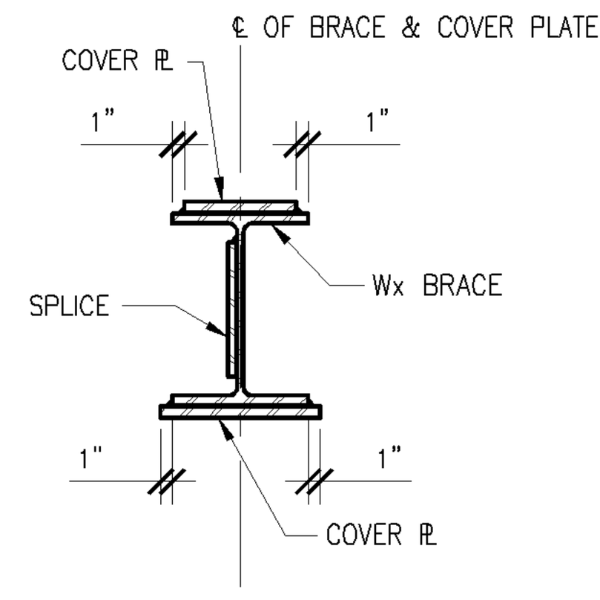
2 BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE L)

3/4" = 1'-0"

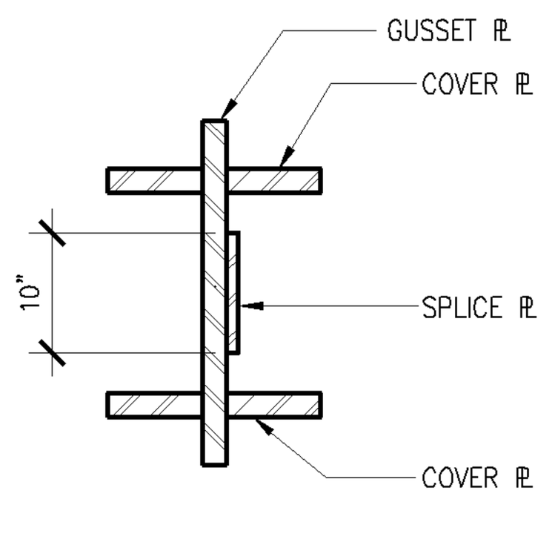
NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.



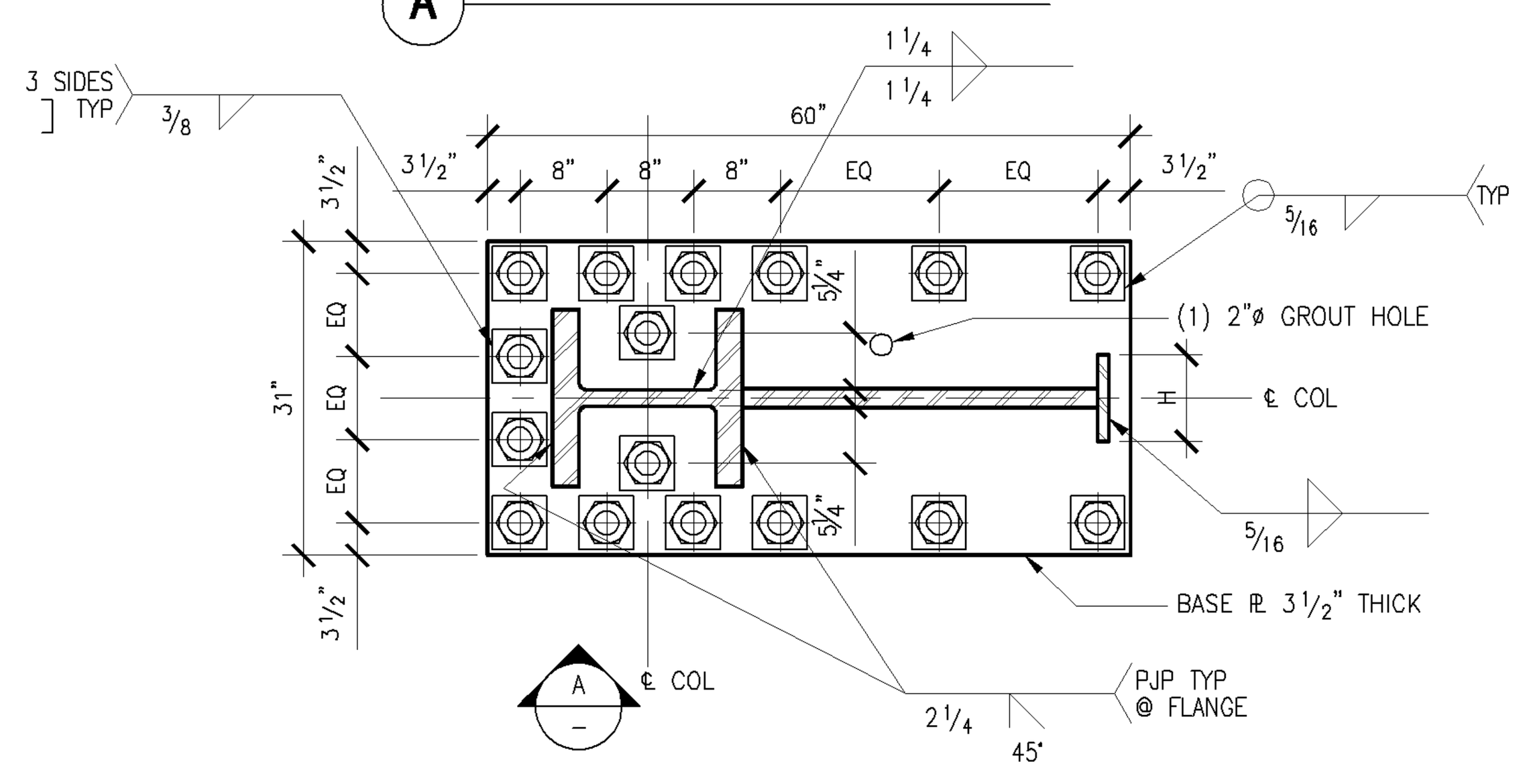
(A) ELEVATION



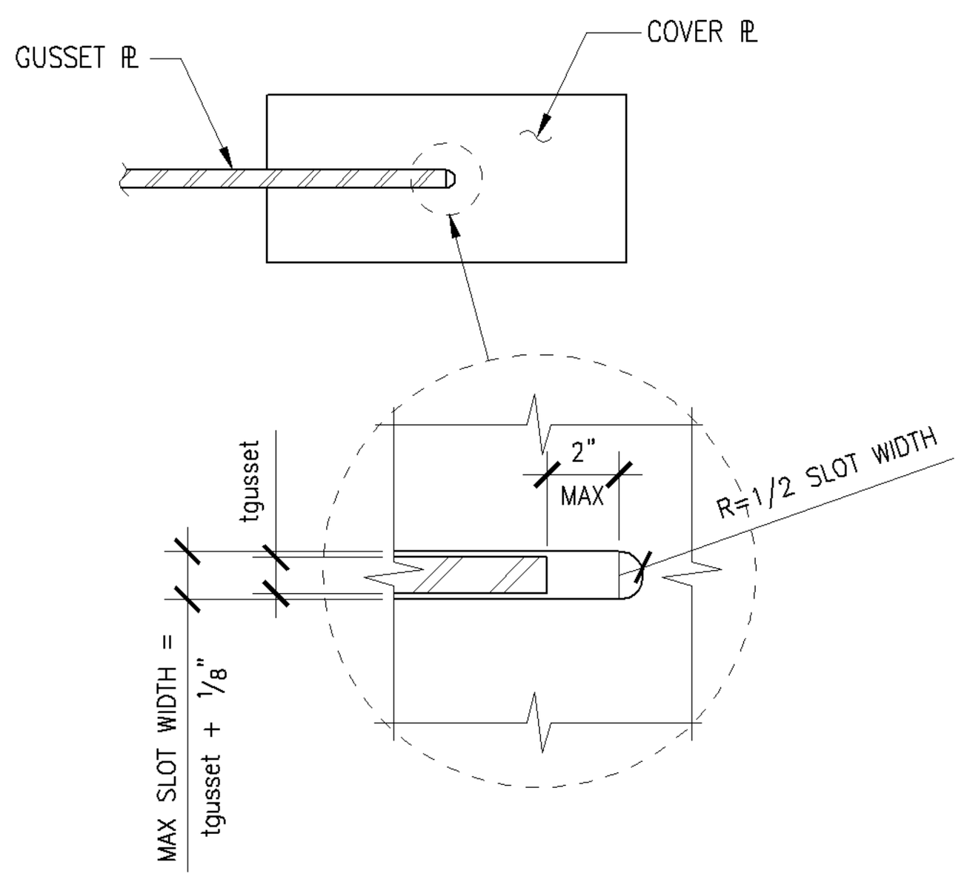
(D) SECTION



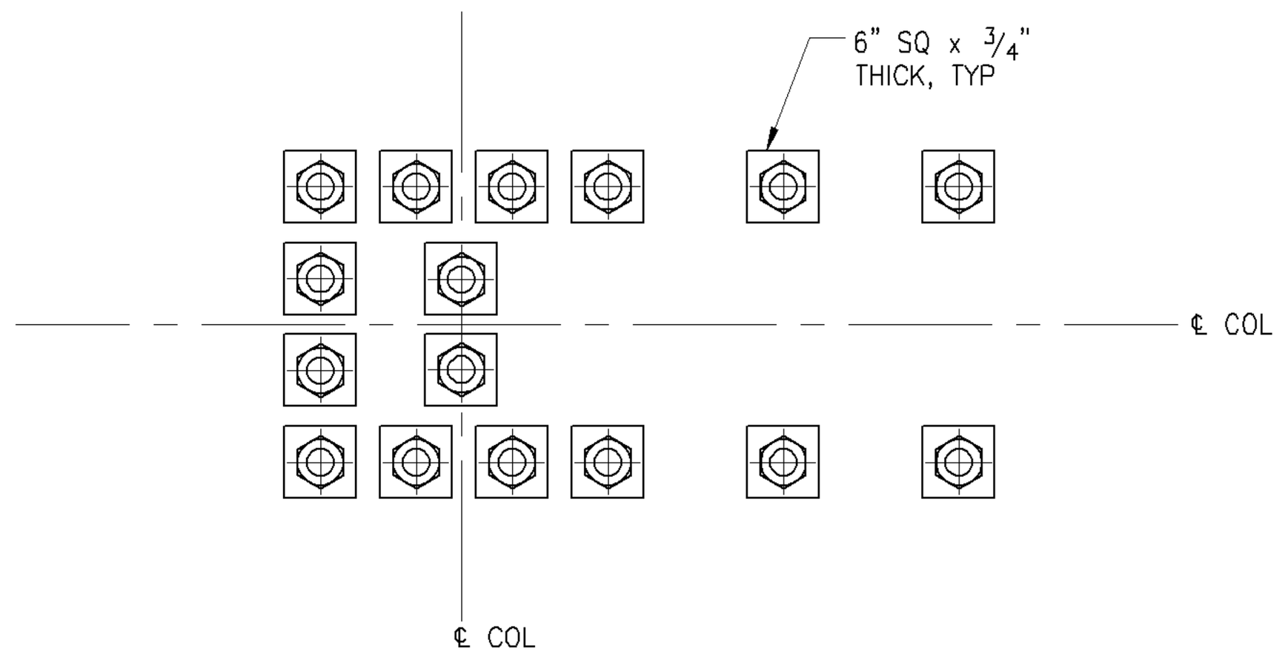
(E) SECTION



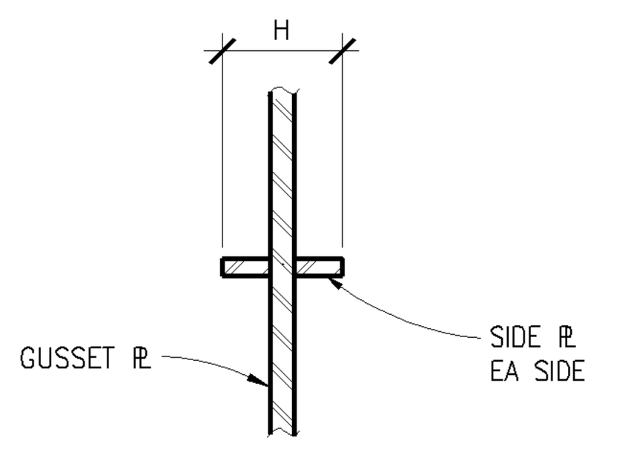
(B) PLAN - BASE PLATE



(F) SECTION



(C) PLAN - BEARING PLATES



(G) SECTION

CONNECTION SCHEDULE K																				
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)					
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
K1	W14x132	–	1 3/4	1 1/2	1	–	–	3/4	5/8	5/8	5/8	5/16	3/4	7/8	28	17	18	17	33	44
K2	W14x132	–	1 3/4	1 1/2	1	–	–	3/4	5/8	3/4	5/8	5/16	5/8	7/8	28	17	17	17	33	45

1 BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE K)

3/4" = 1'-0"

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

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2908 OREGON COURT, SUITE I-7
TORRANCE, CA 90503

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER · STRUCTURAL · STATE OF CALIFORNIA · Exp. 12/31/07

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

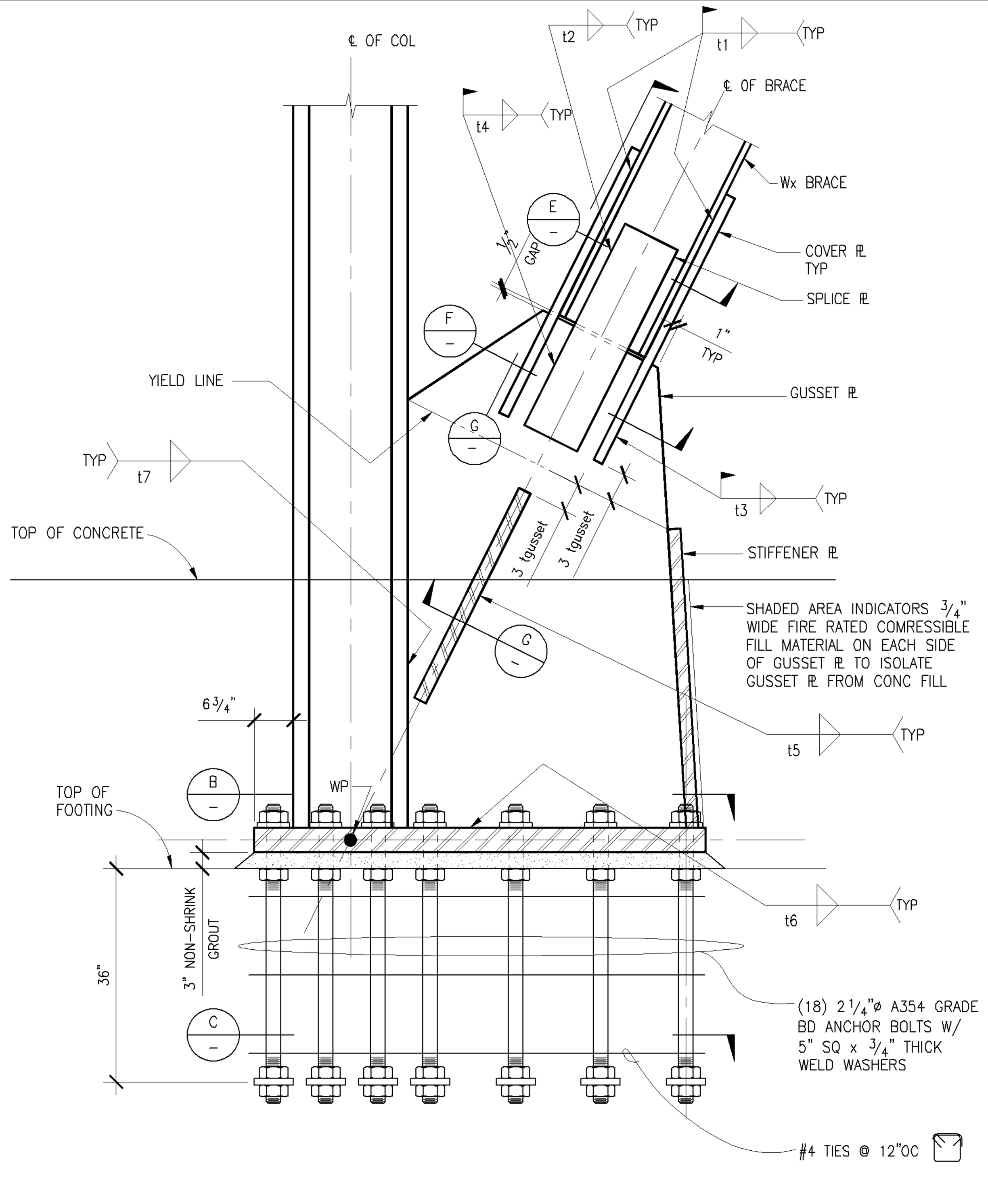
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

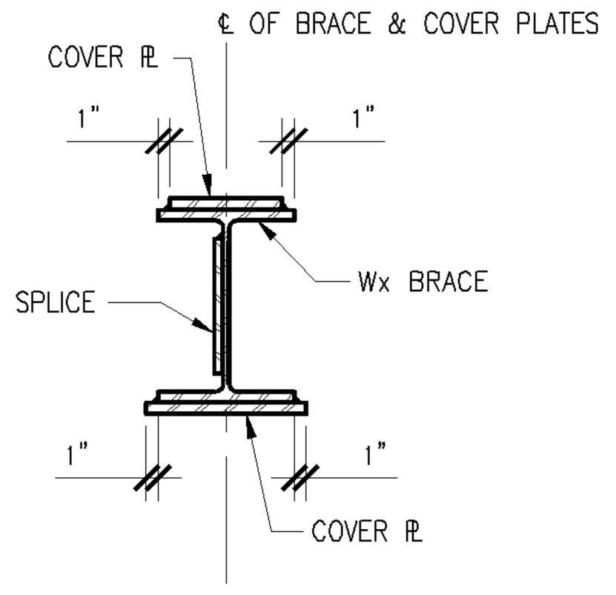
SHEET TITLE
BRACED FRAME DETAILS

S6-108

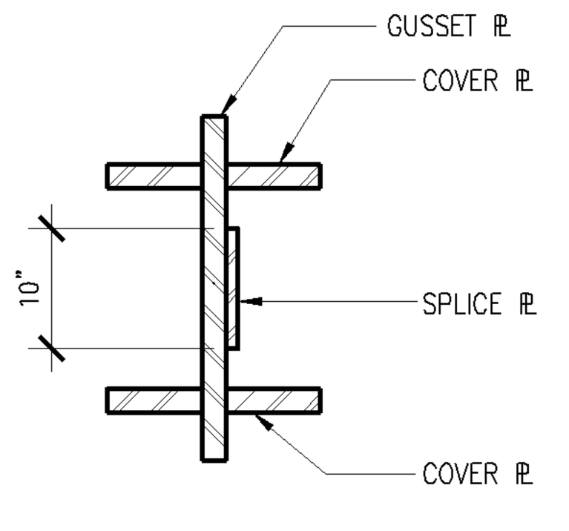
NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.



A ELEVATION



D SECTION



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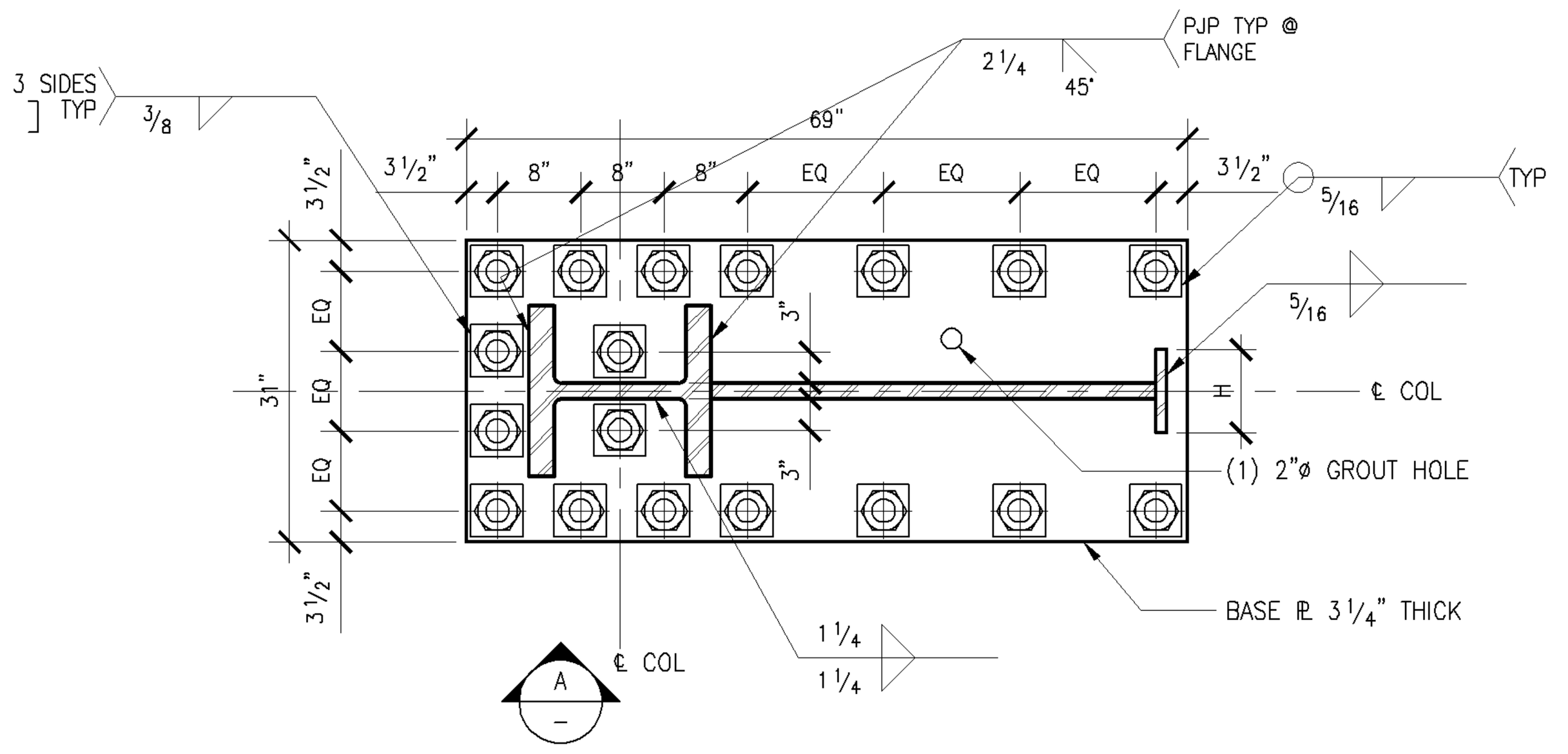
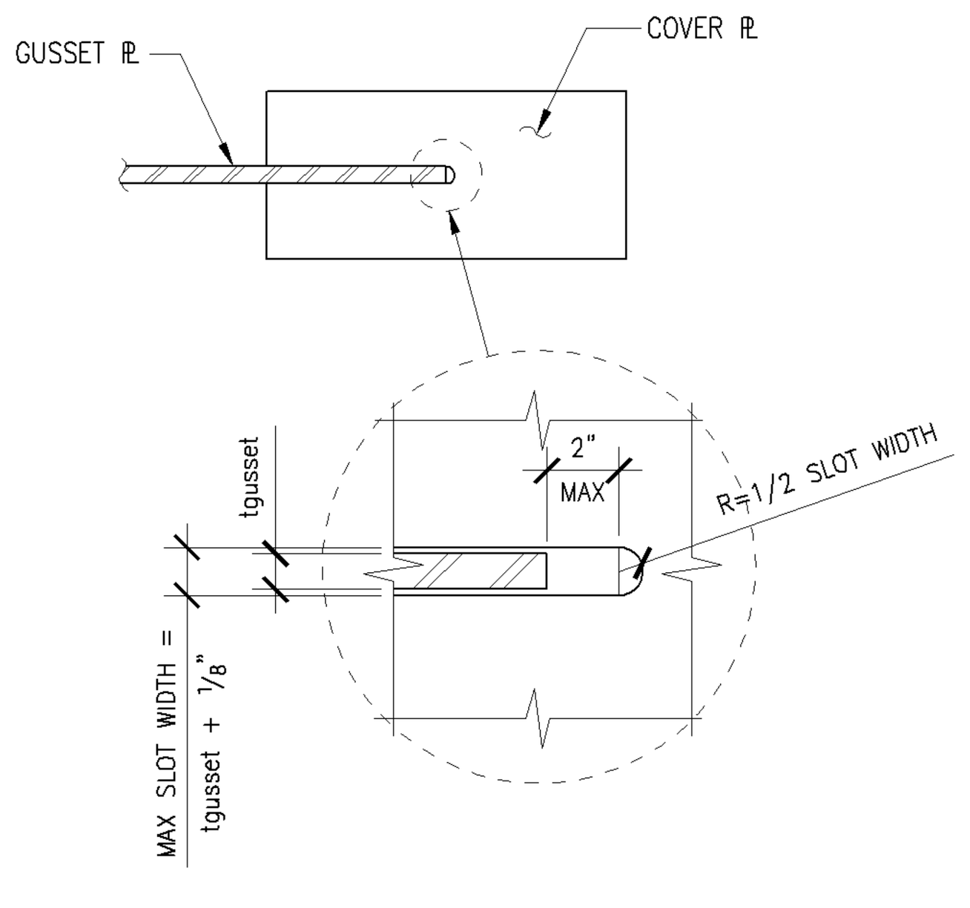


B PLAN - BASE PLATE



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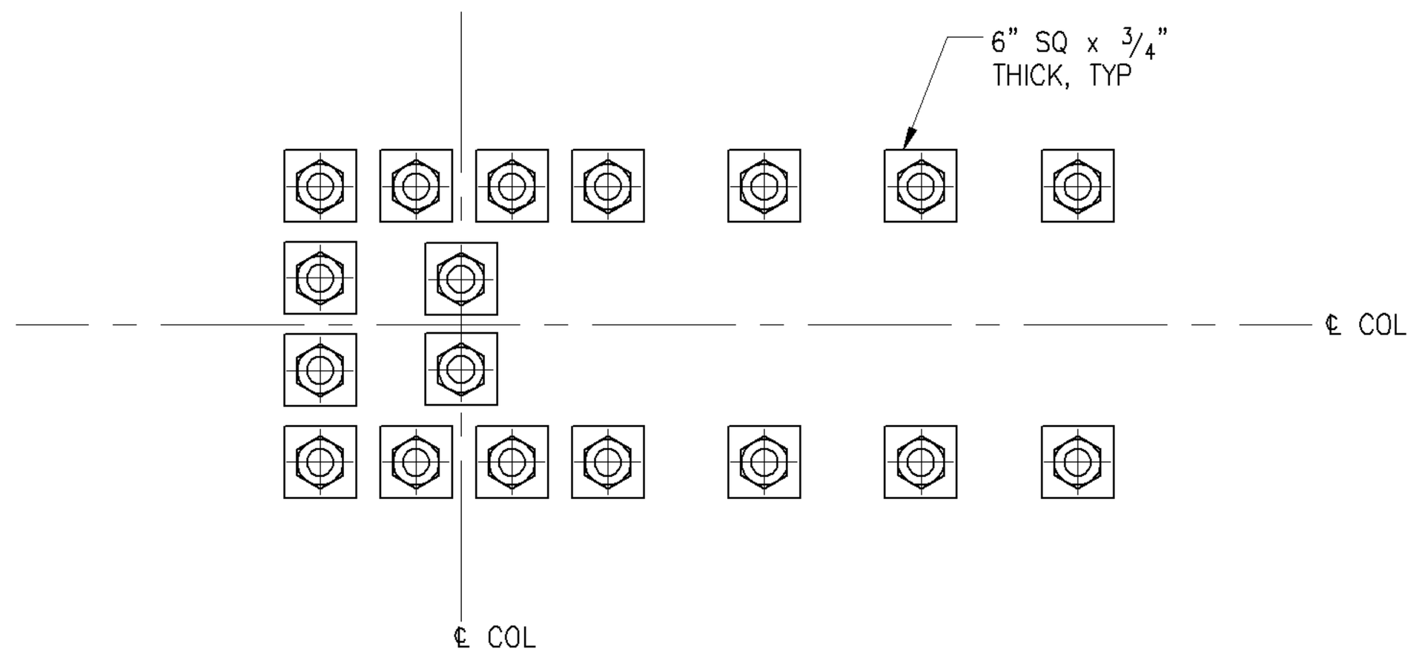
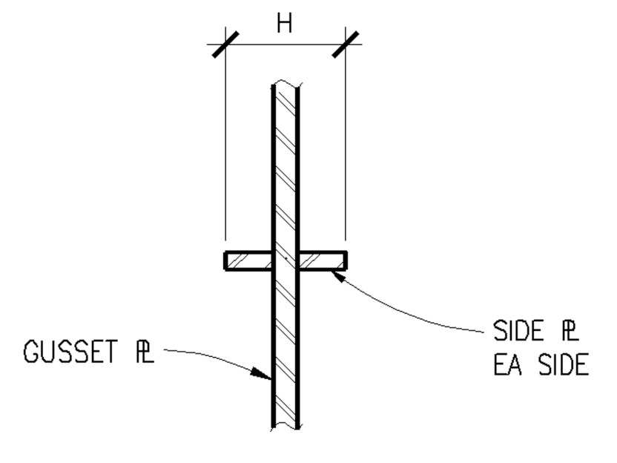


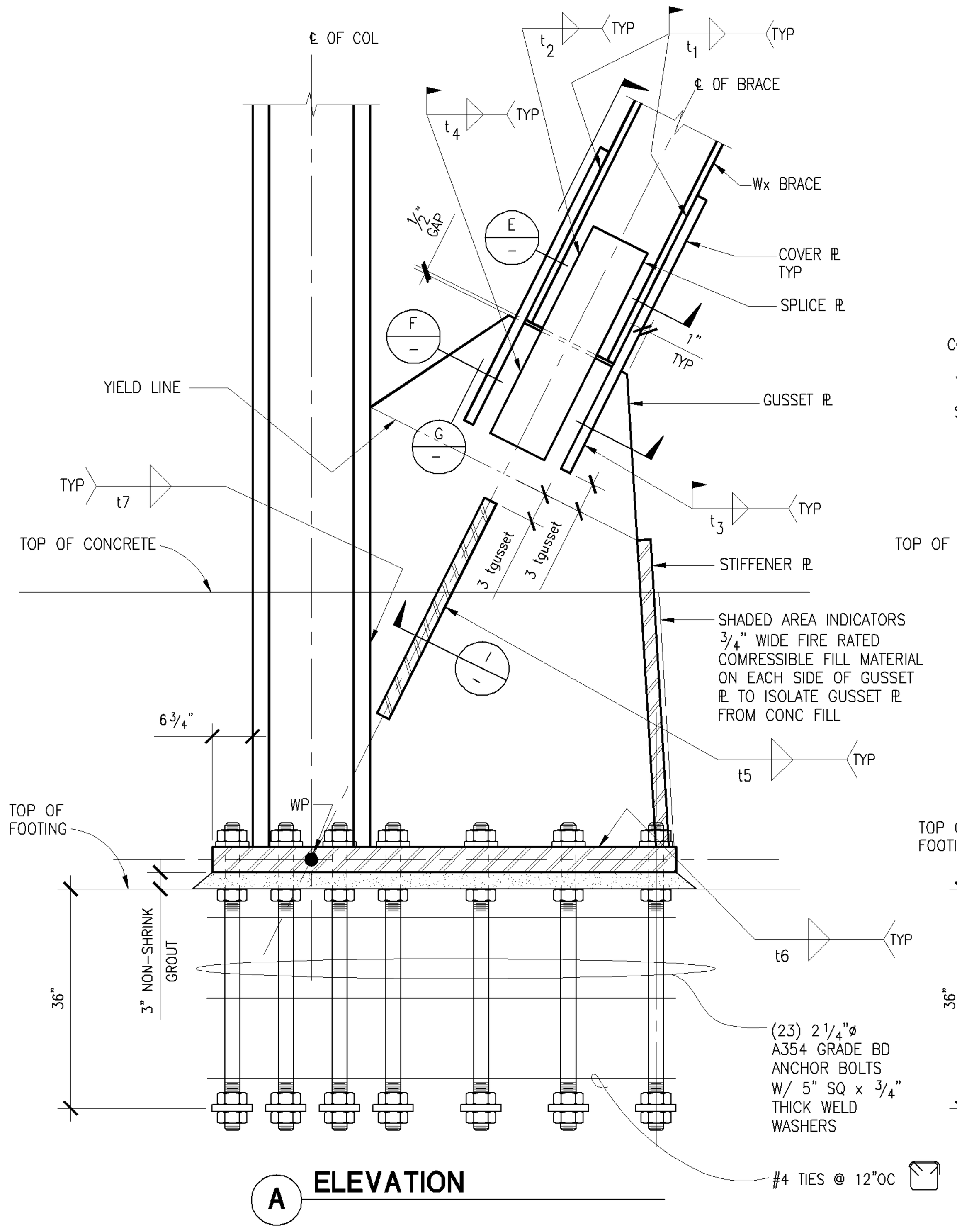
C PLAN - BEARING PLATES



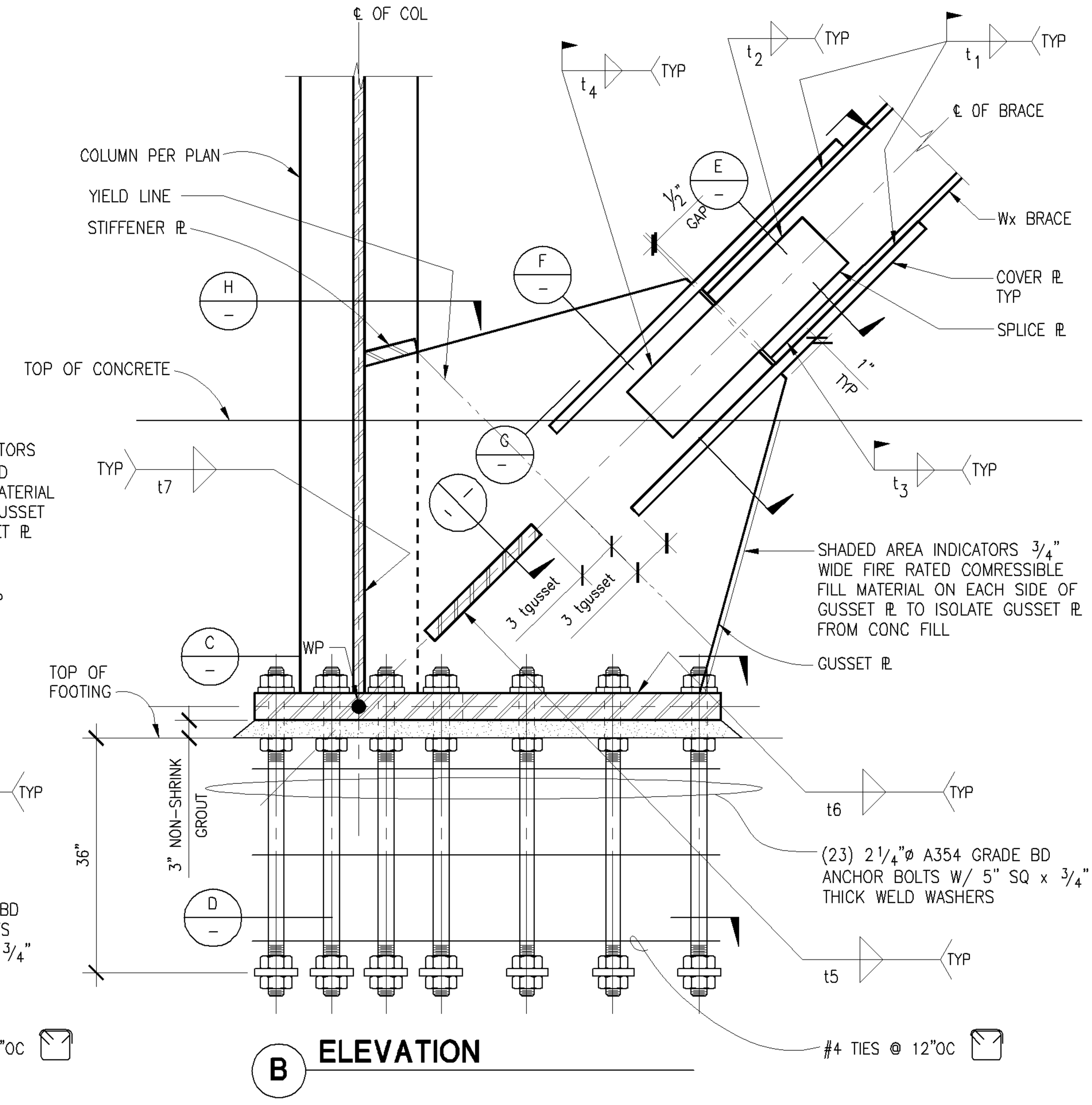
G SECTION

CONNECTION SCHEDULE N																					
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)						
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7	
N1	W14x132	10	1 1/2	1 1/2	3/4	3/4	3/4	3/4	1/2	3/4	1/2	5/16	5/16	5/8	28	17	17	17	43	66	

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE N) 3/4" = 1'-0" 2

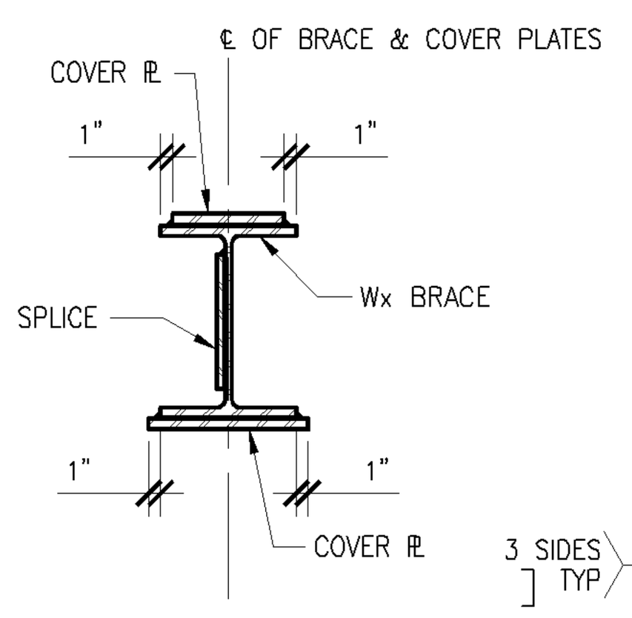


A ELEVATION

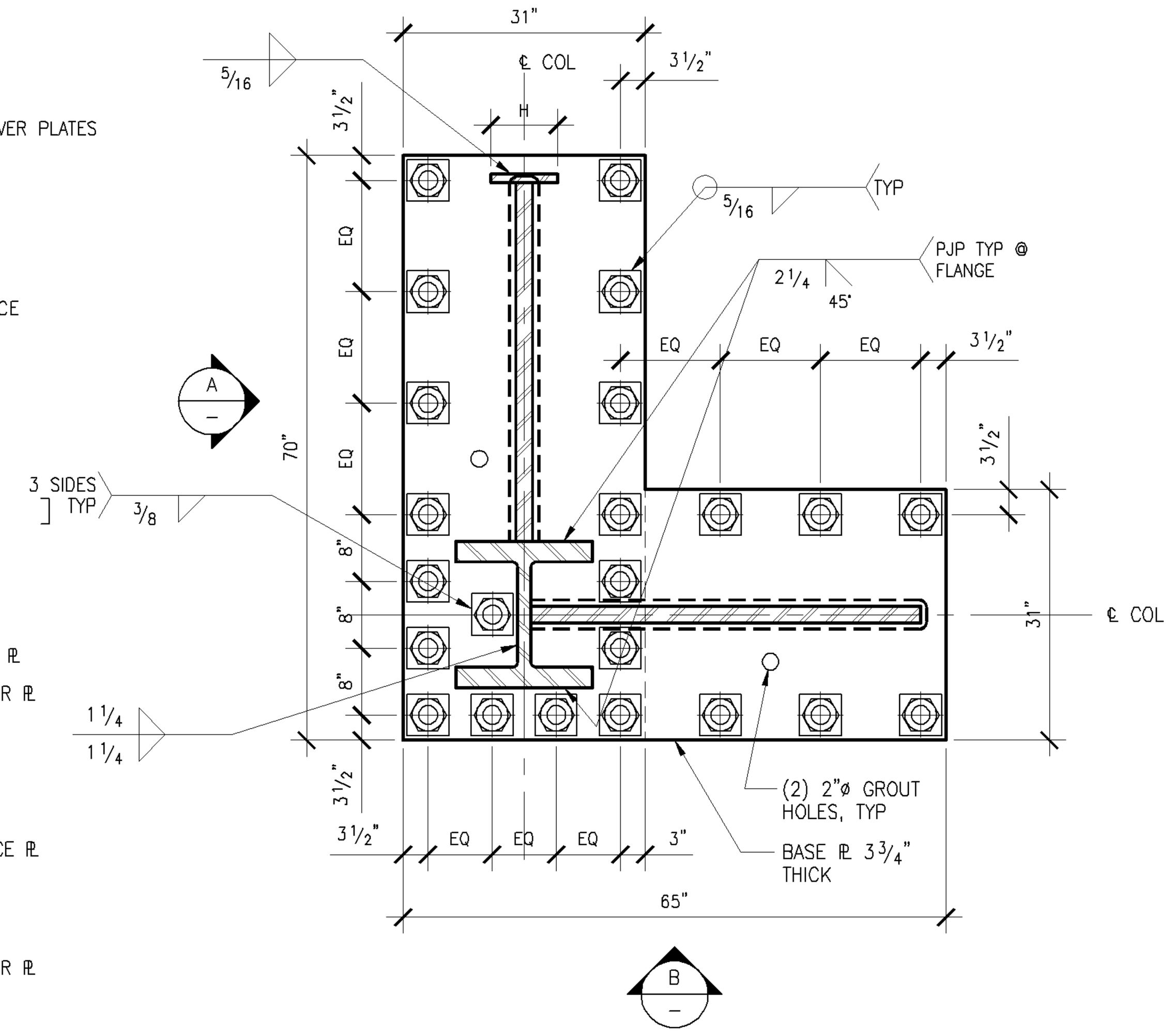


B ELEVATION

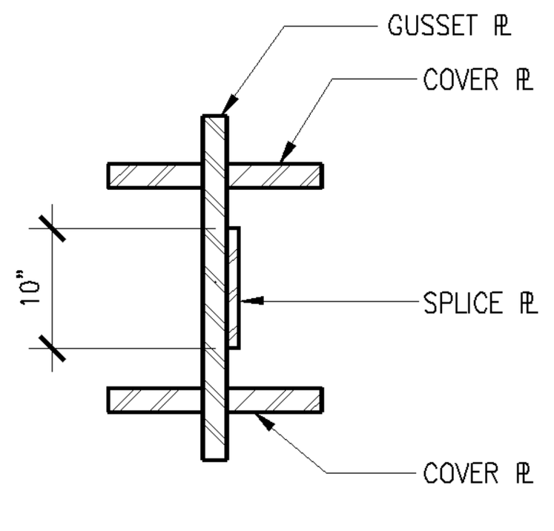
NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.



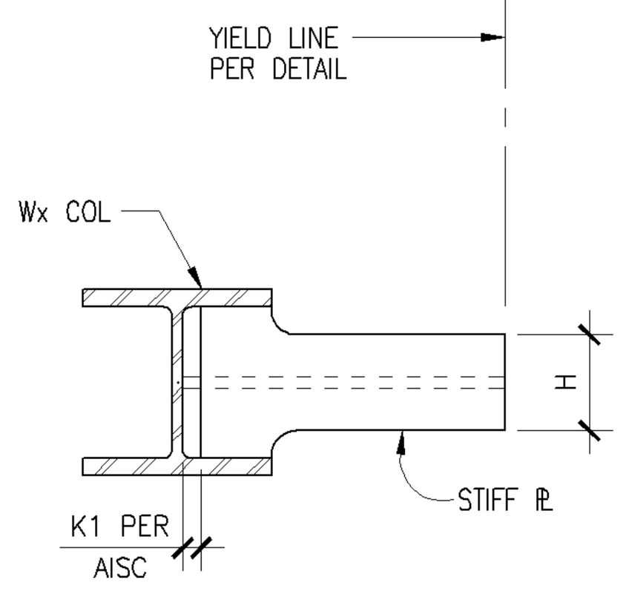
E SECTION



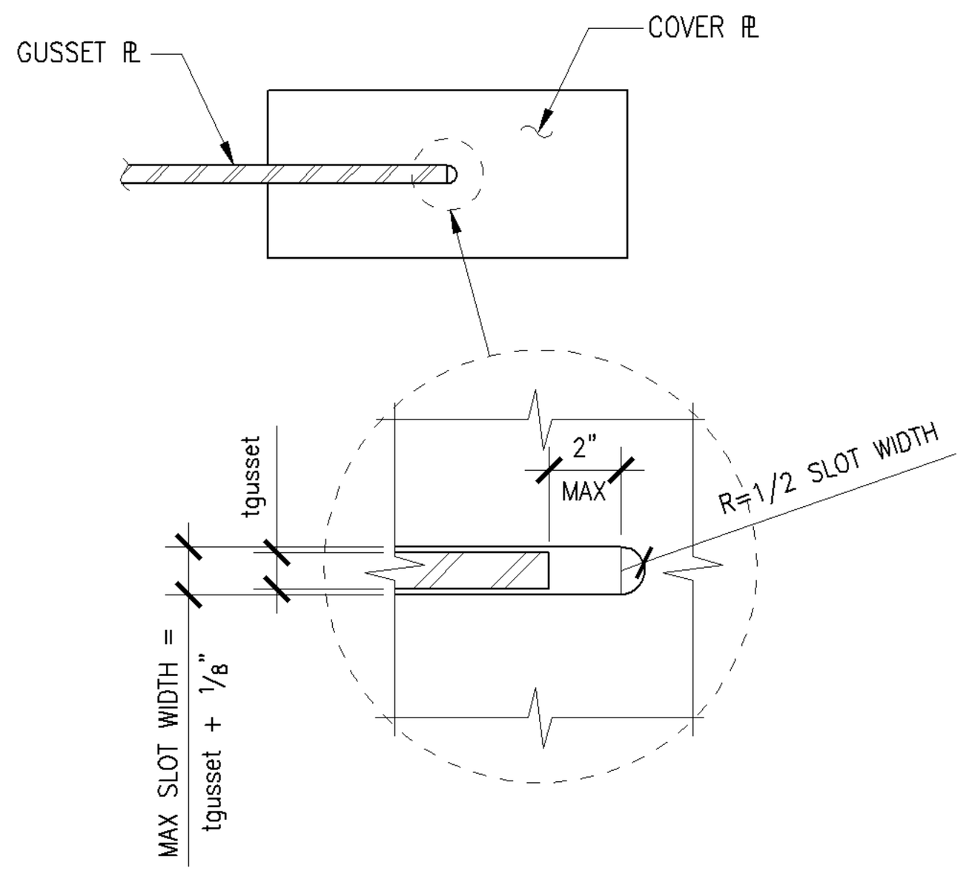
C PLAN - BASE PLATE



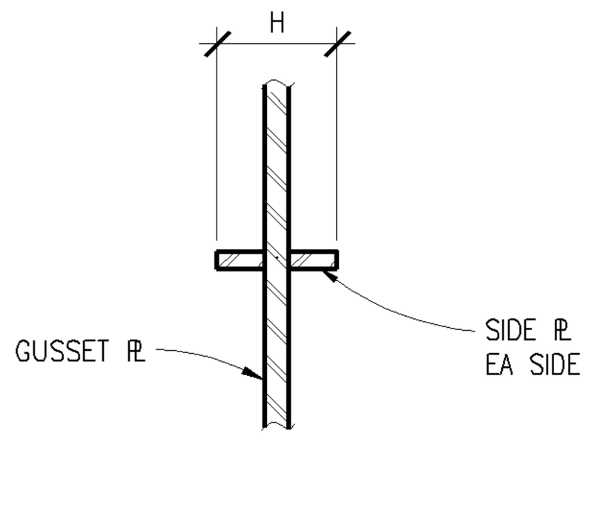
F SECTION



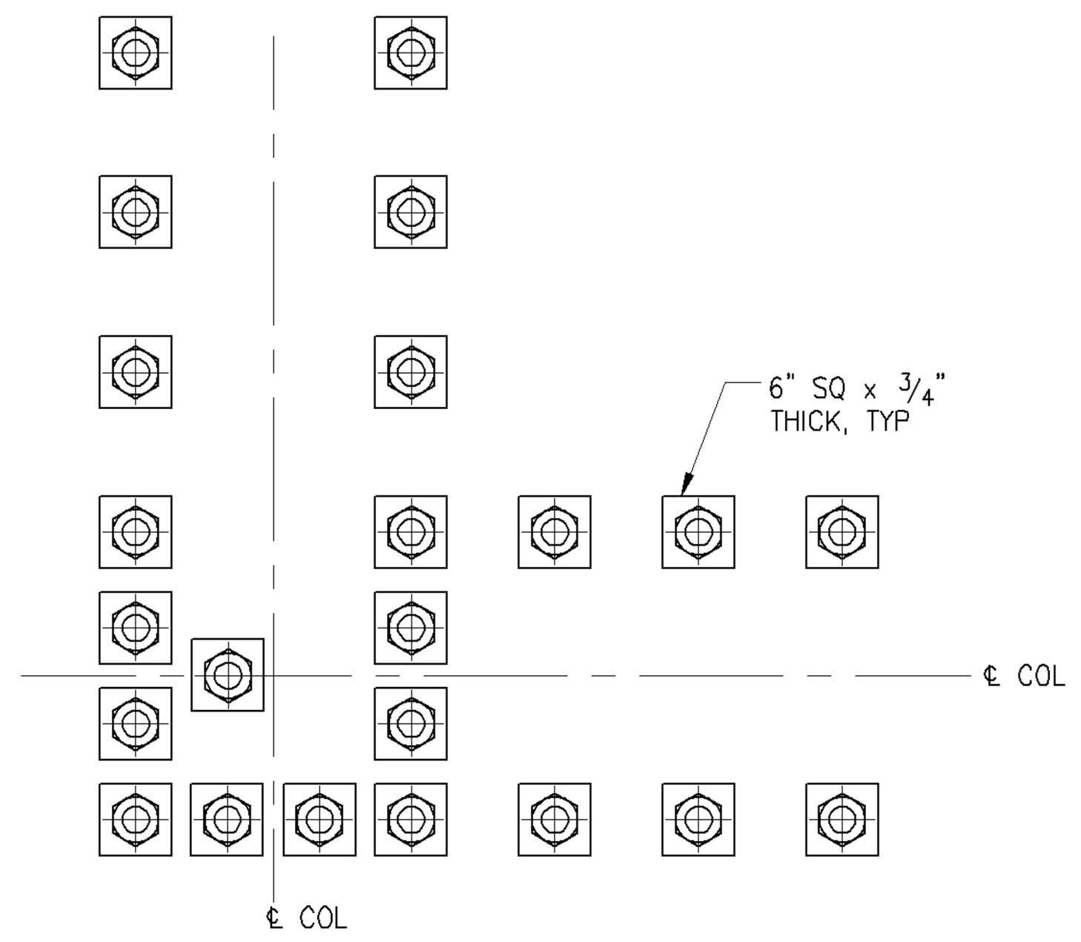
H SECTION



G SECTION



I SECTION



D PLAN - BEARING PLATES

CONNECTION SCHEDULE M																					
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)						
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7	
M1 (ELV A)	W14x132	10	1 1/2	1 1/2	3/4	3/4	3/4	3/4	1/2	3/4	1/2	5/16	5/16	5/8	28	17	17	17	43	66	
M1 (ELV B)	W14x132	–	1 3/4	1 1/2	1	–	–	3/4	5/8	3/8	5/8	5/16	3/4	3/4	28	17	28	17	47	46	

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE M) 3/4" = 1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
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LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
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REGISTRATION

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1	10-23-06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

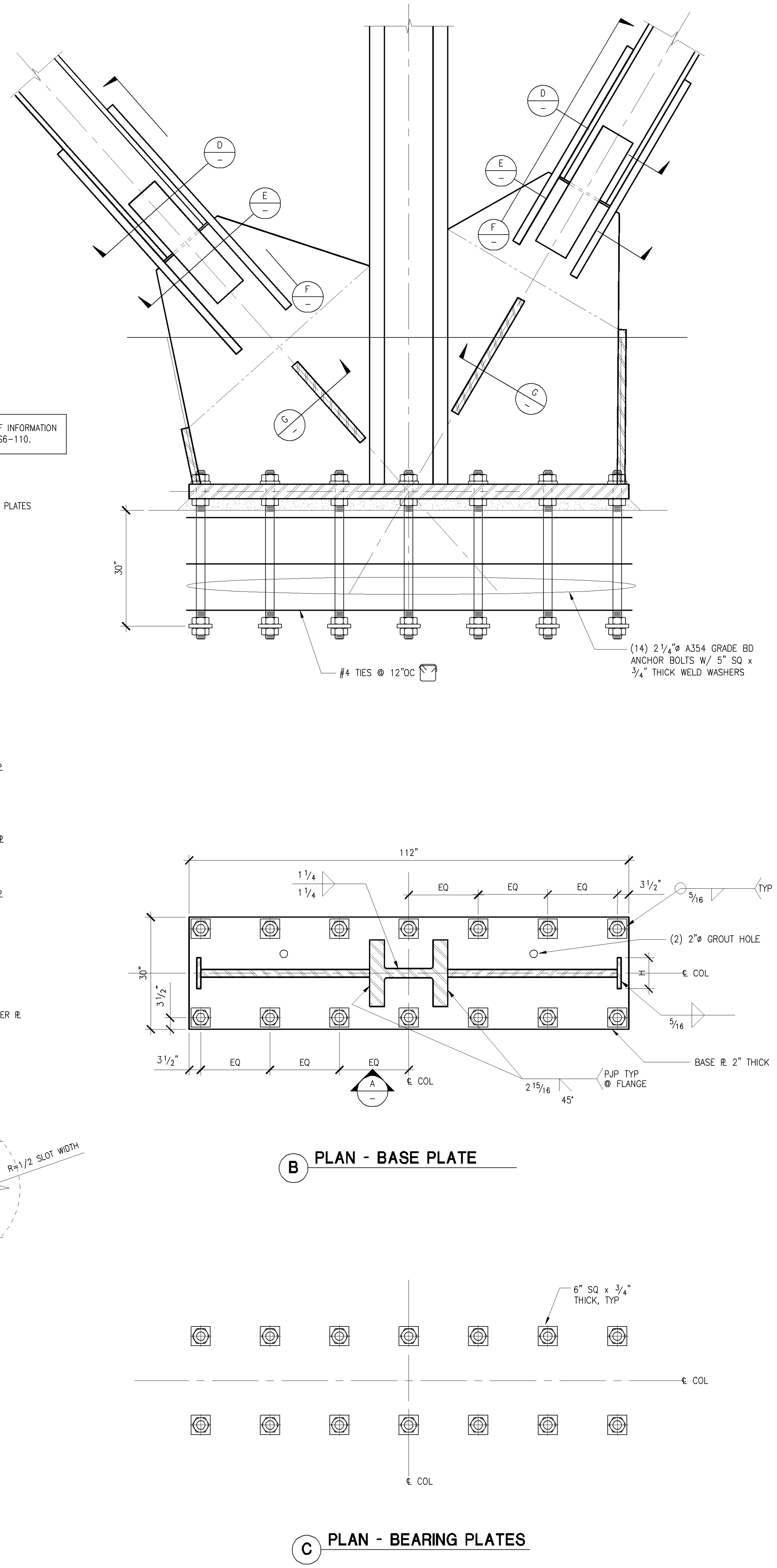
KEY PLAN

SHEET TITLE
BRACED FRAME DETAILS

S6-109

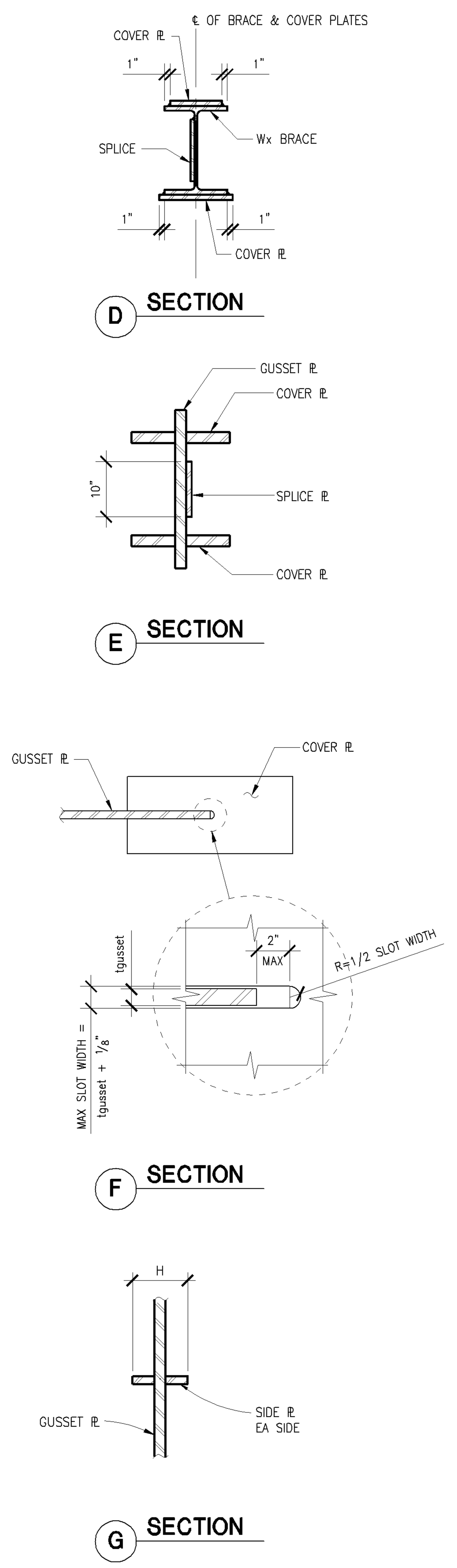
BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE P)

NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.



B PLAN - BASE PLATE

C PLAN - BEARING PLATES



D SECTION

E SECTION

F SECTION

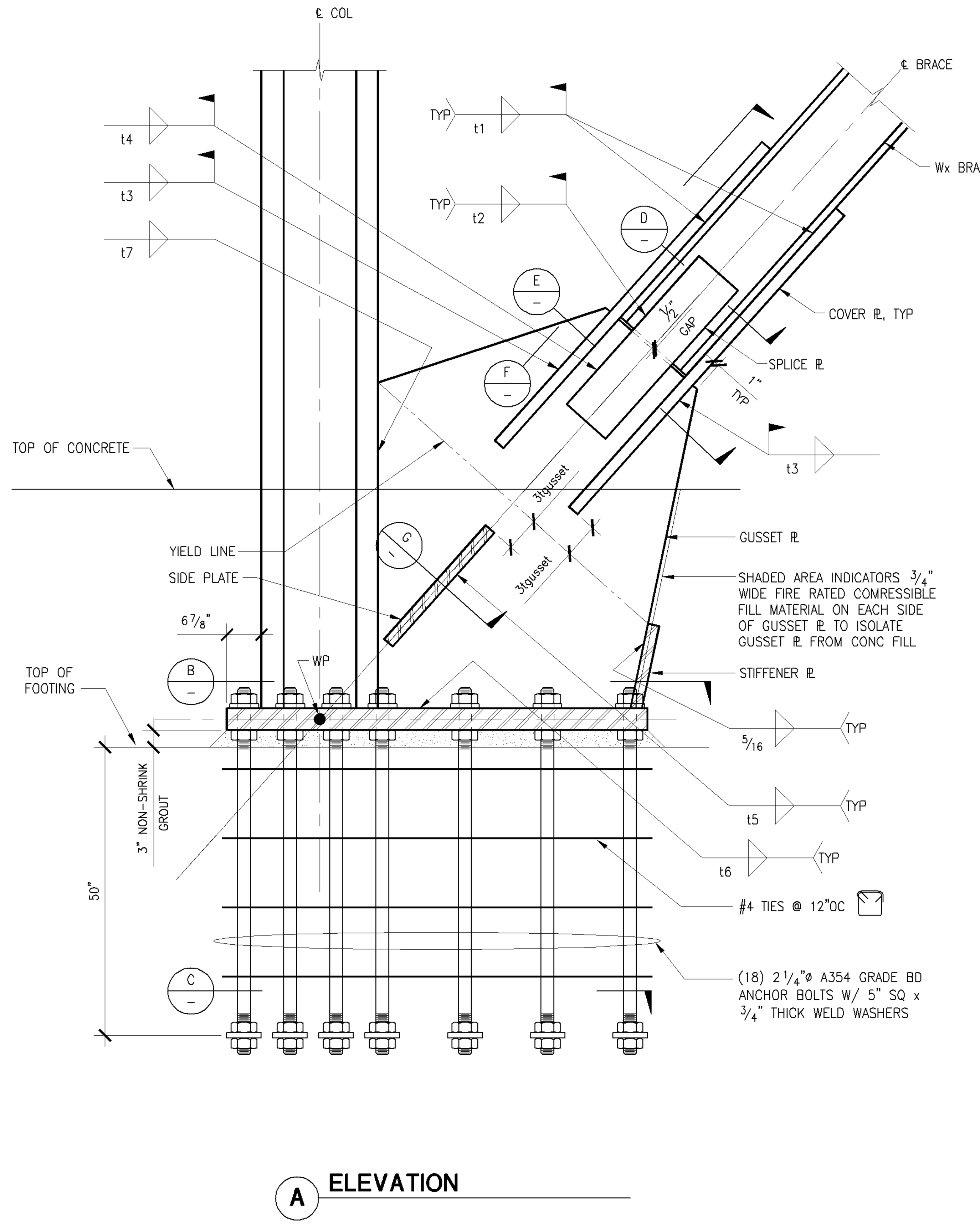
G SECTION

CONNECTION SCHEDULE N																					
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)						
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
P1	W14x176	–	2	1 3/4	1	–	–	3/4	5/8	1/2	5/8	5/16	3/4	7/8	35	17	24	17	40	53
P2	W14x176	10	2	1 3/4	1	3/4	1	3/4	5/8	3/4	5/8	5/16	1/2	7/8	32	17	18	17	42	64
P3	W14x176	–	2	1 3/4	1	–	–	3/4	5/8	1/2	5/8	5/16	3/4	7/8	35	17	24	17	39	51
P4	W14x176	10	2	1 3/4	1	3/4	1	3/4	5/8	3/4	5/8	5/16	1/2	7/8	32	17	18	17	40	61

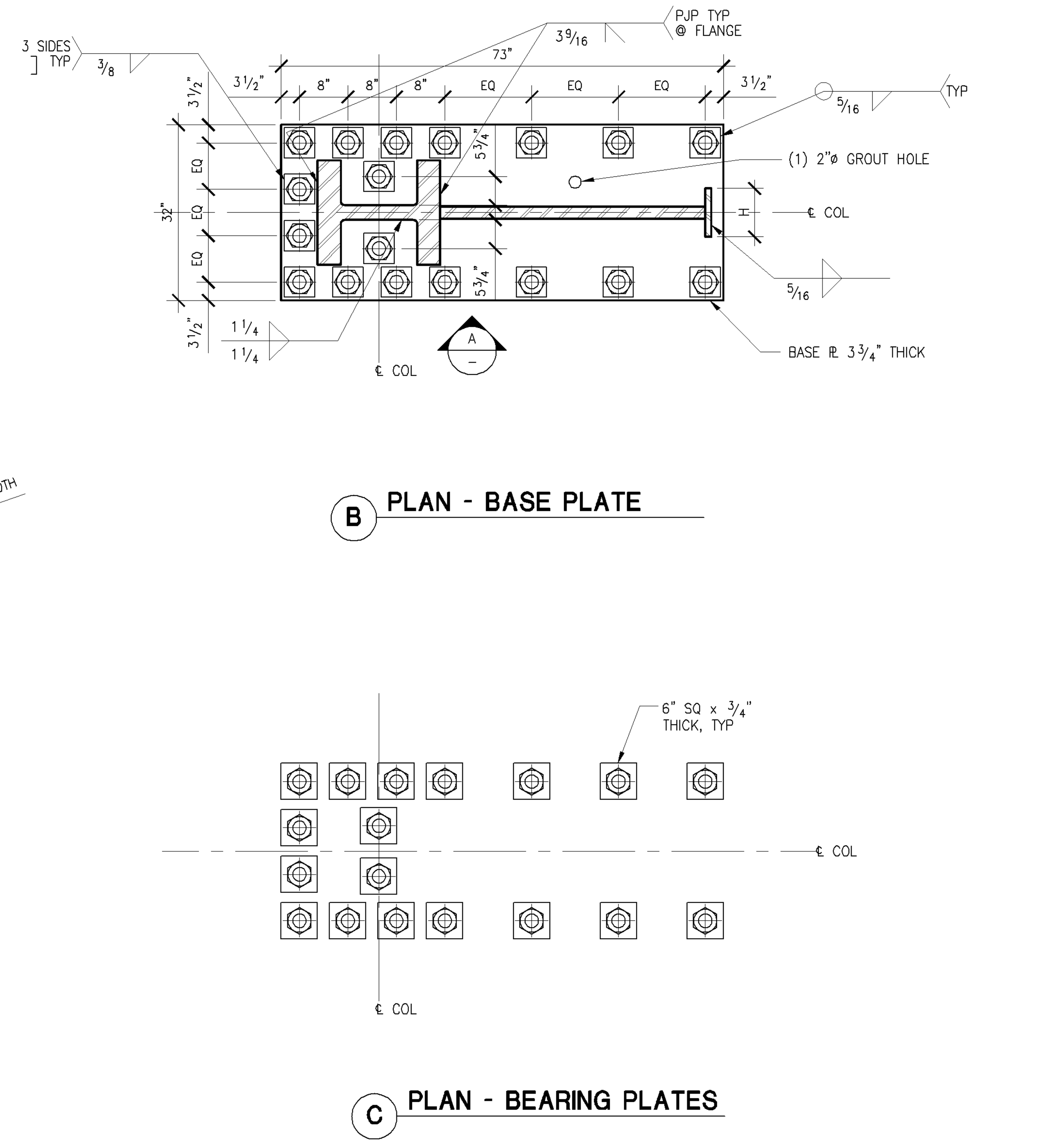
BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE P) 3/4" = 1'-0" 2

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE O)

NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.

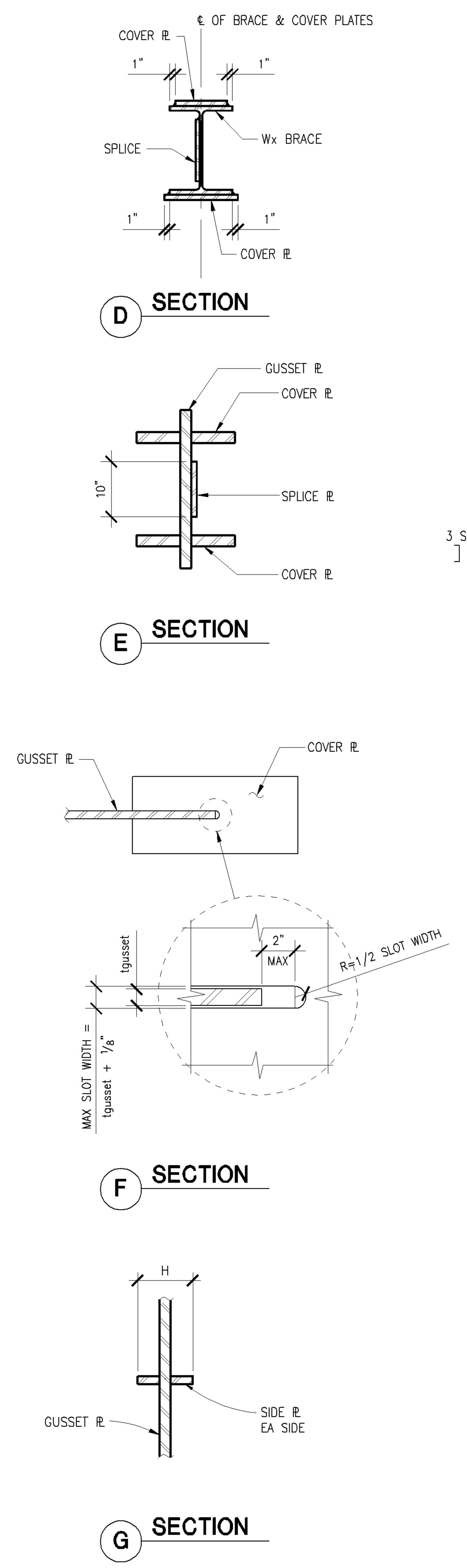


A ELEVATION



B PLAN - BASE PLATE

C PLAN - BEARING PLATES



D SECTION

E SECTION

F SECTION

G SECTION

CONNECTION SCHEDULE M																					
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)						
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
O1	W14x176	–	2	1 3/4	1	–	–	3/4	5/8	1/2	5/8	5/16	3/4	7/8	35	17	26	17	40	53
O2	W14x176	–	2	1 3/4	1	–	–	3/4	5/8	1/2	5/8	5/16	3/4	7/8	35	17	26	17	39	51

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE O) 3/4" = 1'-0" 1

OWNER/CLIENT
Abbott Vascular
TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT
DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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TORRANCE, CA 90503

REGISTRATION

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1	10-23-06	BLDG & SAFETY SUBMITTAL

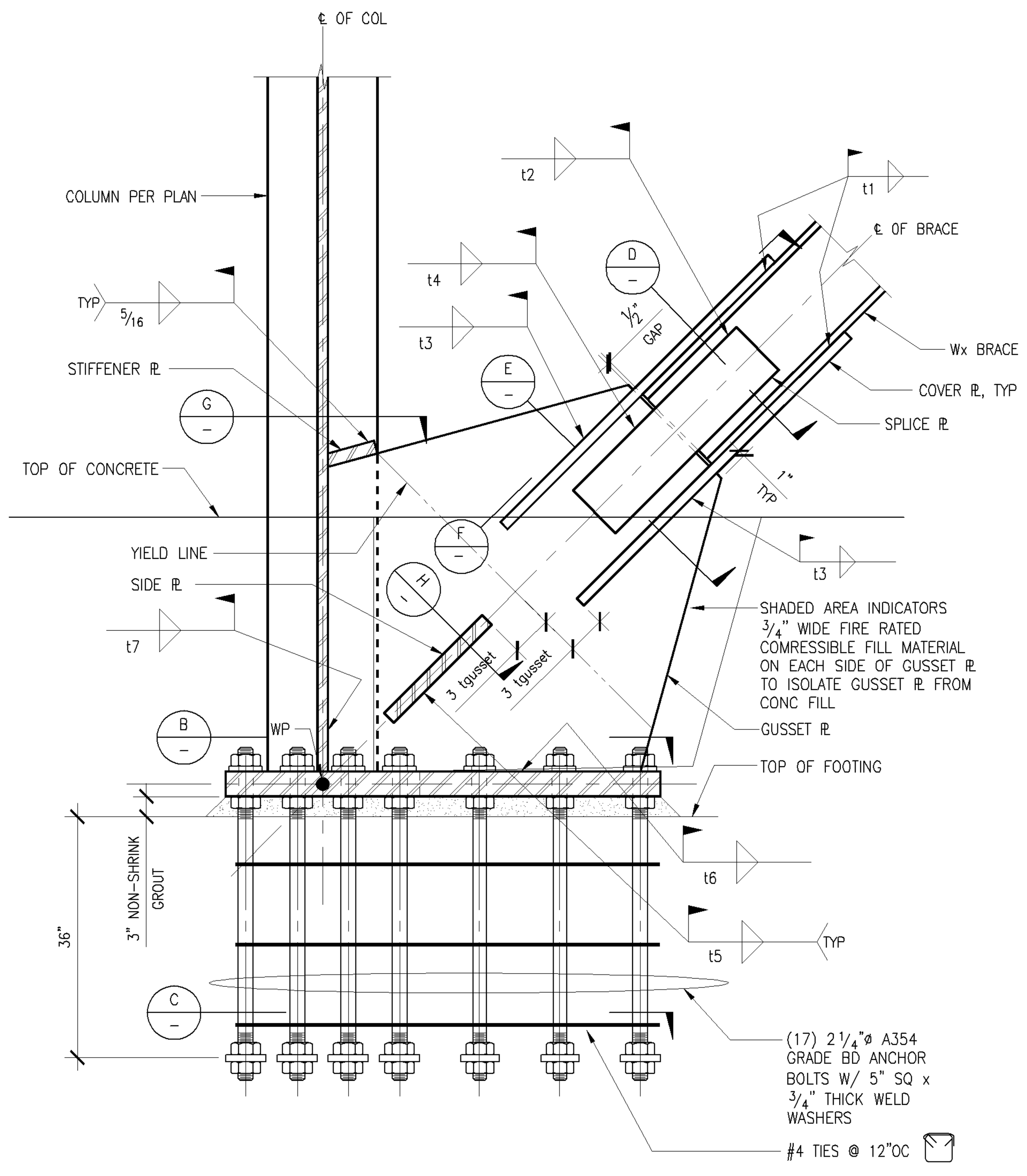
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

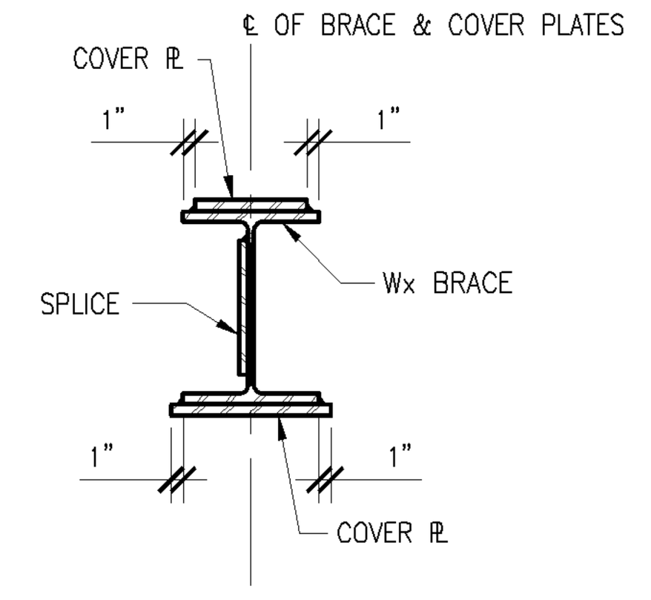
SHEET TITLE
BRACED FRAME DETAILS

S6-110

NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.



A ELEVATION



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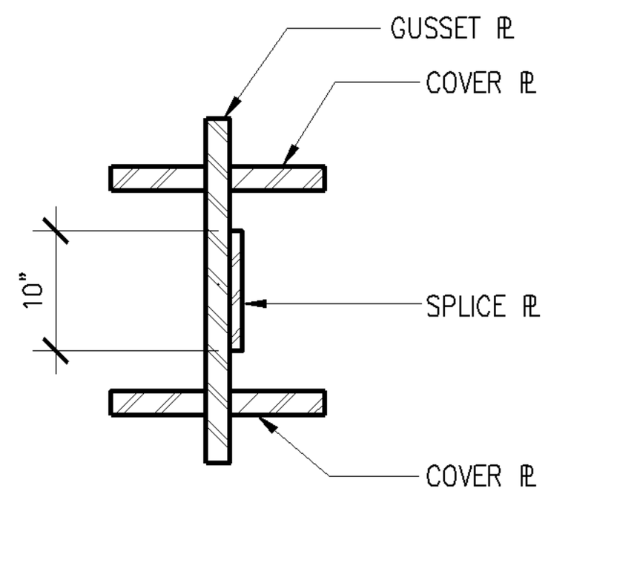
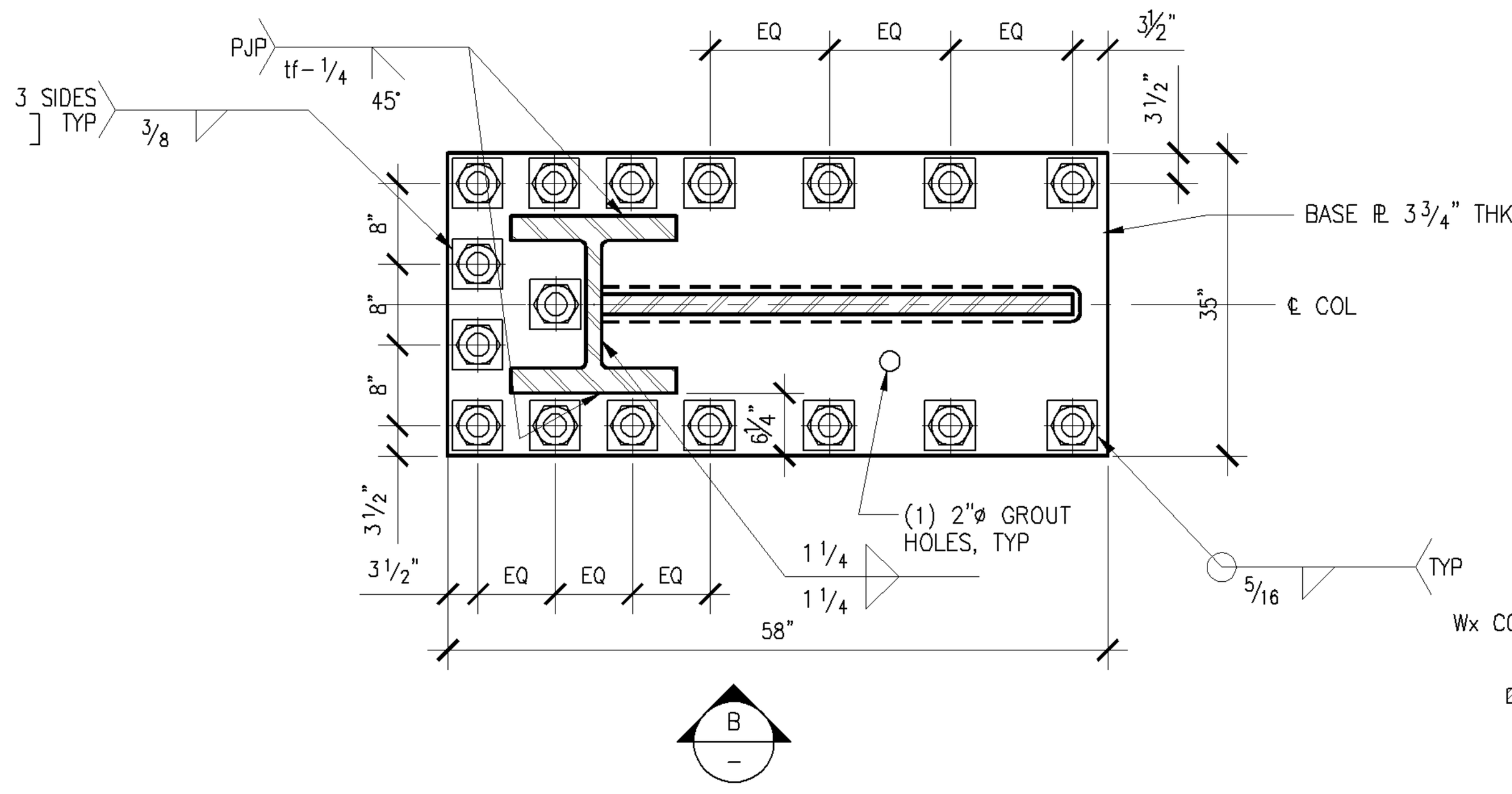
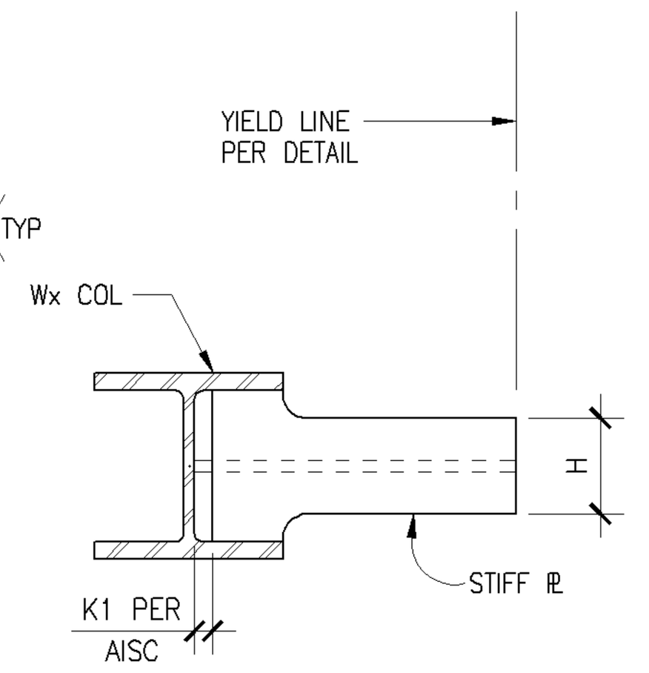


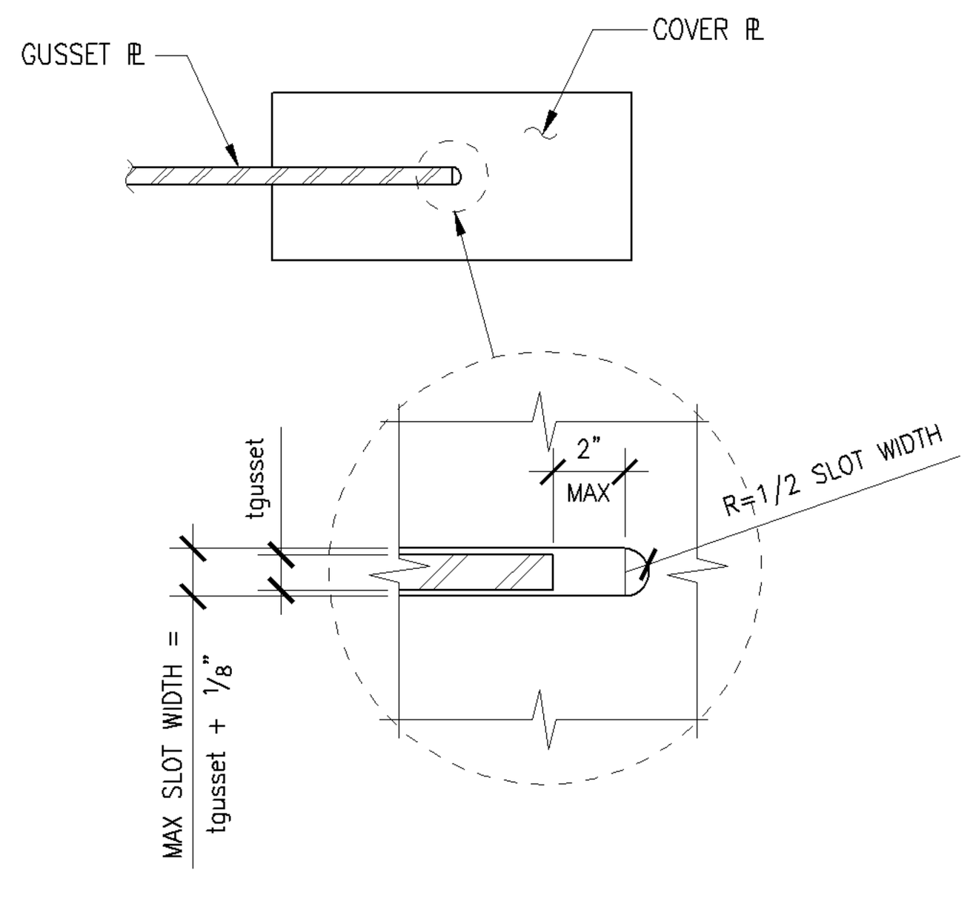
E SECTION



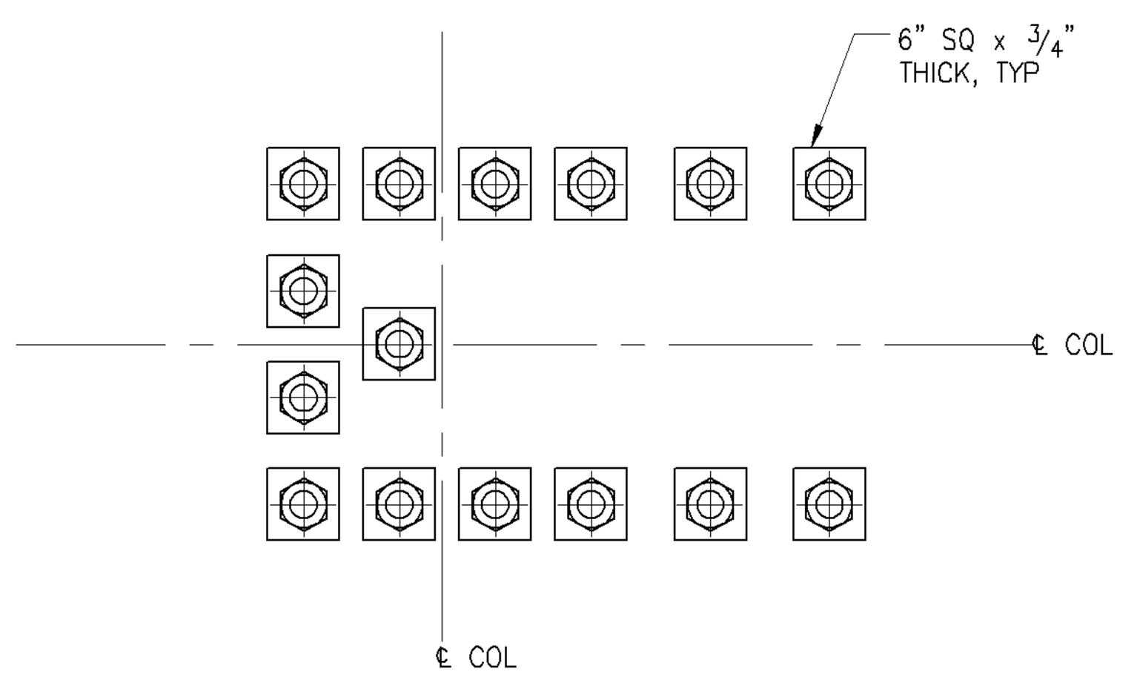
B PLAN - BASE PLATE



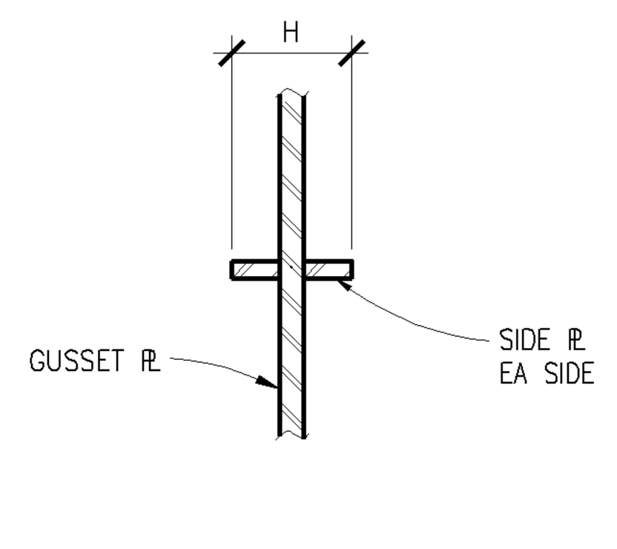
G SECTION



F SECTION



C PLAN - BEARING PLATES



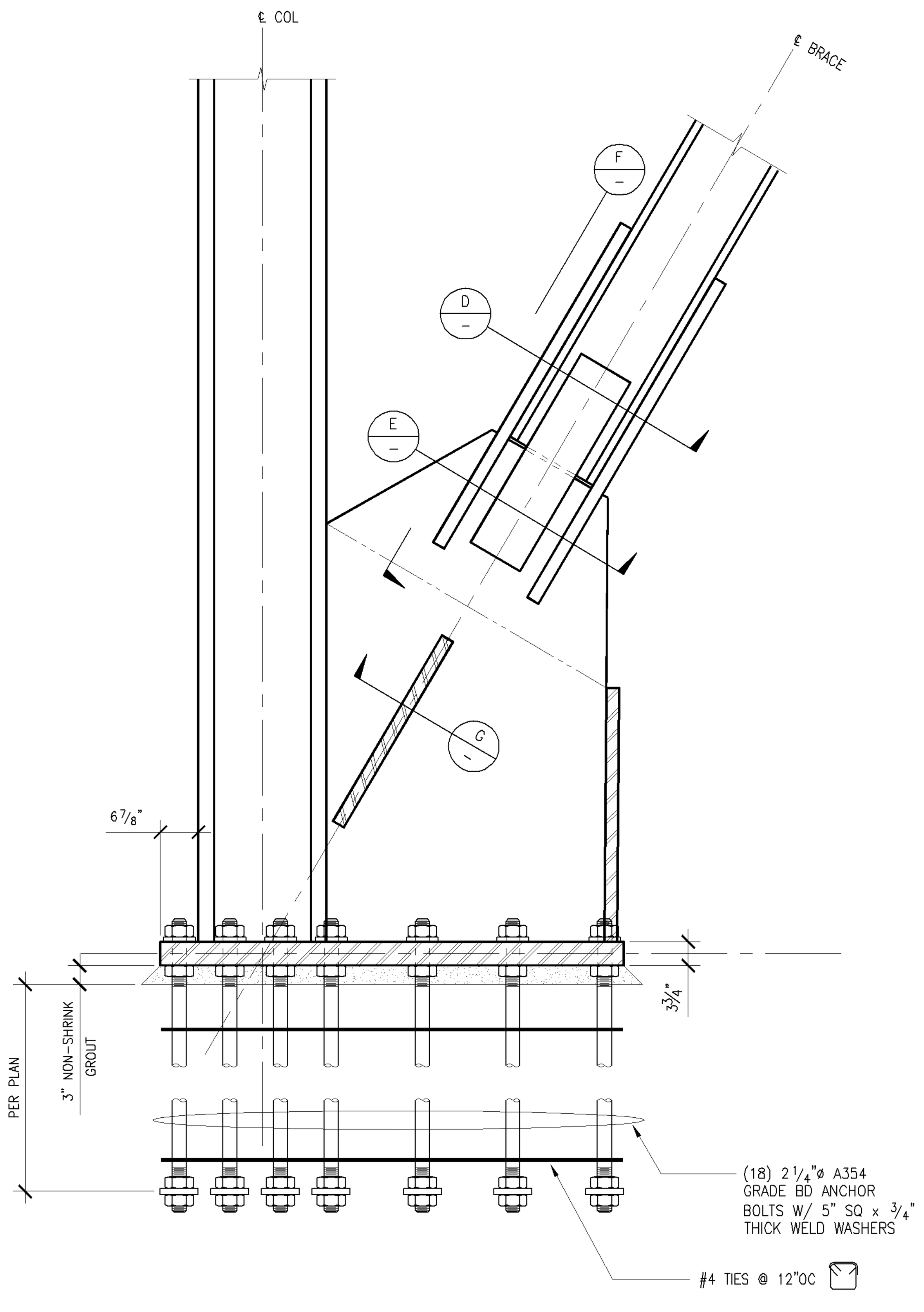
H SECTION

CONNECTION SCHEDULE R																				
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)					
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
R1	W14x120	–	1 3/4	1 1/4	1/2	–	–	3/4	1/2	5/8	1/2	5/16	7/8	7/8	21	17	17	17	38	37
R2	W14x120	–	1 3/4	1 1/4	1/2	–	–	3/4	1/2	5/8	1/2	5/16	7/8	7/8	21	17	17	17	38	38

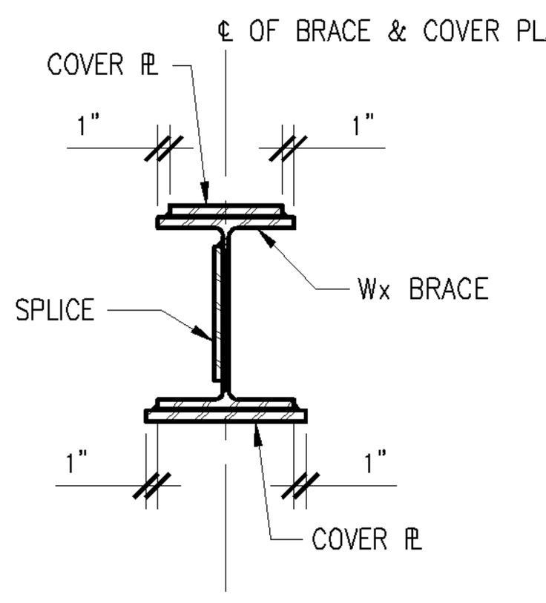
BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE R) 3/4" = 1'-0" 2

NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.

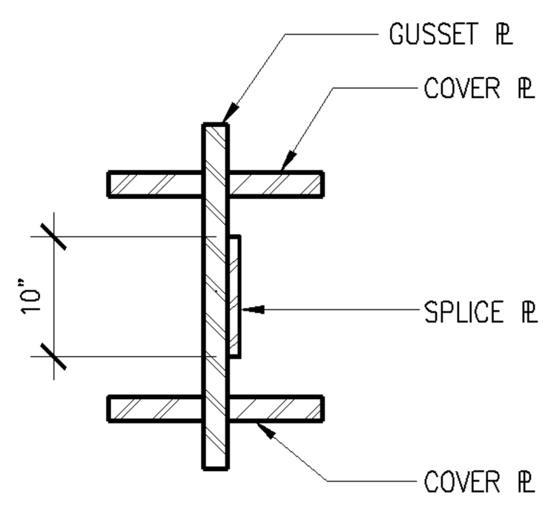
FOR BALANCE OF INFORMATION SEE DETAIL 1A/S6-110.



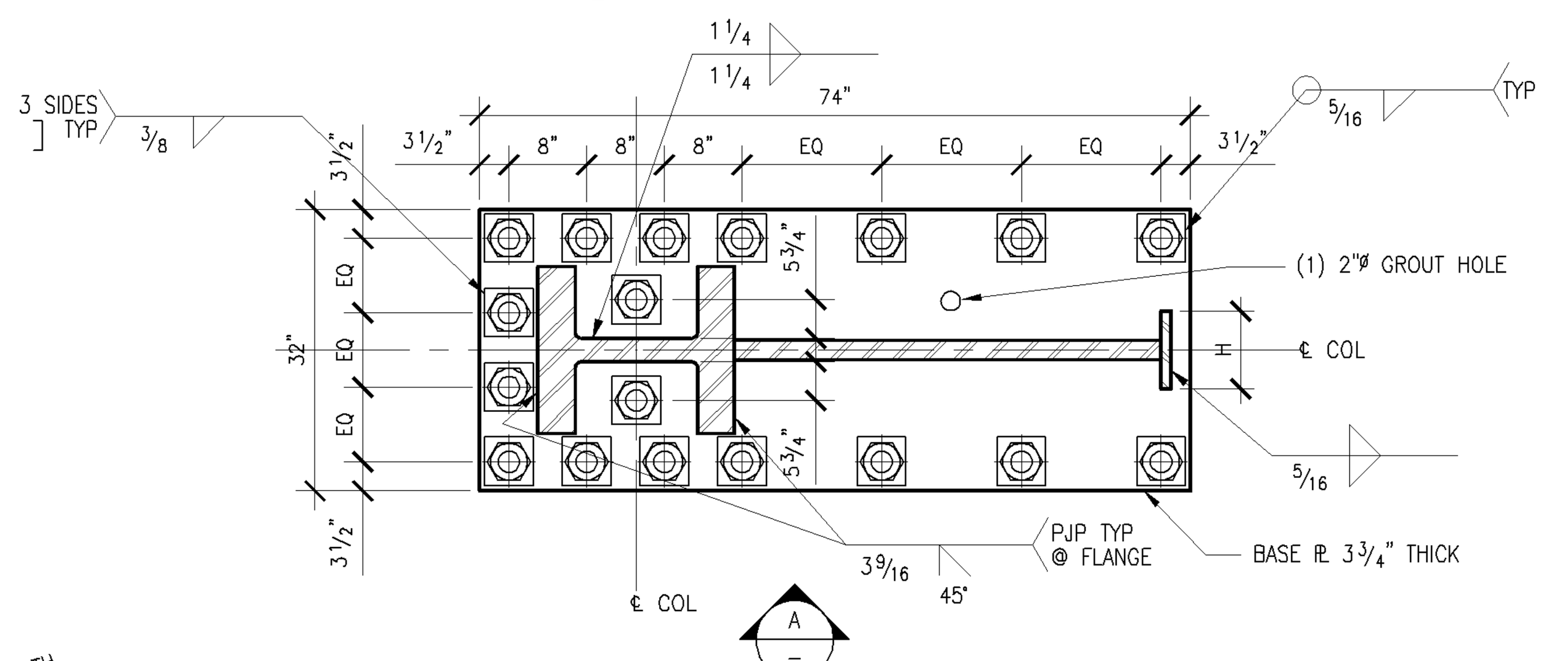
A ELEVATION



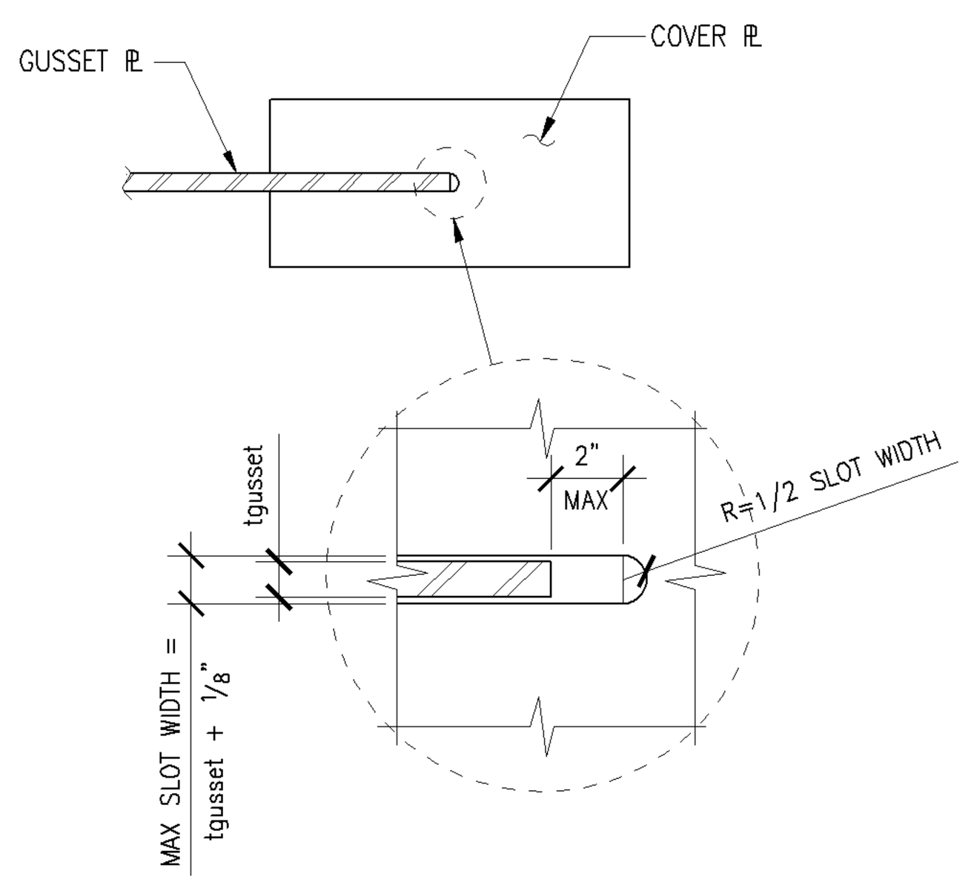
D SECTION



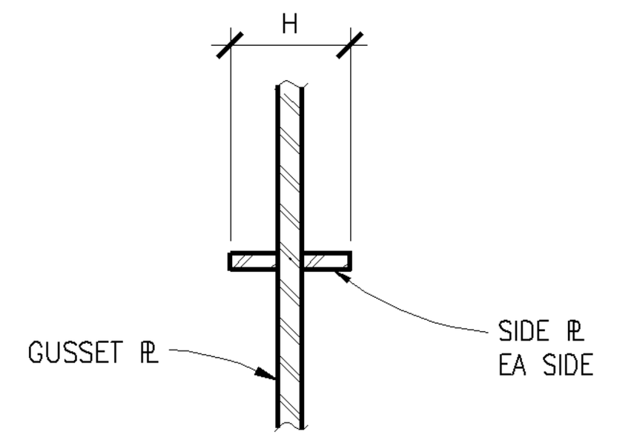
E SECTION



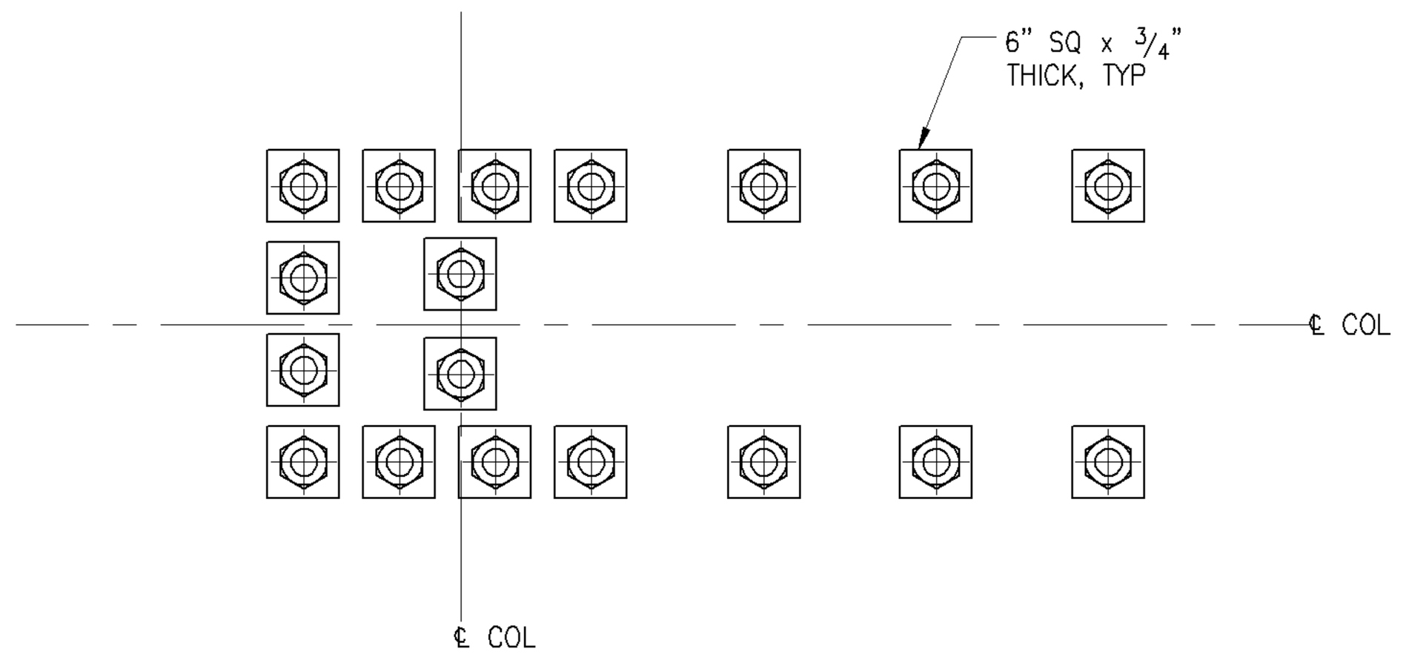
B PLAN - BASE PLATE



F SECTION



G SECTION



C PLAN - BEARING PLATES

CONNECTION SCHEDULE Q																				
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)					
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
Q1	W14x176	10	2	1 3/4	1	3/4	1	3/4	5/8	3/4	5/8	5/16	1/2	7/8	32	17	18	17	42	64
Q2	W14x176	10	2	1 3/4	1	3/4	1	3/4	5/8	3/4	5/8	5/16	1/2	7/8	32	17	18	17	40	61

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE Q) 3/4" = 1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
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CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

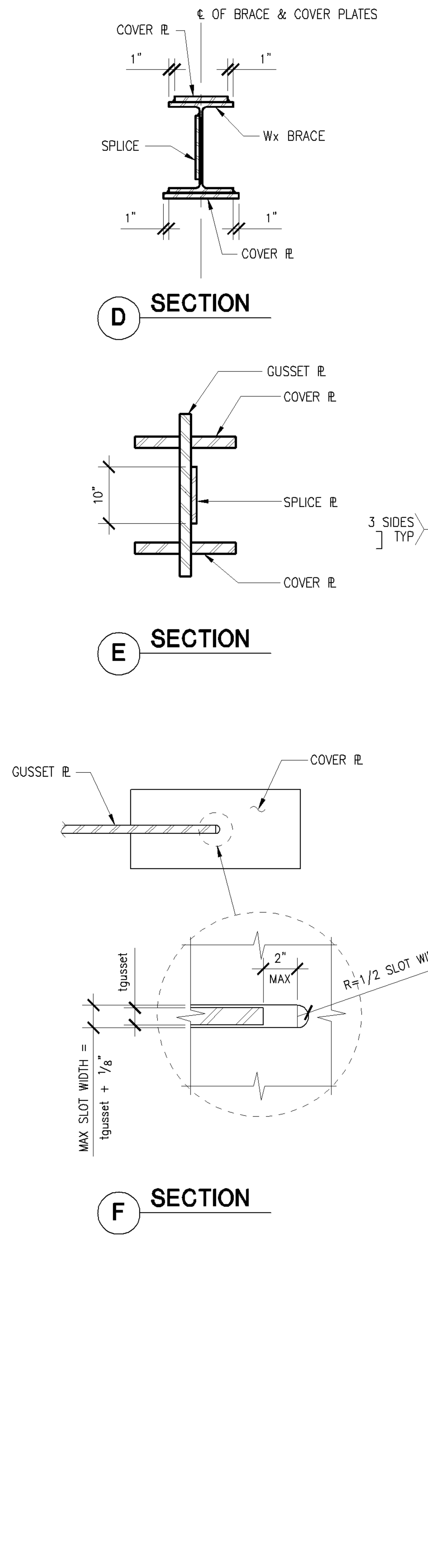
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

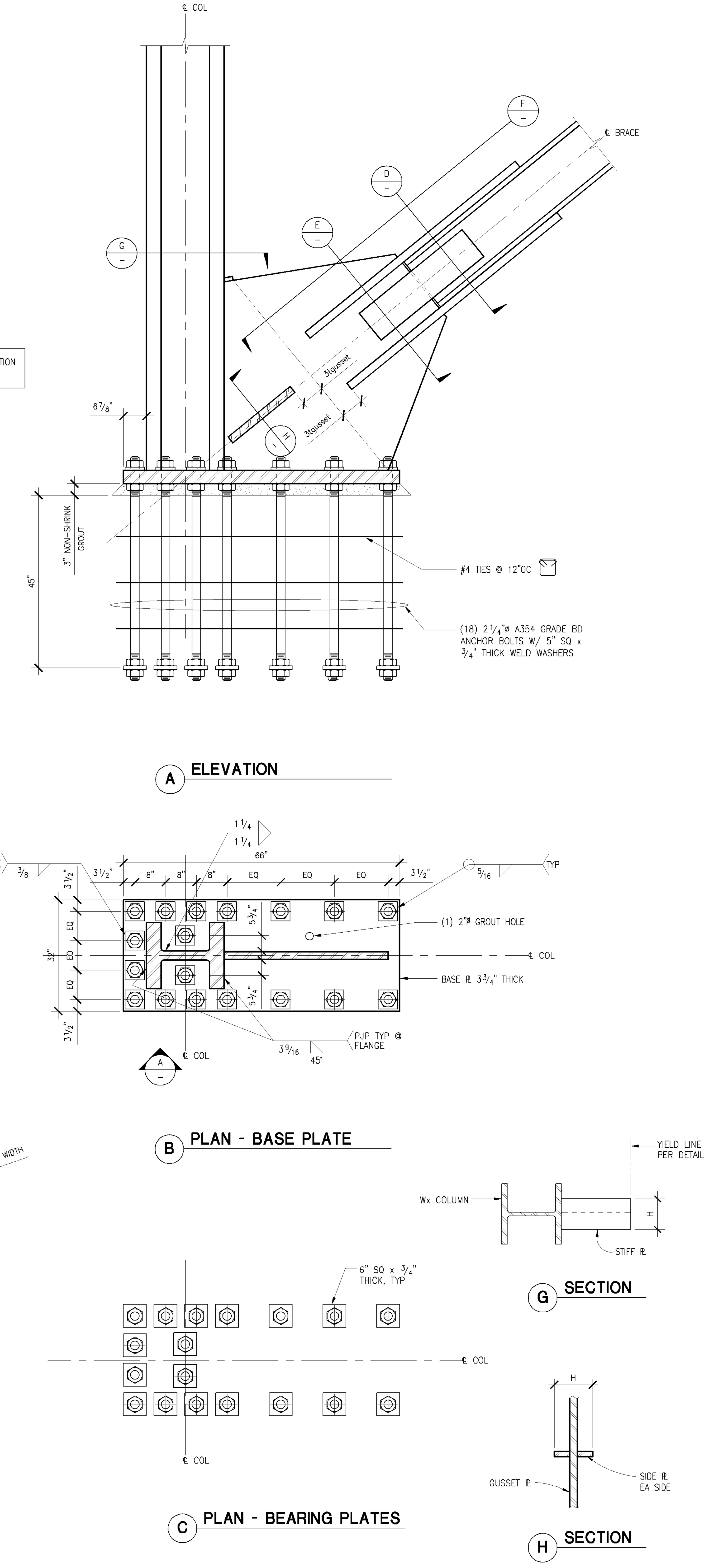
KEY PLAN

SHEET TITLE
BRACED FRAME DETAILS

S6-111

NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.

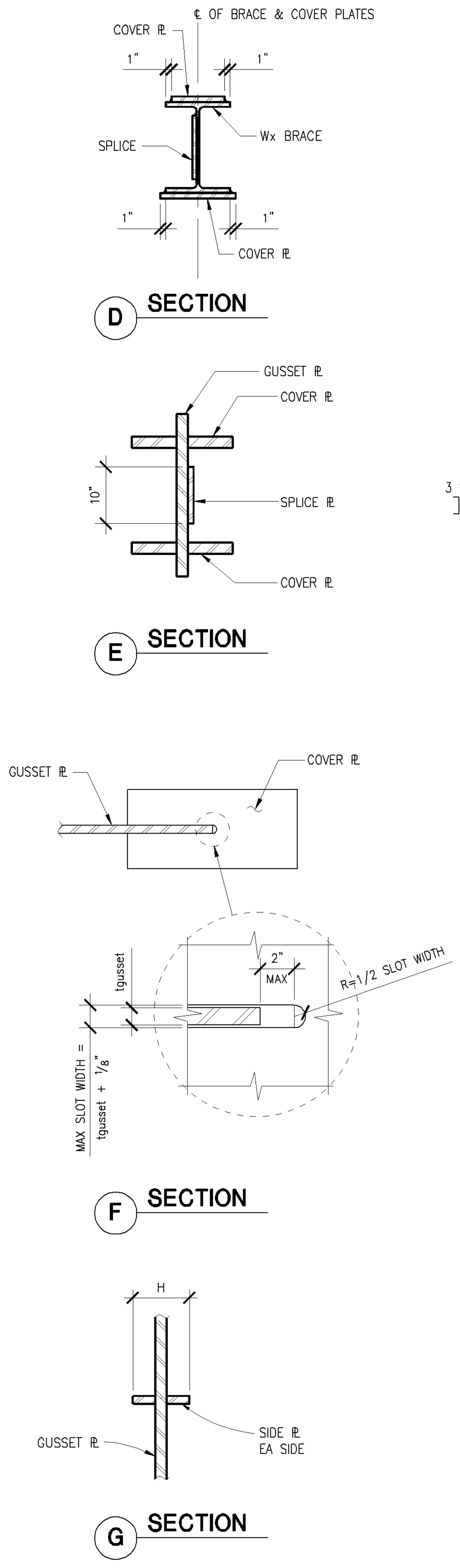


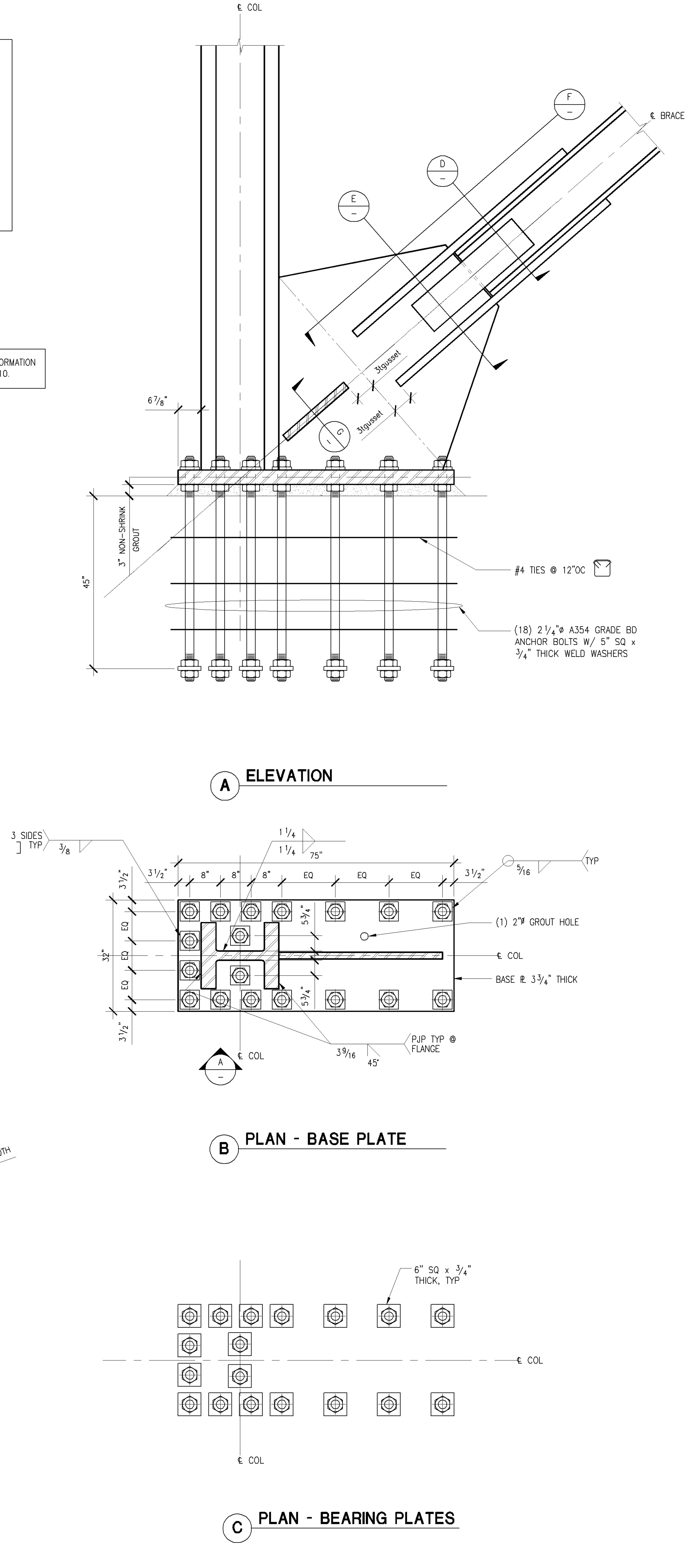


CONNECTION SCHEDULE T																				
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)					
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
T1	W14x176	–	2	1 1/2	1	–	–	3/4	5/8	5/8	5/8	5/16	7/8	7/8	31	17	22	17	34	41
T2	W14x176	–	2	1 1/2	1	–	–	3/4	5/8	5/8	5/8	5/16	7/8	7/8	31	17	22	17	34	41

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE T) 3/4" = 1'-0" 2

NOTES:
1. ALL PLATE MATERIAL TO BE ASTM A572 GR50.
2. GUSSET PLATE AND SIDE PLATE THICKNESS PER THE CONNECTION SCHEDULE.
3. SEE S6-100 SIM FOR DIMENSIONS LT THRU L7
4. SEE S6-100 FOR SHOP DRAWING SUBMITTAL REQUIREMENTS.
5. ALL CONNECTION ELEMENTS SHALL BE WELDED ALONG THE FULL PLATE INTERFACE LENGTH INDICATED IN THE SCHEDULE.
6. SEE 3/S6-103 FOR DEEPENED FOOTING AT ANCHOR BOLTS IF REQUIRED.





CONNECTION SCHEDULE S																				
CONNX ID	BRACE SIZE	H (IN)	PLATE THICKNESS (IN)					WELD THICKNESS (IN)							PLATE INTERFACE LENGTH (IN)					
			GUSSET	COVER	SPLICE	SIDE	STIFF	t1	t2	t3	t4	t5	t6	t7	L1	L2	L3	L4	L6	L7
S1	W14x132	–	1 3/4	2	1	–	–	3/4	5/8	1/2	5/8	5/16	7/8	7/8	39	17	33	17	43	50
S2	W14x132	–	1 3/4	2	1	–	–	3/4	5/8	1/2	5/8	5/16	7/8	7/8	39	17	33	17	43	50

BRACED FRAME FOUNDATION CONNECTION SCHEDULE AND DETAILS (TYPE S) 3/4" = 1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
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LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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ESCONDIDO, CA 92029

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PASADENA, CA 91101

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2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER
ALDRIN J. ...
No. S4399
Exp. 12/31/07
STRUCTURAL
STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
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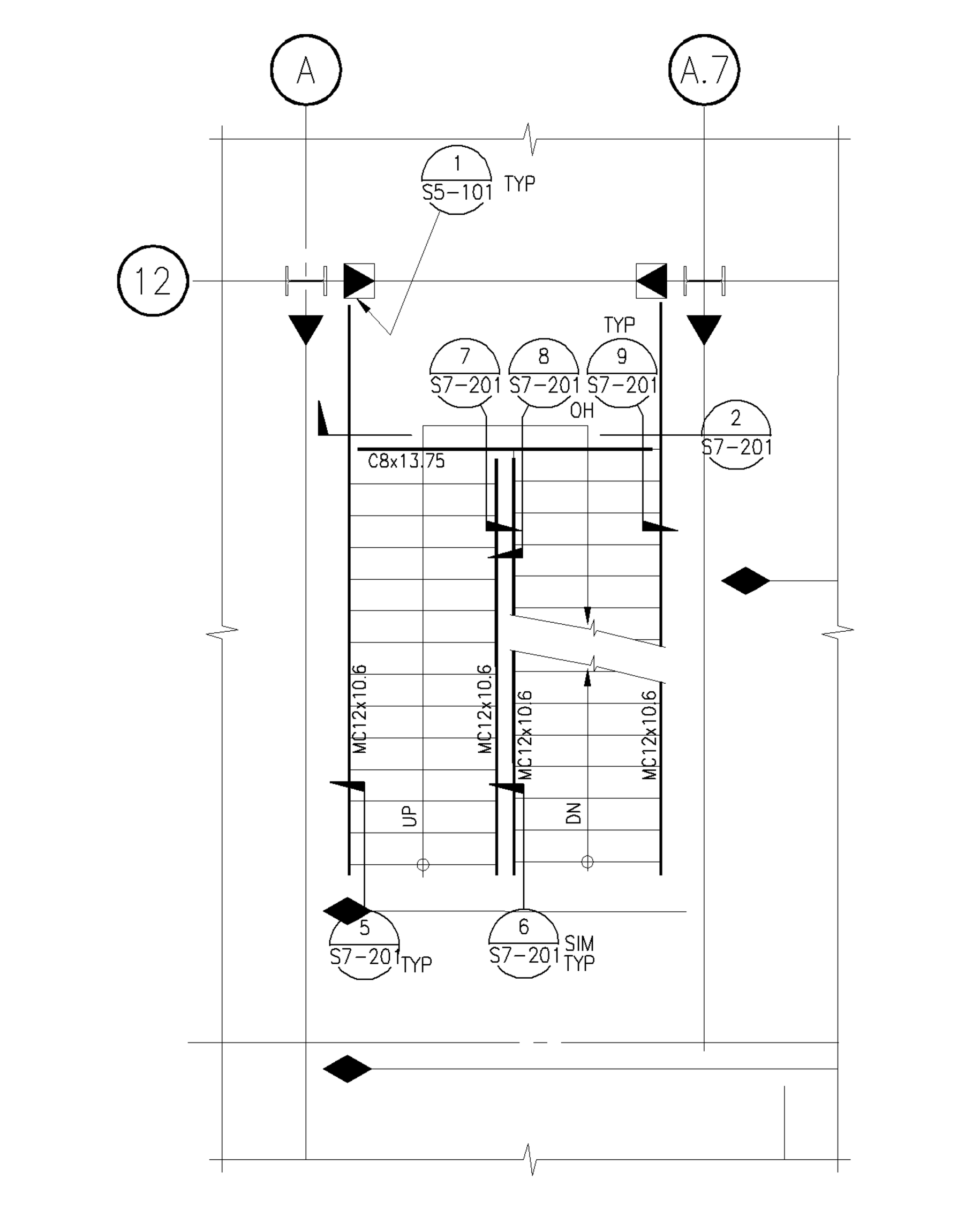
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

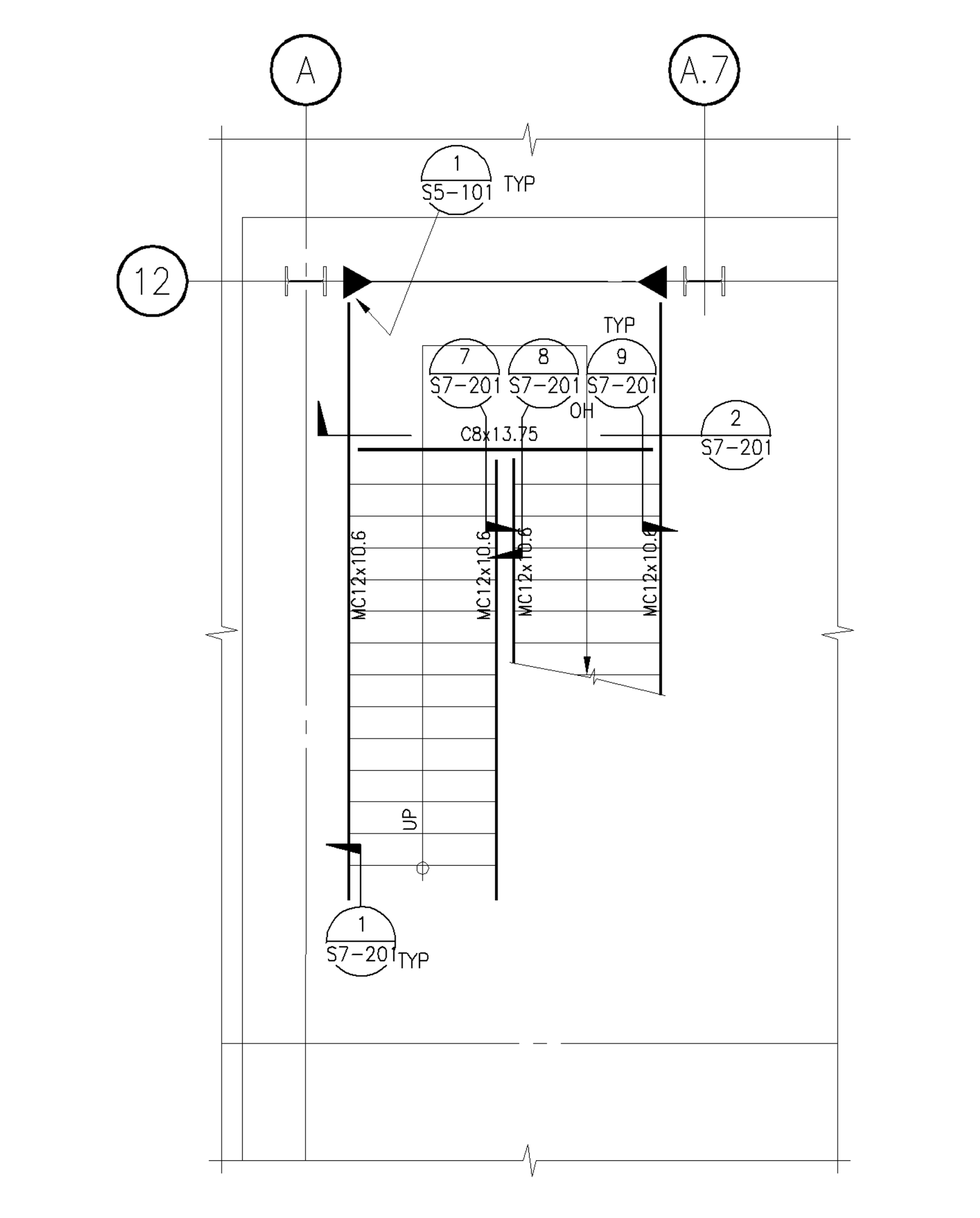
SHEET TITLE

BRACED FRAME DETAILS

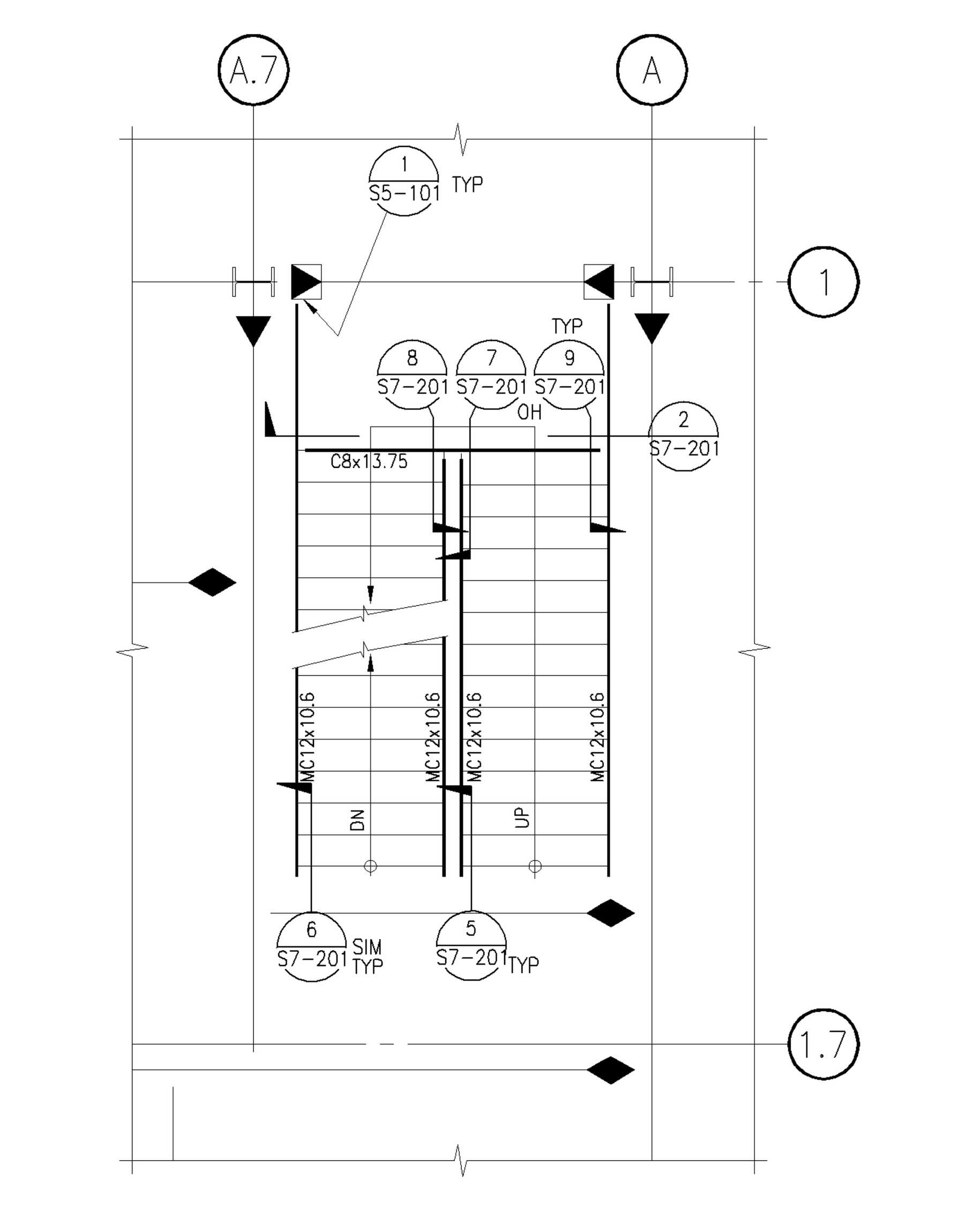
S6-112



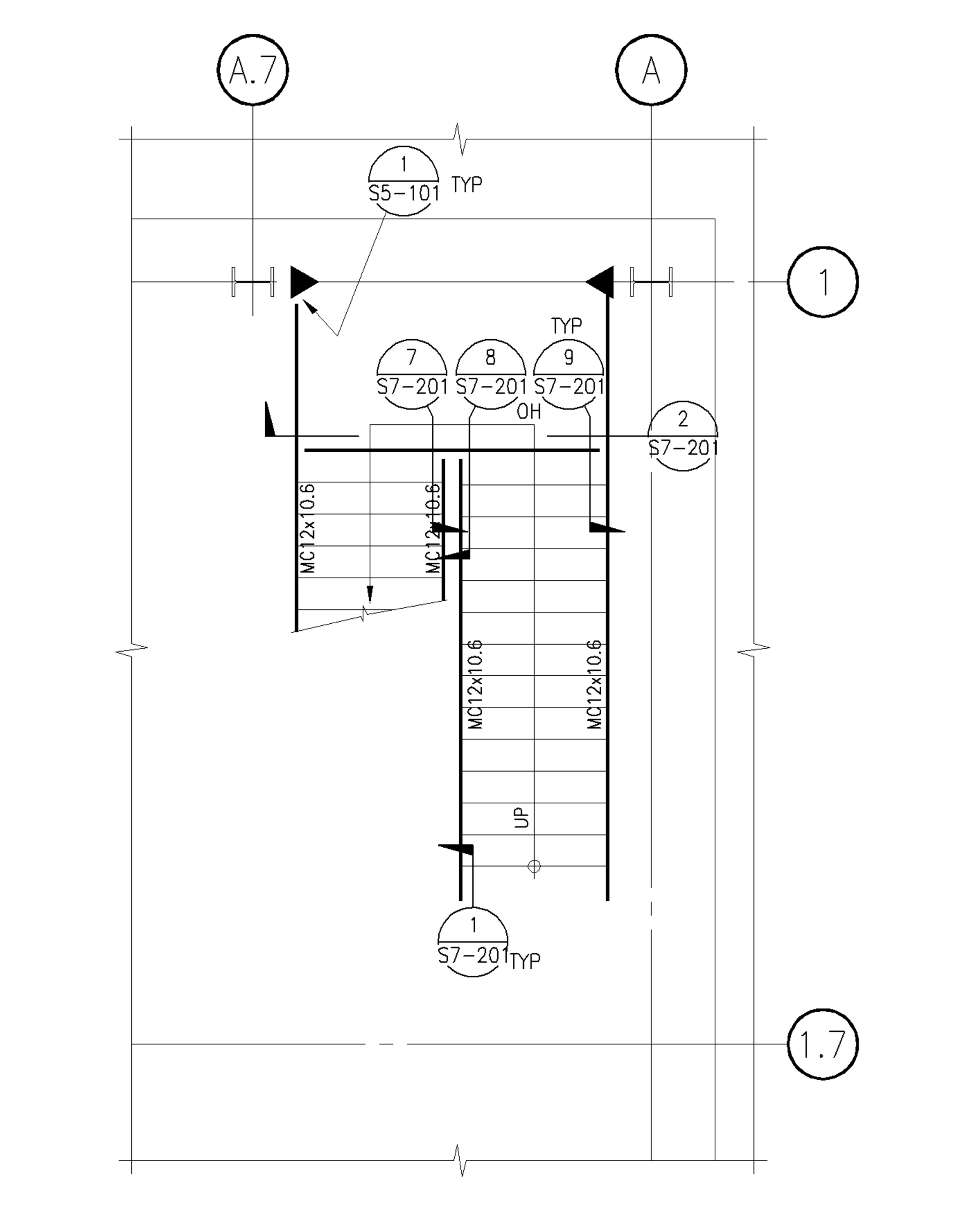
STAIR #2 - LEVEL 4 1/4"=1'-0" 10



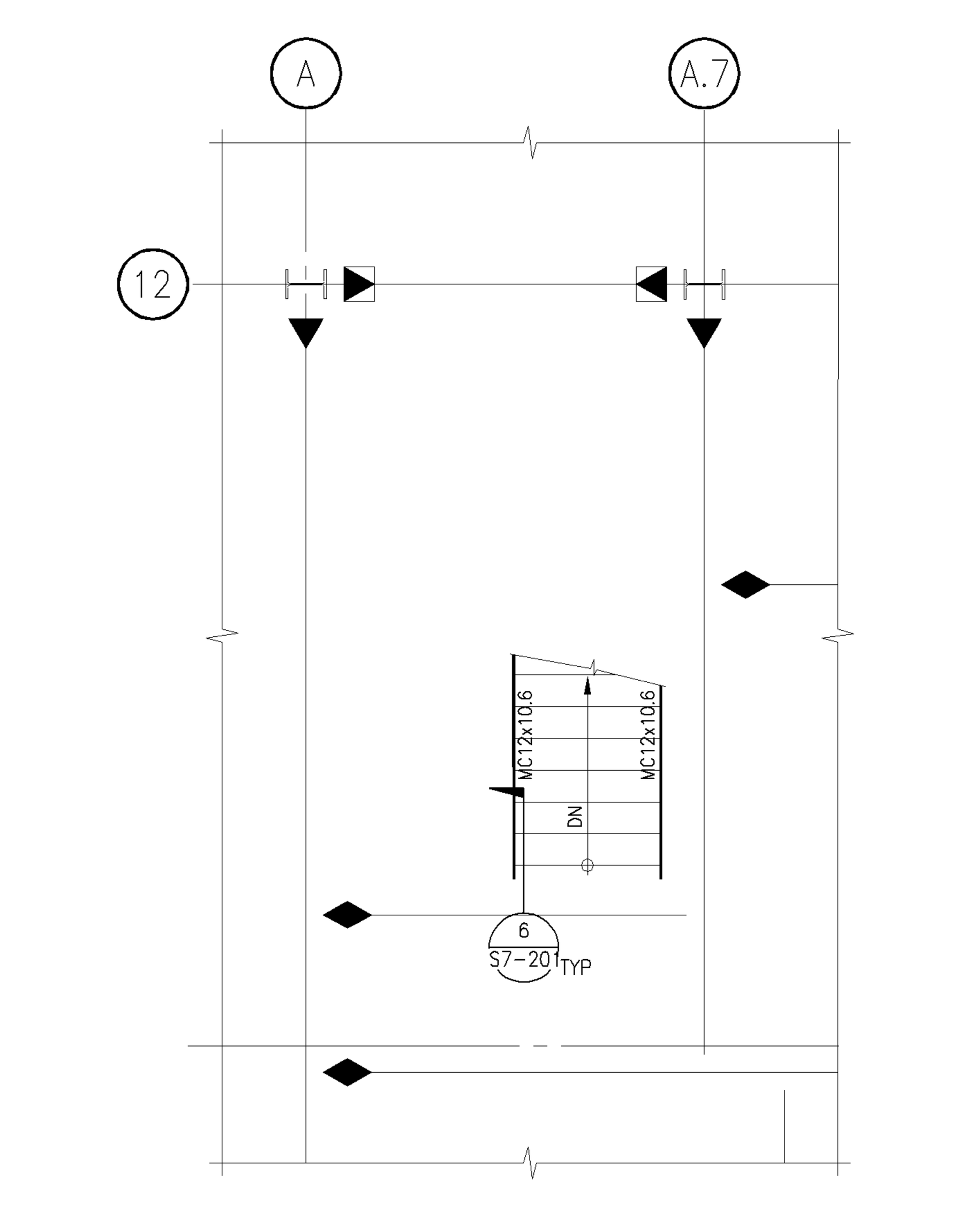
STAIR #2 - LEVEL 1 1/4"=1'-0" 7



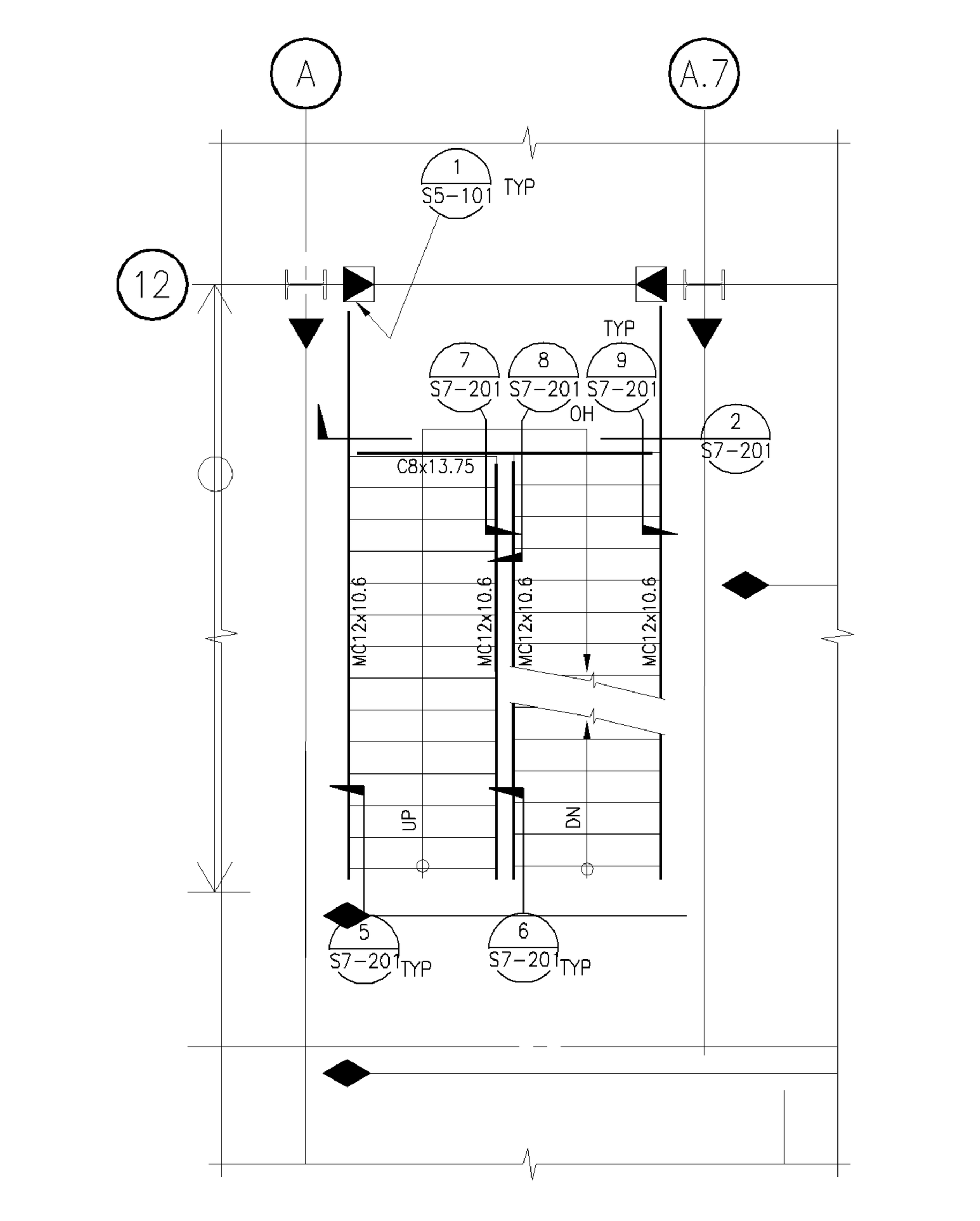
STAIR #1 - LEVEL 4 1/4"=1'-0" 4



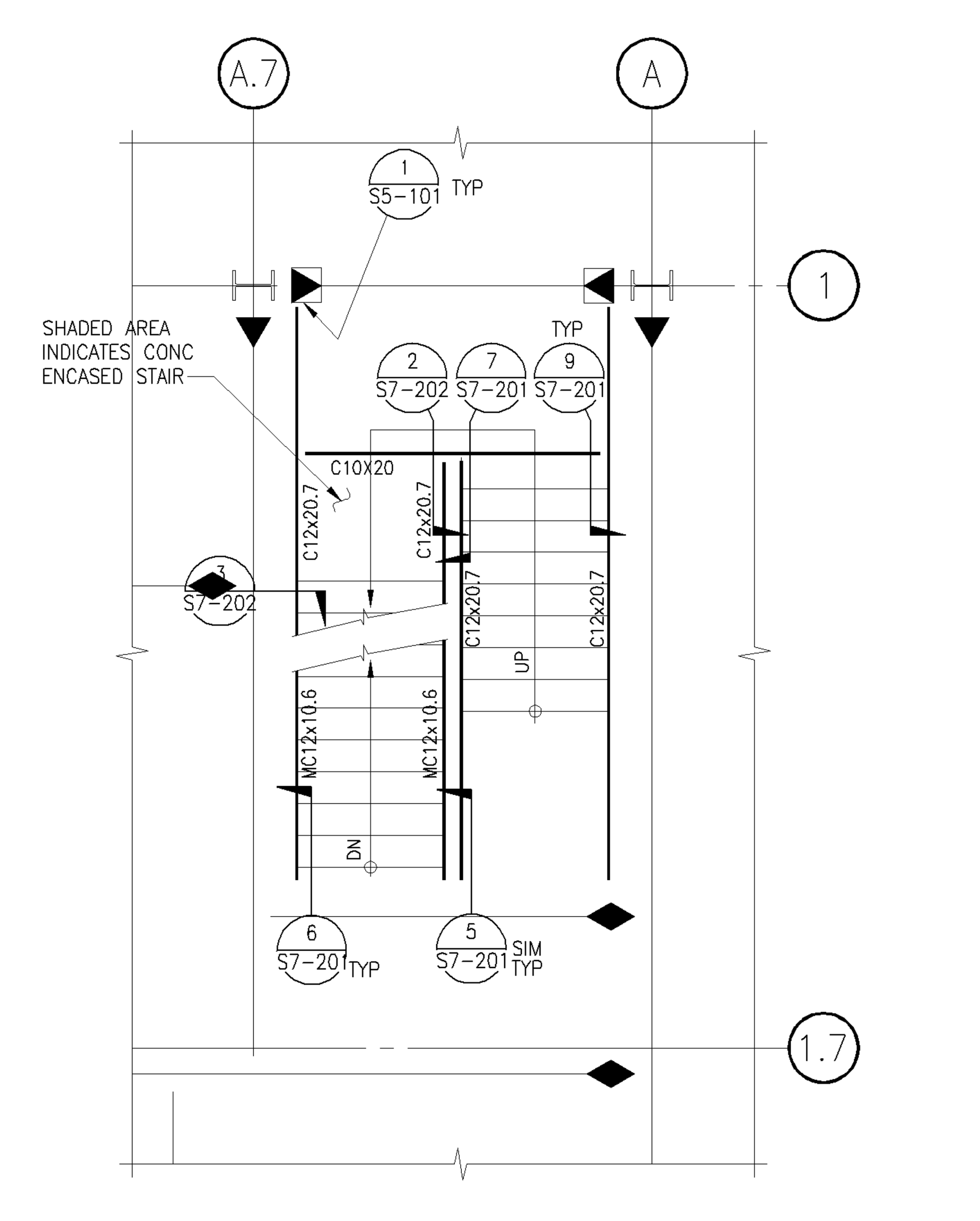
STAIR #1 - LEVEL 1 1/4"=1'-0" 1



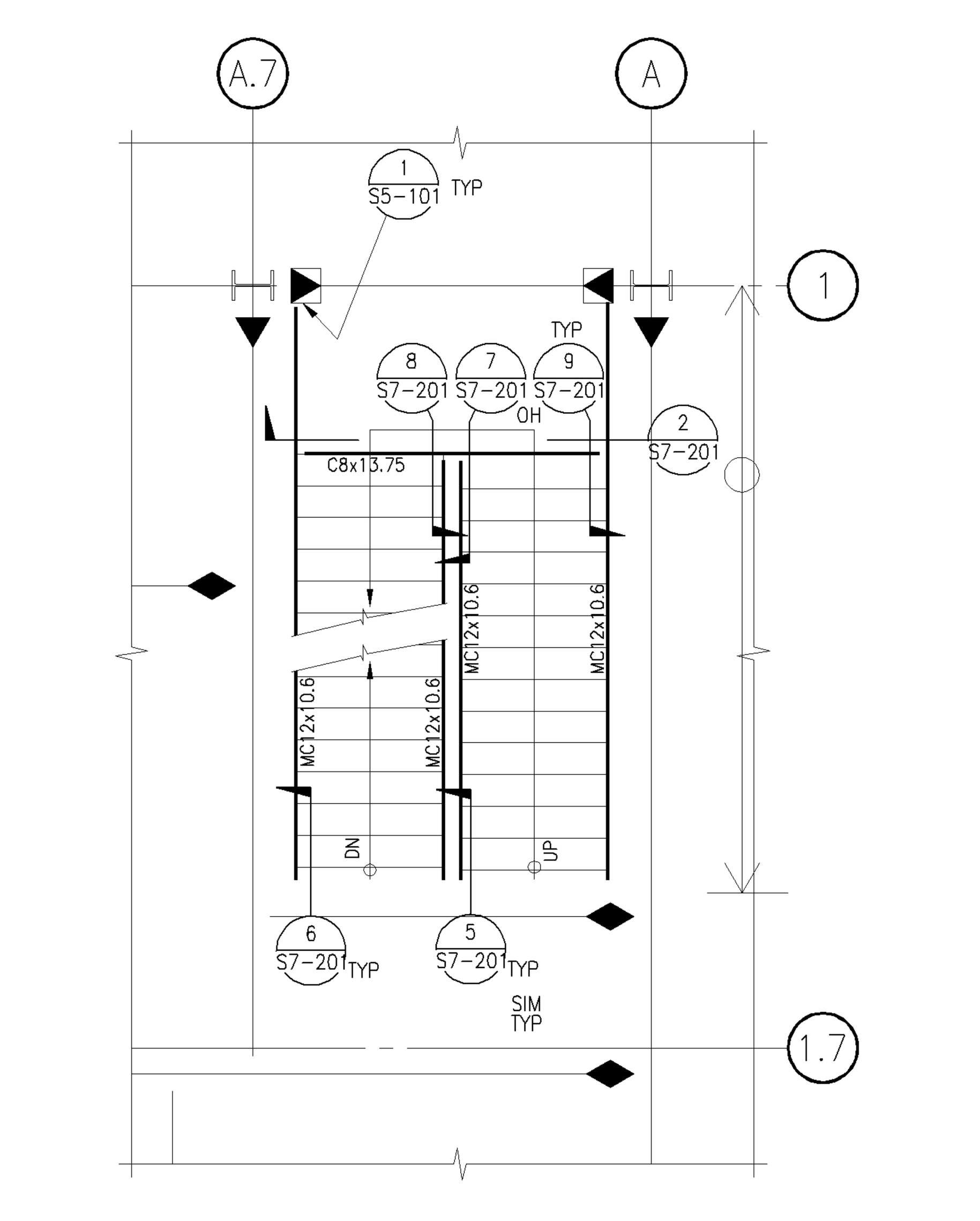
STAIR #2 - LEVEL 5 1/4"=1'-0" 11



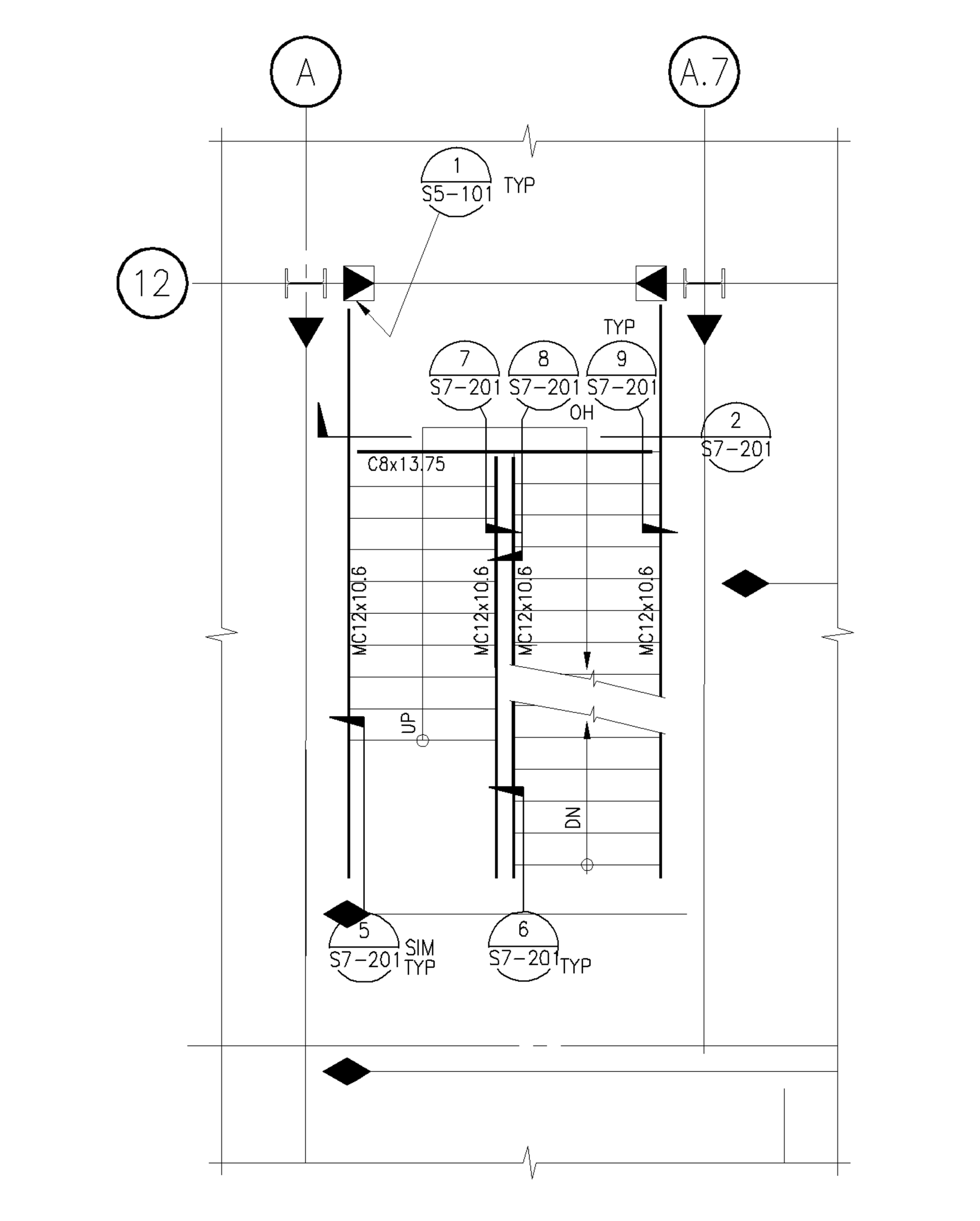
STAIR #2 - LEVEL 2 1/4"=1'-0" 8



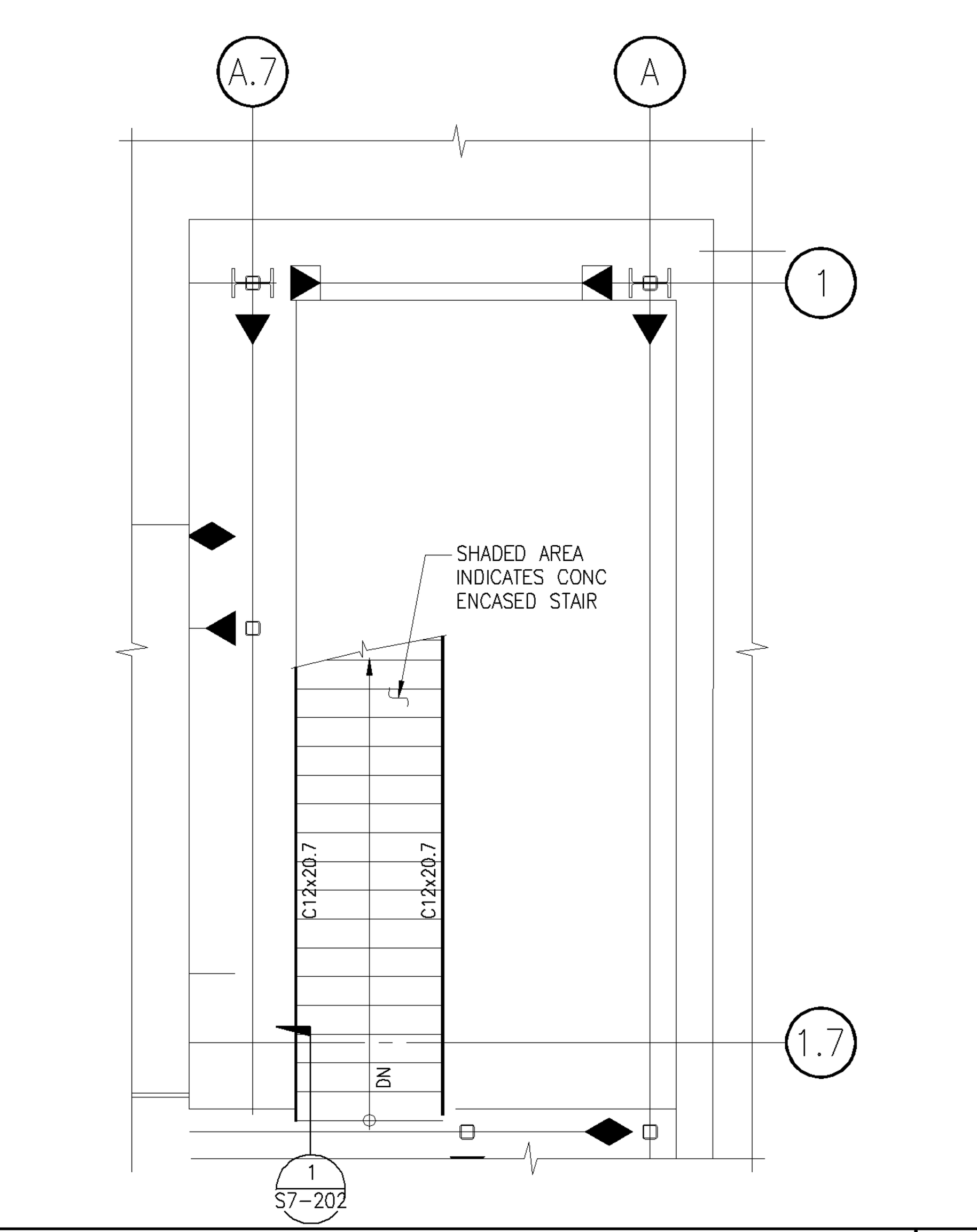
STAIR #1 - LEVEL 5 1/4"=1'-0" 5



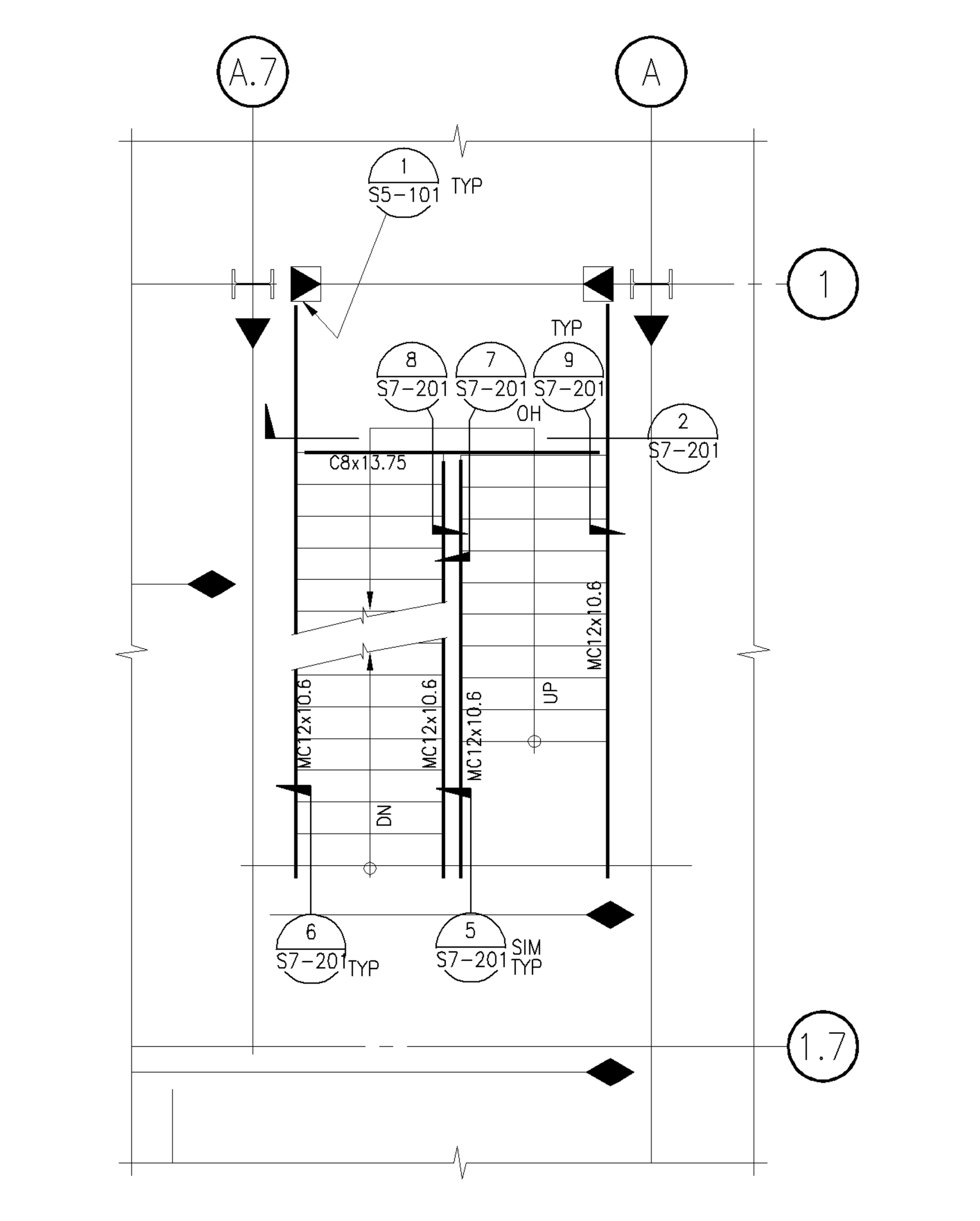
STAIR #1 - LEVEL 2 1/4"=1'-0" 2



STAIR #2 - LEVEL 3 1/4"=1'-0" 9



STAIR #1 - ROOF LEVEL 1/4"=1'-0" 6



STAIR #1 - LEVEL 3 1/4"=1'-0" 3

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

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CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
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BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE I-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

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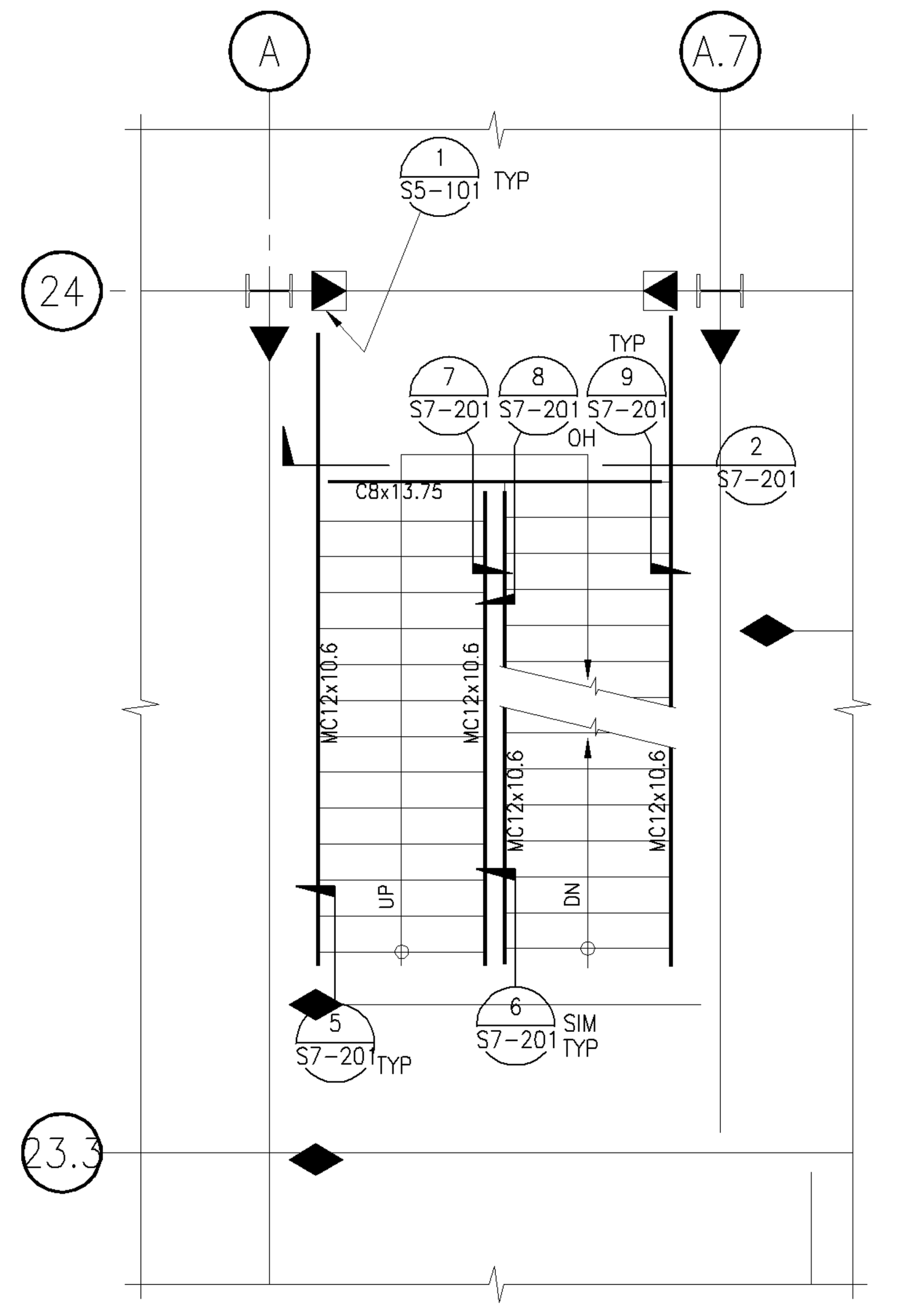
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

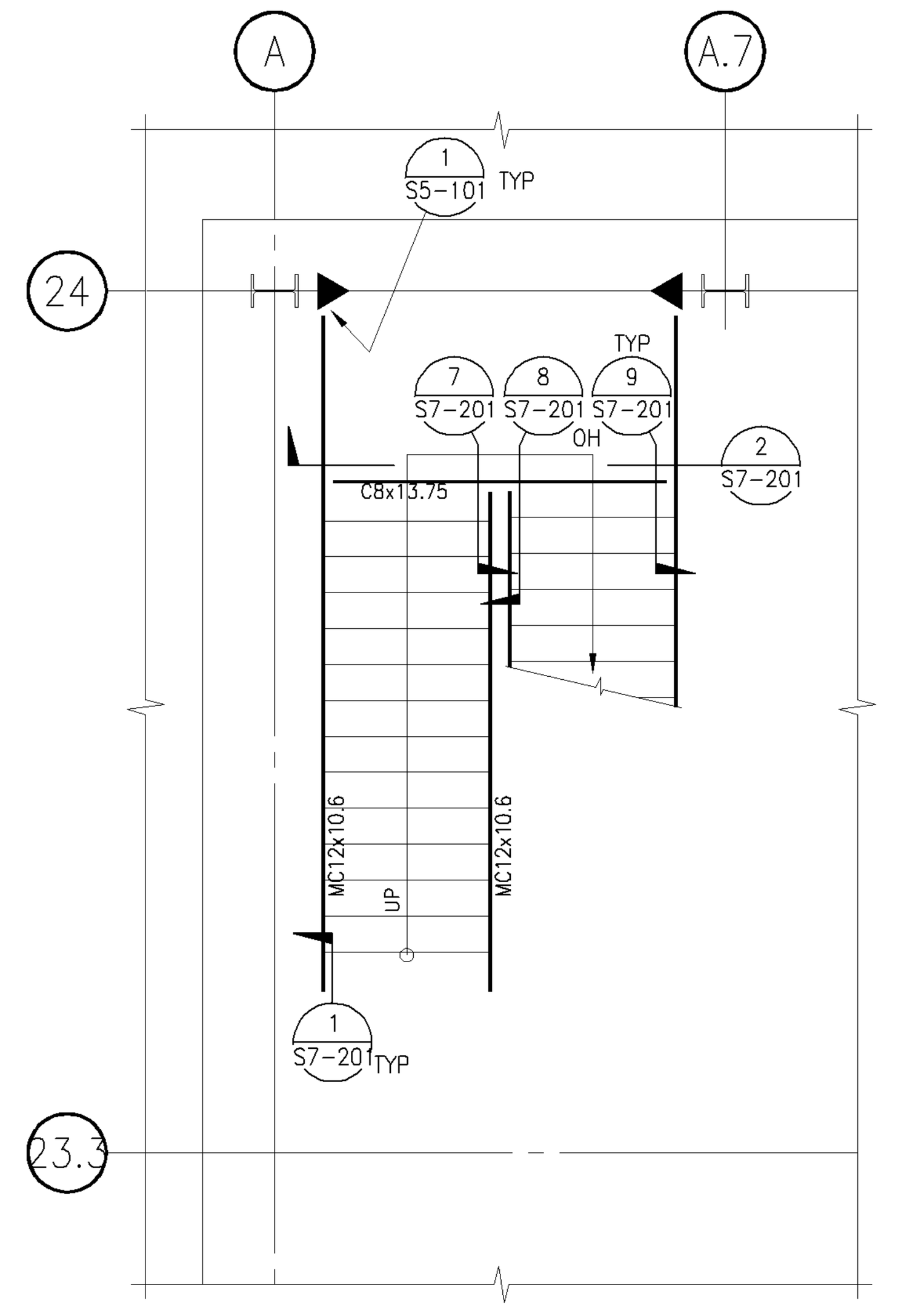
SHEET TITLE

STAIR FRAMING PLANS BUILDING F

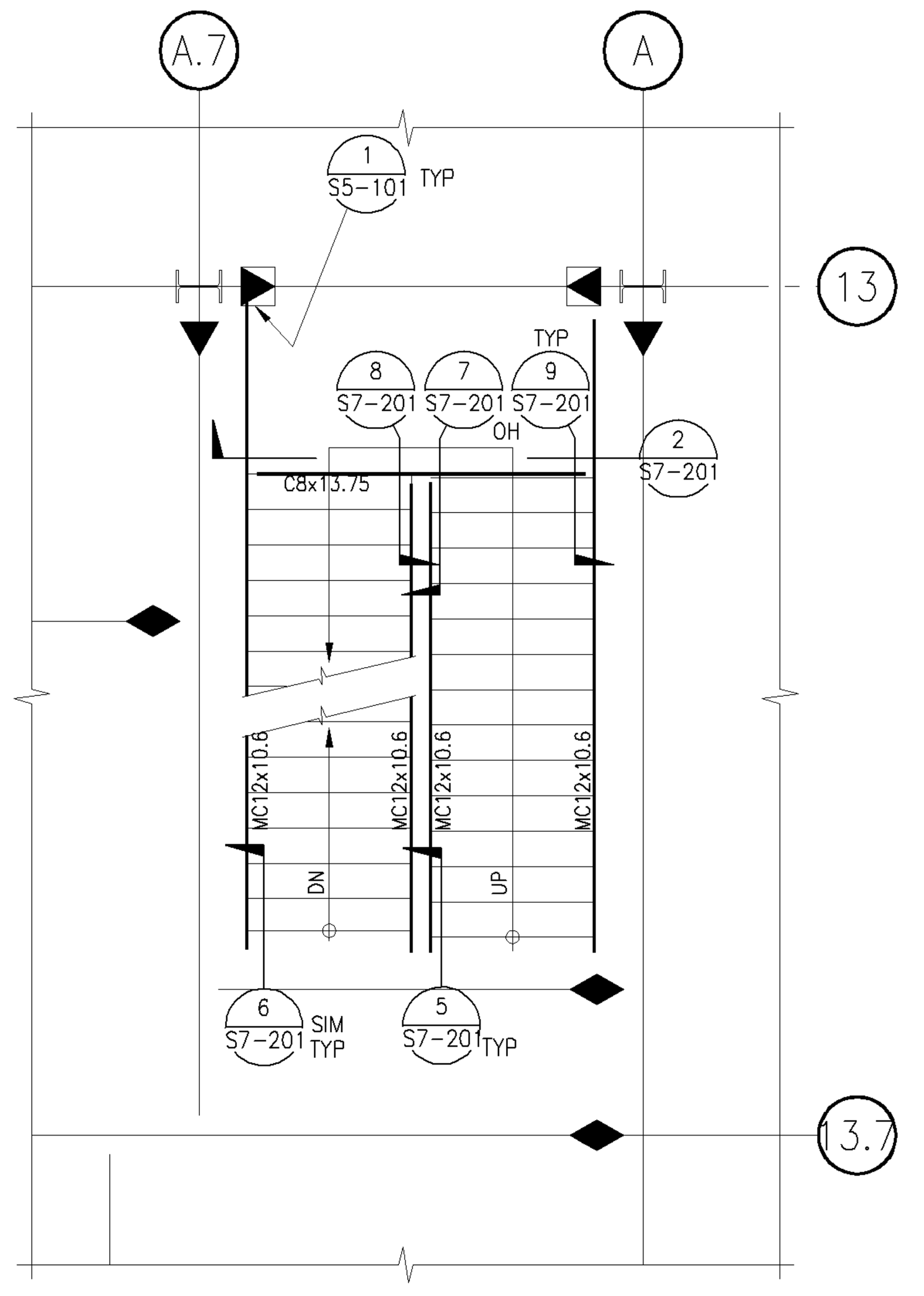
S7-101



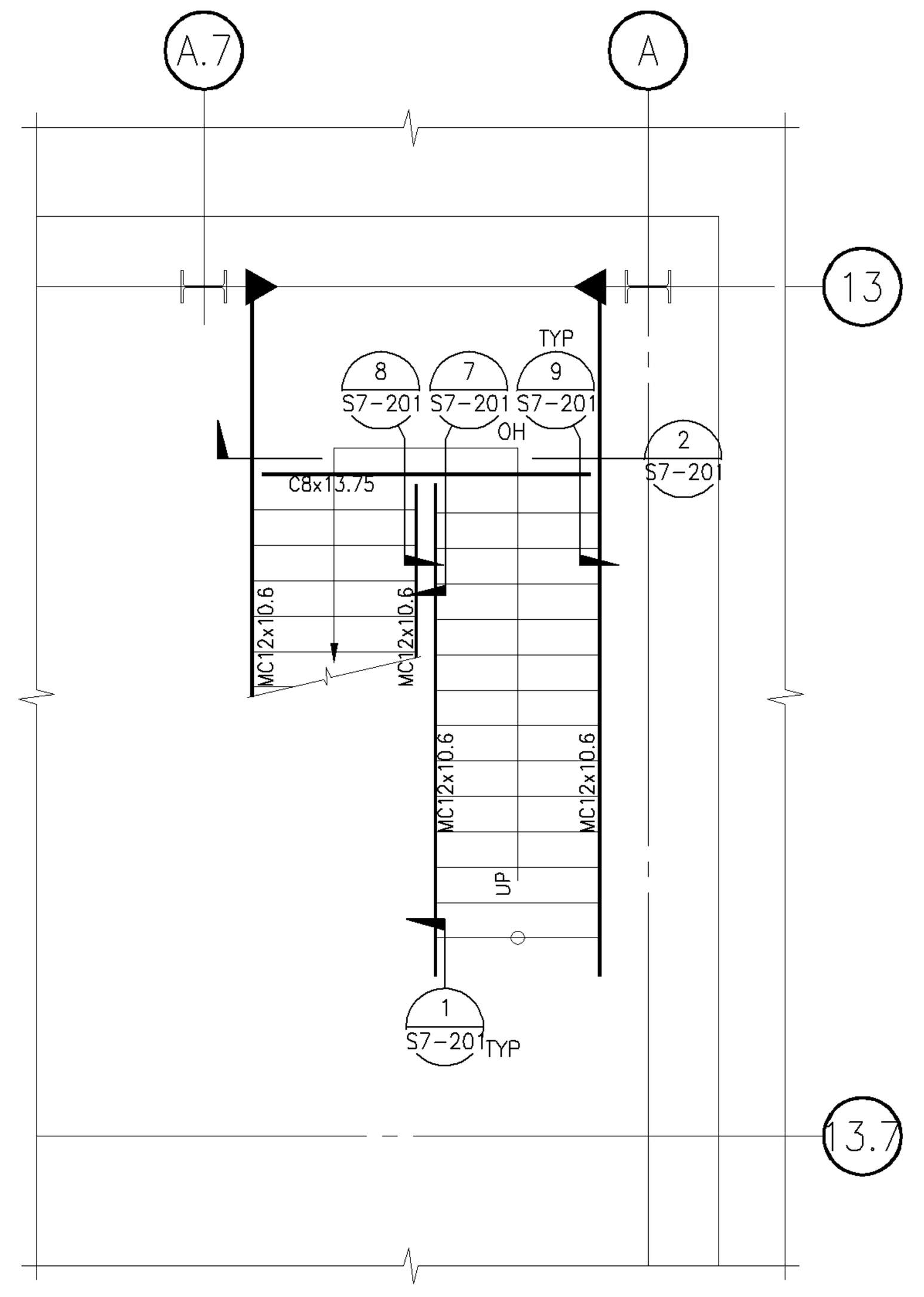
STAIR #4 - LEVEL 4 1/4"=1'-0" 10



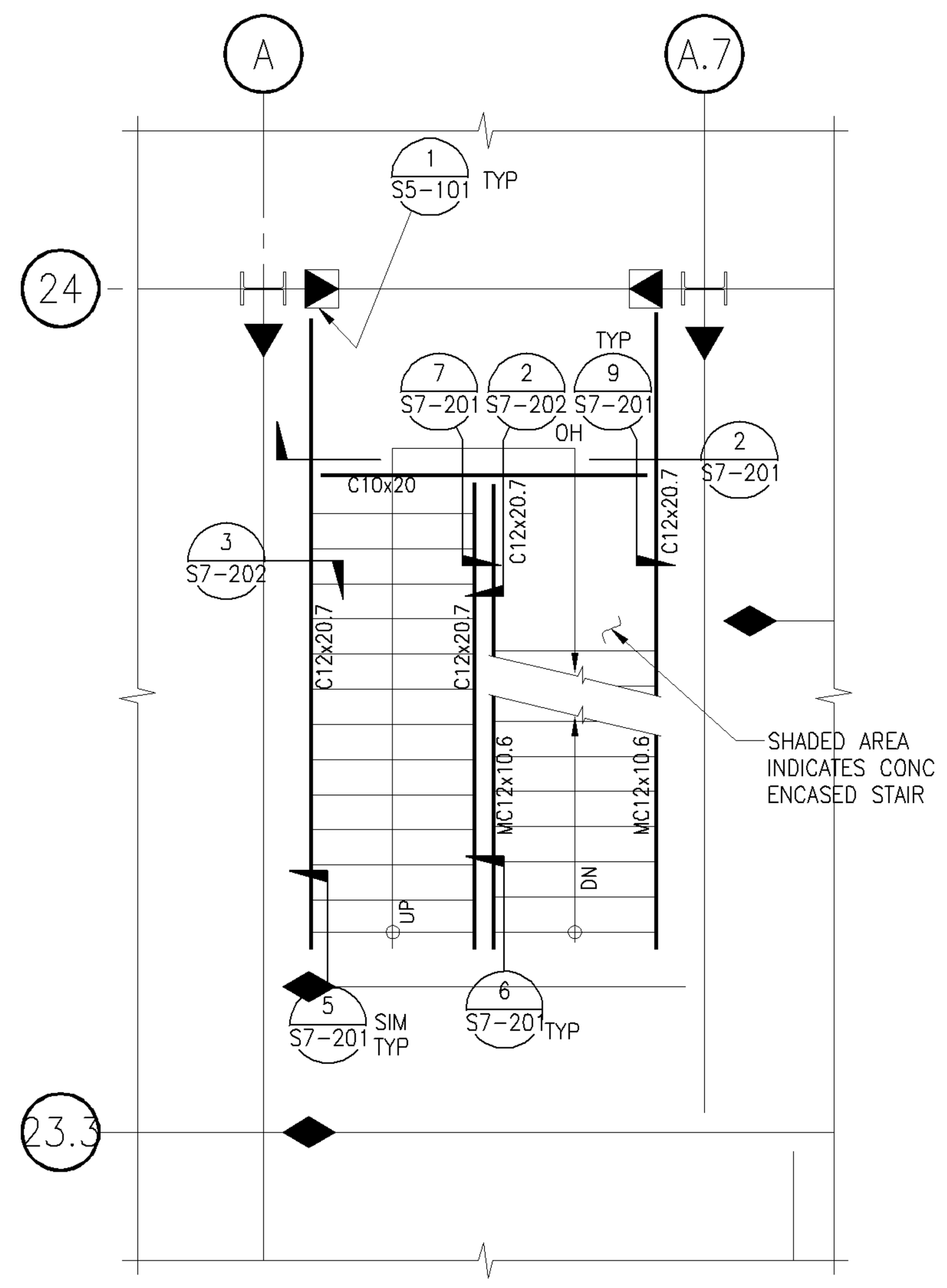
STAIR #4 - LEVEL 1 1/4"=1'-0" 7



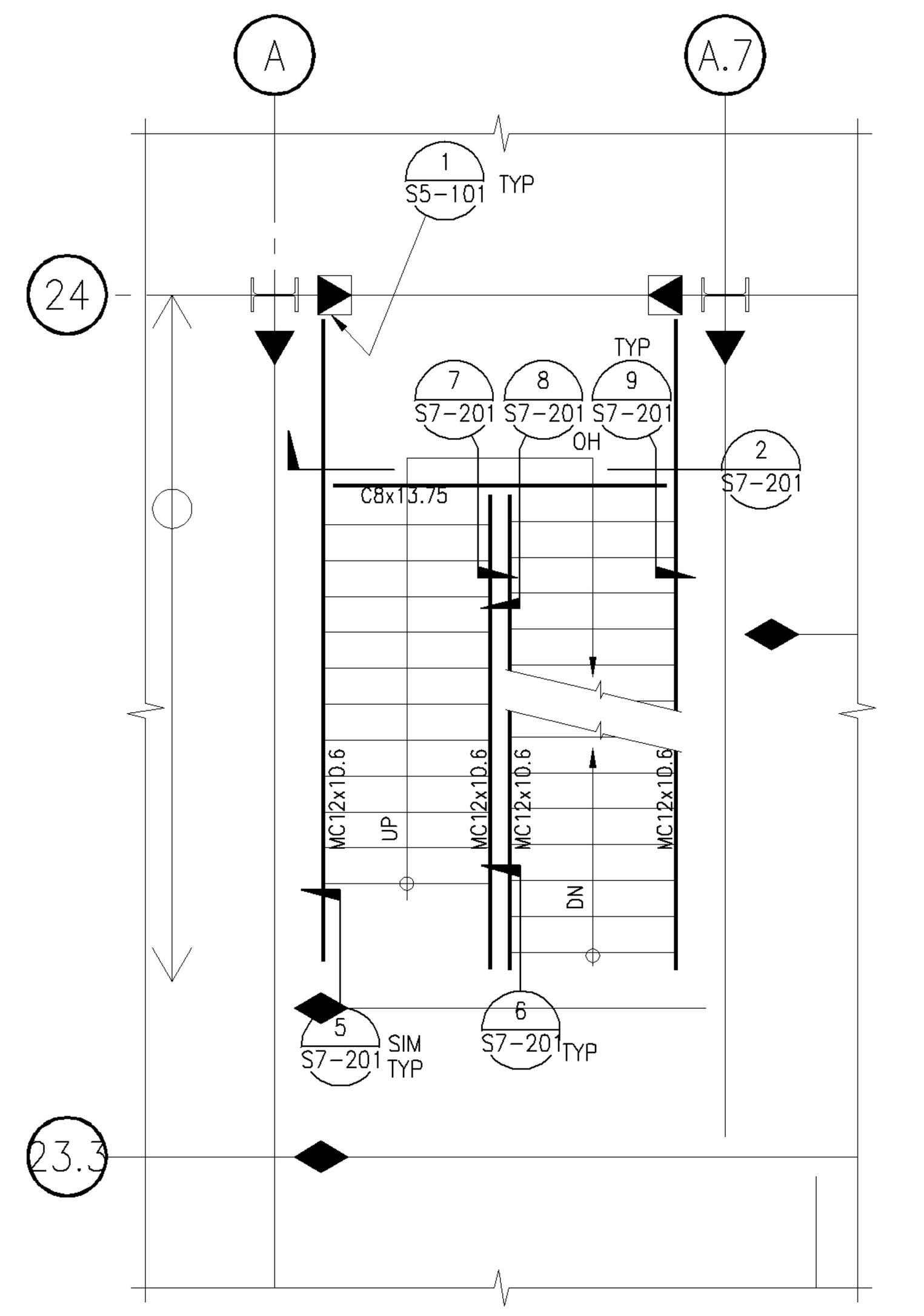
STAIR #3 - LEVEL 4 1/4"=1'-0" 4



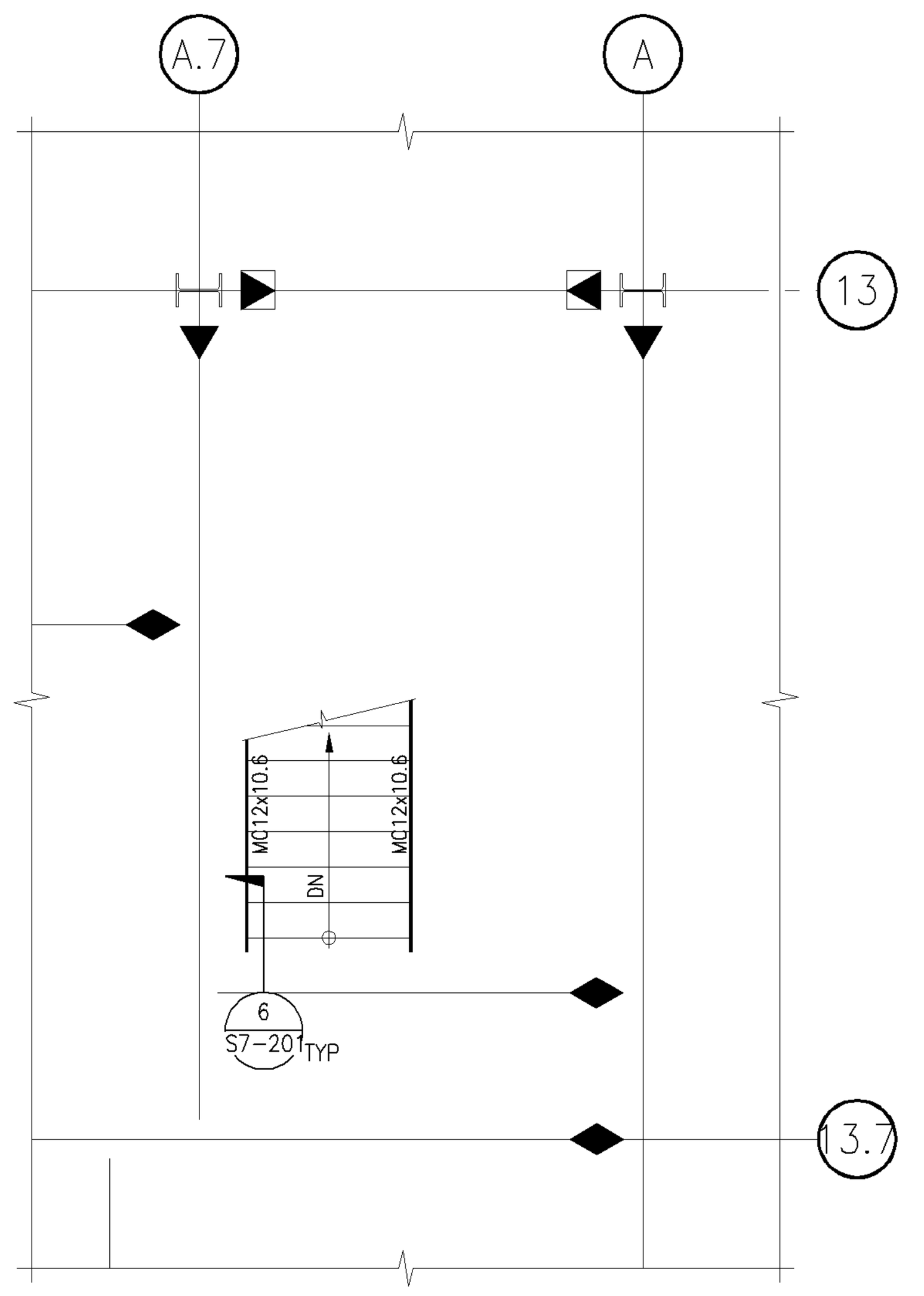
STAIR #3 - LEVEL 1 1/4"=1'-0" 1



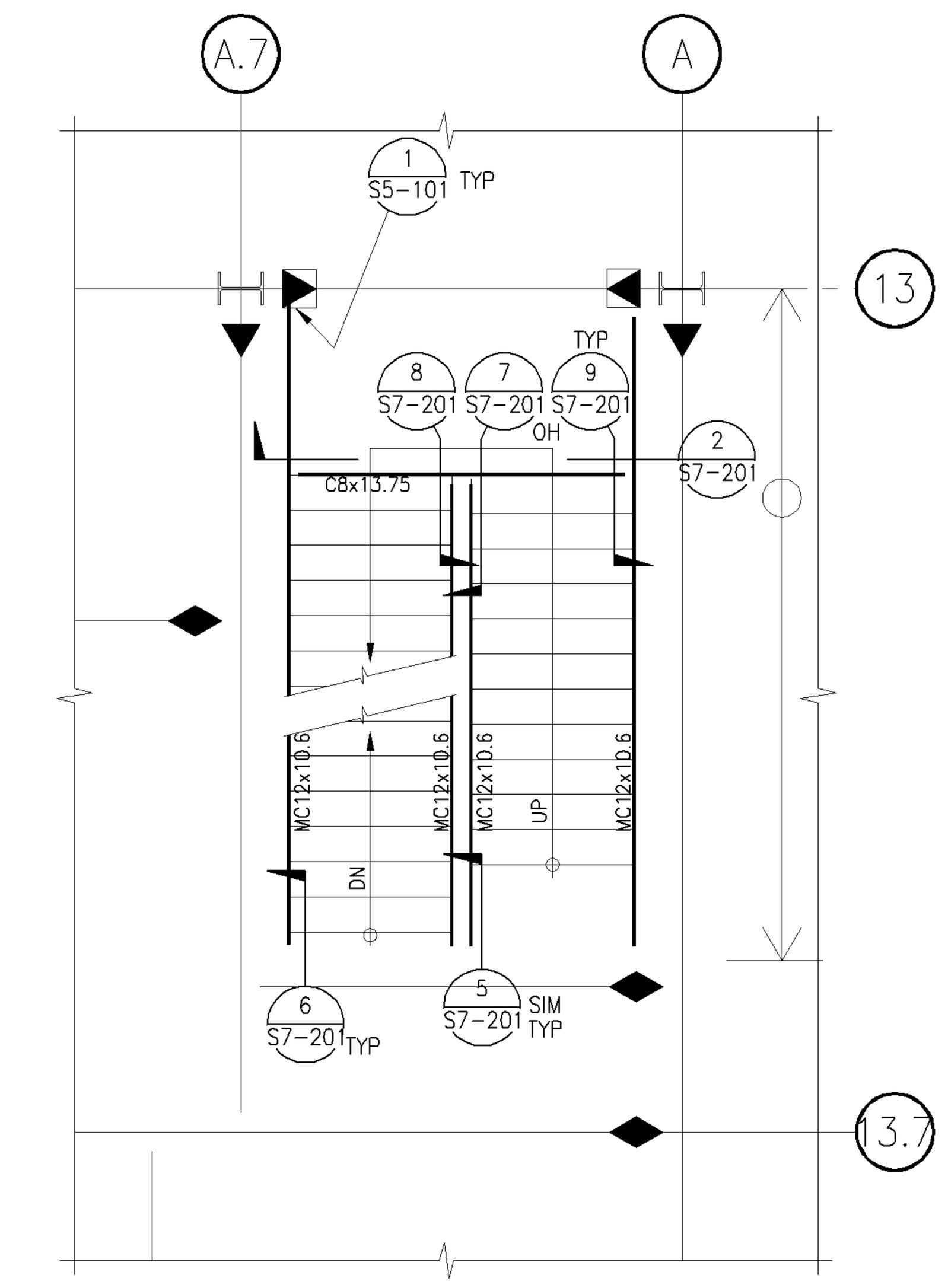
STAIR #4 - LEVEL 5 1/4"=1'-0" 11



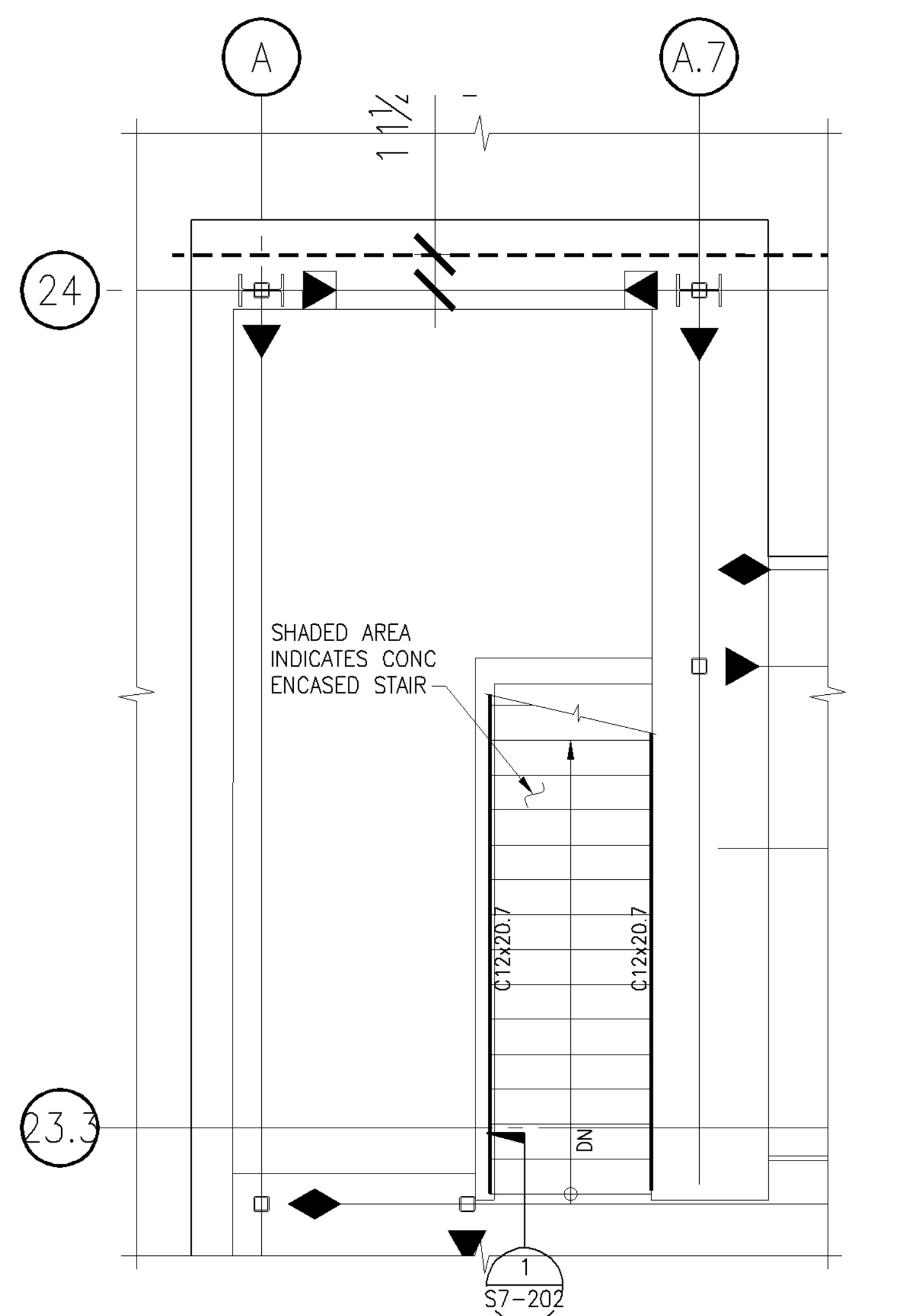
STAIR #4 - LEVEL 2 1/4"=1'-0" 8



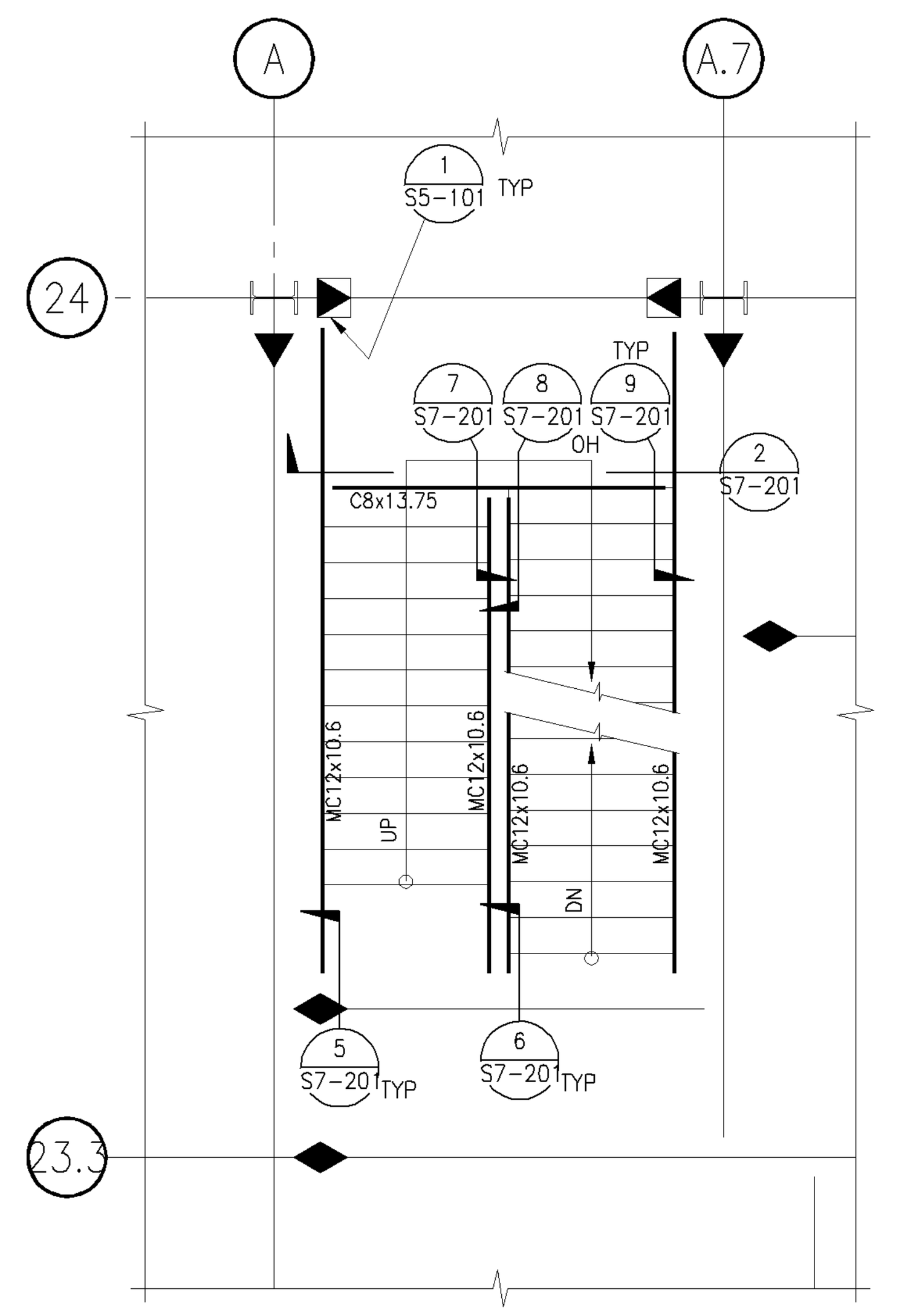
STAIR #3 - LEVEL 5 1/4"=1'-0" 5



STAIR #3 - LEVEL 2 1/4"=1'-0" 2

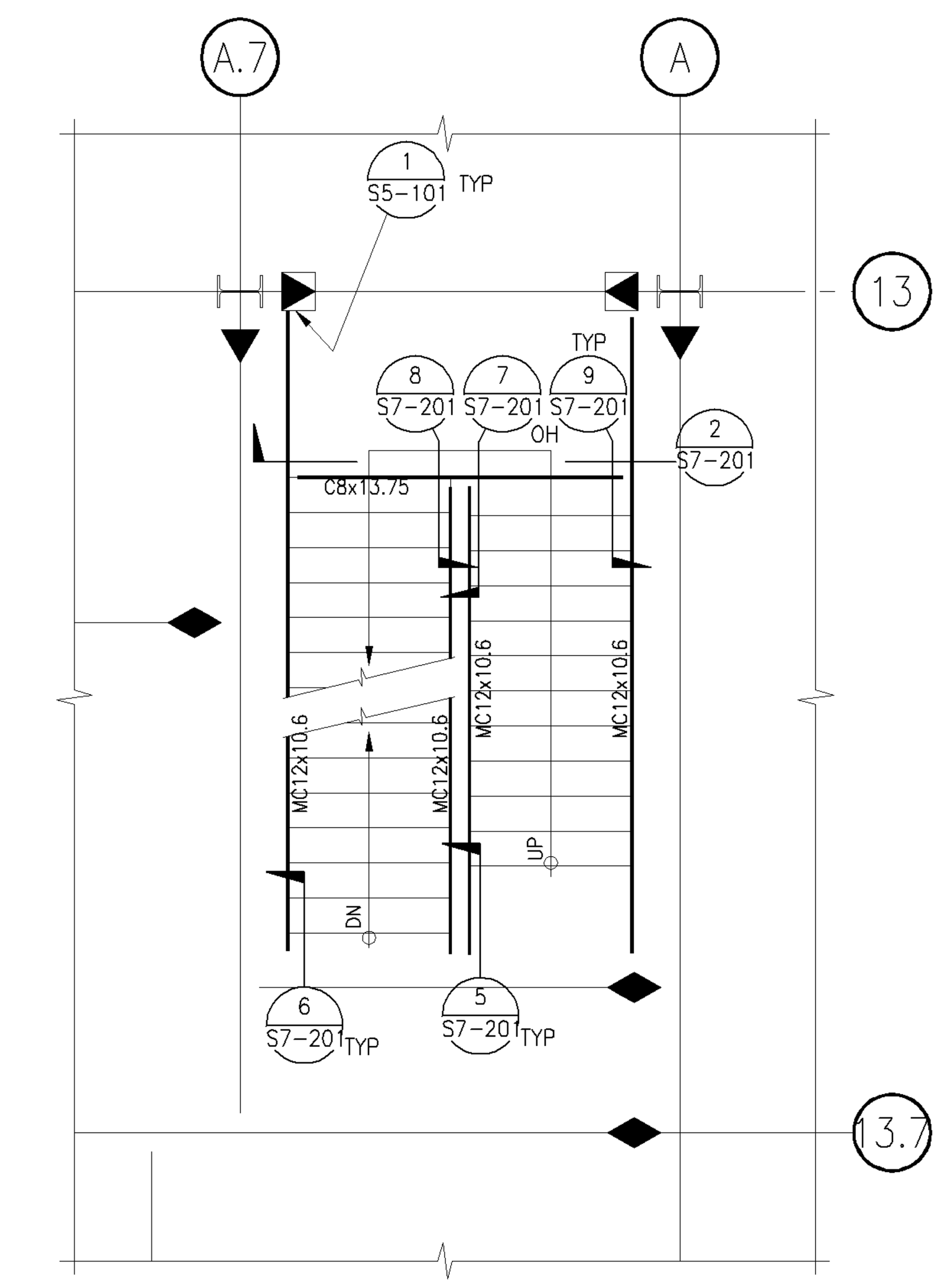


STAIR #4 - ROOF LEVEL 1/4"=1'-0" 12



STAIR #4 - LEVEL 3 1/4"=1'-0" 9

NOT USED 6



STAIR #3 - LEVEL 3 1/4"=1'-0" 3

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
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2701 LOKER AVENUE WEST, SUITE 130
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2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503

REGISTRATION

ISSUE

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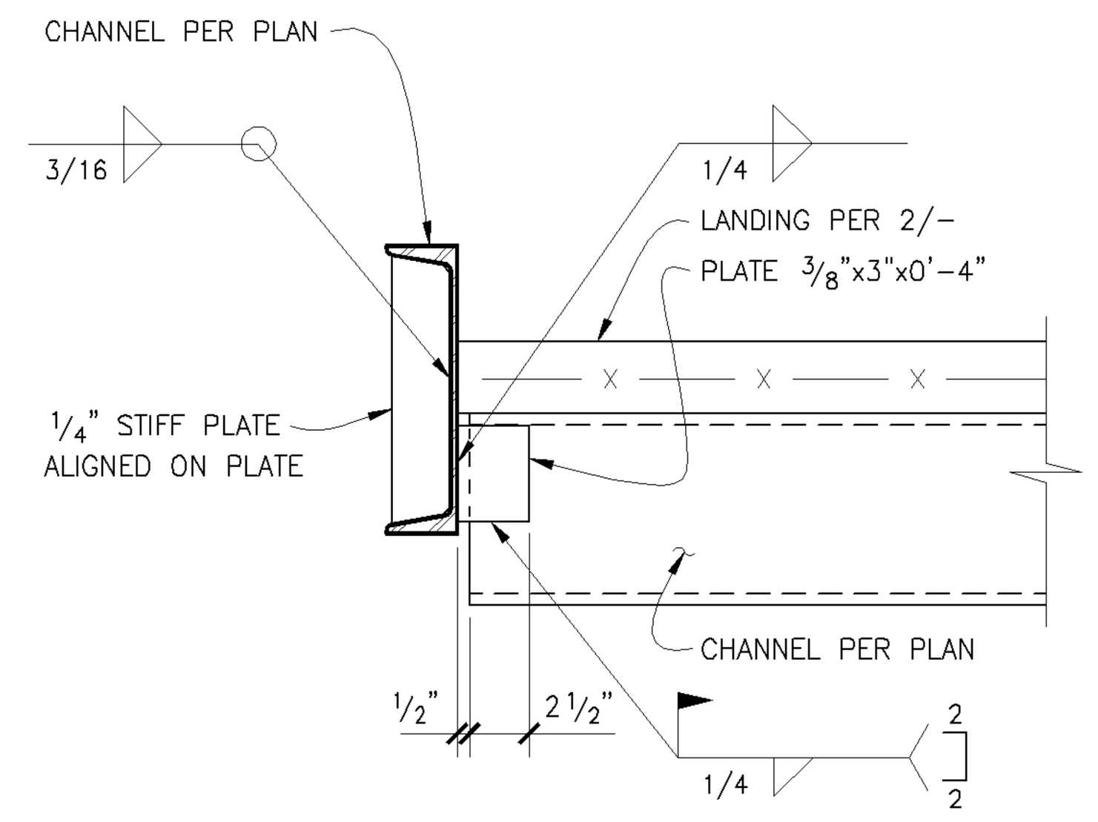
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

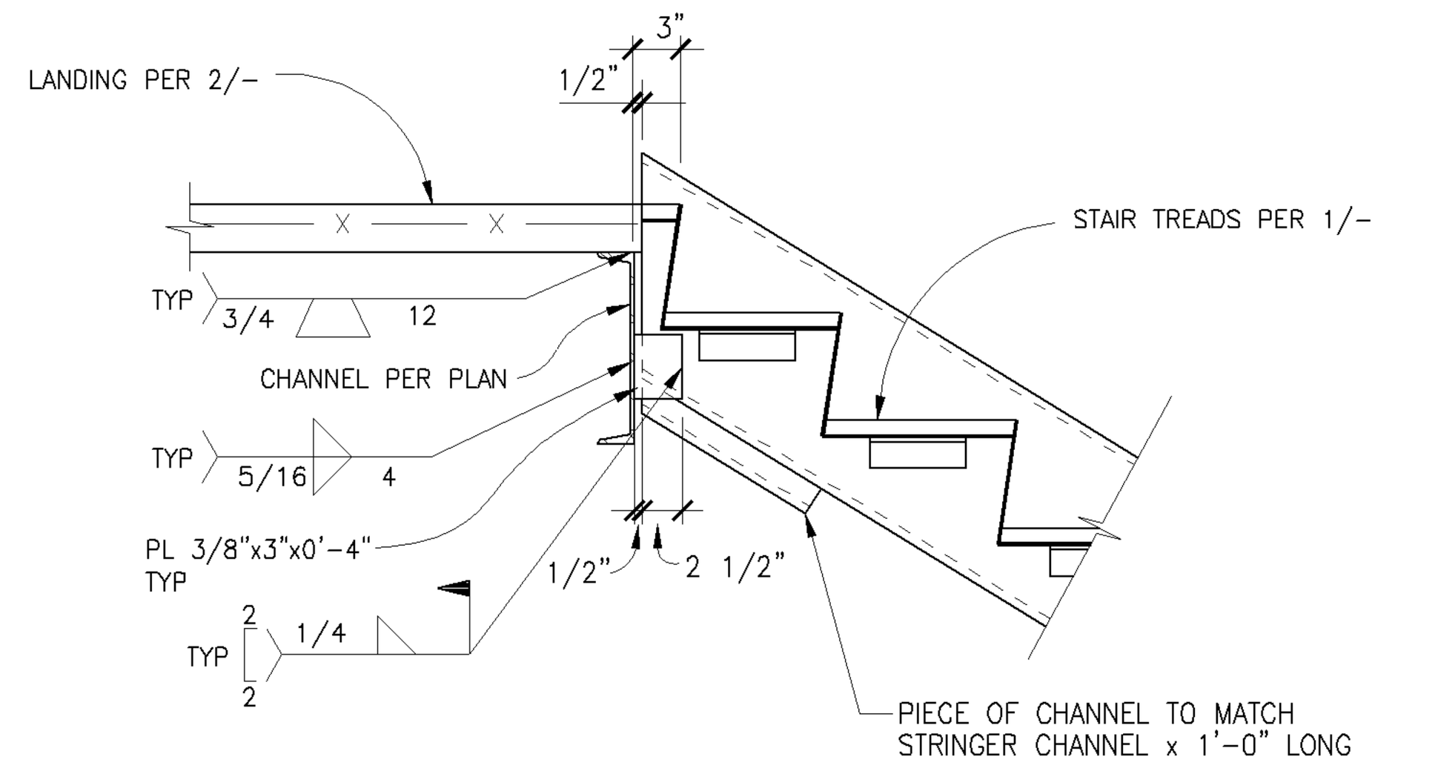
SHEET TITLE

STAIR FRAMING PLANS
BUILDING G

S7-111

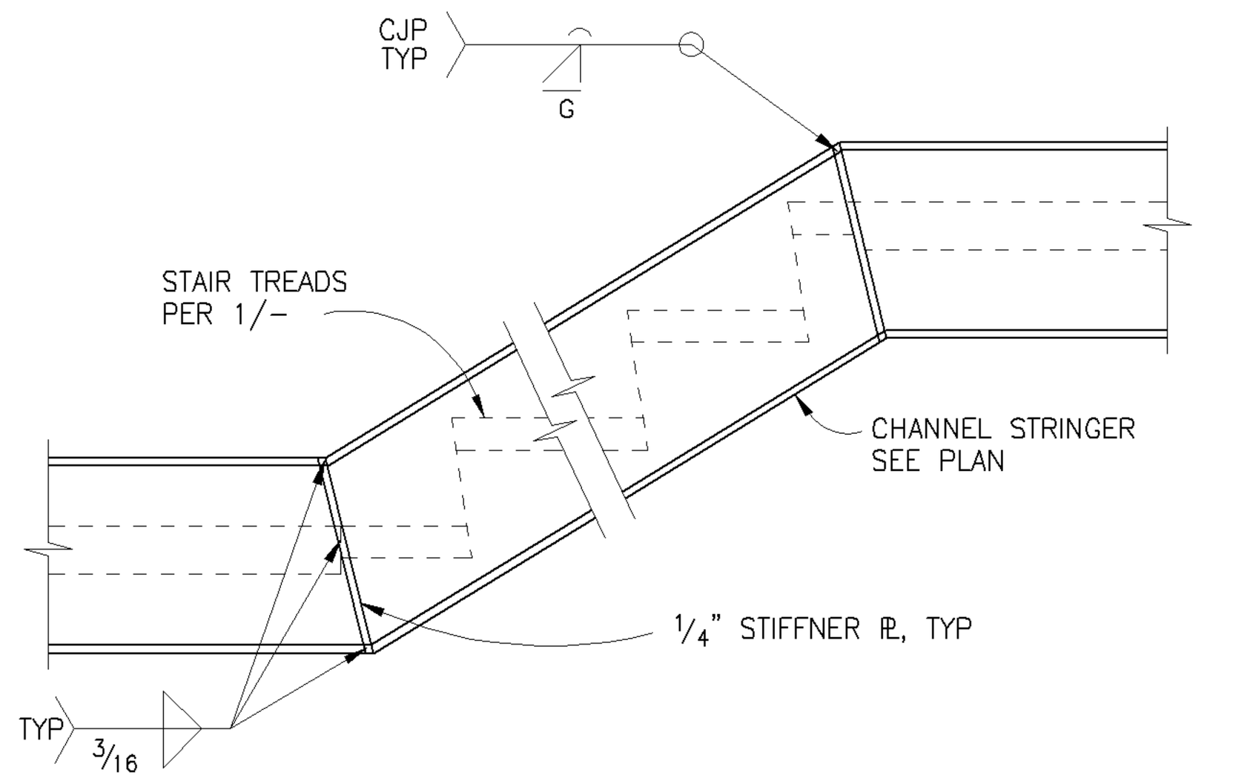


STRINGER CONNECTION
1"=1'–0" 10

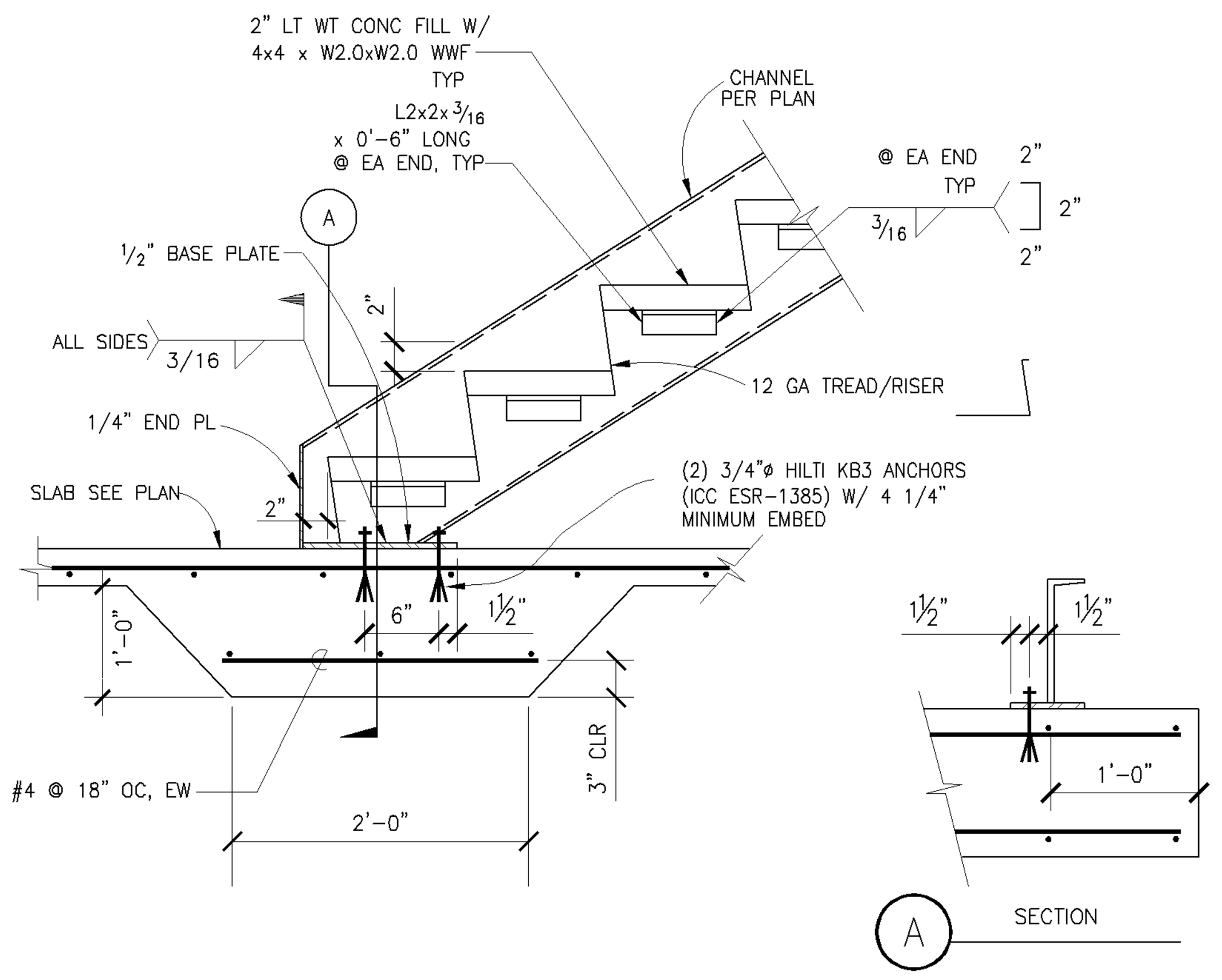


NOTE: SEE DETAIL 11/– FOR HANDRAIL DETAIL.

STRINGER CONNECTION AT LANDING
1"=1'–0" 7

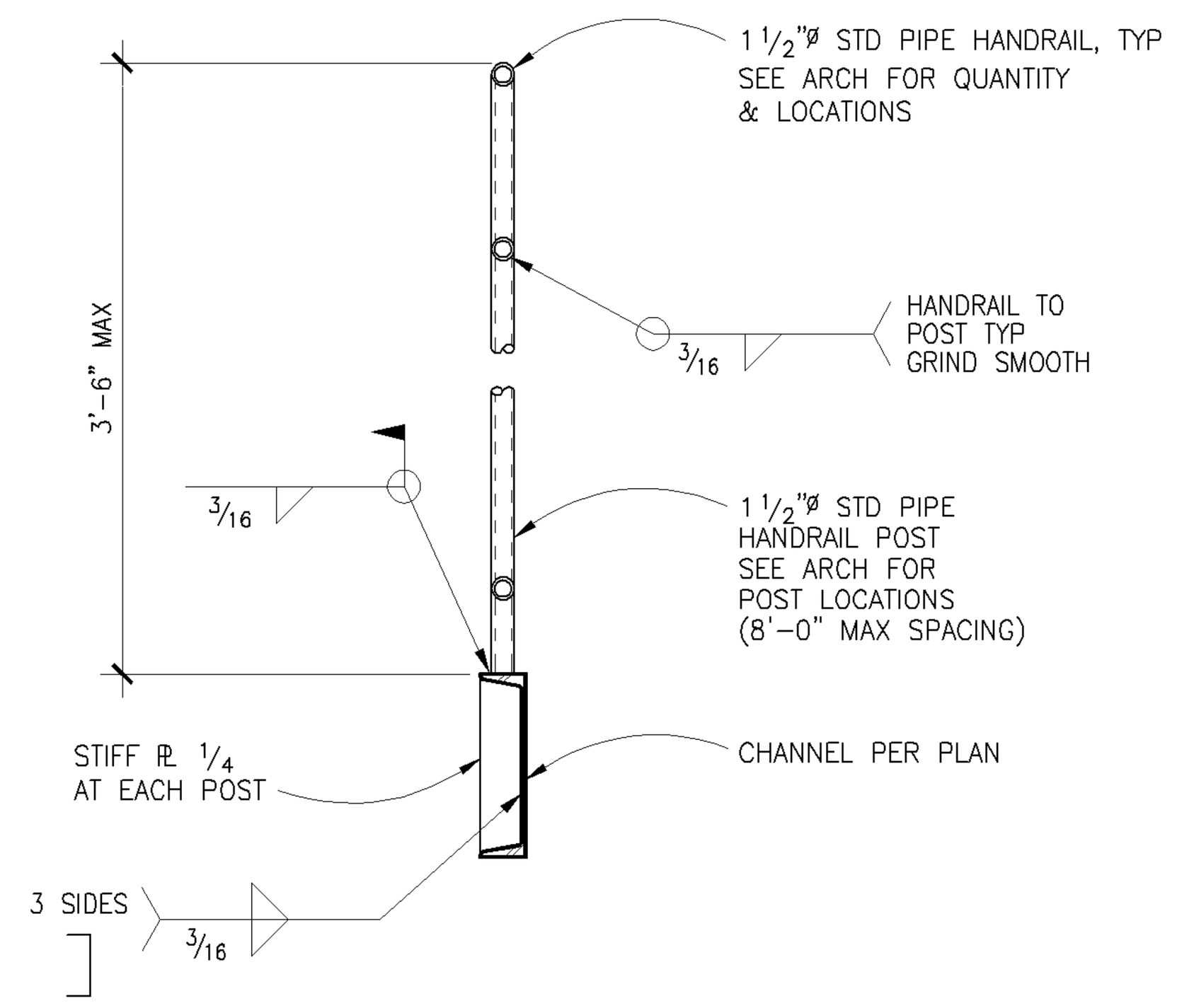


CHANNEL BENT STAIR STRINGER
1"=1'–0" 4

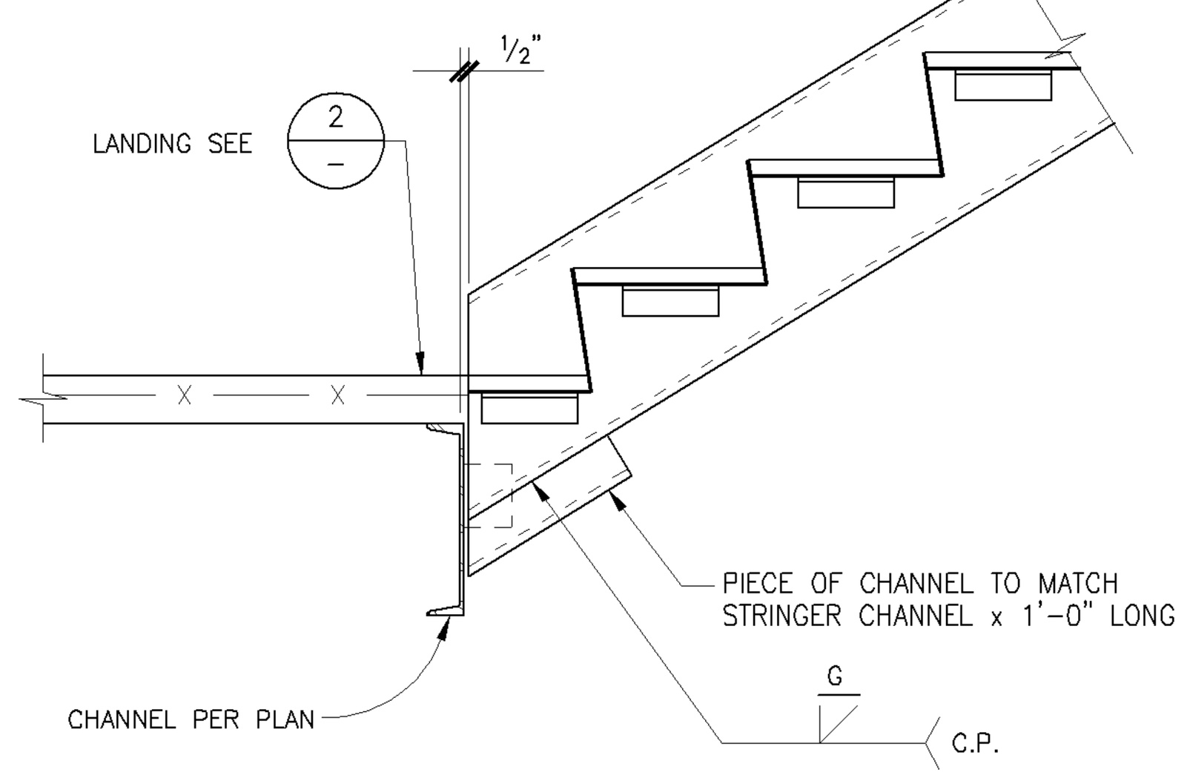


NOTE: SEE DETAIL 11/– FOR HANDRAIL DETAIL.

STRINGER TO SLAB ON GRADE CONNECTION
1"=1'–0" 1

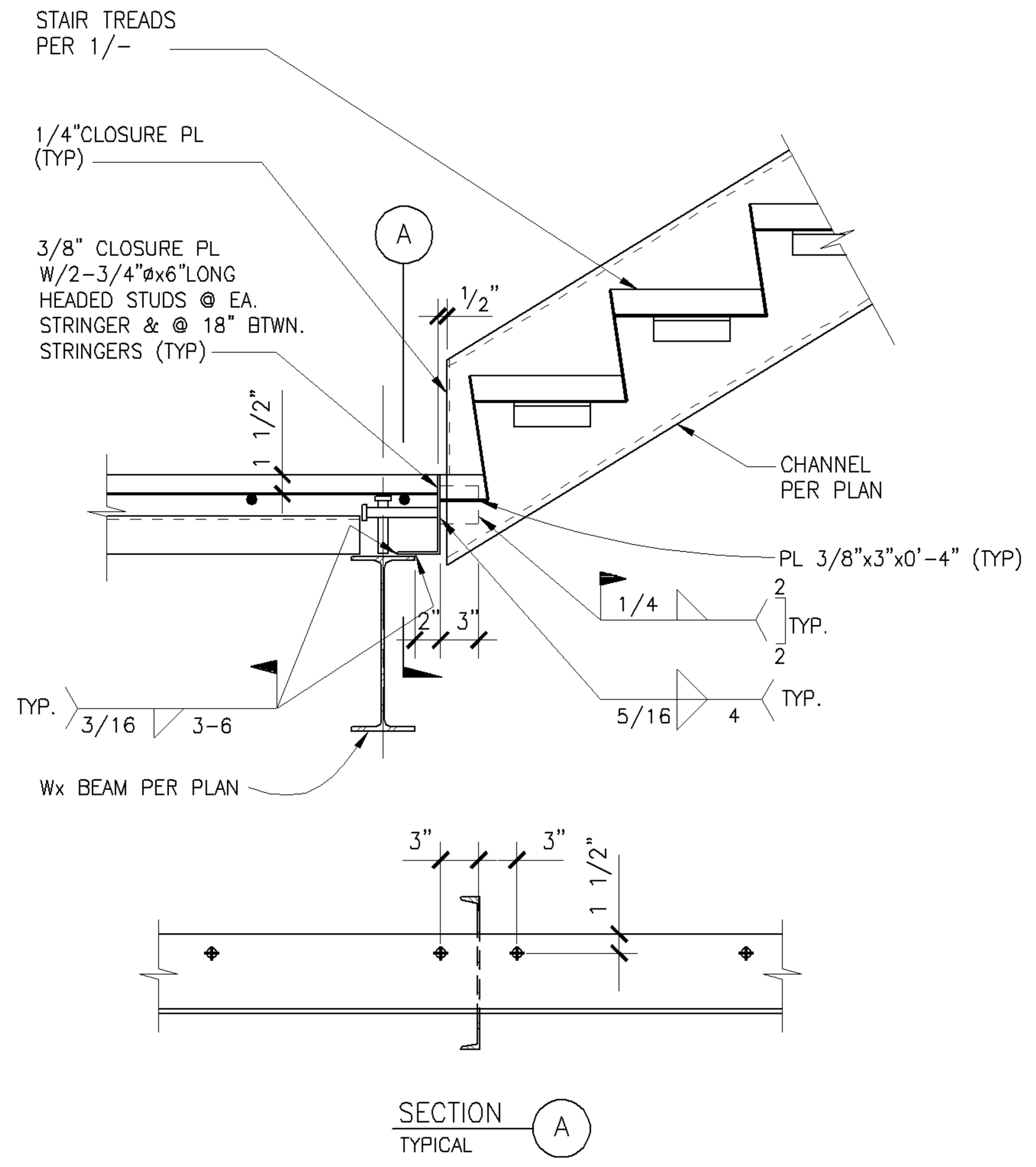


HANDRAIL DETAIL
1"=1'–0" 11



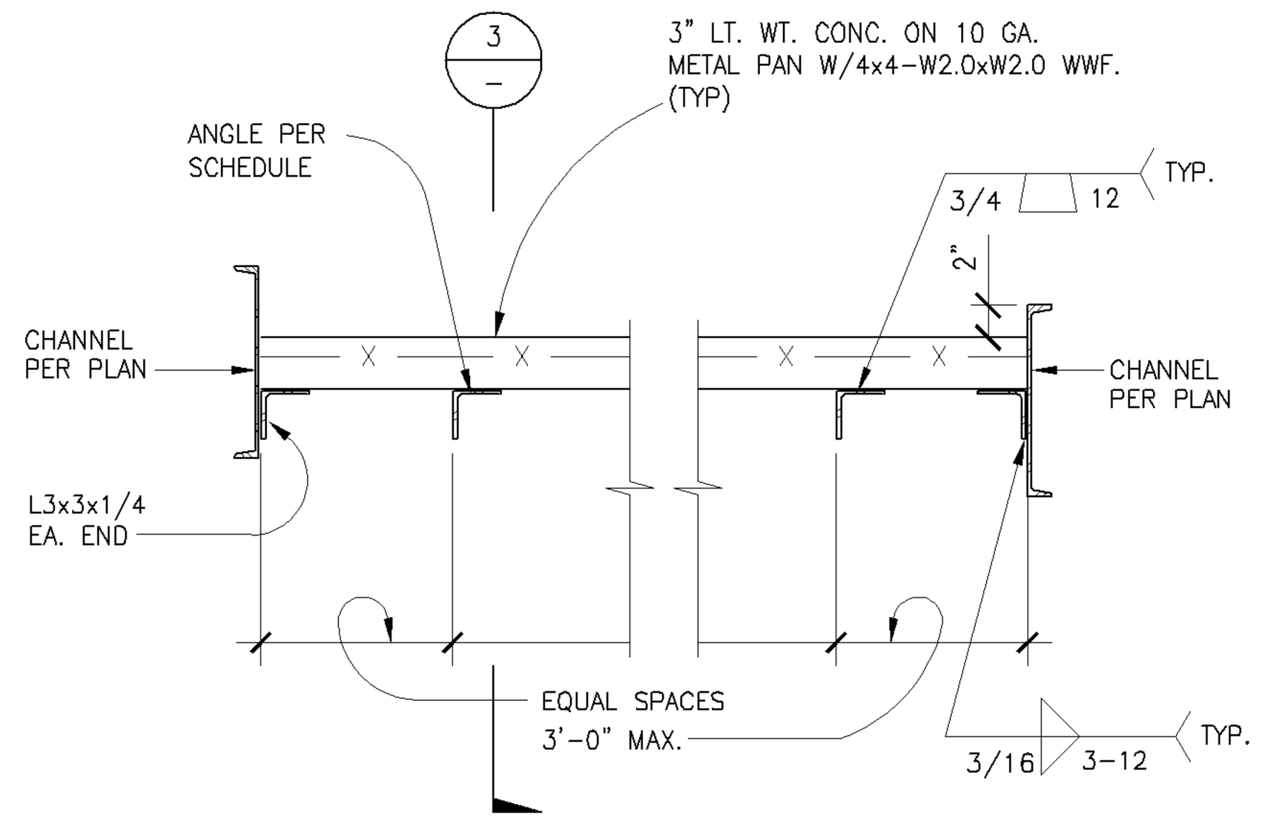
FOR TYPICAL NOTES SEE DETAIL 7/–

STRINGER CONNECTION AT LANDING
1"=1'–0" 8



NOTE: SEE DETAIL 11/– FOR HANDRAIL DETAIL.

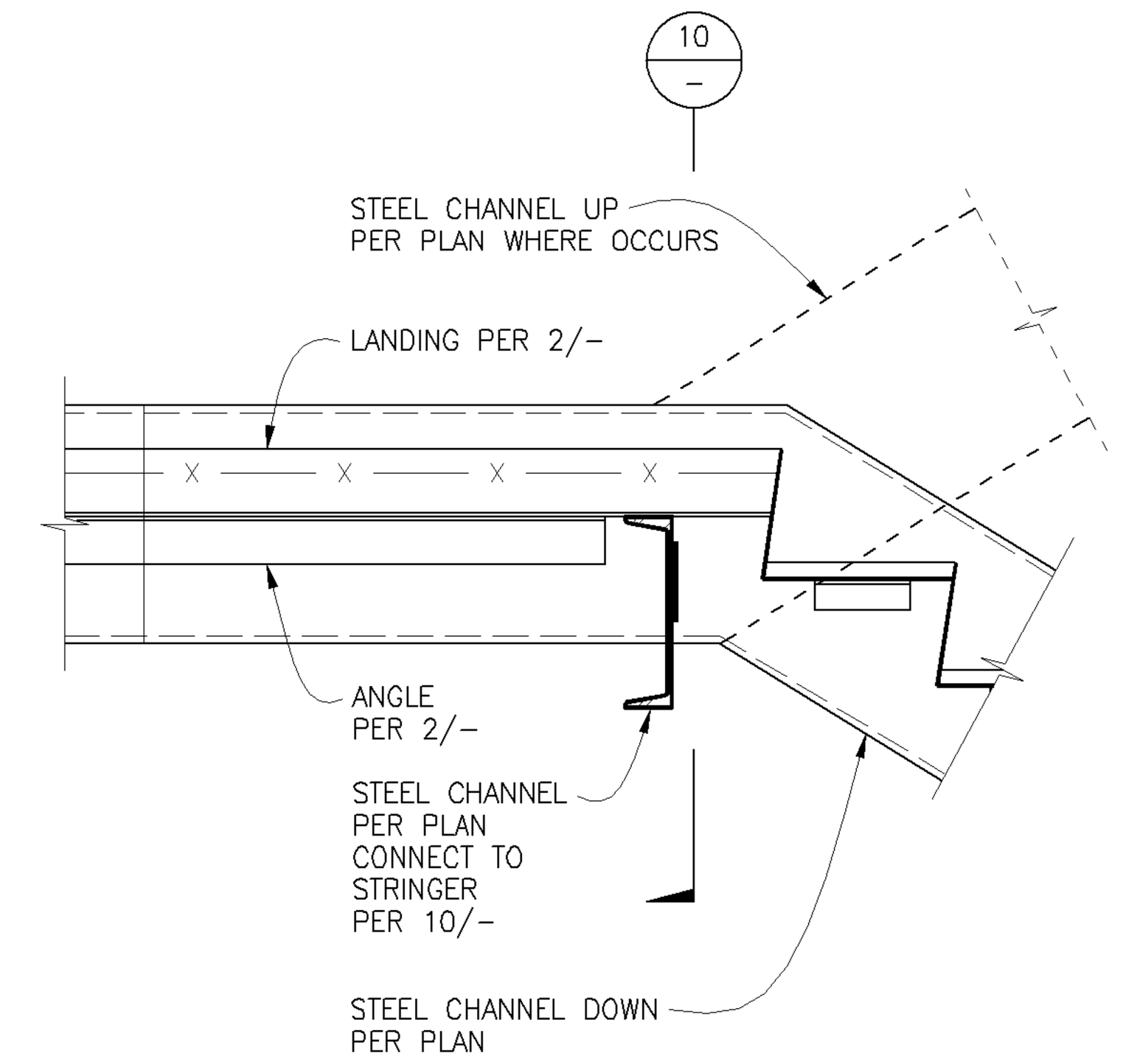
STRINGER CONNECTION AT FLOOR
1"=1'–0" 5



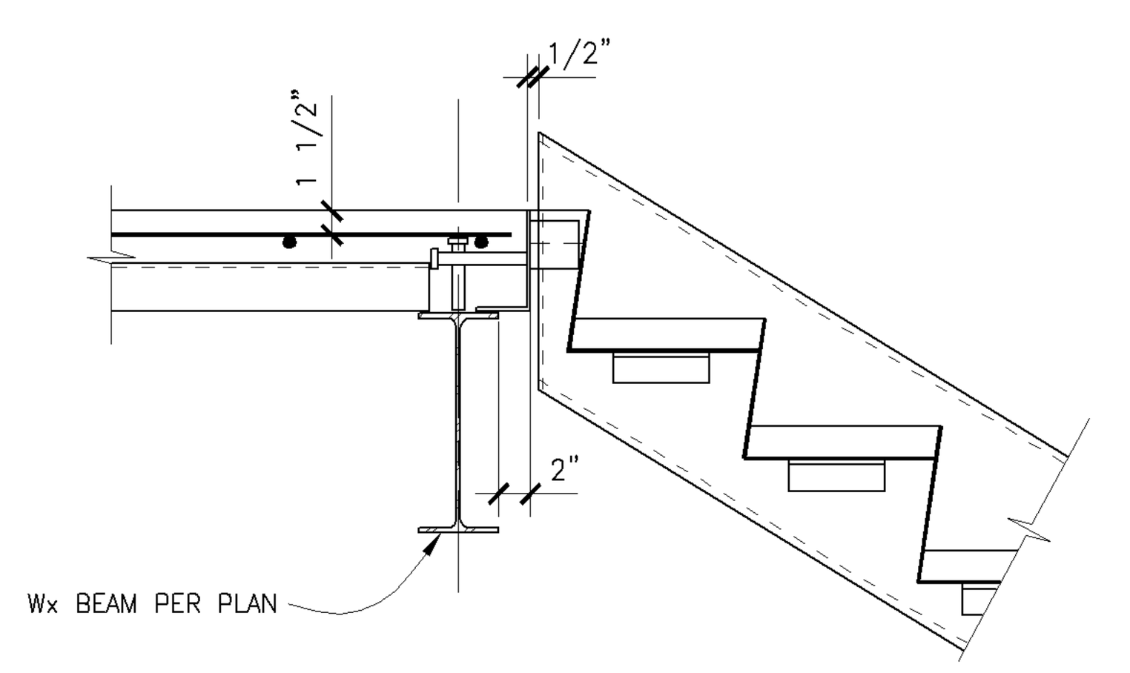
SCHEDULE	
SPAN	STIFFENER ANGLE
UP TO 4'–0"	L3x3x1/4
4'–0" TO 5'–0"	L3 1/2x3 1/2x1/4
5'–0" TO 6'–0"	L4x4x1/4

NOTE: SEE DETAIL 11/– FOR HANDRAIL DETAIL.

LANDING SECTION
1"=1'–0" 2

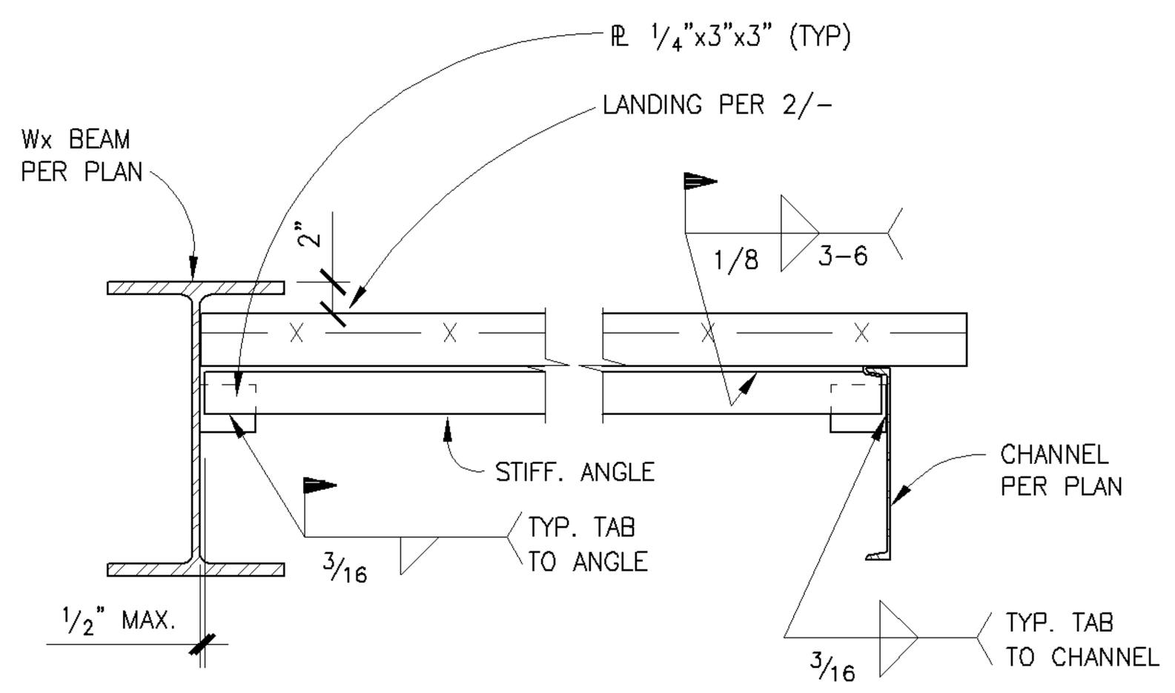


STRINGER UP AND DOWN AT LANDING
1"=1'–0" 9



FOR TYPICAL NOTES SEE DETAIL 5/–

STRINGER CONNECTION AT FLOOR
1"=1'–0" 6



TYPICAL INTERMEDIATE LANDING FRAME
1"=1'–0" 3

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591–4628

ARCHITECT

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1	10–23–06	BLDG & SAFETY SUBMITTAL

PROJECT NO: 60004775
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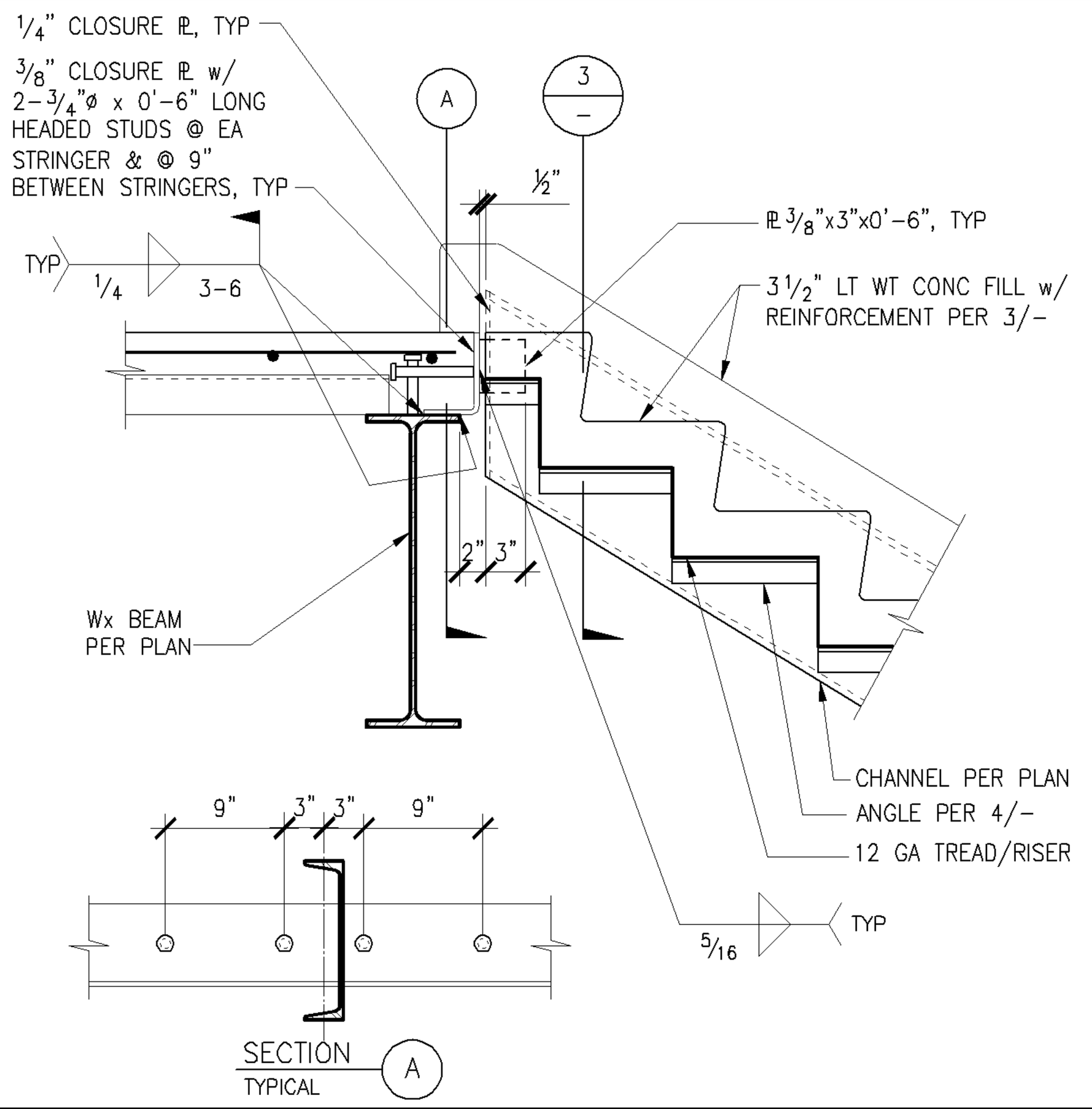
CHECKED BY:

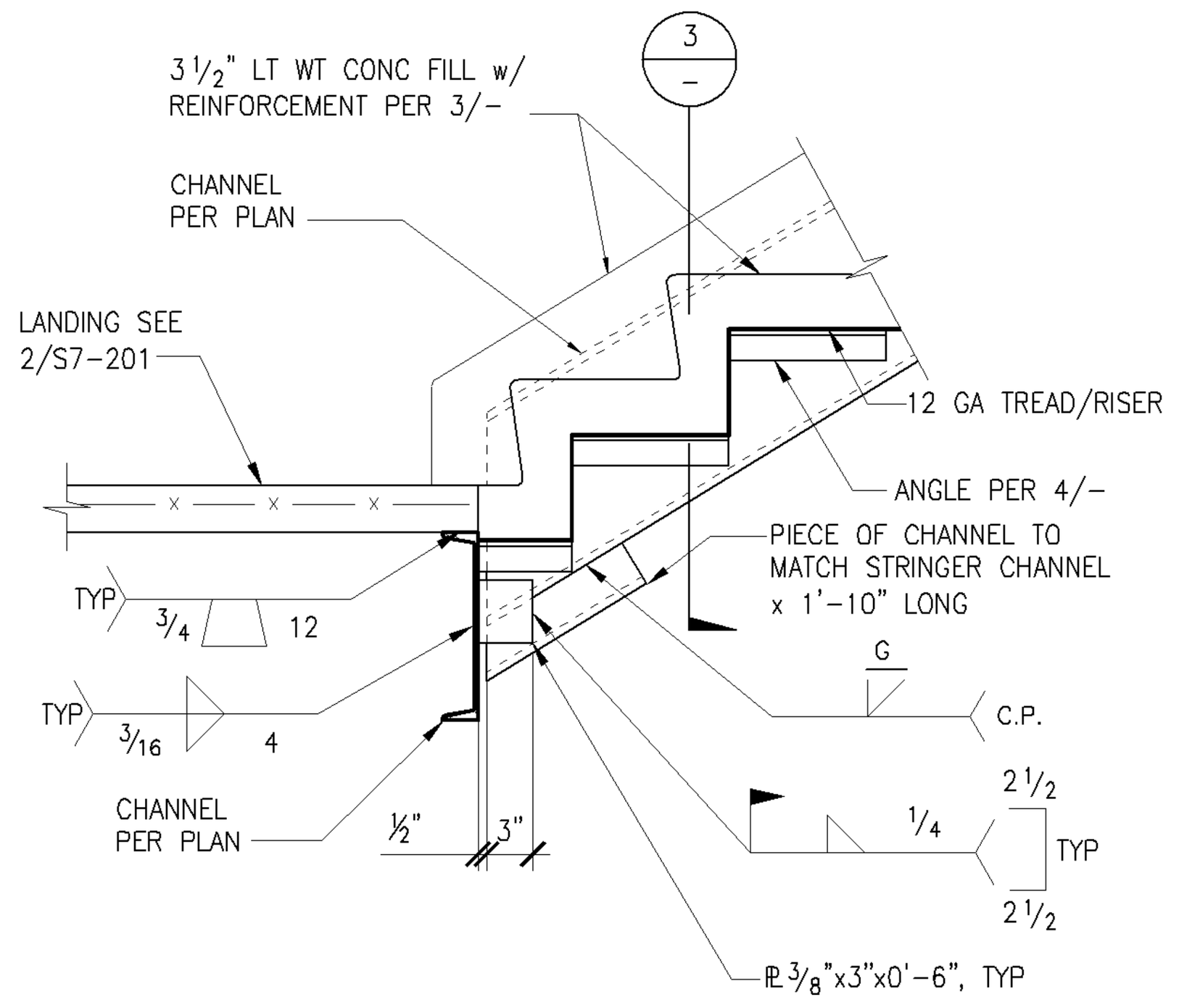
KEY PLAN

SHEET TITLE
STAIR DETAILS

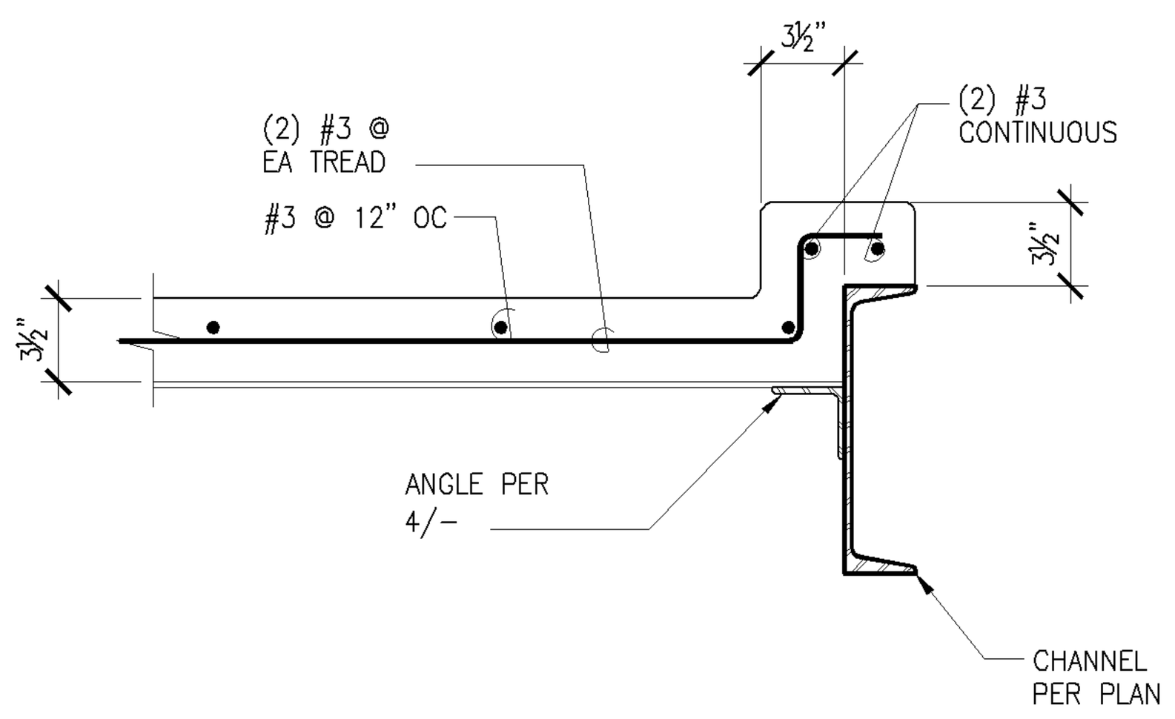
S7-201



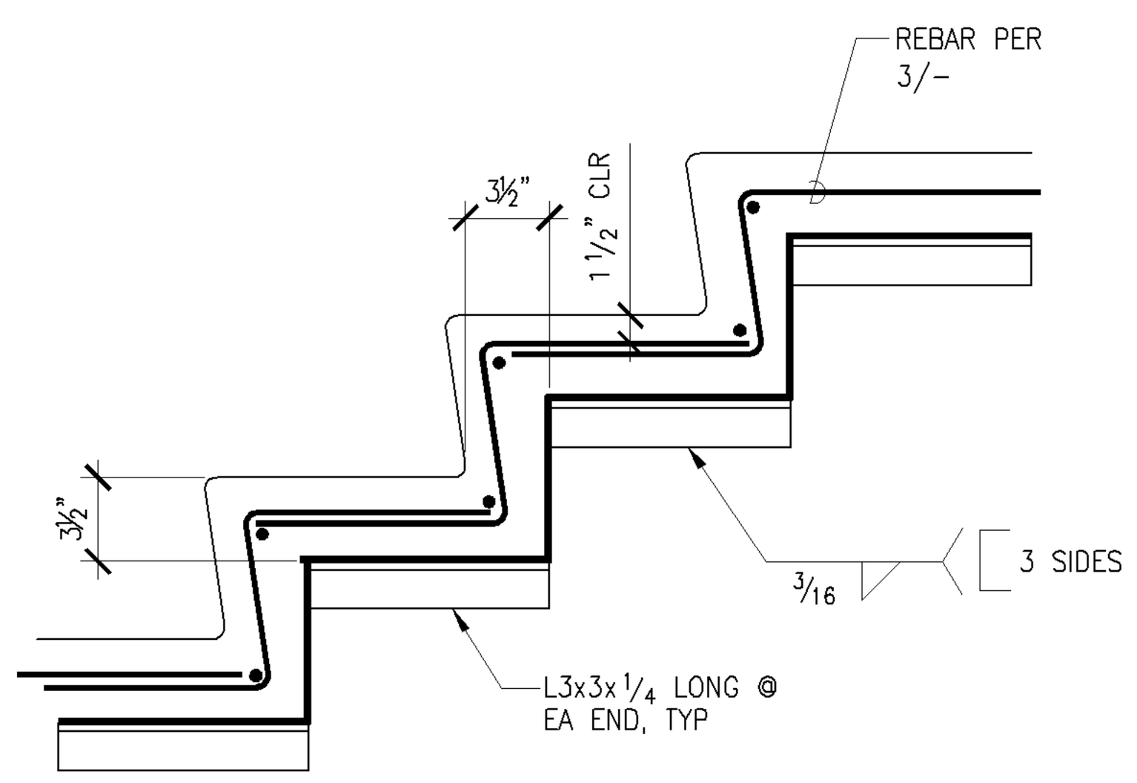
STRINGER CONNECTION AT FLOOR 1"=1'-0" | 1



STRINGER CONNECTION AT LANDING 1"=1'-0" | 2



STAIR DETAIL 1 1/2"=1'-0" | 3



STAIR DETAIL 1 1/2"=1'-0" | 4

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

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515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
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2908 OREGON COURT, SUITE 1-7
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3	03-22-07	FOR CONSTRUCTION
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1	10-23-06	BLDG & SAFETY SUBMITTAL

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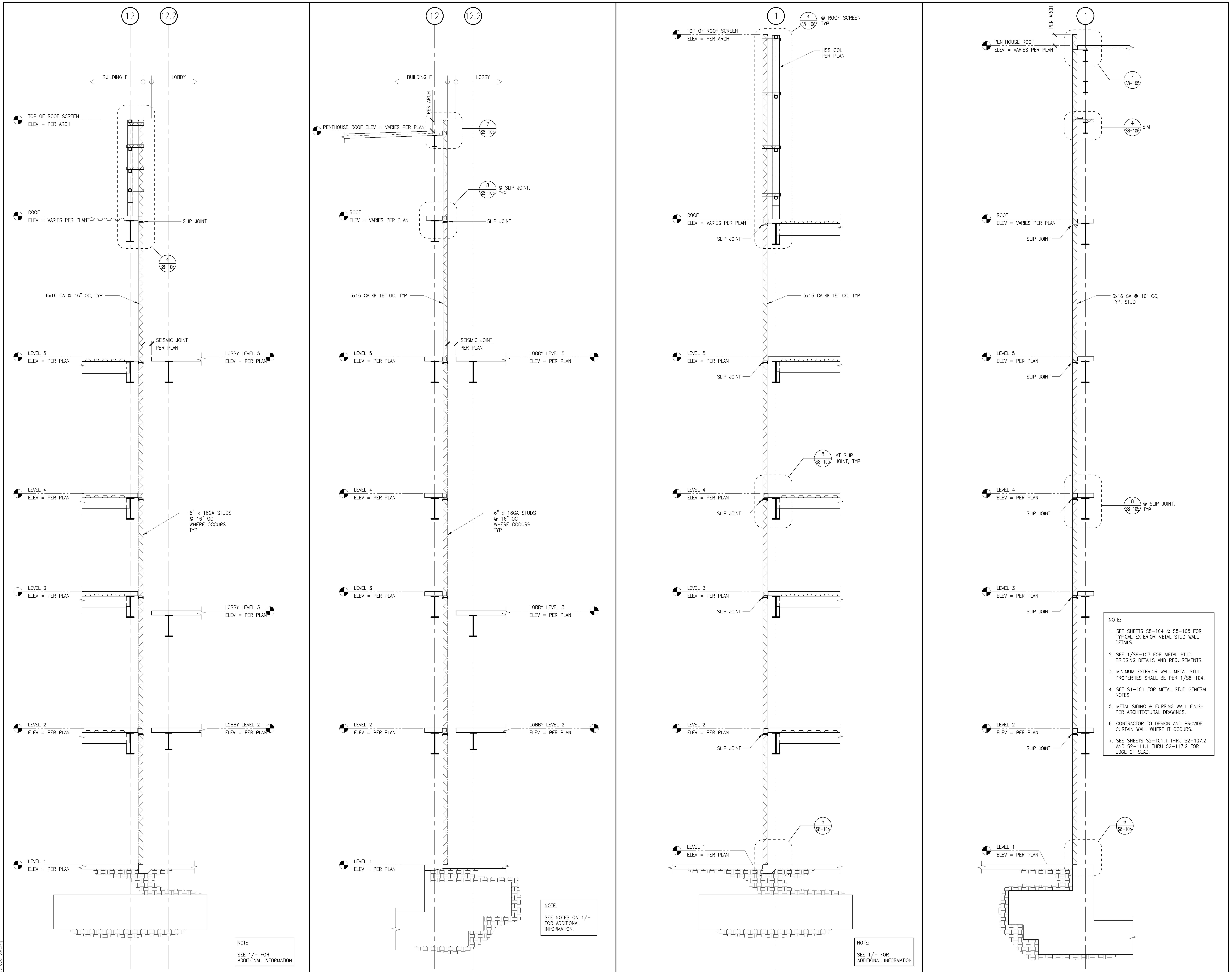
KEY PLAN

SHEET TITLE

STAIR DETAILS

S7-202



EXTERIOR SECTION AT BUILDING F (NORTH) 1/4"=1'-0" 4

EXTERIOR SECTION AT BUILDING F (NORTH) 1/4"=1'-0" 3

EXTERIOR SECTION AT BUILDING F (SOUTH) 1/4"=1'-0" 2

EXTERIOR SECTION AT BUILDING F (SOUTH) 1/4"=1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

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LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

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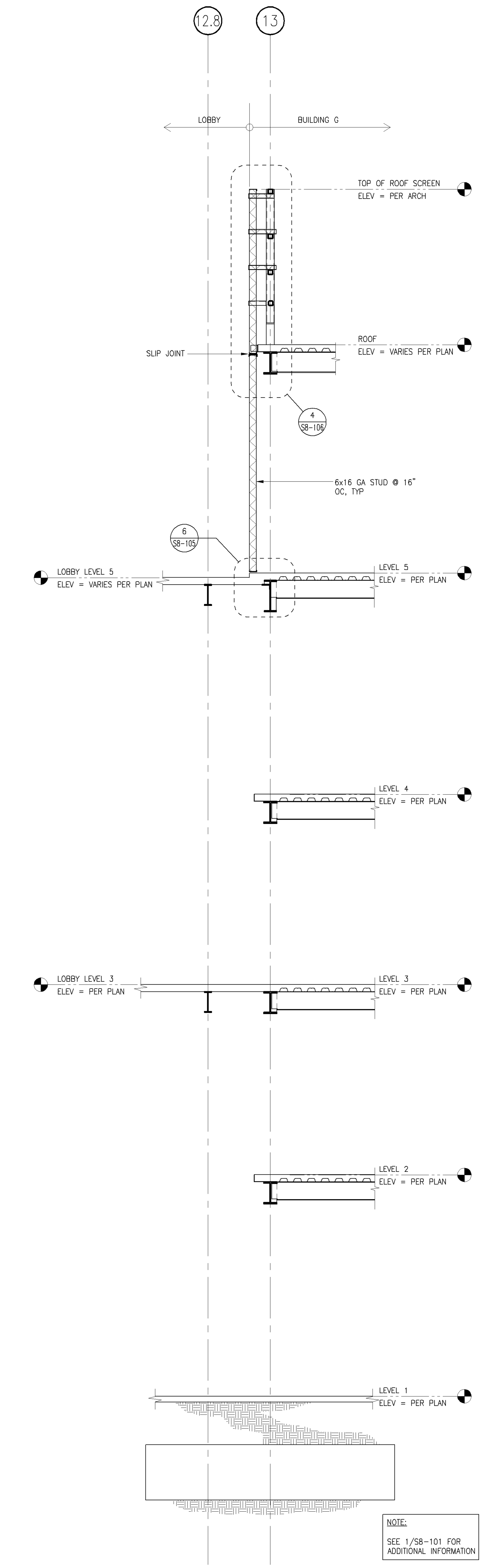
KEY PLAN

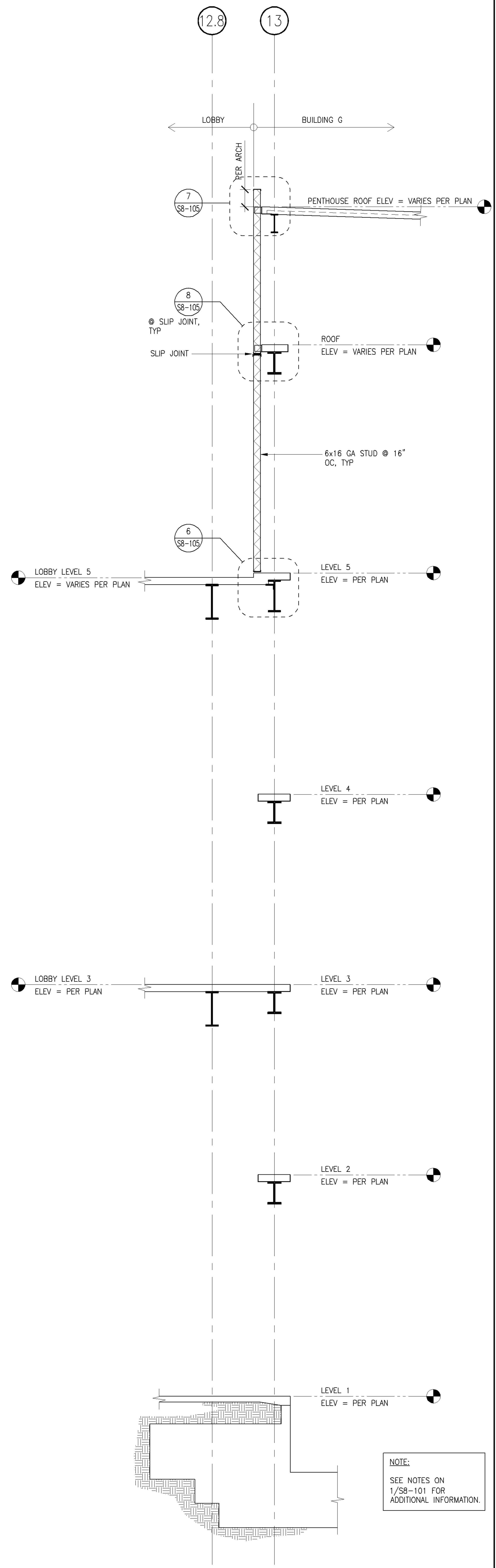
SHEET TITLE

EXTERIOR METAL STUD WALL SECTIONS

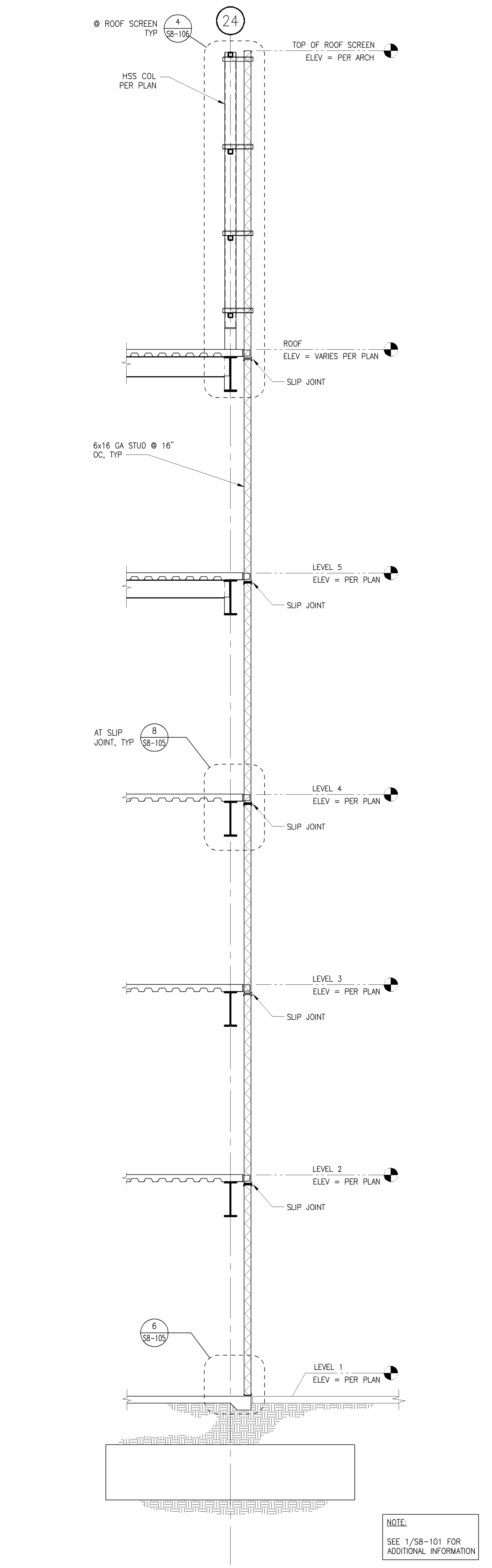
S8-101



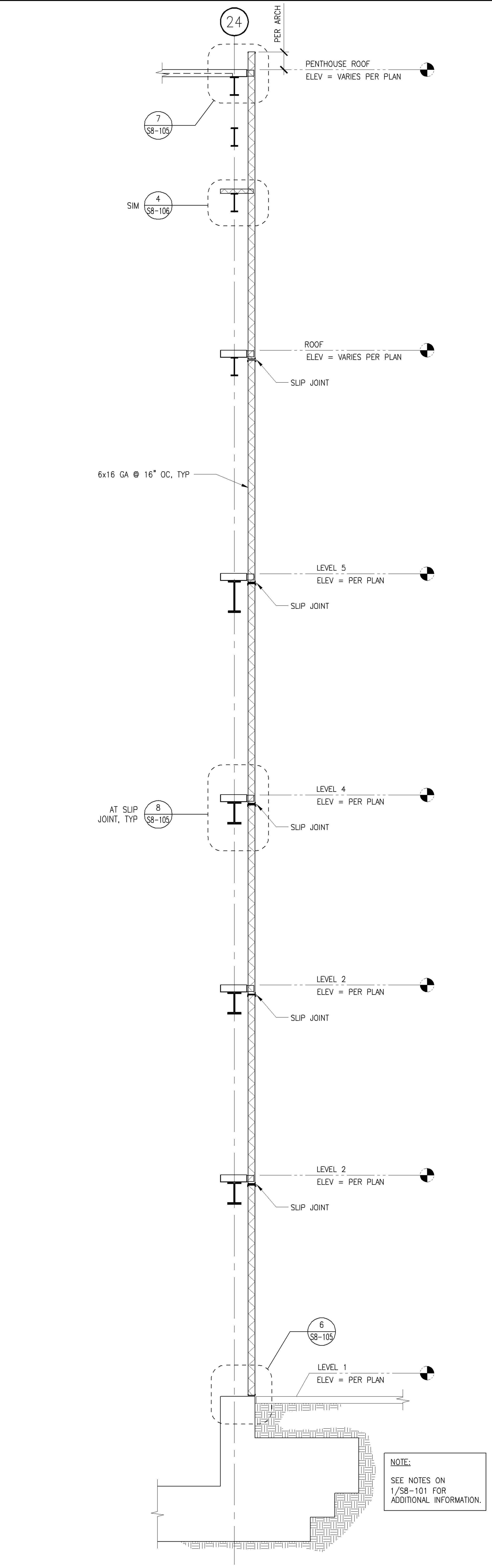
EXTERIOR SECTION AT BUILDING G (WEST) 1/4"=1'-0" 4



EXTERIOR SECTION AT BUILDING G (WEST) 1/4"=1'-0" 3



EXTERIOR SECTION AT BUILDING G (EAST) 1/4"=1'-0" 2



EXTERIOR SECTION AT BUILDING G (EAST) 1/4"=1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

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LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

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MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

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LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

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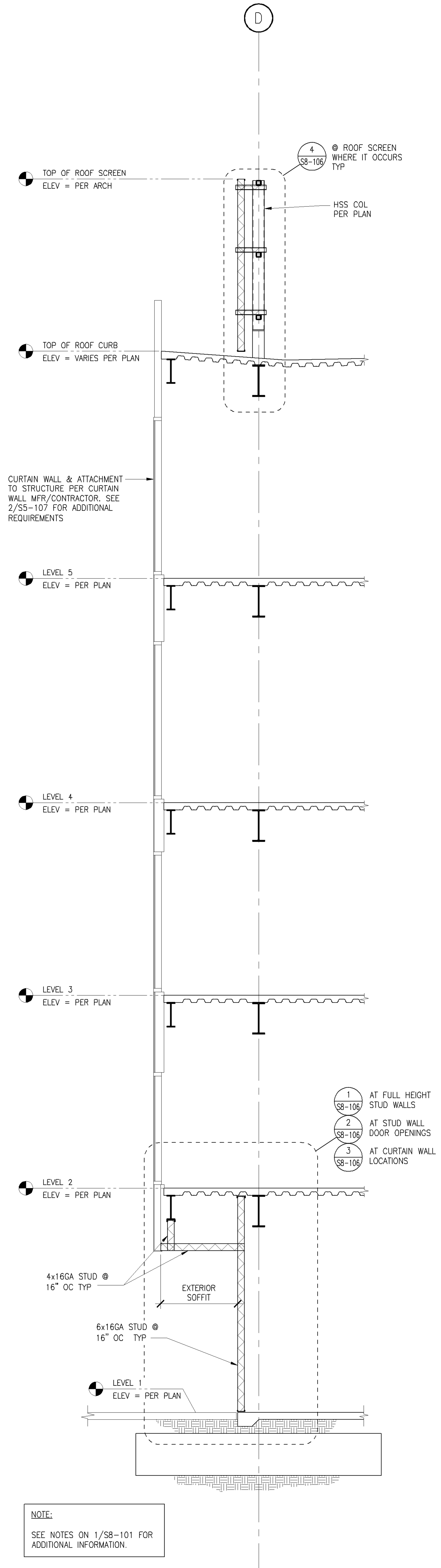
KEY PLAN

SHEET TITLE

EXTERIOR METAL STUD WALL SECTIONS

S8-102

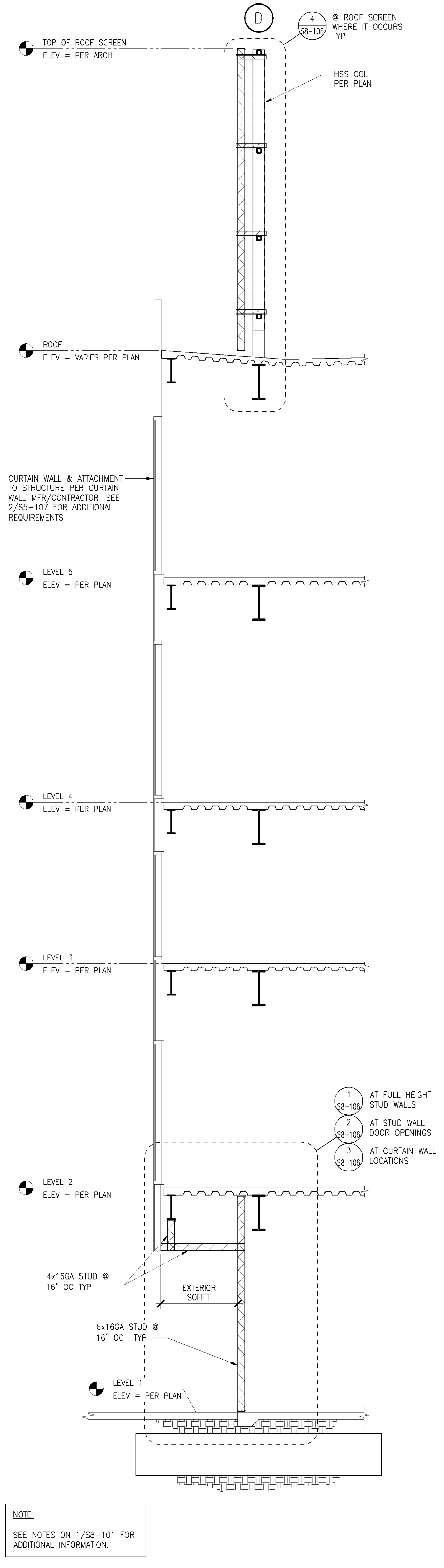


NOTE:

SEE NOTES ON 1/S8-101 FOR ADDITIONAL INFORMATION.

EXTERIOR SECTION AT BUILDING G

1/4"=1'-0" 2



NOTE:

SEE NOTES ON 1/S8-101 FOR ADDITIONAL INFORMATION.

EXTERIOR SECTION AT BUILDING F

1/4"=1'-0" 1

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

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LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029

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2 N LAKE AVENUE, SUITE 820
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2908 OREGON COURT, SUITE 1-7
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REGISTERED PROFESSIONAL ENGINEER ALDRIN J. ... No. S4399 Exp. 12/31/07 STRUCTURAL STATE OF CALIFORNIA

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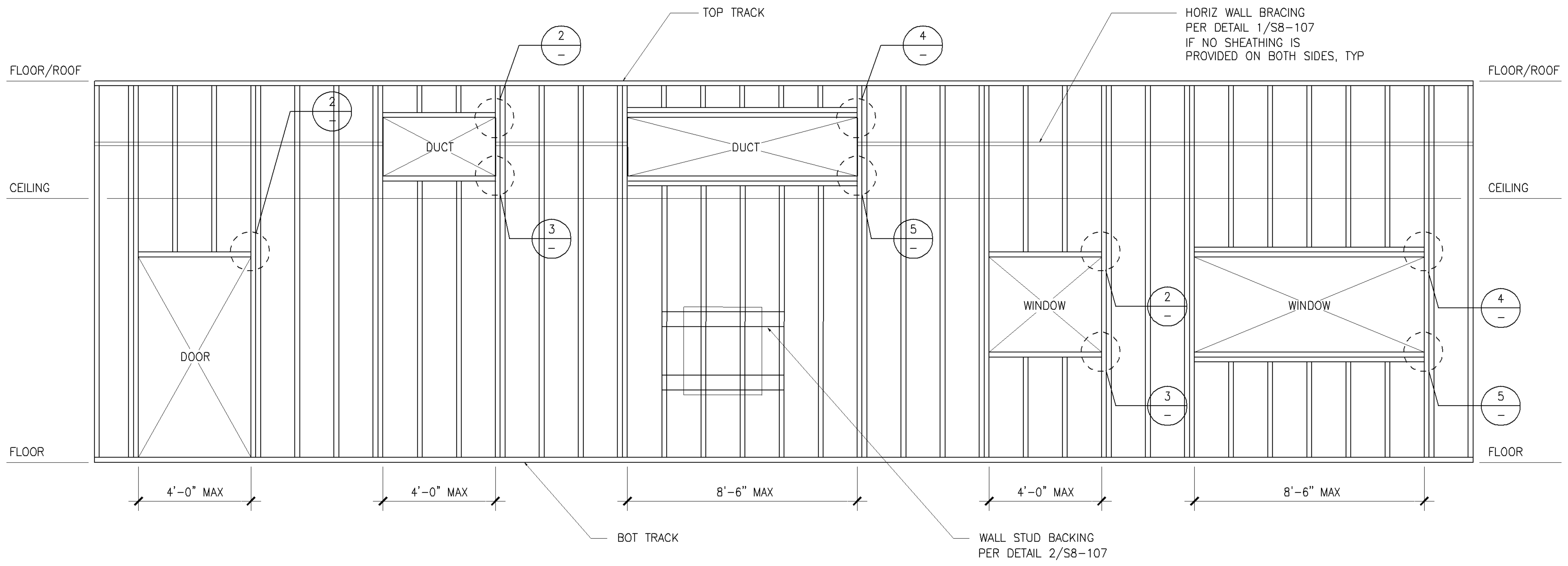
KEY PLAN

SHEET TITLE

EXTERIOR METAL STUD WALL SECTIONS

S8-103



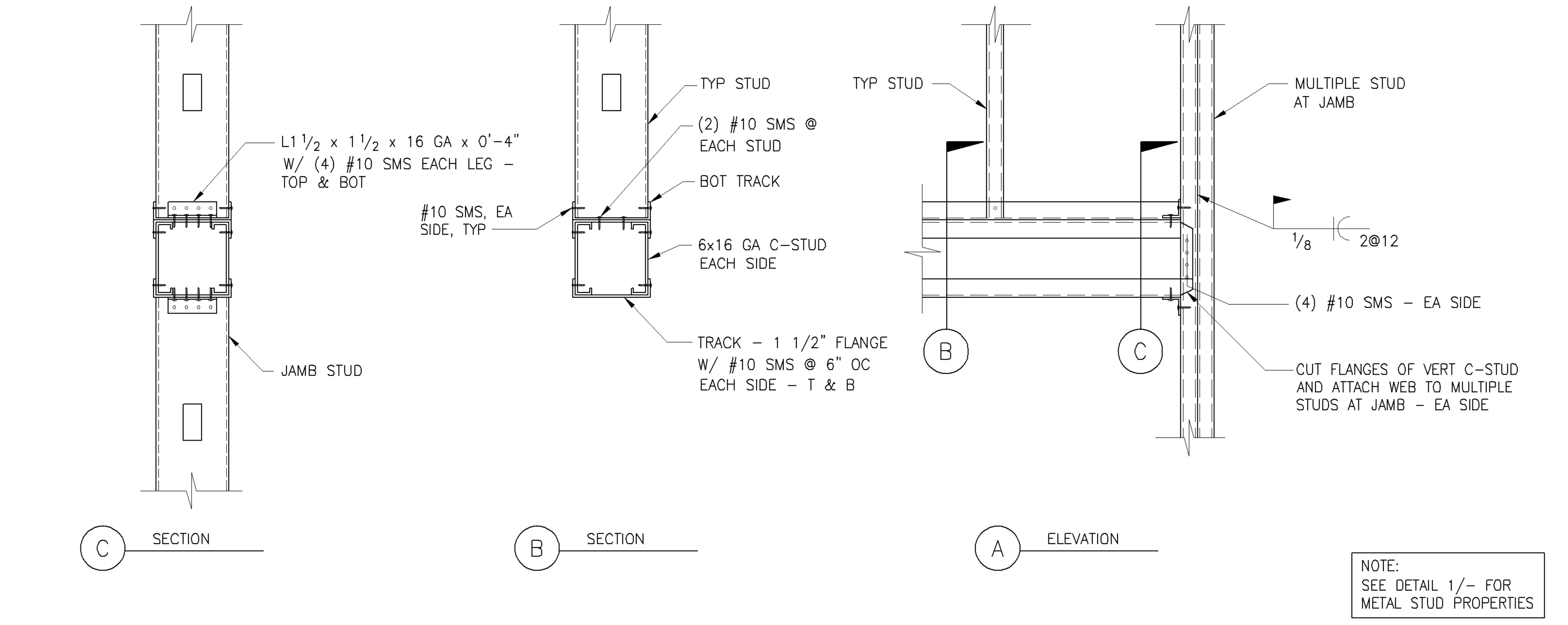
TYPICAL WALL STUD PROPERTIES 6" WALL STUDS @ 16" OC MAX		DEPTH	THICKNESS	FLANGE WIDTH	GROSS AREA (in^2)	EFFECTIVE Sxx (in^3)	EFFECTIVE Ixx (in^4)
TYP 6" EXTERIOR WALL	C-STUD	6"	16 GA	1 3/8"	0.514	0.809	2.518
	TOP TRACK	6"	16 GA	1 1/2"	0.509	0.714	2.464
	BOTTOM TRACK	6"	16 GA	1 1/2"	0.509	0.714	2.464
	DEFLECTION TRACK	6"	12 GA	2"	1.015	1.568	5.558
	NESTED TRACK	6"	16 GA	2 1/2"	0.622	0.733	3.447

TYPICAL WALL STUD PROPERTIES 4" WALL STUDS @ 16" OC MAX		DEPTH	THICKNESS	FLANGE WIDTH	GROSS AREA (in^2)	EFFECTIVE Sxx (in^3)	EFFECTIVE Ixx (in^4)
TYP 4" EXTERIOR WALL	C-STUD	4"	16 GA	1 3/8"	0.401	0.428	0.929
	TOP TRACK	4"	16 GA	1 1/2"	0.396	0.374	0.918
	BOTTOM TRACK	4"	16 GA	1 1/2"	0.396	0.374	0.918
	DEFLECTION TRACK	4"	16 GA	2"	0.452	0.397	1.037
	NESTED TRACK	4"	16 GA	2 1/2"	0.509	0.445	1.205

NOTES:
1. ALL METAL STUD FRAMING SHALL BE IN COMPLIANCE WITH SSMA – STEEL STUD MANUFACTURERS ASSOCIATION (ICBO ER-4943P).
2. ALL METAL STUD MEMBERS SHALL COMPLY WITH THE MINIMUM SECTION PROPERTIES AS INDICATED IN THE TABLE ABOVE (UNO).
3. SEE ARCHITECTURAL DRAWINGS FOR DEPTH AND LOCATION OF WALLS.
4. ALL WALL MOUNT EQUIPMENT, CABINETS, & PLUMBING FIXTURES USE 16 GA WALL STUD PROPERTIES.

TYPICAL EXTERIOR METAL STUD WALL FRAMING

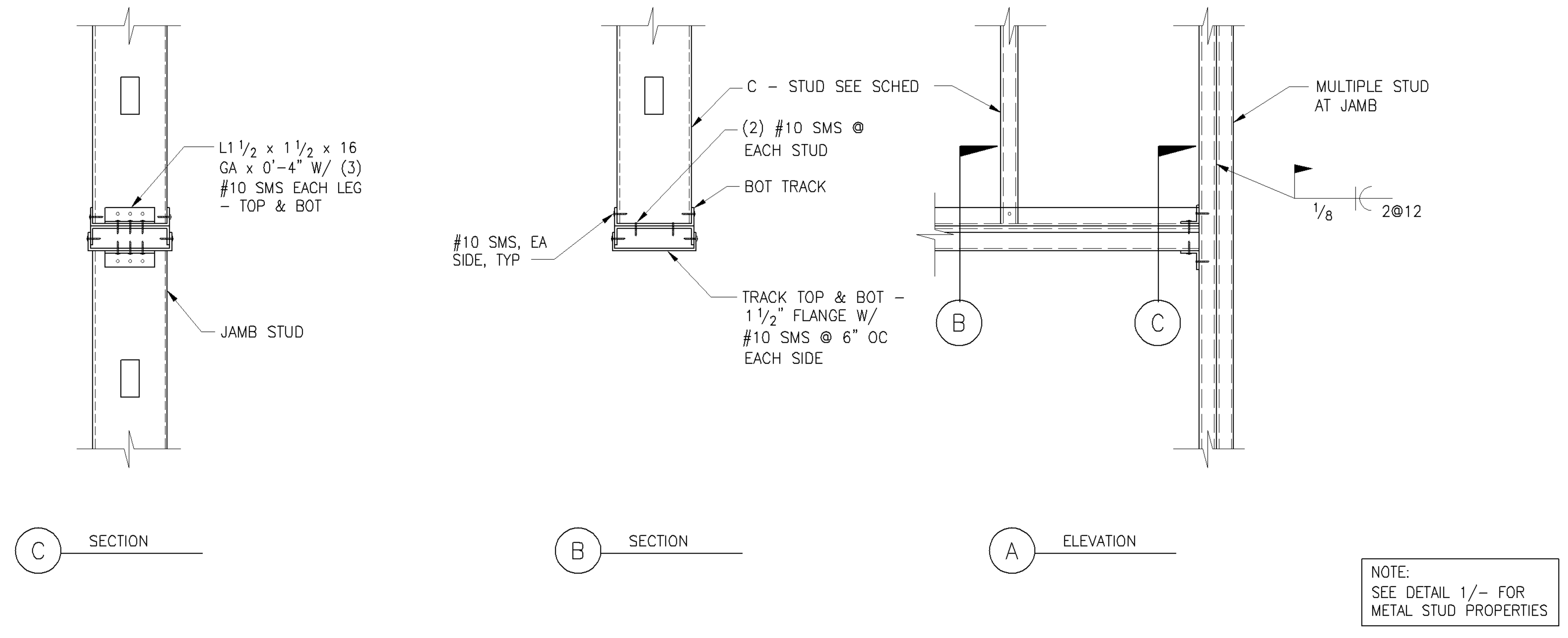
3/8"=1'-0" | 1



NOTE:
SEE DETAIL 1/– FOR METAL STUD PROPERTIES

HEADER & JAMB DETAIL FOR 8'-6" MAX OPENING

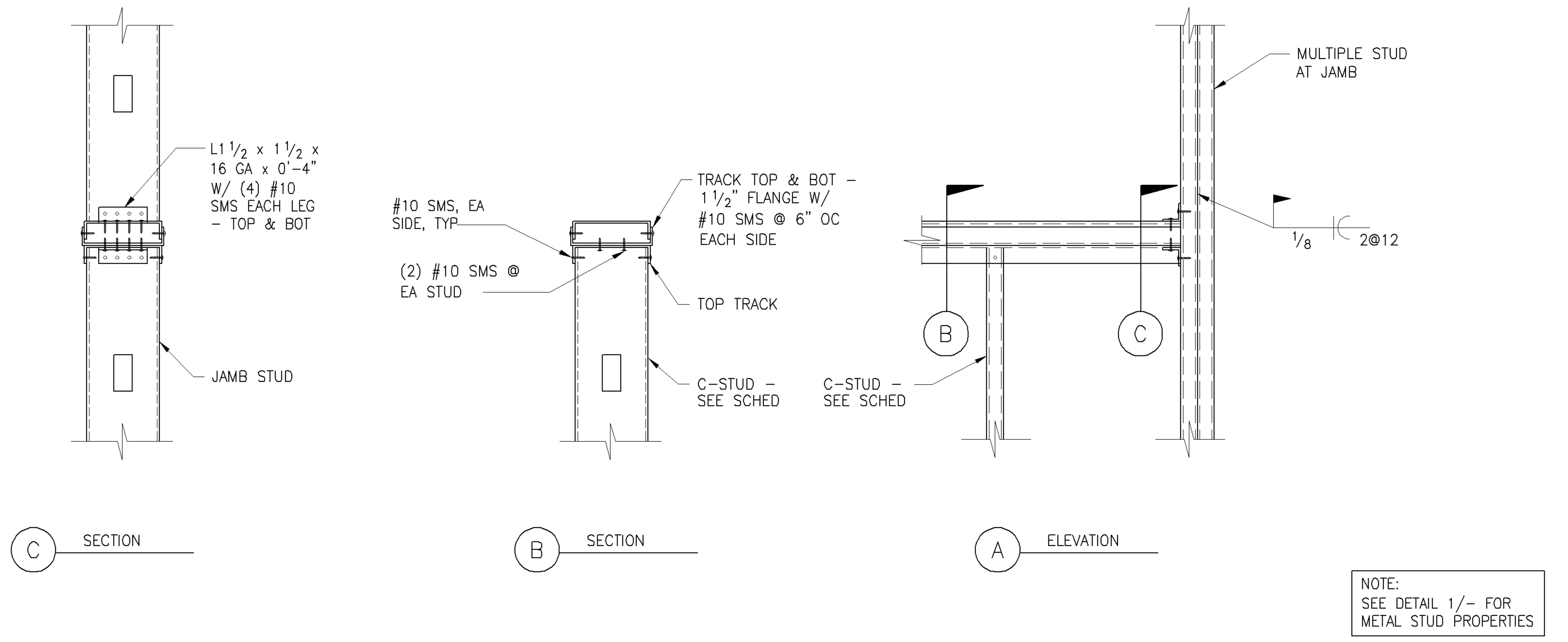
1 1/2"=1'-0" | 4



NOTE:
SEE DETAIL 1/– FOR METAL STUD PROPERTIES

HEADER & JAMB DETAIL FOR 4'-0" MAX OPENING

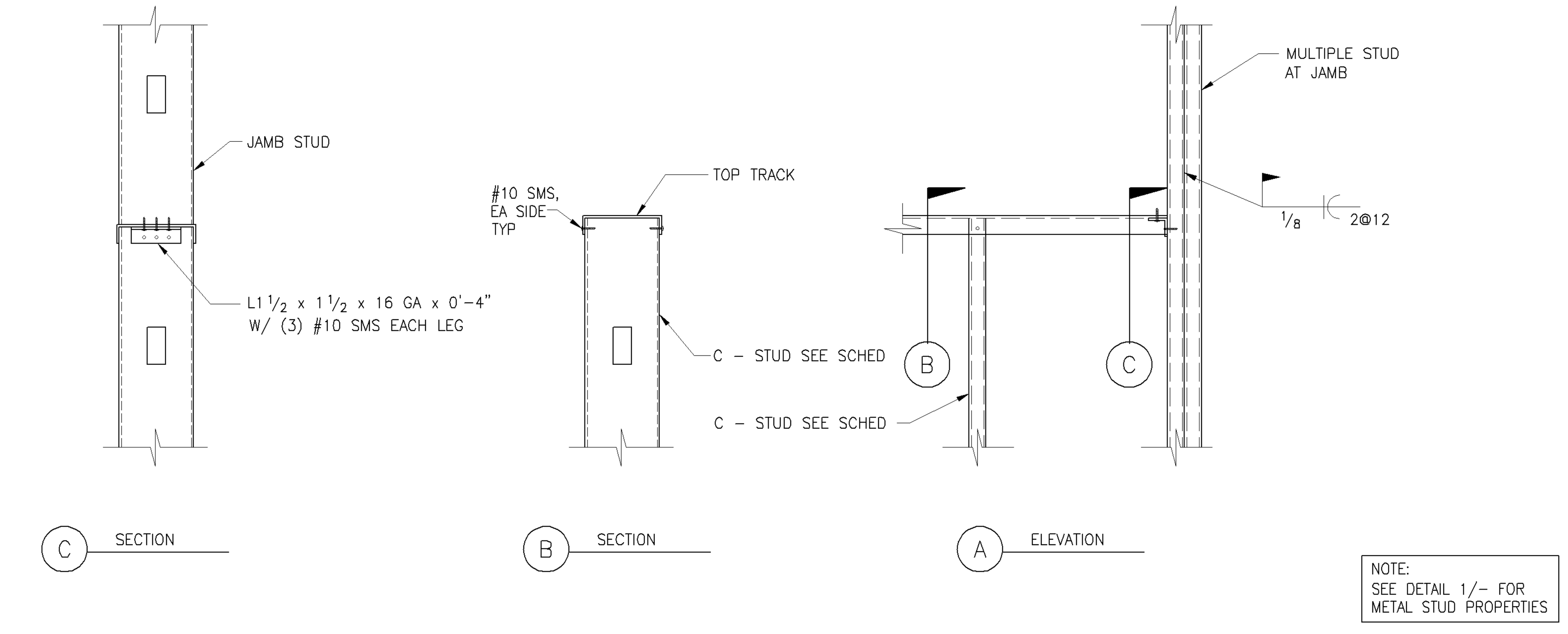
1 1/2"=1'-0" | 2



NOTE:
SEE DETAIL 1/– FOR METAL STUD PROPERTIES

SILL & JAMB DETAIL FOR 8'-6" MAX OPENING

1 1/2"=1'-0" | 5



NOTE:
SEE DETAIL 1/– FOR METAL STUD PROPERTIES

SILL & JAMB DETAIL FOR 4'-0" MAX OPENING

1 1/2"=1'-0" | 3

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

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LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
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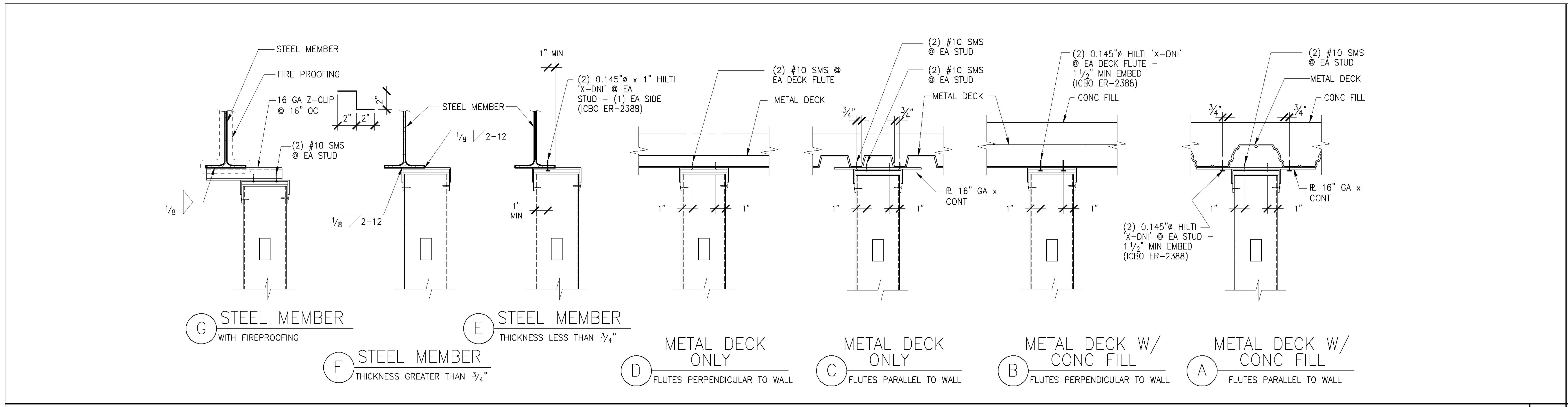
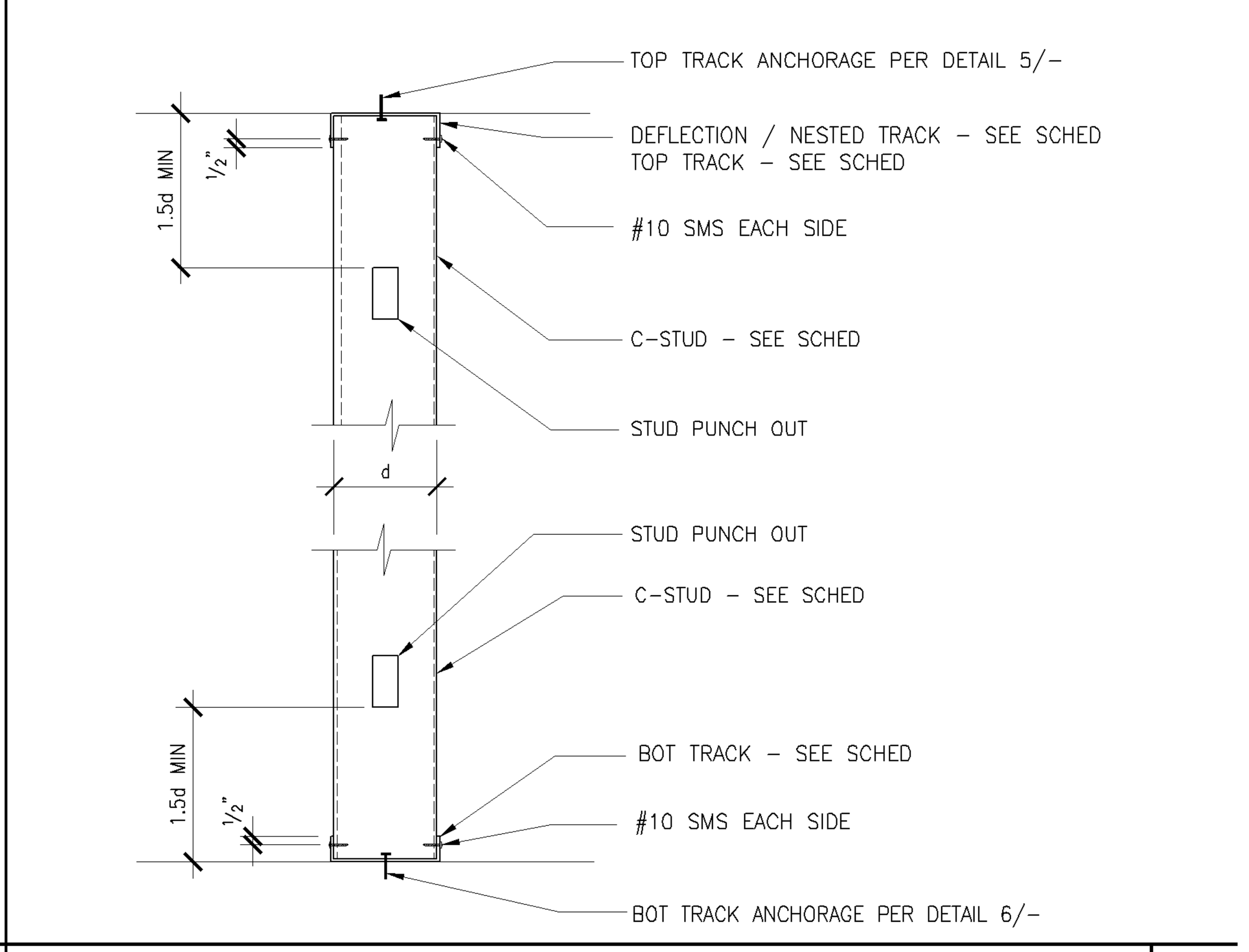
SHEET TITLE

TYPICAL EXTERIOR METAL STUD WALL DETAILS

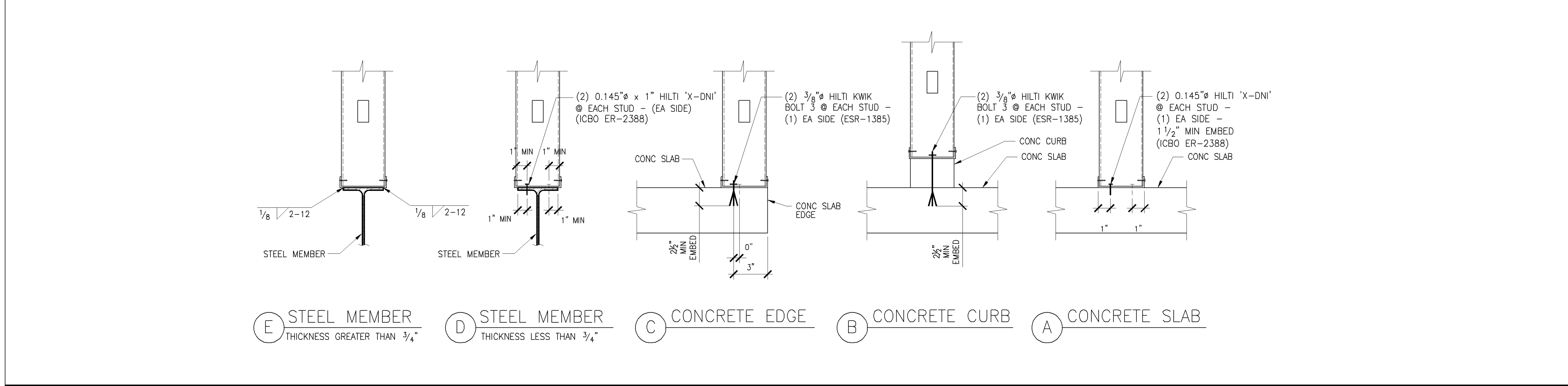
S8-104



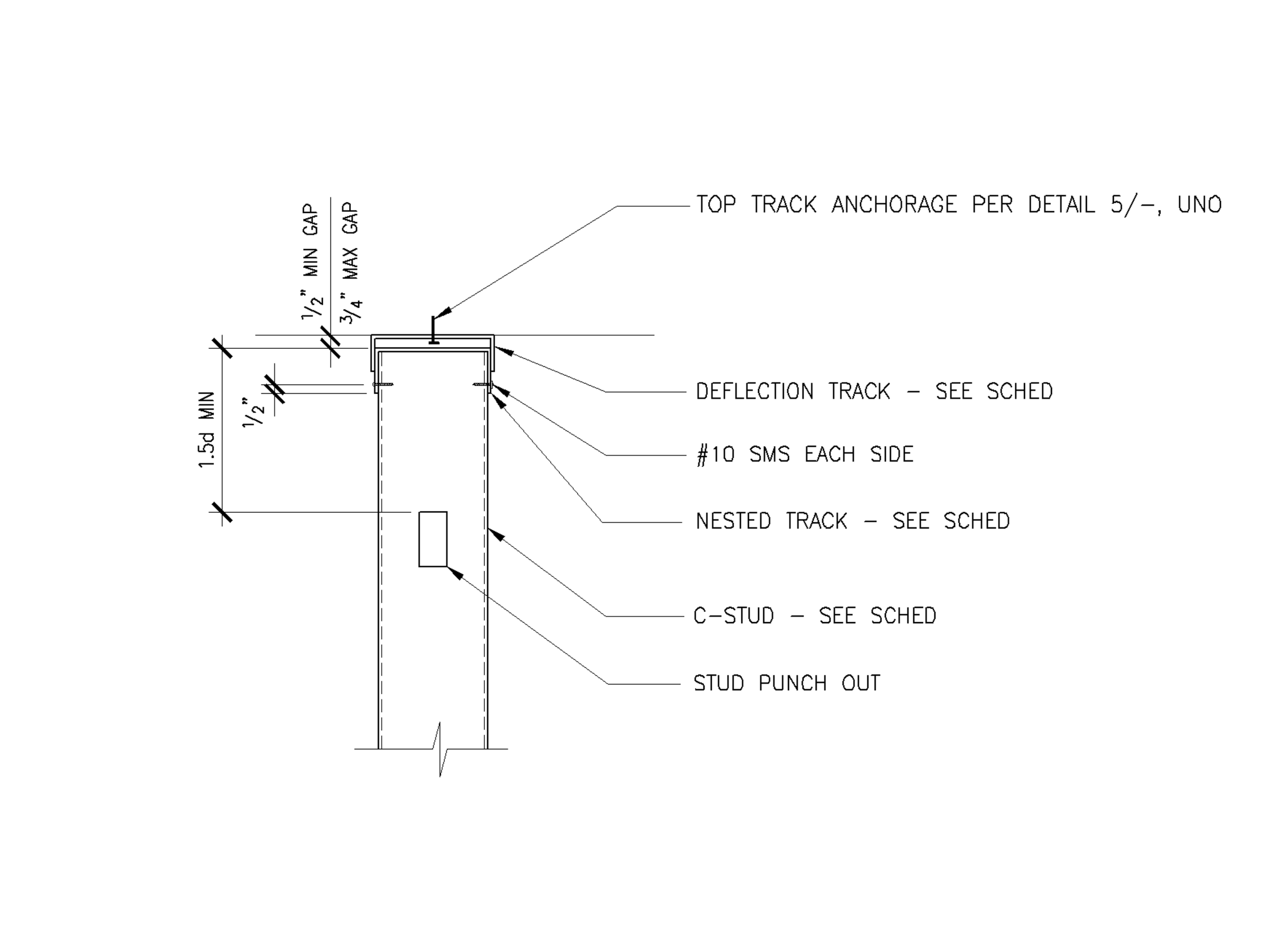
TYPICAL EXTERIOR TOP DEFLECTION TRACK ANCHORAGE 1 1/2"=1'-0" 5



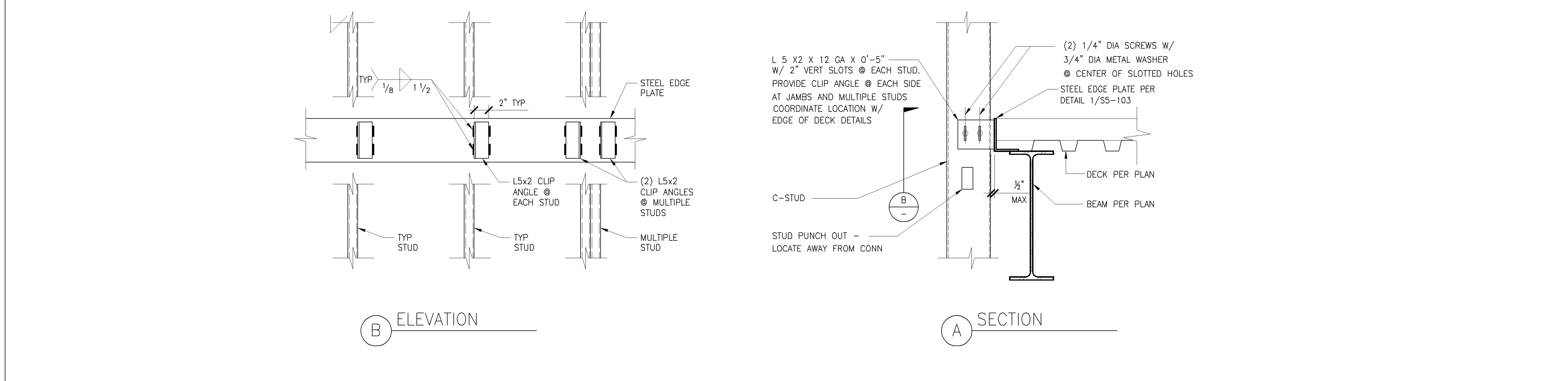
TYPICAL EXTERIOR STUD WALL STUD 1 1/2"=1'-0" 1



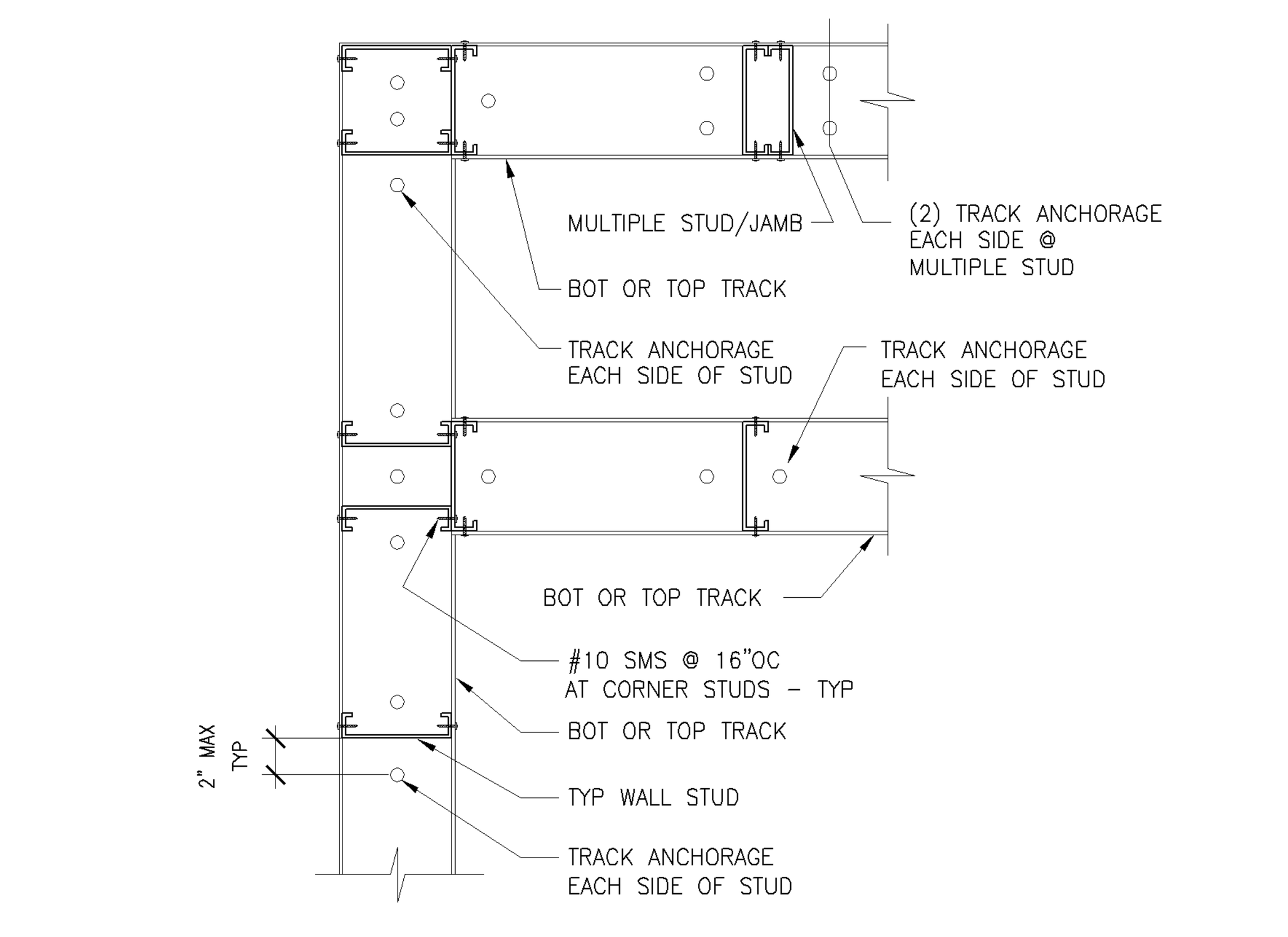
TYPICAL EXTERIOR BOT TRACK ANCHORAGE 1 1/2"=1'-0" 6



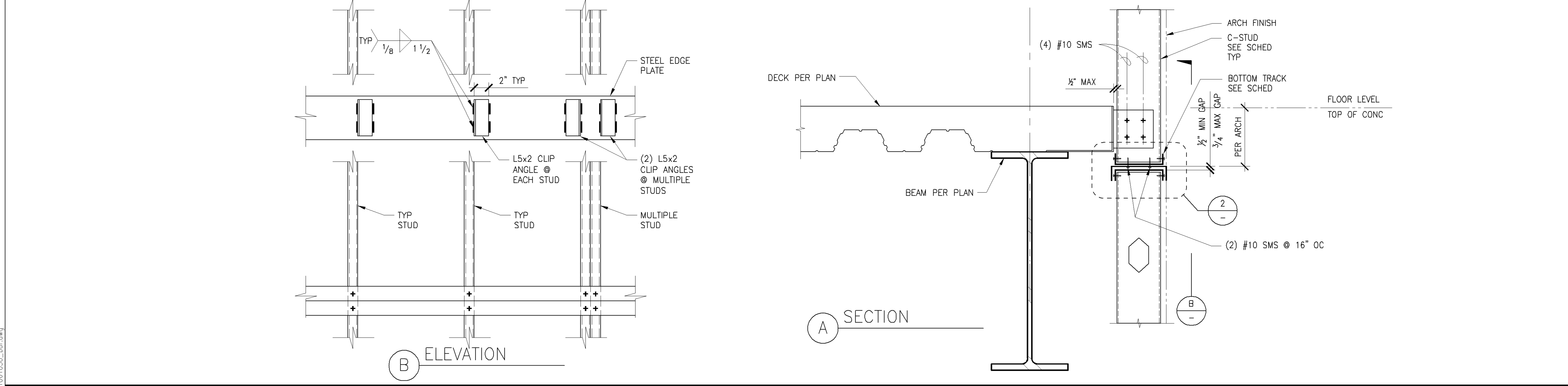
TYPICAL EXTERIOR DEFLECTION TRACK ASSEMBLY 1 1/2"=1'-0" 2



TYPICAL EXTERIOR STUD WALL SLIP CLIP CONNECTION 1 1/2"=1'-0" 7



TYPICAL EXTERIOR STUD WALL FRAMING - PLAN VIEW 1 1/2"=1'-0" 3



TYPICAL EXTERIOR STUD WALL FIXED CLIP CONNECTION 1 1/2"=1'-0" 8

NOT USED - 4

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

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440 STATE PLACE
ESCONDIDO, CA 92029

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PASADENA, CA 91101

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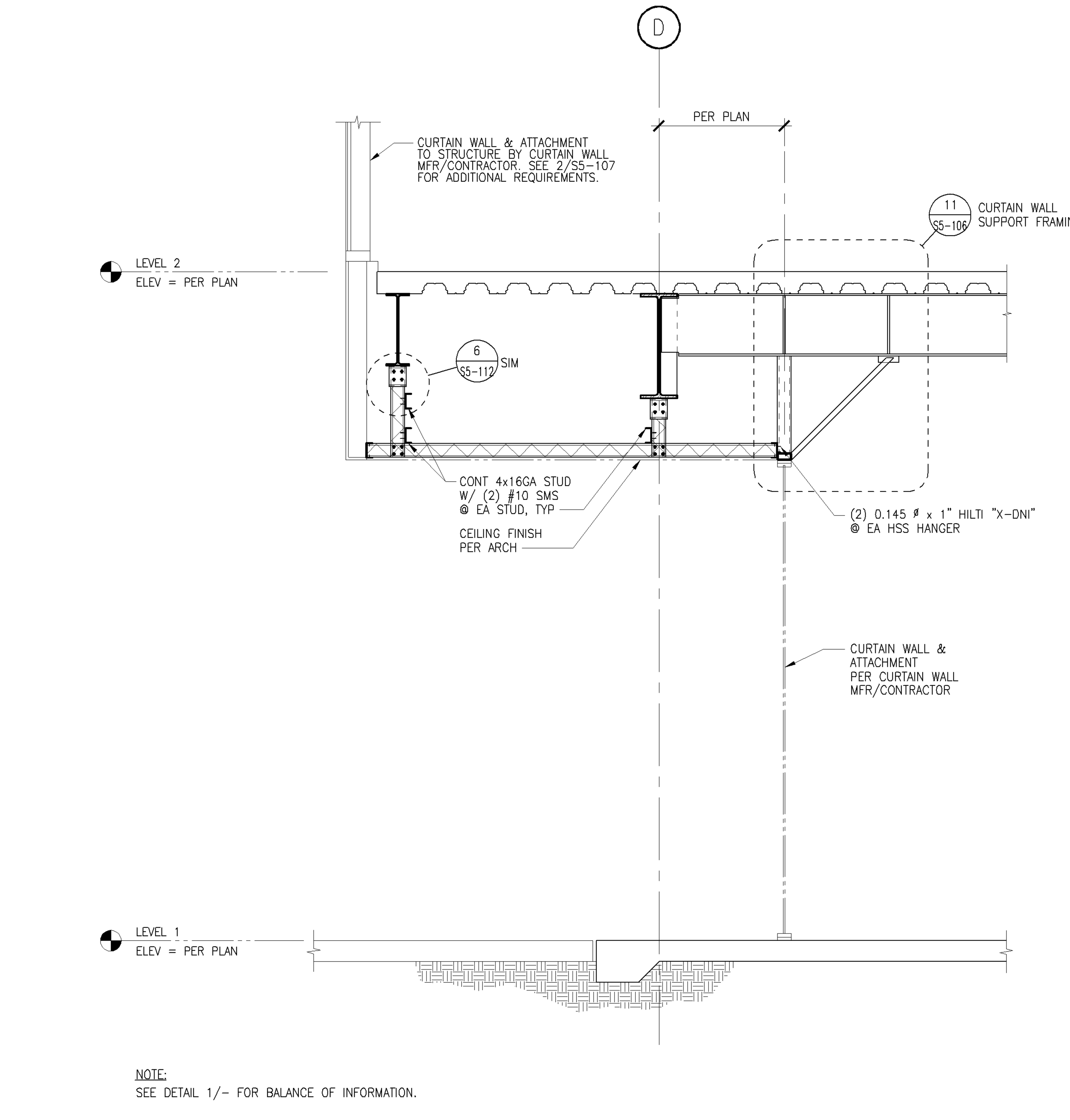
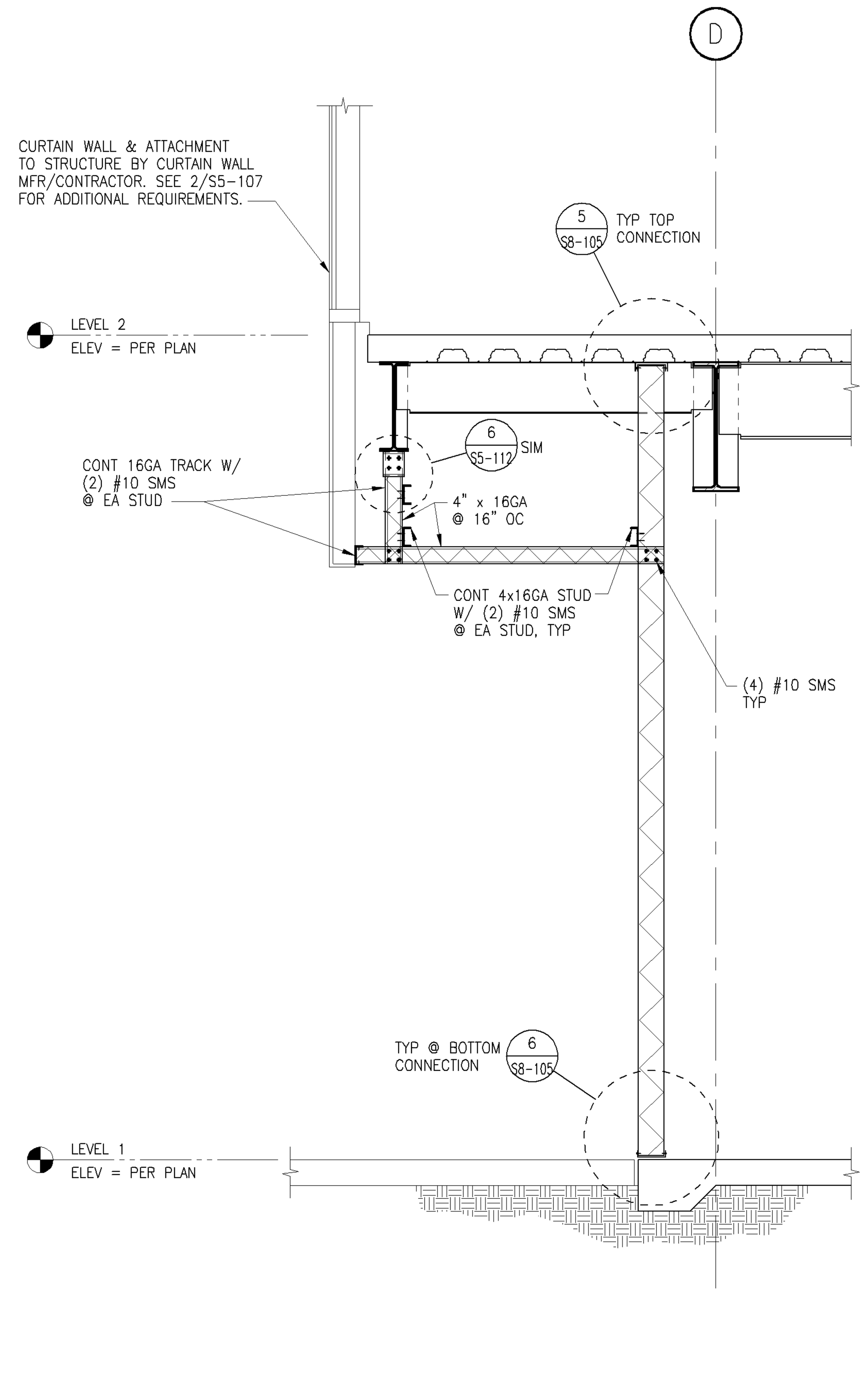
SHEET TITLE

TYPICAL EXTERIOR METAL STUD WALL DETAILS

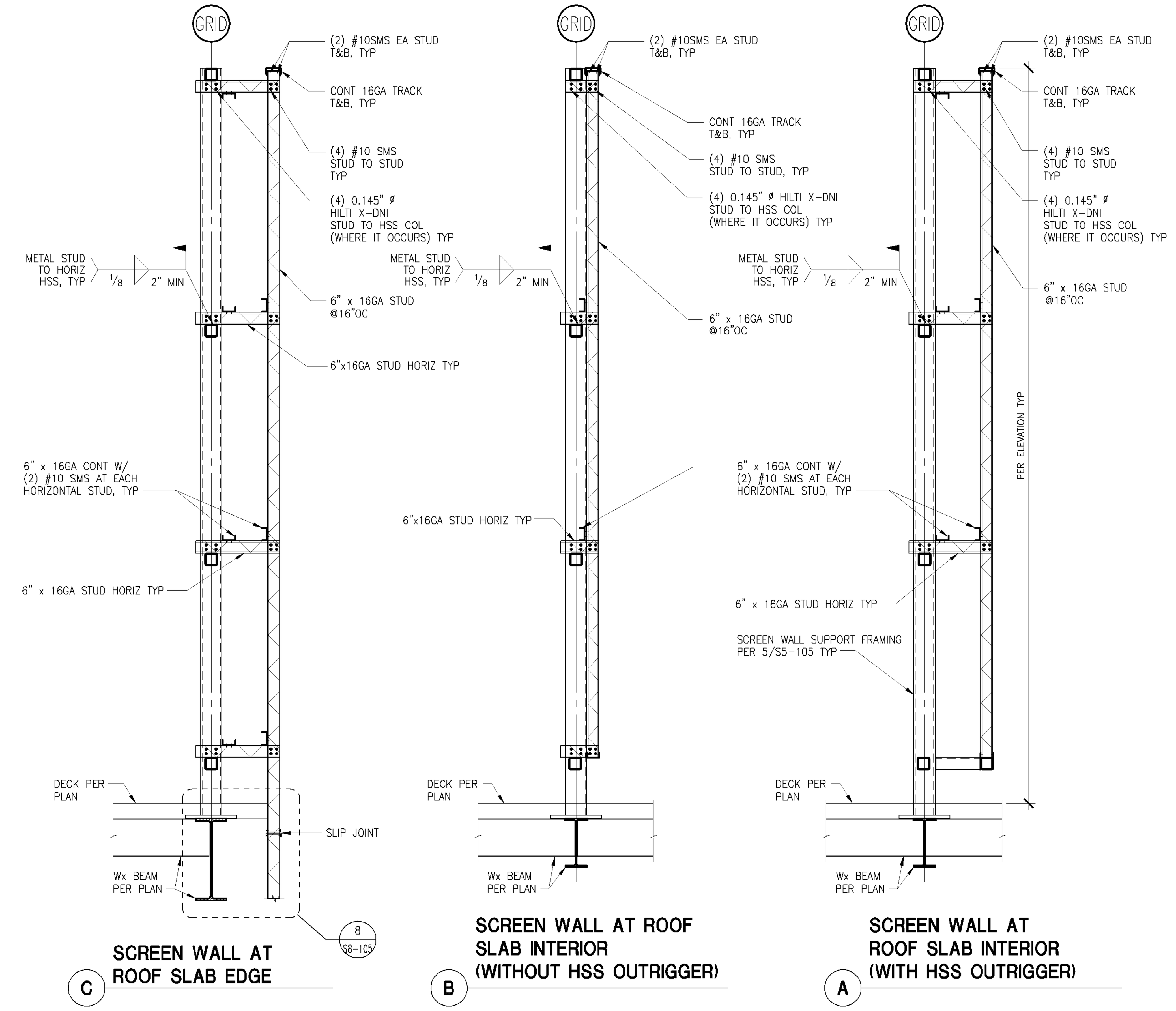
S8-105



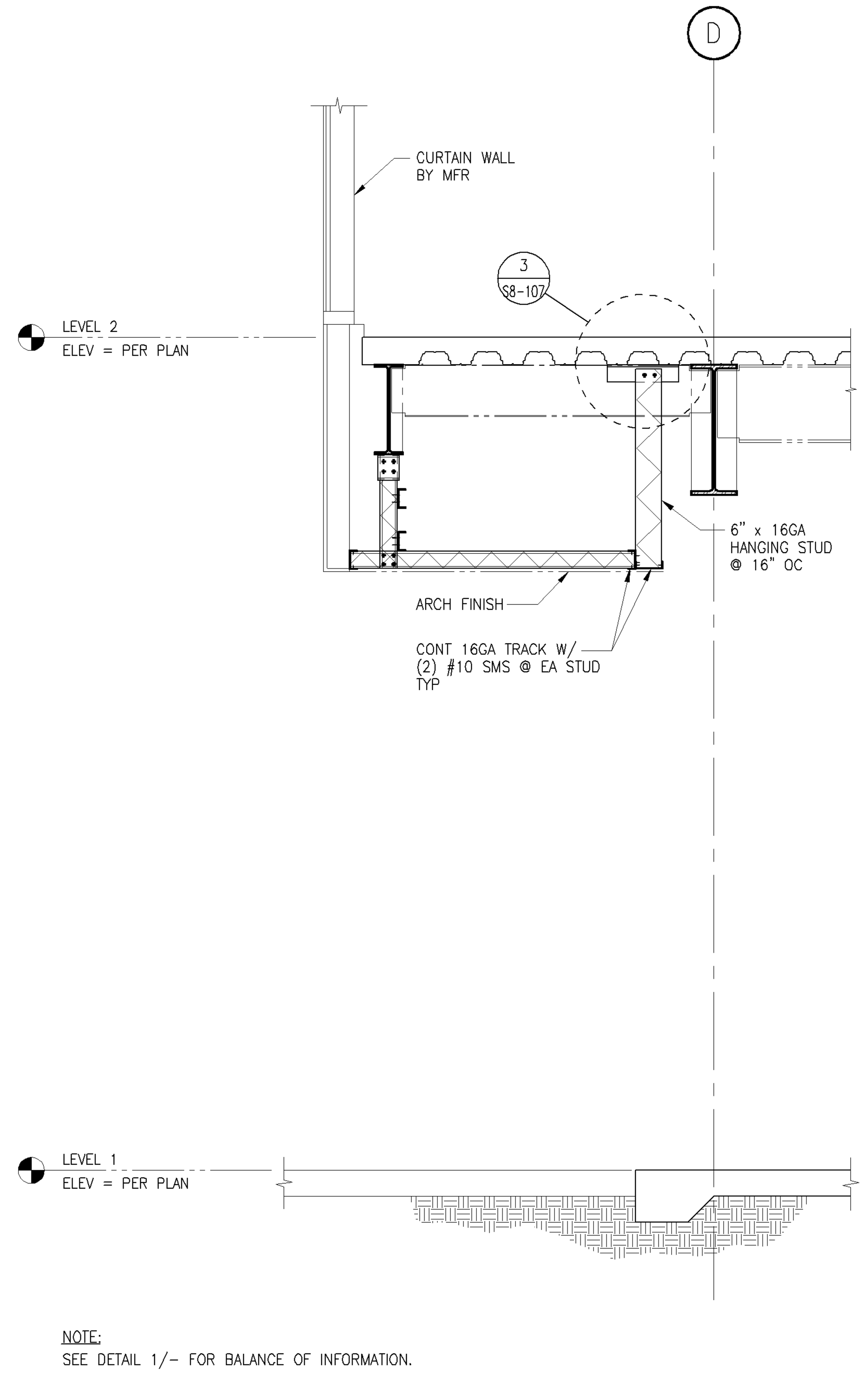
EXTERIOR SOFFIT SECTION AT CURTAIN WALL SUPPORT FRAMING ½"=1'-0" 3



EXTERIOR SOFFIT SECTION AT STUD WALL ½"=1'-0" 1



METAL STUD FRAMING AT ROOF SCREEN WALLS ½"=1'-0" 4



EXTERIOR SOFFIT SECTION AT STUD WALL OPENING ½"=1'-0" 2

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
26531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM
515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
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CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

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STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

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LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE I-7
TORRANCE, CA 90503

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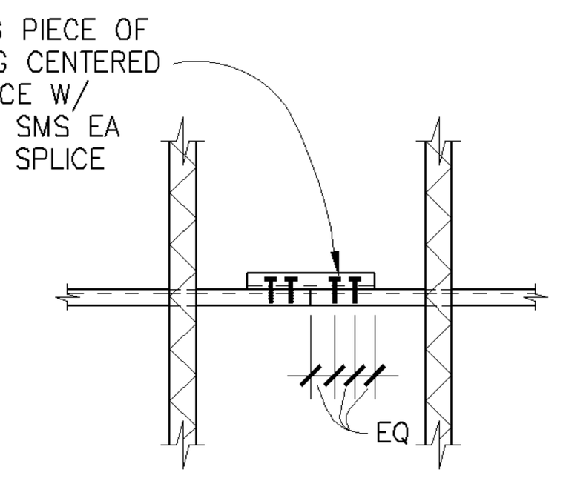
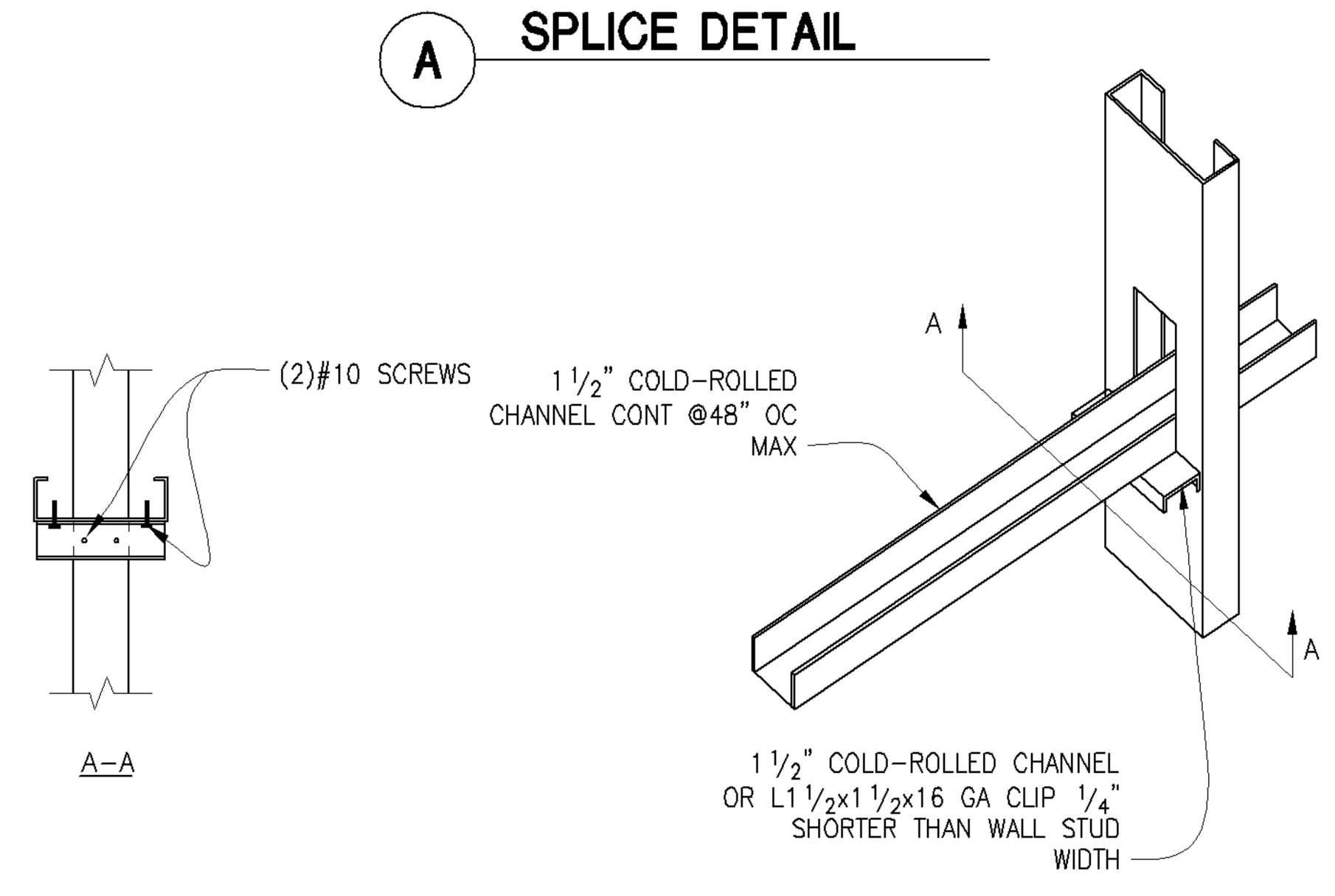
KEY PLAN

SHEET TITLE
EXTERIOR METAL STUD WALL DETAILS

S8-106



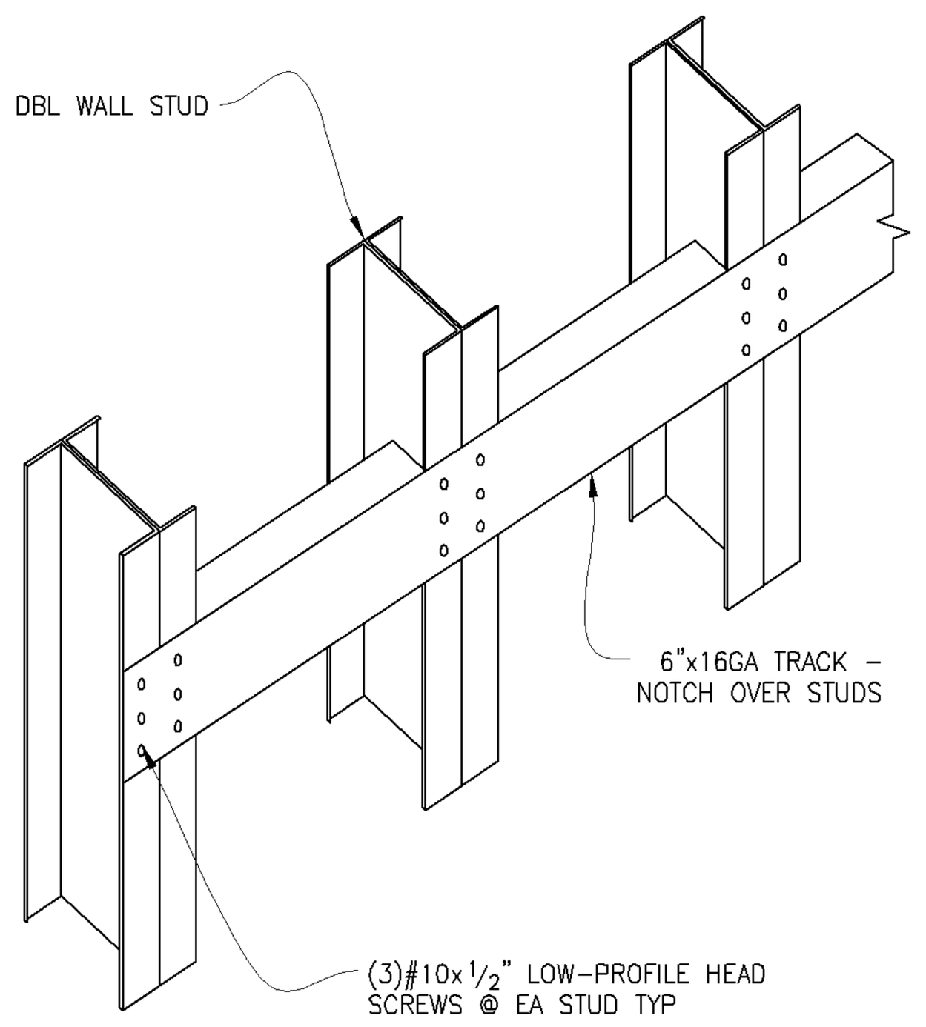
(A) SPLICE DETAIL



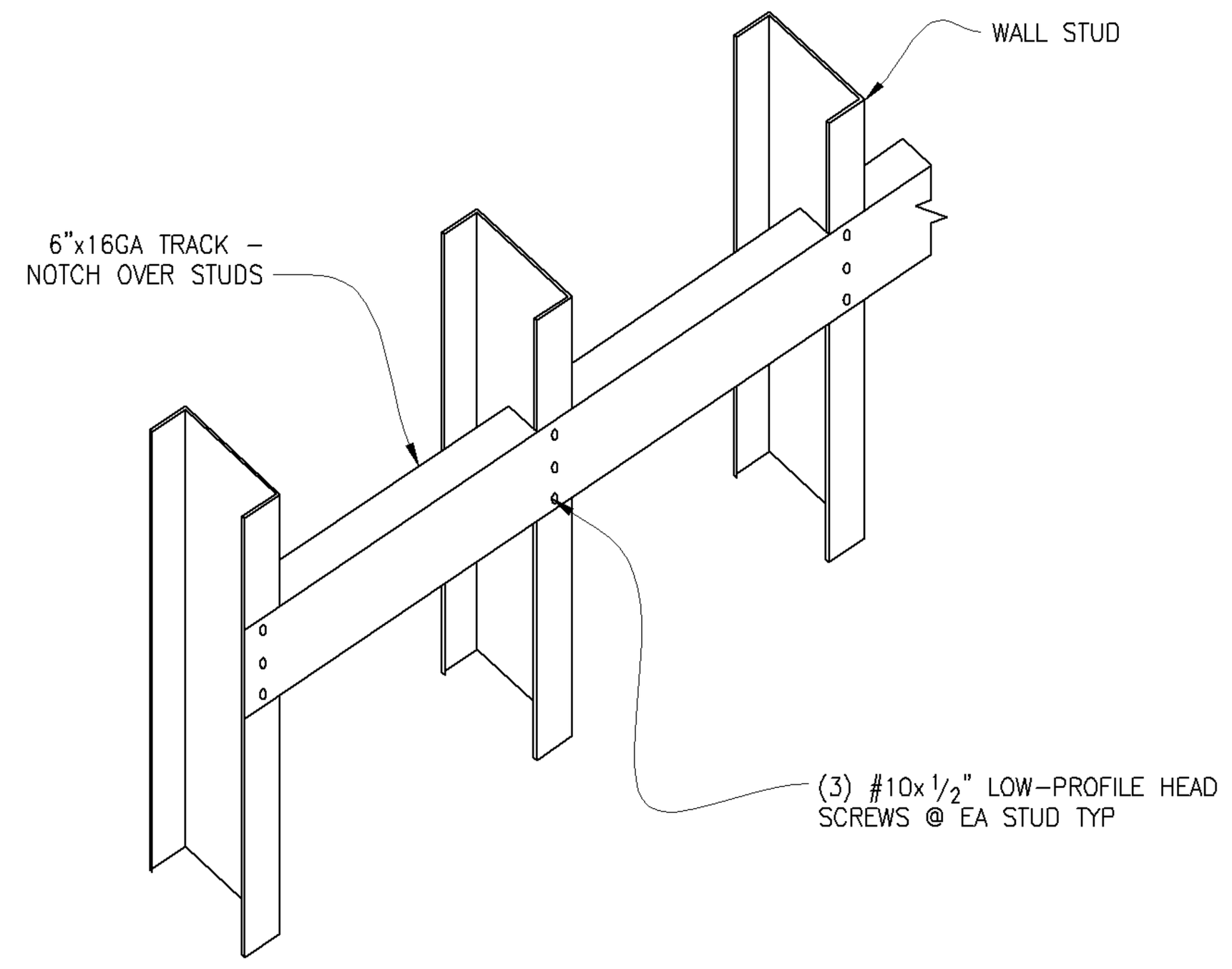
(B) BRIDGING OPTION

METAL STUD FRAMING

1"=1'-0" | 1



(B) WALL MOUNTED ITEMS (150 LBS TO 300 LBS)

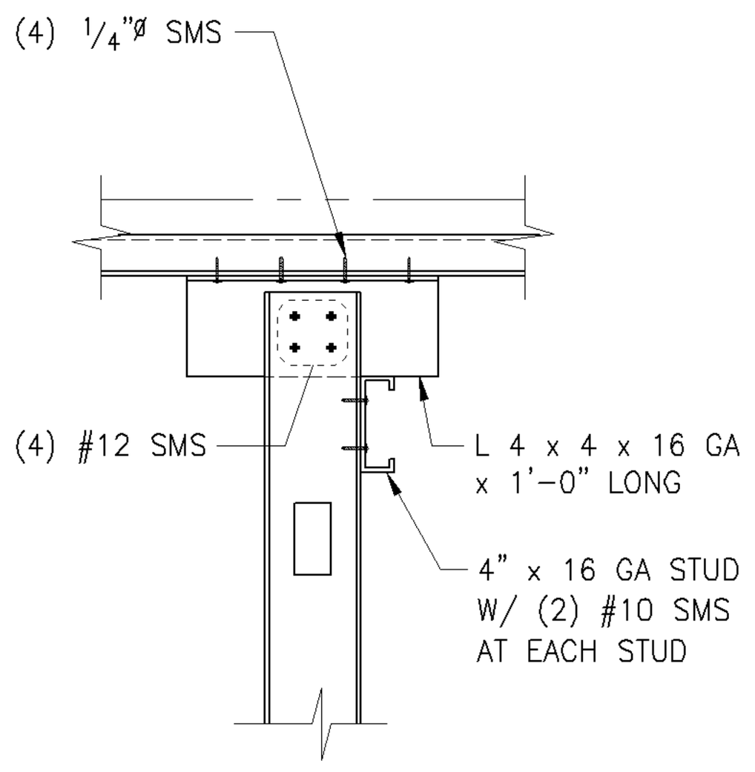


(A) WALL MOUNTED ITEMS (150 LBS – MAX)

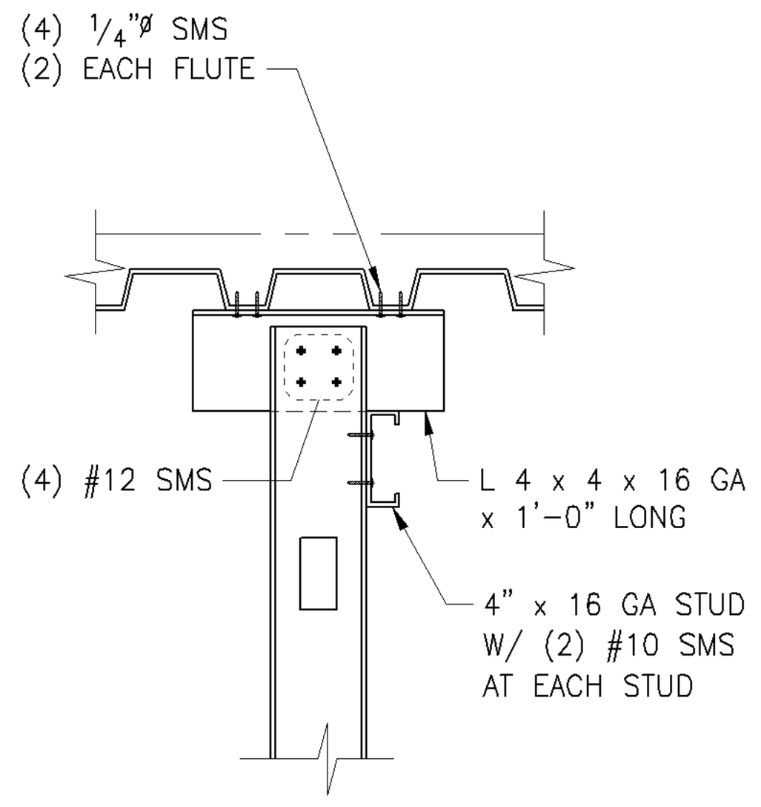
BACKING AT EQUIPMENT

METAL STUD BACKING DETAILS

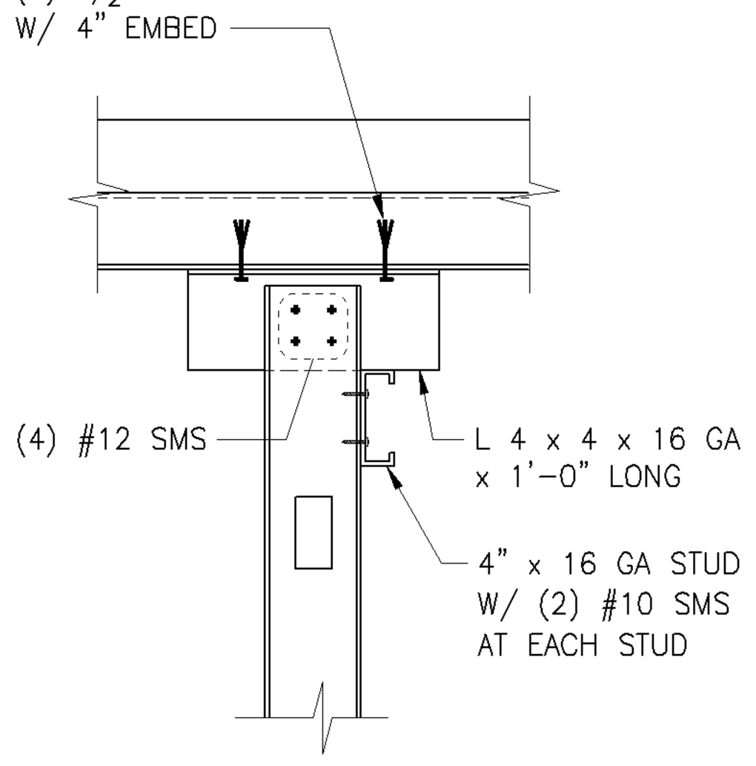
1"=1'-0" | 2



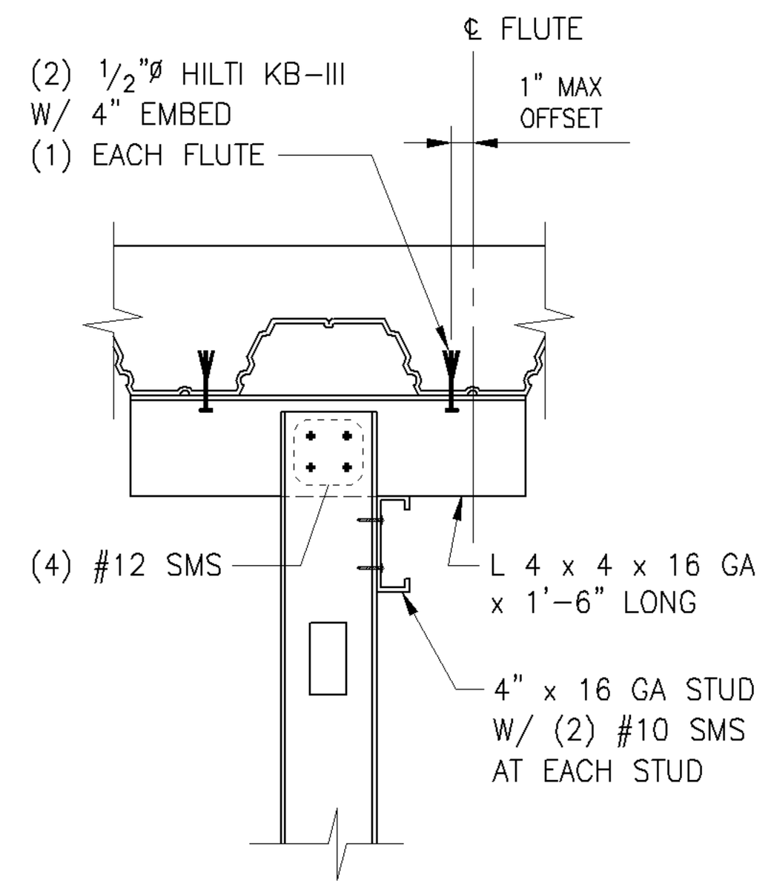
(D) METAL DECK ONLY
FLUTES PERPENDICULAR TO WALL



(C) METAL DECK ONLY
FLUTES PARALLEL TO WALL



(B) METAL DECK W/ CONC FILL
FLUTES PARALLEL TO BRACE



(A) METAL DECK W/ CONC FILL
FLUTES PERPENDICULAR TO BRACE

TYPICAL HANGING STUD DETAIL AT DECK

1 1/2"=1'-0" | 3

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS
25531 YNEZ ROAD
Temecula, CA 92591-4628

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCANDIDO, CA 92029
Tel: 760.745.8118
Fax: 760.745.1890

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101
Tel: 626.578.1121
Fax: 626.578.9121

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010
Tel: 760.496.3714
Fax: 760.496.3711

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
2908 OREGON COURT, SUITE 1-7
TORRANCE, CA 90503
Tel: 310.328.4734
Fax: 310.328.4708

REGISTRATION

REGISTERED PROFESSIONAL ENGINEER
No. S4399
Exp. 12/31/07
STRUCTURAL
STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTAL

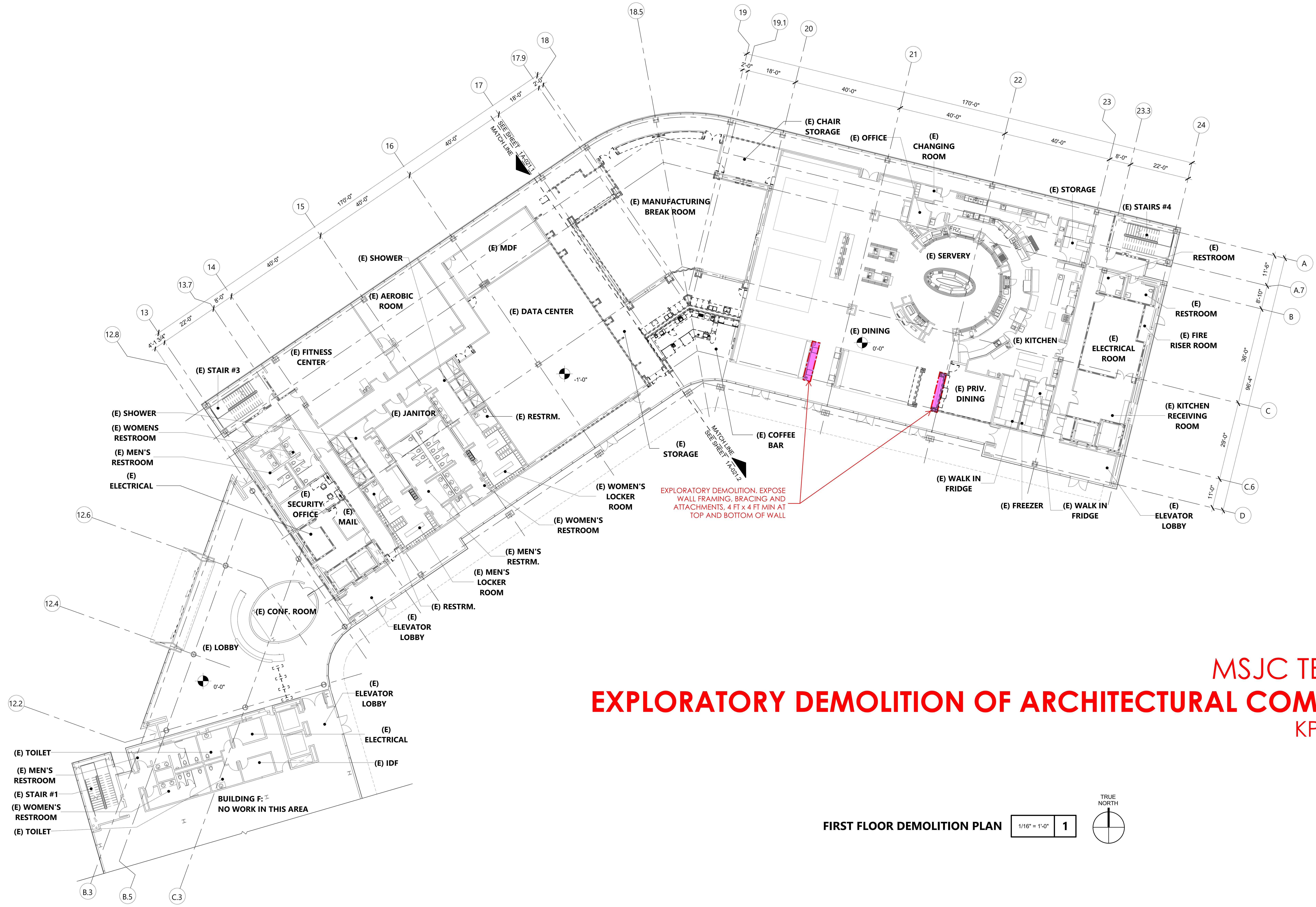
PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN

SHEET TITLE

EXTERIOR METAL STUD WALL DETAILS

S8-107



MSJC TEMECULA
EXPLORATORY DEMOLITION OF ARCHITECTURAL COMPONENTS
KPFF, 2/8/2019

FIRST FLOOR DEMOLITION PLAN 1/16" = 1'-0" | 1

TRUE NORTH

KEYNOTES

DEMOLITION LEGEND

- (E) WALL, TO REMAIN, PROTECT IN PLACE
- (E) 1HR. WALL, TO REMAIN, PROTECT IN PLACE
- (E) 2HR. RATED WALL, TO REMAIN, PROTECT IN PLACE
- (E) WALL TO BE REMOVED, COMPLETE
- (E) FLOOR TO BE REMOVED, COMPLETE
- (E) RAISED FLOOR SYSTEM TO REMAIN, PROTECT IN PLACE

GENERAL NOTES

1. SEE CIVIL, LANDSCAPE, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR ADDITIONAL DEMOLITION SCOPE OF WORK.

PMSM ARCHITECTS

19·6 ARCHITECTS
802 EAST COTA STREET, SUITE A
SANTA BARBARA, CA 93103
TEL (805) 963-1955

CONSULTANTS

CIVIL ENGINEER
KPFF
700 South Flower Street, Suite 2100
Los Angeles, CA 90017
TEL (213) 418-0201

STRUCTURAL ENGINEER
KPFF
700 South Flower Street, Suite 2100
Los Angeles, CA 90017
TEL (213) 310-8536

MECHANICAL ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

ELECTRICAL ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

FIRE PROTECTION ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

LABORATORY DESIGN CONSULTANT
RFD
3965 Fifth Avenue, Suite 400
San Diego, CA 92103
TEL (619) 756-2681

ARCHITECT STAMP | CONSULTANT STAMP

LICENSED ARCHITECT, STATE OF CALIFORNIA — NOT FOR CONSTRUCTION

AGENCY APPROVAL

REVISIONS

NO.	DATE	DESCRIPTION

KEY PLAN

AREA 1 AREA 2

PROJECT OWNER & TITLE

MT. SAN JACINTO COMMUNITY COLLEGE DISTRICT

TEMECULA VALLEY CAMPUS
41888 Motor Car Pkwy,
Temecula, CA 92591

SHEET TITLE

DEMOLITION FIRST FLOOR PLAN

DRAWN BY: Author JOB NUMBER: 018103.01

SHEET NO.

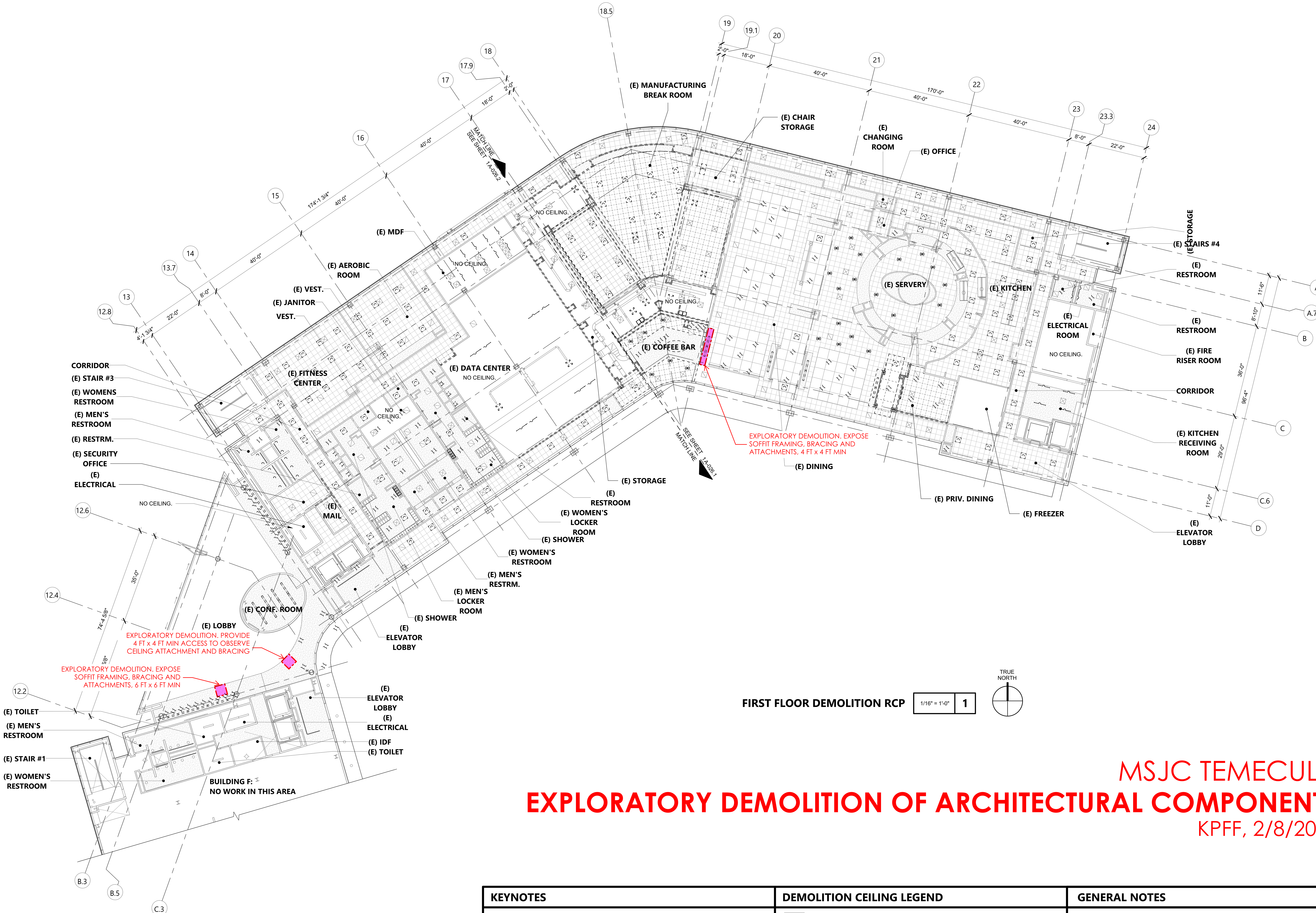
A-021

DATE: JANUARY 21, 2019

DESIGN DEVELOPMENT

FILE LOCATION: BIM 360://MSJC Temecula Valley Campus/A17_MSJC_TEMECULA VALLEY COLLEGE_Building G.rvt

DATE PLOTTED: 1/25/2019 11:01:50 AM



FIRST FLOOR DEMOLITION RCP | 1/16" = 1'-0" | 1

MSJC TEMECULA
EXPLORATORY DEMOLITION OF ARCHITECTURAL COMPONENTS
KPFF, 2/8/2019

KEYNOTES

DEMOLITION CEILING LEGEND

- (E) GYP. BD. - HARD LID CEILING
- REMOVE (E) GYP. BD. CEILING
- (E) SUSPENDED 2X2 ACOUSTICAL CEILING , TO REMAIN
  ACT 1 - "ARMSTRONG" MESA 687
  ACT 2 - "ARMSTRONG" OPEN PLAN 3355
  ACT 3 - "ARMSTRONG" CLEANROOM VL
  ACT4 - "ARMSTRONG" OPEN PLAN NON STANDARD SIZE
- REMOVE (E) SUSPENDED 2X2 ACOUSTICAL CEILING
- (E) LIGHT FIXTURES TO BE REMOVED
- (E) SUPPLY REGISTER TO BE REMOVED
- SUPPLY REGISTER PER MECHANICAL DRAWINGS
- EXHAUST FAN OR GRILLE PER MECHANICAL DRAWINGS
- RETURN AIR GRILLE PER MECHANICAL DRAWINGS
- (E) WALL TO BE DEMOLISHED

GENERAL NOTES

1. SEE CIVIL, LANDSCAPE, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR ADDITIONAL DEMOLITION SCOPE OF WORK.

PMSM ARCHITECTS

19·6 ARCHITECTS
802 EAST COTA STREET, SUITE A
SANTA BARBARA, CA 93103
TEL (805) 963-1955

CONSULTANTS

CIVIL ENGINEER
KPFF
700 South Flower Street, Suite 2100
Los Angeles, CA 90017
TEL (213) 418-0201

STRUCTURAL ENGINEER
KPFF
700 South Flower Street, Suite 2100
Los Angeles, CA 90017
TEL (213) 310-8536

MECHANICAL ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

ELECTRICAL ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

FIRE PROTECTION ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

LABORATORY DESIGNCONSULTANT
RFD
3965 Fifth Avenue, Suite 400
San Diego, CA 92103
TEL (619) 756-2681

ARCHITECT STAMP | CONSULTANT STAMP

LICENSED ARCHITECT MONISHA ADNANI, AIA — NOT FOR CONSTRUCTION — STATE OF CALIFORNIA

AGENCY APPROVAL

REVISIONS

NO.	DATE	DESCRIPTION

KEY PLAN
AREA 1 AREA 2

PROJECT OWNER & TITLE

MT. SAN JACINTO COMMUNITY COLLEGE DISTRICT

TEMECULA VALLEY CAMPUS
41888 Motor Car Pkwy, Temecula, CA 92591

SHEET TITLE

DEMO FIRST FLOOR RCP

DRAWN BY: Author JOB NUMBER: 018103.01

SHEET NO.

A-026

DATE: JANUARY 21, 2019

DESIGN DEVELOPMENT

FILE LOCATION: BIM 360://MSJC Temecula Valley Campus/A17_MSJC_TEMECULA VALLEY COLLEGE_Building G.rvt

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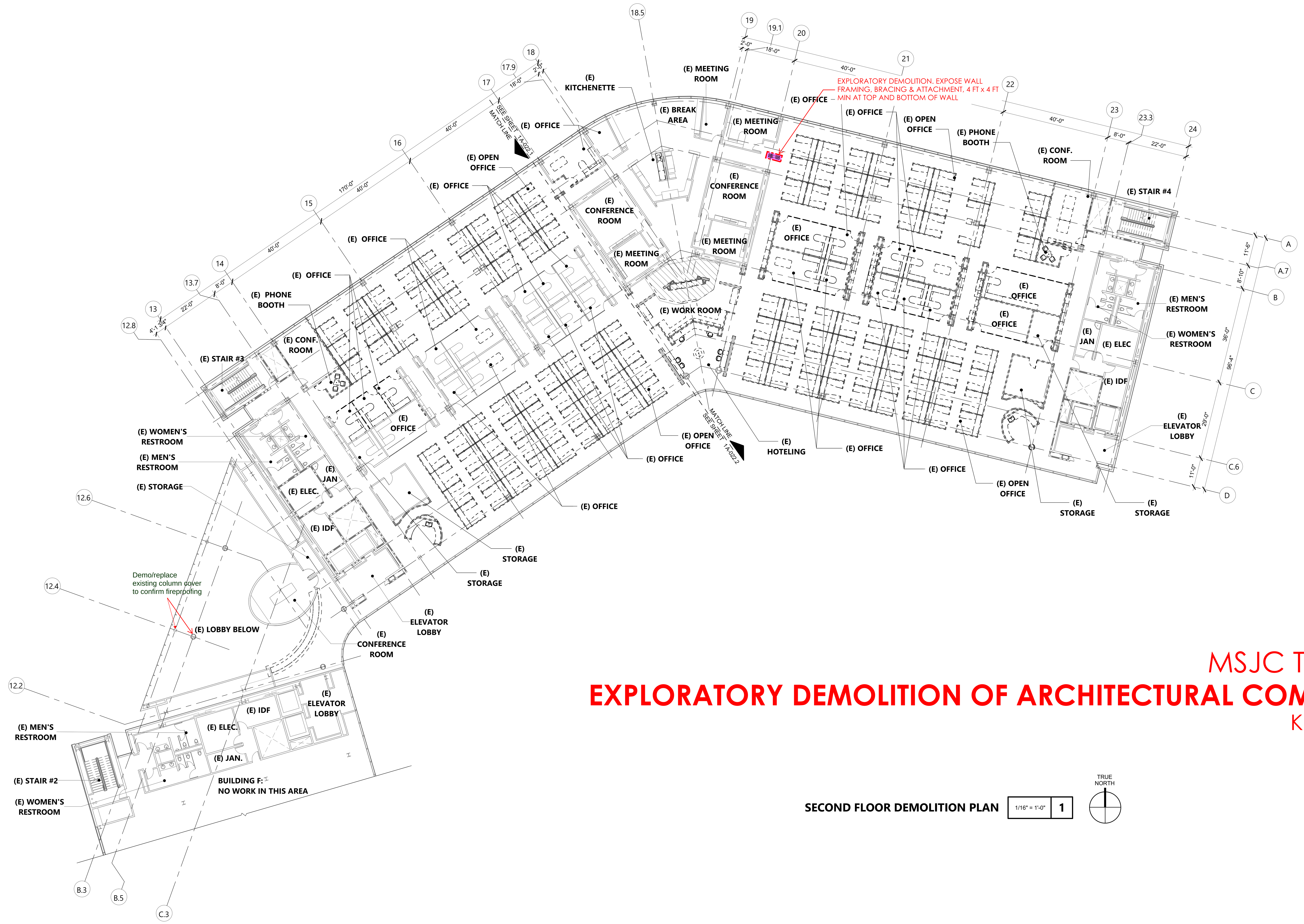

MSJC TEMECULA

EXPLORATORY DEMOLITION OF ARCHITECTURAL COMPONENTS

KPFF, 2/8/2019

SECOND FLOOR DEMOLITION PLAN | 1/16" = 1'-0" | 1

TRUE NORTH

KEYNOTES

DEMOLITION LEGEND

- (E) WALL, TO REMAIN, PROTECT IN PLACE
- (E) 1HR. WALL, TO REMAIN, PROTECT IN PLACE
- (E) 2HR. RATED WALL, TO REMAIN, PROTECT IN PLACE
- (E) WALL TO BE REMOVED, COMPLETE
- (E) FLOOR TO BE REMOVED, COMPLETE
- (E) RAISED FLOOR SYSTEM TO REMAIN, PROTECT IN PLACE

GENERAL NOTES

1. SEE CIVIL, LANDSCAPE, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR ADDITIONAL DEMOLITION SCOPE OF WORK.

PMSM ARCHITECTS

19six ARCHITECTS
802 EAST COTA STREET, SUITE A
SANTA BARBARA, CA 93103
TEL (805) 963-1955

CONSULTANTS

CIVIL ENGINEER
KPFF
700 South Flower Street, Suite 2100
Los Angeles, CA 90017
TEL (213) 418-0201

STRUCTURAL ENGINEER
KPFF
700 South Flower Street, Suite 2100
Los Angeles, CA 90017
TEL (213) 310-8536

MECHANICAL ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

ELECTRICAL ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

FIRE PROTECTION ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

LABORATORY DESIGN CONSULTANT
RFD
3965 Fifth Avenue, Suite 400
San Diego, CA 92103
TEL (619) 756-2681

ARCHITECT STAMP | CONSULTANT STAMP

LICENSED ARCHITECT — NOT FOR CONSTRUCTION — STATE OF CALIFORNIA

AGENCY APPROVAL

REVISIONS

NO.	DATE	DESCRIPTION

KEY PLAN: AREA 1, AREA 2

PROJECT OWNER & TITLE

MT. SAN JACINTO COMMUNITY COLLEGE DISTRICT

TEMECULA VALLEY CAMPUS
41888 Motor Car Pkwy,
Temecula, CA 92591

SHEET TITLE

DEMOLITION SECOND FLOOR PLAN

DRAWN BY: Author | JOB NUMBER: 018103.01

SHEET NO.

A-022

DATE: JANUARY 21, 2019

DESIGN DEVELOPMENT

FILE LOCATION: BIM 360://MSJC Temecula Valley Campus/A17_MSJC_TEMECULA VALLEY COLLEGE_Building G.rvt

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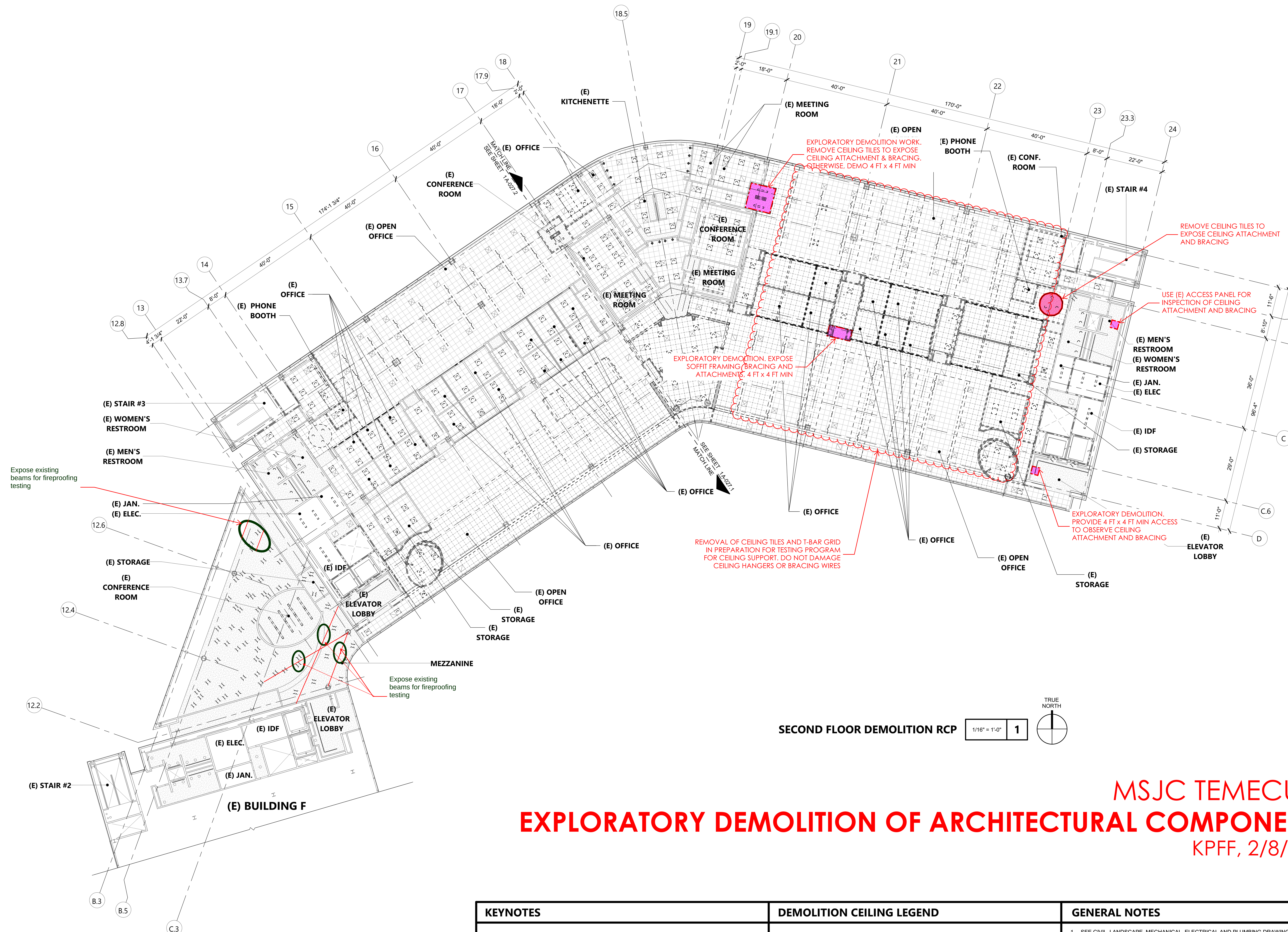

SECOND FLOOR DEMOLITION RCP | 1/16" = 1'-0" | 1

MSJC TEMECULA

EXPLORATORY DEMOLITION OF ARCHITECTURAL COMPONENTS

KPFF, 2/8/2019

KEYNOTES

DEMOLITION CEILING LEGEND

- (E) GYP. BD. - HARD LID CEILING
- REMOVE (E) GYP. BD. CEILING
- (E) SUSPENDED 2X2 ACOUSTICAL CEILING , TO REMAIN
  - ACT 1 - "ARMSTRONG" MESA 687
  - ACT 2 - "ARMSTRONG" OPEN PLAN 3355
  - ACT 3 - "ARMSTRONG" CLEANROOM VL
  - ACT4 - "ARMSTRONG" OPEN PLAN NON STANDARD SIZE
- REMOVE (E) SUSPENDED 2X2 ACOUSTICAL CEILING
- (E) LIGHT FIXTURES TO BE REMOVED
- (E) SUPPLY REGISTER TO BE REMOVED
- SUPPLY REGISTER PER MECHANICAL DRAWINGS
- EXHAUST FAN OR GRILLE PER MECHANICAL DRAWINGS
- RETURN AIR GRILLE PER MECHANICAL DRAWINGS
- (E) WALL TO BE DEMOLISHED

GENERAL NOTES

1. SEE CIVIL, LANDSCAPE, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR ADDITIONAL DEMOLITION SCOPE OF WORK.

PMSM ARCHITECTS

19·6 ARCHITECTS
802 EAST COTA STREET, SUITE A
SANTA BARBARA, CA 93103
TEL (805) 963-1955

CONSULTANTS

CIVIL ENGINEER
KPFF
700 South Flower Street, Suite 2100
Los Angeles, CA 90017
TEL (213) 418-0201

STRUCTURAL ENGINEER
KPFF
700 South Flower Street, Suite 2100
Los Angeles, CA 90017
TEL (213) 310-8536

MECHANICAL ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

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P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

FIRE PROTECTION ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

LABORATORY DESIGNCONSULTANT
RFD
3965 Fifth Avenue, Suite 400
San Diego, CA 92103
TEL (619) 756-2681

ARCHITECT STAMP | CONSULTANT STAMP

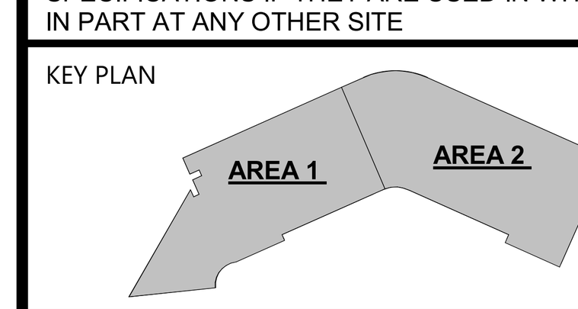
LICENSED ARCHITECT MONISHA ADNANI, AIA C-32128 1-31-19 STATE OF CALIFORNIA — NOT FOR CONSTRUCTION

AGENCY APPROVAL

REVISIONS

NO.	DATE	DESCRIPTION

KEY PLAN



PROJECT OWNER & TITLE

MT. SAN JACINTO COMMUNITY COLLEGE DISTRICT

TEMECULA VALLEY CAMPUS
41888 Motor Car Pkwy,
Temecula, CA 92591

SHEET TITLE

DEMO SECOND FLOOR RCP

DRAWN BY: Author JOB NUMBER: 018103.01

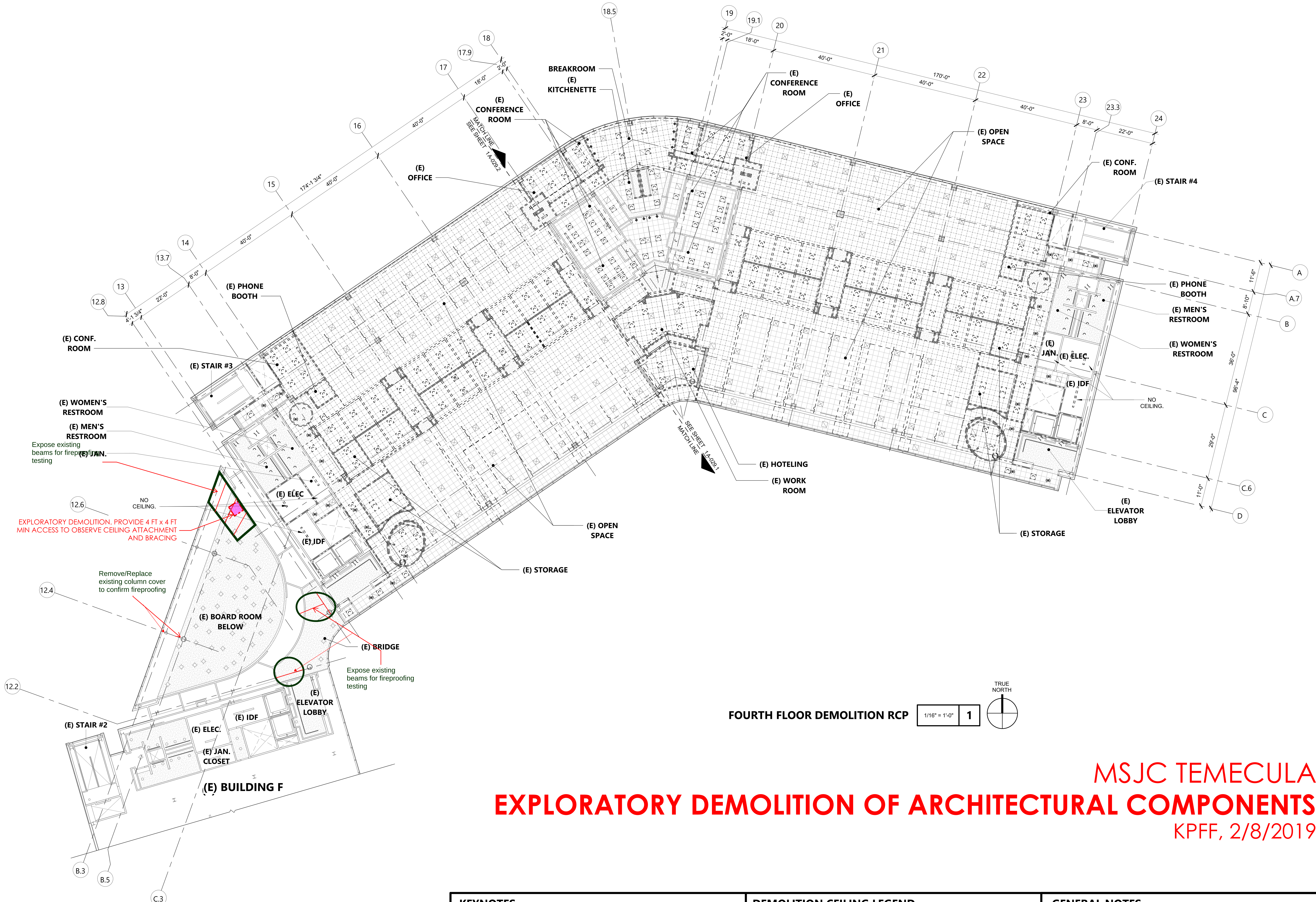
SHEET NO.

A-027

DATE: JANUARY 21, 2019

DESIGN DEVELOPMENT

DATE PLOTTED: 1/25/2019 11:03:48 AM FILE LOCATION: BIM 360://MSJC Temecula Valley Campus/A17_MSJC_TEMECULA VALLEY COLLEGE_Building G.rvt



FOURTH FLOOR DEMOLITION RCP 1/16" = 1'-0" 1

MSJC TEMECULA

EXPLORATORY DEMOLITION OF ARCHITECTURAL COMPONENTS

KPFF, 2/8/2019

KEYNOTES

DEMOLITION CEILING LEGEND

- (E) GYP. BD. - HARD LID CEILING
- REMOVE (E) GYP. BD. CEILING
- (E) SUSPENDED 2X2 ACOUSTICAL CEILING , TO REMAIN
  - ACT 1 - "ARMSTRONG" MESA 687
  - ACT 2 - "ARMSTRONG" OPEN PLAN 3355
  - ACT 3 - "ARMSTRONG" CLEANROOM VL
  - ACT4 - "ARMSTRONG" OPEN PLAN NON STANDARD SIZE
- REMOVE (E) SUSPENDED 2X2 ACOUSTICAL CEILING
- (E) LIGHT FIXTURES TO BE REMOVED
- (E) SUPPLY REGISTER TO BE REMOVED
- SUPPLY REGISTER PER MECHANICAL DRAWINGS
- EXHAUST FAN OR GRILLE PER MECHANICAL DRAWINGS
- RETURN AIR GRILLE PER MECHANICAL DRAWINGS
- (E) WALL TO BE DEMOLISHED

GENERAL NOTES

1. SEE CIVIL, LANDSCAPE, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR ADDITIONAL DEMOLITION SCOPE OF WORK.
2. ALL (E) LIGHT FIXTURES TO BE REMOVED

PMSM ARCHITECTS

19·6 ARCHITECTS
802 EAST COTA STREET, SUITE A
SANTA BARBARA, CA 93103
TEL (805) 963-1955

CONSULTANTS

CIVIL ENGINEER
KPFF
700 South Flower Street, Suite 2100
Los Angeles, CA 90017
TEL (213) 418-0201

STRUCTURAL ENGINEER
KPFF
700 South Flower Street, Suite 2100
Los Angeles, CA 90017
TEL (213) 310-8536

MECHANICAL ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

ELECTRICAL ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

FIRE PROTECTION ENGINEER
P2S INC
5000 East Spring Street, Suite 800
Long Beach, CA 90815
TEL (562) 497-2999

LABORATORY DESIGNCONSULTANT
RFD
3965 Fifth Avenue, Suite 400
San Diego, CA 92103
TEL (619) 756-2681

ARCHITECT STAMP | CONSULTANT STAMP

LICENSED ARCHITECT MONISHA ADNANI, AIA C-32128 REN. 1-31-19 STATE OF CALIFORNIA — NOT FOR CONSTRUCTION

AGENCY APPROVAL

REVISIONS

NO.	DATE	DESCRIPTION

KEY PLAN

AREA 1 AREA 2

PROJECT OWNER & TITLE

MT. SAN JACINTO COMMUNITY COLLEGE DISTRICT

TEMECULA VALLEY CAMPUS
41888 Motor Car Pkwy,
Temecula, CA 92591

SHEET TITLE

DEMO FOURTH FLOOR RCP

DRAWN BY: Author JOB NUMBER: 018103.01

SHEET NO.

A-029

DATE: JANUARY 21, 2019

DESIGN DEVELOPMENT

FILE LOCATION: BIM 360://MSJC Temecula Valley Campus/A17_MSJC_TEMECULA VALLEY COLLEGE_Building G.rvt
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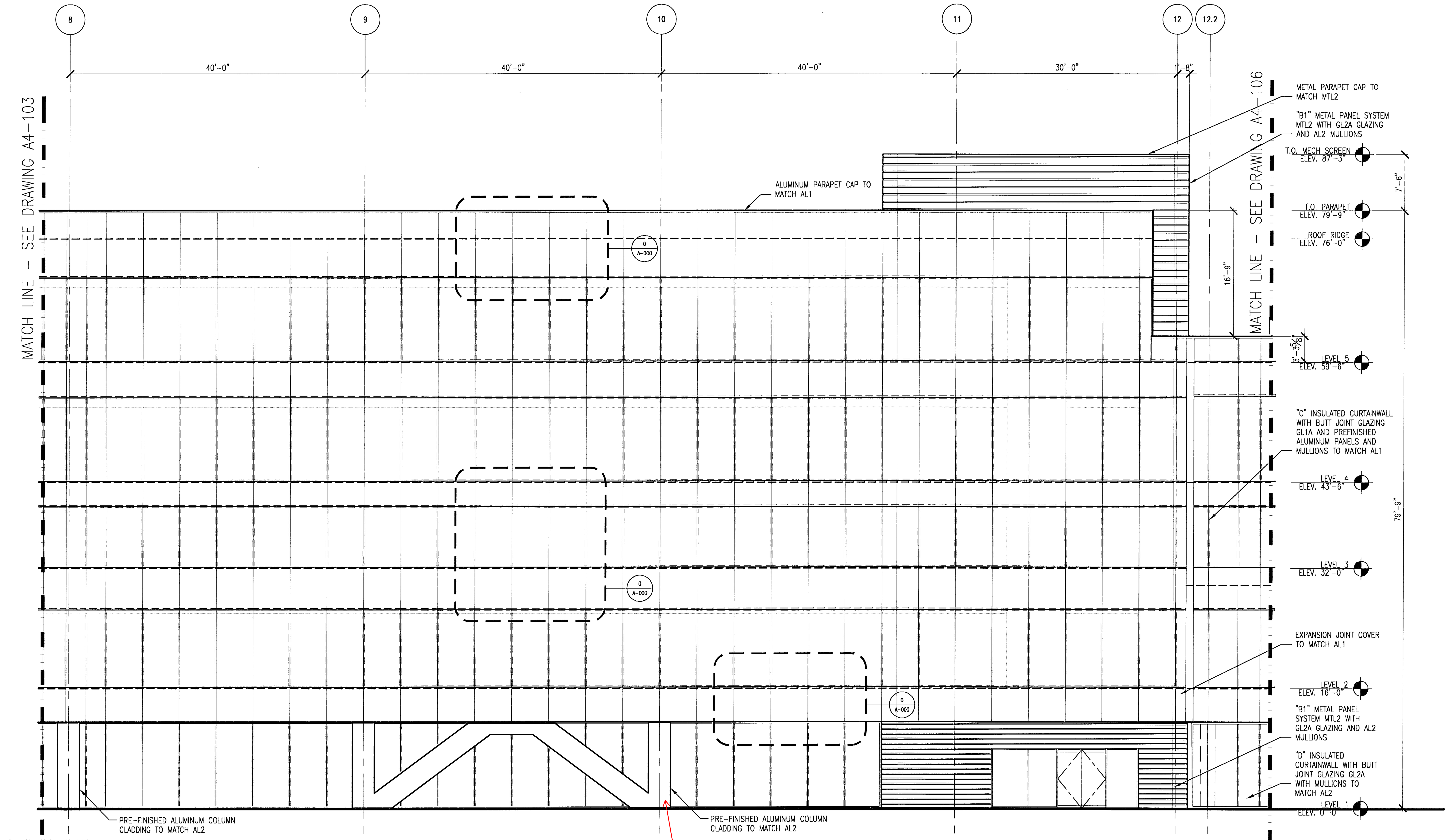
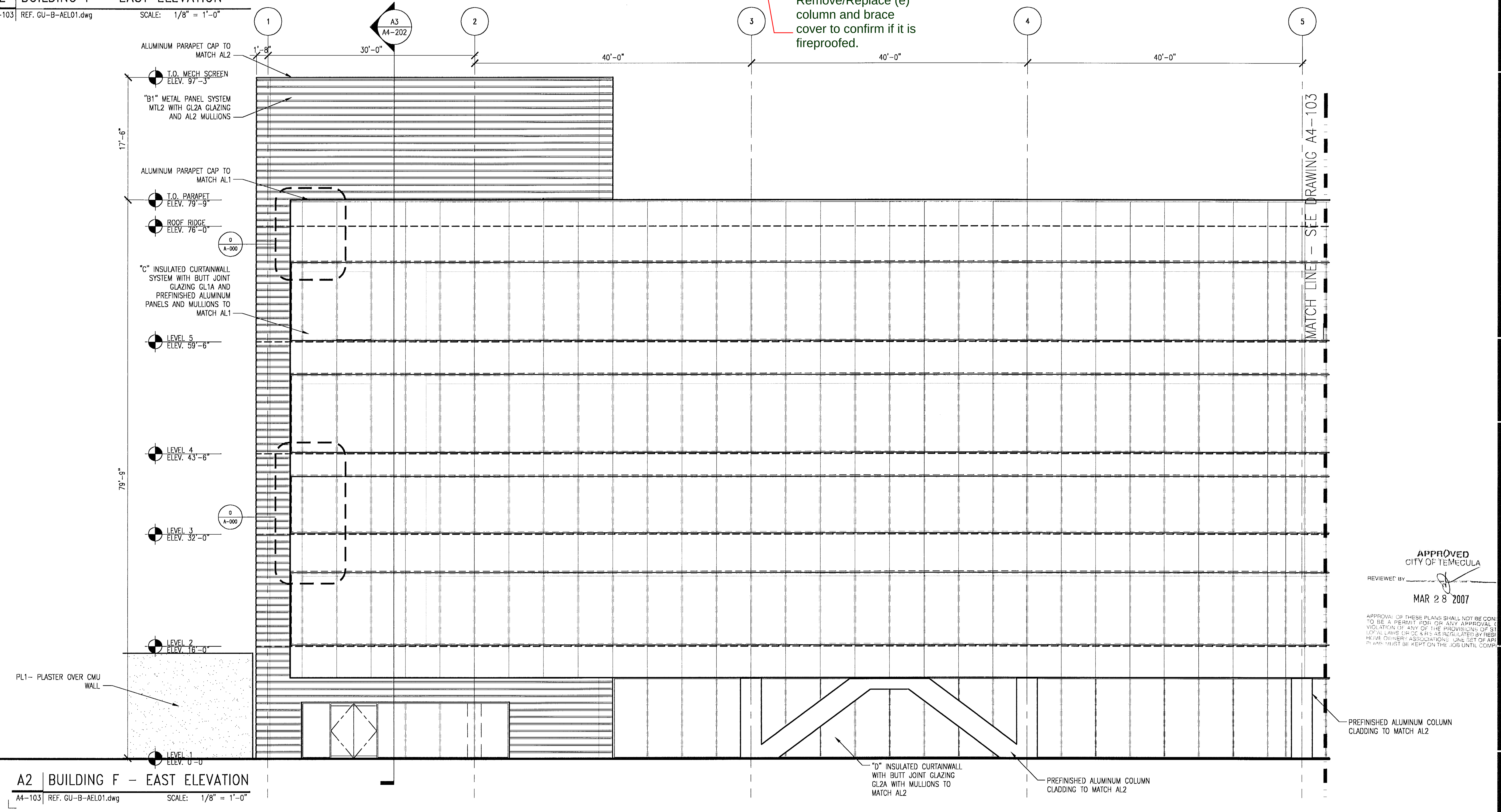


C2 | BUILDING F - EAST ELEVATION
A4-103 | REF. GU-B-AEL01.dwg SCALE: 1/8" = 1'-0"



A2 | BUILDING F - EAST ELEVATION
A4-103 | REF. GU-B-AEL01.dwg SCALE: 1/8" = 1'-0"

Abbreviation	Description
GL1A	VRE 1-59 1" INSULATING LOW-E
GL2A	VRE 1-30 1" INSULATING LOW-E
GL3A	VRE 1-59 1" INSULATING LOW-E WITH SIMULATED ACID ETCHED PATTERN
MTL1	INSULATED METAL PANEL SYSTEM, CENTRIA FORMAWALL DIMENSION SERIES 2" HORIZONTAL FLAT SYSTEM, COLOR: PT1
MTL1A	METAL LOUVER, COLOR: PT1
MTL2	METAL PANEL SYSTEM, CENTRIA CONCEPT SERIES CS-620 COLOR: PT2
AL1	PREFINISHED ALUMINUM, COLOR: PT1
AL2	PREFINISHED ALUMINUM, COLOR: PT2
AL3	PREFINSIHED ALUMINUM, COLOR: PT3
PT1	DURANAR XL "REGAL WHITE"
PT2	DURANAR XL "PEWTER"
PT3	DURANAR XL "BRIGHT SILVER"
PL1	PLASTER OVER CMU, COLOR: TO MATCH PT1

APPROVED
CITY OF TEMECULA
REVIEWED BY
MAR 28 2007

OWNER/CLIENT

Abbott Vascular

TEMECULA EAST CAMPUS

ARCHITECT

DMJM DESIGN | AECOM

515 SOUTH FLOWER ST., 8TH FLOOR
LOS ANGELES, CA 90071
T: 213.593.8100 F: 213.593.8608
www.dmjmhn.aecom.com

CONSULTANTS

EXCEL ENGINEERING
CIVIL ENGINEER
440 STATE PLACE
ESCONDIDO, CA 92029

KPFF CONSULTING ENGINEERS
STRUCTURAL ENGINEER
2 N LAKE AVENUE, SUITE 820
PASADENA, CA 91101

CRB CONSULTING ENGINEERS, INC
MECHANICAL, ELECTRICAL & PLUMBING ENGINEER
2701 LOKER AVENUE WEST, SUITE 130
CARLSBAD, CA 92010

BENNITT + MITCHELL
LANDSCAPE ARCHITECT
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TORRANCE, CA 90503

REGISTRATION

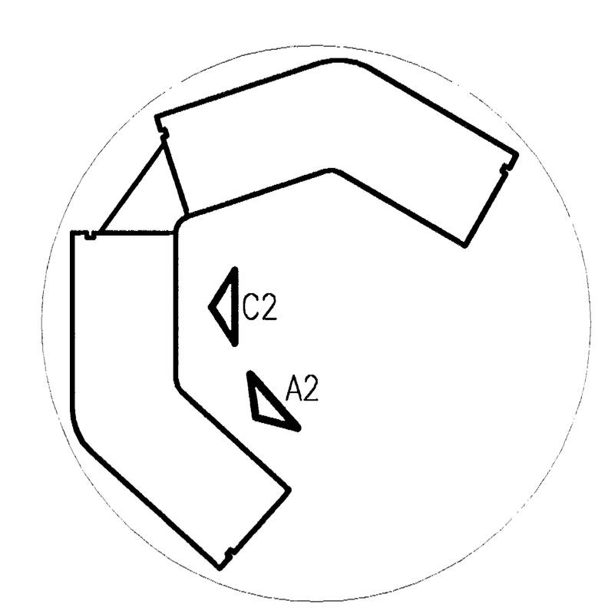
LICENSED ARCHITECT
STATE OF CALIFORNIA

ISSUE

MARK	DATE	DESCRIPTION
3	03-22-07	FOR CONSTRUCTION
2	01-31-07	B & S RESUBMITTAL
1	10-23-06	BLDG & SAFETY SUBMITTA

PROJECT NO: 60004775
DRAWN BY:
CHECKED BY:

KEY PLAN



SHEET TITLE

EXTERIOR ELEVATION: EAST (BUILDING F)

A4-103